



2020 Standard Specifications for Highway Construction



Ministry of
Transportation
and Infrastructure

Volume 1 of 2
Adopted November 1, 2020



2020 STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION VOLUME 1

Adopted November 1, 2020

Construction and Maintenance Branch

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Installing rock-fall protection mesh
Hoffman's Bluff on Trans-Canada Hwy #1, east of Kamloops, BC
Contractor: Emil Anderson Construction Co. Ltd.
50° 43' 37.26" N; 119° 45' 55.49" W

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NOTICE TO USERS

Generally, text significantly changed or added since the 2016 edition is shown with single underlining and a vertical bar in the margin; deletions are marked only by the vertical change bars. Not all changes are marked. Minor corrections such as typos have not been marked.

Extensively modified Sections are watermarked as “Revised Section”; new Sections are watermarked “New Section”, and neither have updates marked.

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Please contact Construction and Maintenance Branch
for questions related to this document.

Construction and Maintenance Branch
Ministry of Transportation and Infrastructure
940 Blanshard Street Level 4C
PO Box 9850 STN PROV GOVT
Victoria BC V8W 9T5
E-mail: TranRehab@gov.bc.ca

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NOTES

000.01 Interpretation – Ministry names may have changed from those identified in the Standard Specifications. A list of current Ministries may be found at the following link

<https://www2.gov.bc.ca/gov/content/governments/organizational-structure/ministries-organizations/ministries>

Upon request by the Contractor, the Ministry Representative will provide clarification as to which Ministry is currently responsible for any particular reference.

000.02 Hyperlinks – In the PDF version of this document, many reference sources have been hyperlinked (such as the reference in Note 000.01) and will show as blue underlined text.

Clicking on the link will open the resource in your web browser.

Generally, links to test methods and specification organizations (like [CSA](#) or [ASTM](#)) will take you to a site where you can purchase and download the associated specifications; a link to a Ministry document, and some others, within the text of a Section should take you to a website where you can download the referenced document free of charge.

Note: Due to MS Word® prioritizing “Track Changes” formatting over other formatting, some newly inserted hyperlinks are not shown in blue. If there is underlined black text that looks like it might warrant a hyperlink, hover your mouse over it to see if it does have an embedded link.

000.02.01 Disclaimer – Any information, including statutes and regulations, that may be found by accessing URLs or websites (including via hyperlinks) in these Standard Specifications are not the official versions and may not be accurate, complete, current or reliable.

000.03 General Condition (GC) References – Where these Standard Specifications make a reference to a General Condition (e.g. GC 38.00 Changes to Work), that reference

is to the applicable clause of the Ministry’s [Major Works General Conditions](#).

Where the Work is being performed under a different set of terms and conditions (e.g the [Minor Works General Conditions](#)), the reference shall be read as being to the most comparable term and condition of the actual Contract.

In the event there is uncertainty in determining the applicable clause, the Ministry Representative will determine how the reference is to be interpreted.

000.04 Engineer of Record (EoR) / Professional of Record (PoR) – These two terms are used, essentially interchangeably, within these Standard Specifications and are based on the Association of Professional Engineers and Geoscientists of the Province of British Columbia’s (APEGBC) definition and recent terminology change from EoR to PoR.

APEGBC’s definition, as expressed within their “Use of Seal” (and other) Quality Management Guidelines, is:

Professional of Record: The Engineering / Geoscience Professional or licensee with the lowest level of direct professional responsibility for the engineering or geoscience work and any related engineering or geoscience documents produced, and whose seal appears on the documents. A test of “direct professional responsibility” is the ability of that Engineering/Geoscience Professional to alter or revise the engineering or geoscience content in the master documents.

Whereas APEGBC is concerned only with Professional Engineers and Geoscientists, usage in these Standard Specifications may also capture other registered professions such as Registered Professional Foresters (RPF), Registered Professional Biologists (RPBio), and other disciplines, as appropriate to the circumstances.

Each EoR/PoR is professionally responsible to the Ministry for the works they have designed.

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QUALITY MANAGEMENT

101.01 General – All work on Ministry projects shall be undertaken through a total quality management process, utilizing Ministry, Contractor and third-party resources, as necessary.

The Contractor shall have a quality management program and staff in place and provide the Ministry with documented assurance that any product meets all applicable requirements of the Contract, at any hold point or witness point (as defined in SS 145.12) and prior to issuance of the Completion Certificate.

The Ministry accepts the following definitions associated with Quality Management:

- **Quality Control (QC):** The process of checking specific product or service results to determine if they comply with relevant quality standards and identifying ways to eliminate causes of unsatisfactory product or service performance.
- **Quality Assurance (QA):** The process of evaluating overall product or service, by persons or companies independent of those doing the Work, on a regular basis to provide confidence that the product or service satisfies the relevant quality standards.

The quality management program has two key components as follows:

- Quality Control – the Contractor’s responsibility
- Quality Assurance – the Ministry’s responsibility

Each component of the program must address materials, processes, products, and documentation, utilizing both construction inspection measures and standard test methodologies where appropriate.

The Contractor shall provide unrestricted access to all Quality Control operations and documentation produced by or on behalf of the Contractor and shall allow the Ministry full access at any time during working hours.

The Ministry Representative will review the Contractor’s performance of the Work and determine the acceptability of the Work based on the Ministry’s Quality Assurance results and, where deemed appropriate by the Ministry Representative, supplemented by the Contractor’s Quality Control results.

Work failing to meet the conditions of the Contract will be considered Unacceptable Work.

The Ministry Representative may consider all Work from the last acceptable Quality Assurance testing as Unacceptable Work. The Contractor will not be entitled to payment for Work that lacks the appropriate Quality

Control documentation, verified by the QC Manager, as required by the Contract.

The Contractor shall implement a well-coordinated approach to all operations related to the Work and shall organize its team and operations in keeping with the goal of doing things right the first time.

101.02 Quality Control Plan (QC Plan)

101.02.01 QC Plan General Requirements – The Contractor shall be responsible for all Quality Control during the performance of the Work. QC work includes monitoring, inspecting and testing the means, methods, materials, workmanship, processes and products of all aspects of the Work as necessary to ensure conformance with the Contract.

The Contractor shall prepare, and submit to the Ministry Representative, a Quality Control Plan (QC Plan) in accordance with the Contract provisions, including SS 101.02.03

The QC Plan shall be structured around the ISO 9001 program although ISO registration is not required, and clearly demonstrate the Contractor’s understanding and commitment to ISO’s principles of quality management:

- Customer focus
- Leadership
- Engagement of people
- Process approach
- Continual improvement
- Evidence-based decision-making
- Relationship Management.

The QC Plan must also include sections detailing the Contractor’s methodology associated with each of the above principles.

No Work shall be undertaken on any element of Work (including payment Items and incidental Work, or submittals for review) for which there are QC Plan submission requirements until the Ministry has accepted the base portion of the QC Plan and the specific details for that element of Work.

Subject to the submission requirements outlined in SS 101.02.03, the QC Plan is required to cover the Work in its entirety, including without limitation all materials the Contractor and Subcontractors are supplying, and all items and phases of construction on the Project.

The plan may be operated wholly or in part by a qualified Subcontractor or an independent agency/organization. However, the plan's administration (including conformance with the plan and its modifications) and the quality of the Work remain the responsibility of the Contractor.

The Contractor's QC program and the Work shall be undertaken in accordance with the QC Plan and shall be well managed, with testing results representative of actual operations. Results will be reported accurately and in a timely manner.

The Contractor shall also ensure that all workers are familiar with the Quality Management Plans, its goals, and their role under it, as well as with the Contract specifications associated with the Work they are to undertake.

101.02.02 QC Plan Quality Control Staff and Equipment Submission Requirements – The Contractor shall provide all resources and take all actions necessary to ensure:

- Provision of sufficient inspection or testing staff, with adequate equipment and technical support to perform all Quality Control functions in an accurate and timely manner.
- That QC staff perform only inspections and tests for which they are qualified.
- All testing equipment is calibrated, properly maintained, and in good operating condition.
- All testing and inspection is performed in accordance with appropriate standards of the Contract.
- Submission to the Ministry Representative, within twenty-four (24) hours, of daily reports for all tests and inspections that indicate non-conformance of the material being tested.
- Production, within forty-eight (48) hours, of daily reports for all tests and inspections that indicate conformance of the material being tested and the availability of back-up documentation to substantiate test results when required.
- Organization, compilation and submission of all project QC documentation within 14 days of the last day on Site and as a condition precedent to the issuance of the Completion Certificate.

The Contractor shall designate one person as the Quality Control Manager (QC Manager) who shall be responsible for the implementation of the QC Plan.

The QC Manager shall be:

- a qualified Professional Engineer,
- a Certified Engineering Technician,
- an Applied Science Technologist, or
- shall have extensive relevant experience in quality management.

and must demonstrate knowledge, skills and abilities acceptable to the Ministry Representative. .

The QC Manager shall be at "arms-length" from the productivity part of the Contractor's organization and specifically shall not be the Project Manager or the Project Superintendent. On Minor Works contracts, this requirement will, upon request by the Contractor and where the candidate is deemed acceptable under the foregoing paragraph, generally be waived by the Ministry Representative.

The Ministry recognizes the Contractor's Project Manager and Superintendent as the personnel responsible for making the product meet the contractual requirements, but the QC Manager's duties include being responsible to measure conformance and to ensure that quality is not compromised by production pressures.

The QC Manager, or a designated replacement acceptable to the Ministry Representative empowered and able to perform all of the QC Manager's relevant duties, shall remain on Site at all times the Contractor is performing Work which must be tested or inspected in-process, and must be readily accessible and able to return when off-Site.

The QC Plan will include the following information:

- the name of the QC Manager and qualifications establishing a proven capability to provide the specific services required for the Project;
- the name of QC testing agencies and their proven capability to provide the specific services required for the Project;
- a listing of QC staff (including names, qualifications and relevant experience) and their assigned roles and work scheduling in performing QC duties;
- a list of testing equipment to be used for the Work.

The QC Plan must include an organizational chart showing details of the flow of information, "hold" points, rectification of deficiencies and other relationships and responsibilities necessary to assure Project quality requirements are met.

The QC Plan should describe how the QC staff are allocated to Project requirements, the tasks assigned to each, and how their work will be coordinated.

The QC Plan must include a commitment to have every submission to the Ministry seeking review or approval to be accompanied by a QC Checksheet, showing that:

- (a) the contents of the submission have been reviewed by QC staff and signed off by the QC Manager; and
- (b) the submission, including any associated information, is
 - (i) fully compliant with all requirements of the Contract, or

- (ii) identifies any non-conformances and
- (A) provides an internal NCR, complete with proposed corrective action, or
- (B) requests a variance from the Contract provisions including any proposed revised standards to be applicable to the Work and any compensation offered to the Ministry should the variance be approved.

Without limitation, the Contractor's QC Manager shall:

- implement the Contractor's QC Plan;
- be responsible for measuring conformance with all aspects of the contract quality;
- stop work when materials, product, processes or submittals are deficient;
- develop inspection and testing plans for each element of Work;
- develop acceptance/non-acceptance reports and quality control checklists for each element of Work in sufficient detail to gauge conformance with all significant contractual requirements;
- undertake quality review activities on the Work and Quality Control activities delegated to any Sub-contractors to ensure that their work is compliant with all terms of the Contract.
- ensure the requirements for quality management (including an overview of how the QC Plan operates, the worker's role in it, contractual specifications for the Work, and work procedures) are known to, understood by, and adhered to by all workers on the Site;
- ensure that all QC checklists are signed-off by competent and responsible parties as close to the actual work as appropriate to the nature of the Work (e.g. by the actual worker or a foreperson for most work; by a Professional Engineer for false work erection; etc.)
- review, sign, and be responsible for all reports (materials and testing results);
- consult with field inspectors regarding materials and testing issues;
- receive notification by inspectors regarding deficiencies and ensure re-testing or rejection;
- provide monthly summary reports on testing and inspection results. Weekly summary reports may be required if the Ministry Representative finds the Contractor's QC to be lacking.
- initiate the non-conformance process when materials or product do not meet the required specifications and, inform the Ministry Representative of such non-conformance;
- consult with the Contractor Representative and initiate corrective action on non-conformance;
- respond to each Non-Conformance Report (NCR) issued by the Ministry Representative within the time specified in the NCR;
- schedule testing and inspection services in coordination with the Contractor's superintendent and forepersons;
- monitor QC testing and inspection procedures including those of the Subcontractors;
- work directly with the Ministry Representative on matters related to QC;
- ensure required approvals and permits from the Ministry Representative and others are obtained as and when required;
- verify that all testing equipment is properly maintained and kept in good working order;
- keep an organized filing system to ensure that quality records are easily accessible so that auditors can obtain necessary information;
- review issued for construction drawings, calculations, and shop drawings and ensure that all concerned Contractor staff have current versions of documents applicable to their part of the Work;
- notify the Ministry Representative of any changes in survey layout, location, line, grade, etc., for approval; notify the company principles of any issues that compromise the integrity or function of the Quality Management System, and
- provide an auditable trail for survey computations to the Ministry Representative.

101.02.03 QC Plan Submission Requirements (Contract-Specific) – Each QC Plan submission to the Ministry shall be accompanied by a QC checklist prepared by the Contractor demonstrating that the submission has been checked for compliance with contractual requirements.

(a) Full Submission

Unless otherwise specified in the Special Provisions, the Contractor's QC plan shall provide details of the means, methods, and frequencies of Quality Control measures for all elements of Work (whether payment Items or incidental) in the Contract.

(b) Partial Submission

On projects considered by the Ministry to be of low complexity and/or risk, and only where explicitly invoked by the Special Provisions, the Ministry will accept a partial QC Plan submission.

Notwithstanding any such reduced submission requirements, the Contractor remains responsible for QC for all aspects of the Work.

The Contractor's partial QC Plan submission to the Ministry Representative is only required to address the details of the following types of Work:

- Traffic Management
- Survey/layout
- Materials incorporated into the Work (concrete barrier, culverts, filter cloth, lock-blocks, etc.)
- Compaction (subgrade, embankments, granular aggregates, culvert backfill, etc.)
- Aggregate gradation
- Plus any other elements identified in the Special Provisions as a submission requirement.

The Contractor shall initiate such other Quality Control procedures as are necessary for ensuring the production of a quality product and may include them in the Quality Control Plan submission.

(c) For Both Full and Partial Submissions

The initial QC Plan shall be submitted to the Ministry Representative a minimum of seven (7) days in advance of the Project pre-construction meeting or fourteen (14) days in advance of undertaking any Work on Site, whichever is earlier, and must provide details of all elements of Work anticipated to be undertaken within the Contractor's first thirty (30) days on Site.

This initial submission must provide complete detail of the QC methodology to be employed with respect to the following elements, regardless of when those tasks are anticipated to commence:

- All elements listed in SS 101.02.02
- Traffic Management
- Site Safety

Detailed submissions for the balance of the Work must be received:

- (i) a minimum of fourteen (14) days prior to the anticipated first day of Work on each element covered by the submission; and
- (ii) not later than 60 days after the earlier of the pre-construction meeting and the date the Contractor first occupied the Site.

The initial submission, as well as any subsequent submission or revision, must be accompanied by the Contractor's QC checklist for Quality Management, verifying that the submission meets all relevant contractual requirements.

Improved procedures may be introduced after the start of work as necessary as amendments to the Quality Control Plan. All amendments require the written acceptance of the Ministry Representative.

The type and frequency of QC tests shall be established by the Contractor and shall be in conformance with the requirements of the Contract, including the minimum frequencies specified in the Special Provisions and/or Standard Specifications (for those listed items applicable to the Work), and the current acceptable practice of the industry.

When materials or equipment are specified by standard or performance specifications the Contractor shall obtain from suppliers or manufacturers independent test reports, or test certificates stating that the materials or equipment meet or exceed specified requirements. The Contractor shall provide back-up documentation of actual testing results upon request by the Ministry Representative.

101.03 Ministry Quality Assurance – The Ministry may prepare and implement a Quality Assurance Plan, based in part on the effectiveness and reliability of the Contractor's Quality Control Plan. The Ministry may also undertake random and systematic inspections of the Work and of the Contractor's QC documentation.

The purpose of the QA Plan and inspectional activities is to ensure that payment is made only for acceptable works in place and may be based on a limited amount of sampling and testing.

The Ministry Representative will monitor the Contractor's operations and the Quality Control program to assure that standards are being met and to assess what payments have been earned under the terms of the Contract.

Any instances of Unacceptable Work discovered may result in a Non-Conformance Report being issued to the Contractor.

The QA program activities will not relieve the Contractor of Quality Control responsibilities under the terms of the Contract.

The frequency of QA inspection and testing will generally be approximately zero to ten percent (0 – 10%) of the frequencies undertaken by the Contractor in its QC Plan and will initially be set at a level commensurate with the Ministry Representative's confidence in the anticipated effectiveness of the Contractor's QC program.

The Ministry Representative may increase or decrease the frequency of QA inspection and testing during the course of the Work, based in part on the actual effectiveness of the Contractor's QC Plan.

The Ministry's Quality Assurance may also include random and/or targeted quality audits.

101.04 Independent Quality Audit – The Ministry may have one or more independent auditors on the Project, supplementing the work of the Ministry QA staff. When utilized, the auditor(s) will report to the Ministry Representative and provide a systematic and independent assessment of whether or not the materials and Project

activities and related results comply with the Contract, the Contractor's QC Plan, and the Ministry's QA Plan. These independent auditors may be Ministry employees but will be assigned from resources which have not otherwise been involved with the Work.

The objective of the independent Quality Auditing is to have an impartial opinion on both QC and QA activities and be proactive in avoiding or reducing quality related issues by requiring the process of conformance verification to be systematic.

The auditor(s) will be allowed unrestricted access to the Site and all activities therein, to all testing and documentation of the work done by the Ministry, Contractor and their agents and suppliers.

101.05 Non-Conformance Reports (NCRs) – The Contractor shall and the Ministry may review the Work to determine conformance with the contractual requirements.

Non-conformances found shall be dealt with as follows.

101.05.01 Contractor's Internal NCR – Should the Contractor's QC reporting indicate that the Work is not in conformance, the QC Manager shall issue an internal Non-Conformance Report (NCR) to the Contractor, with a copy to the Ministry, including a response time.

The Contractor shall then respond to the QC Manager, with a copy to the Ministry Representative, with respect to the NCR, within the specified time, with proposed resolutions and corrective actions. The Contractor and/or the QC Manager may consult with the Ministry Representative on the resolutions but is not required to do so.

Payment for a Quality Management will not be affected by internal NCRs, as long as the issue is diligently pursued and resolved.

Payment for the Work itself may be withheld until the NCR issue is resolved.

101.05.02 Ministry-Issued NCR – Should the Ministry's QA reporting indicate that the Work is not in conformance, or that the Contractor has failed to address an internal NCR within a reasonable period of time, the Ministry Representative may issue to the Contractor a NCR, including a response time. Where appropriate, the Ministry may issue two NCR's related to a single issue – one for the product itself, and the other for the Contractor's failure to perform Quality Management.

The Contractor shall then respond to that NCR, within the specified time, with proposed resolutions and corrective actions.

The Ministry Representative will accept or reject the proposed resolution and corrective action proposal.

Assurance testing and inspection will be performed to determine if the corrective action has provided an acceptable product. Acceptance and rejection will continue

until the Ministry Representative determines that a quality product has been achieved.

A portion of the payment for a Quality Management may be withheld until the NCR issue is resolved or, in accordance with SS 101.07, may be withheld permanently.

Payment for the Work itself may be withheld until the NCR issue is resolved.

101.05.03 Opportunity for Improvement – Should the QA review indicate that the Work is not in conformance, but the variance is deemed minor by the Ministry Representative, the Ministry Representative may issue an Opportunity for Improvement (OFI) report.

The Contractor is encouraged to review the findings and undertake such modifications to the QC Plan and the work procedures as necessary to address the issue.

An OFI will not affect payment for Quality Management or for the Work itself.

101.05.04 Stop Work Order – Should the Ministry Representative determine that the non-conformance is significant, the Ministry Representative may issue a Stop Work Order.

101.06 Appeal – If the Contractor disputes the validity of any testing or inspection finding in an NCR, the Contractor may file an appeal with the Ministry Representative. An appeal under this SS 101.06 is available only for issues of assessing compliance of a material, product or process against the requirements of the Contract, and not any decision the Ministry may make regarding rectification of the non-compliance.

The Ministry Representative and the Contractor Representative will use all reasonable efforts to refine the area of dispute and to resolve the determination of conformance with the Contract.

If the Ministry Representative and the Contractor Representative cannot come to a mutually agreeable resolution, the Work that is the subject of the Non-Conformance Report shall be re-evaluated by an independent third-party, selected by the Ministry Representative in consultation with the Contractor.

If the appeal is of test results, the work in question shall be retested at a test frequency equivalent to twice that specified in the Contract or to such other frequency as may be mutually agreed between the Ministry Representative and the Contractor.

If the appeal relates to an inspection, the work in question shall be fully re-evaluated for strict compliance with all applicable terms of the Contract.

The specific retest or reinspection locations shall be determined by the Ministry Representative, in consultation with the Contractor and the independent third-party.

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If the appeal confirms the non-conformance determination, all appeal testing and inspection costs will be borne by the Contractor. If the appeal shows that the Work did in fact meet the requirements of the Contract, all appeal testing and inspection costs will be borne by the Ministry.

101.07 Payment – The Lump Sum Price bid for Quality Management shall be full compensation for all costs resulting from the Quality Management requirements set out in the Contract.

Payment will be made on a monthly basis prorated for the percentage of the total Work completed as determined by the Ministry Representative, subject to the Contractor being totally compliant with the requirements of this Section and with its own Quality Control Plan.

The Ministry Representative, in consultation with the Ministry construction manager following the issuance of a Ministry-issued NCR pursuant to SS 101.05.02 may deduct an amount from any monthly payment so computed, for any quality management work required but not satisfactorily undertaken during that month.

QUALITY MANAGEMENT

The Ministry Representative may also reduce the total Lump Sum payable by the value of any quality management work required but not satisfactorily undertaken during the Term of the Contract. The foregoing determinations will be made in the sole discretion of the Ministry Representative. When the value bid for the Quality Management Item is insufficient to cover the payment reductions, the Ministry Representative may recover the reductions as a back charge against a Site Modifications Item or any other Item in the Contract.

Inspection or testing by the Ministry Representative will be at the Ministry's cost. However, re-inspection or re-testing by the Ministry for repaired or replaced Work items that originally did not meet contract requirements shall be at the Contractor's cost.

Work that is deemed Unacceptable Work will not be eligible for payment.

The Completion Certificate will not be issued if there are any unresolved Non-Conformance Reports.

SECTION 108

SAMPLE FORMS

GENERAL

108.01 General – This Section provides sample copies of forms which the Contractor may be required to execute and submit to the Ministry in accordance with the Contract. Blank copies of these forms are available from the Ministry Representative.

STATUTORY DECLARATIONS

108.11 Major Works – Final Progress Payment and Interim Holdback Release – The form attached as Appendix 108-A (Ministry form H0394A) is used to meet the Statutory Declaration requirements of the Major Works General Conditions where the final progress payment or interim holdback release is to be paid by the Ministry to the Contractor.

108.12 Major Works, Minor Works and Operational Services – Holdback or Security Release – The form attached as Appendix 108-B (Ministry form H0394F) is used to meet the Statutory Declaration requirements of the Major Works or Minor Works General Conditions or the Operational Services Contract where final holdback amounts or security (in a form other than a bond) are to be released by the Ministry.

MINISTRY FORMS

108.20 Ministry Forms – Electronic copies of many Ministry forms (including the Statutory Declarations noted above and most forms with an “H-” prefix) are publicly available on the following website:

<https://www.th.gov.bc.ca/forms/search.aspx>

APPENDIX 108-A – MAJOR WORKS – FINAL PROGRESS PAYMENT



BRITISH COLUMBIA

Ministry of Transportation and Infrastructure

APPENDIX A – MAJOR WORKS FINAL PROGRESS PAYMENT / INTERIM HOLDBACK RELEASE

IN THE MATTER OF THE EVIDENCE ACT, 1996 RSBC c.124 AND IN THE MATTER OF CERTAIN DISBURSEMENTS TO BE MADE IN CONNECTION WITH THE CONTRACT

dated the _____ day of _____, _____ between:

HER MAJESTY THE QUEEN IN RIGHT OF THE PROVINCE OF BRITISH COLUMBIA, AS REPRESENTED BY THE MINISTER OF TRANSPORTATION AND INFRASTRUCTURE (the Province)

and:

(the Contractor)

pertaining to Project Number _____ Contract ID _____
the "Project" the "Contract"

I, _____, being a
(PRINT OR TYPE FULL NAME AND POSITION OR TITLE)

duly authorized representative of and agent for the Contractor on the above-referenced Project do solemnly declare and attest that,

- i. all of the Contractor's obligations under the Contract arising as of this date have been performed;
- ii. all accounts for labour, subcontracts, products, materials, construction machinery, equipment and any other indebtedness and obligations which may have been incurred by the Contractor in connection with the Project and/or the Contract up to the date the immediately preceding progress estimate have been paid in full or remain in good standing; and
- iii. I have the knowledge and information and have conducted all such enquiries necessary to enable me to make this statutory declaration.

I MAKE THESE SOLEMN DECLARATIONS conscientiously believing them to be true and knowing that they are of the same legal force and effect as if made under Oath.

DECLARED BEFORE ME AT THE

_____ of _____, in the Province of British Columbia, on this

_____ day of _____, _____

A Commissioner for taking Affidavits for British Columbia

Signature of Contractor's Authorized Representative

APPENDIX 108-B – HOLDBACK / SECURITY RELEASE



BRITISH COLUMBIA

Ministry of Transportation and Infrastructure

APPENDIX B – HOLDBACK/SECURITY RELEASE STATUTORY DECLARATION

IN THE MATTER OF THE EVIDENCE ACT, 1996 RSBC c/124 AND IN THE MATTER OF CERTAIN DISBURSEMENTS MADE IN CONNECTION WITH THE CONTRACT

dated the _____ day of _____, _____ between:

HER MAJESTY THE QUEEN IN RIGHT OF THE PROVINCE OF BRITISH COLUMBIA, AS REPRESENTED BY THE MINISTER OF TRANSPORTATION AND INFRASTRUCTURE (the Province)

and:

(the Contractor)

pertaining to Project Number _____ Contract ID _____
the "Project" the "Contract"

I, _____, being a
(PRINT OR TYPE FULL NAME AND POSITION OR TITLE)

duly authorized representative of and agent for the Contractor do solemnly declare and attest that,

- i. all of the Contractor's obligations under the Contract have been performed as of this date and;
- ii. I have the knowledge and information and have conducted all such enquiries necessary to enable me to make this statutory declaration.

I MAKE THIS SOLEMN DECLARATION, conscientiously believing it to be true and knowing that it is of the same legal force and effect as if made under Oath.

DECLARED BEFORE ME AT THE _____

of _____, in the Province of British Columbia, on this

_____ day of _____,

A Commissioner for taking Affidavits for British Columbia

Signature of Contractor's Authorized Representative

SECTION 125

VALUE ENGINEERING PROPOSALS

125.01 Definitions

125.01.01 VEP means a value engineering proposal which has been set out in writing by the Contractor to the Ministry in accordance with these guidelines.

125.01.02 Summary VEP means a brief summary of a VEP, including the anticipated cost and timing of developing the VEP, submitted by the Contractor to the Ministry.

125.01.03 Implemented VEP means a value engineering proposal which has been accepted and implemented by means of a Supplemental Agreement or Work Order.

125.01.04 Net Cost Savings means an amount determined by the Ministry on the basis of the Contract Price before implementation of the VEP minus the adjusted Contract Price determined after the implementation of the VEP less the following costs, which are subject to negotiation:

- (a) the Contractor's costs of developing the VEP;
- (b) the Ministry's costs of evaluating the VEP; and
- (c) the anticipated additional administration, engineering, quality management, and project management costs to the Ministry resulting from the implementation of the VEP.

125.01.05 Value Engineering Incentive means an amount calculated pursuant to written agreement between the Ministry and the Contractor, which will generally be equal to fifty percent (50%) of the Net Cost Savings realized by the Ministry due to the implementation of the VEP.

125.02 Guiding Principles for VEP Evaluation

125.02.01 To be considered, a VEP must be perceived by the Ministry to provide a tangible benefit to the Ministry, such as:

- (a) a reduction in the total cost of the Project; or
- (b) an improvement to the product which the Ministry expects will reduce the life cycle costs; or
- (c) a change in the Construction Schedule which will be beneficial to the Ministry; or
- (d) a reduction or transfer of risk which will benefit the Ministry;

and should result in a product which is functionally equal or superior to that which would result in the absence of the VEP.

125.02.02 A VEP should not:

- (a) impair the functionality and characteristics of the Project, including, but not limited to, the service life,

economy of operation, ease of maintenance, aesthetic considerations, preliminary engineering commitments, or design and safety standards.

- (b) be similar to a change in the Drawings or specifications under consideration by the Ministry for the Work at the time the VEP is submitted to the Ministry.
- (c) increase the anticipated life cycle costs of the Project.
- (d) seek a variance to the Contract for an existing non-conforming product. Non-conforming Work cannot be made acceptable using this VEP process.

125.02.03 To be eligible for a Value Engineering Incentive, a VEP must propose a change to the Work which meets the same constraints, assumptions and considerations as those which governed the original design of the Work, as it is defined in the Agreement. Where Changed Conditions, or other changes, have occurred and have already created the need for a Change To Work, a VEP which responds to this need will not normally be eligible for a Value Engineering Incentive, and in any case will be eligible only for a reduced incentive.

125.02.04 A VEP may involve:

- (a) a material or product substitution.
- (b) a change to the Work method.
- (c) a Work sequencing change, including changes to traffic routing or detour sequences, changes to structure or roadway construction sequences.
- (d) a change to the design or engineering of the Project which provides a marked departure from the product as defined in the Contract Document Package.

125.02.05 Any request by the Contractor for a design change, or a substitution of material or method, will only be considered if submitted as a VEP.

125.03 Value Engineering Proposal Submissions

125.03.01 Prior to presenting a VEP, the Contractor will prepare a Summary VEP and submit it to the Ministry for consideration.

125.03.02 The Ministry will evaluate the Summary VEP and will advise the Contractor whether or not the Ministry would be willing to consider, without obligation, the VEP described in the Summary VEP.

125.03.03 A VEP should contain the following information:

- (a) a statement that the proposal is submitted as a VEP.
- (b) a detailed description of the VEP and the manner in which the VEP should be implemented (by Supplemental Agreement or Work Order).

SECTION 125

- (c) a detailed description of the Items and quantities, Drawings, specifications, designs, Completion Date, Milestone Dates or other elements under the Agreement which would be changed, altered, modified or replaced by the implementation of the VEP.
- (d) plans and working drawings needed to analyze the VEP and determine the impact of the VEP on the Project.
- (e) An assessment of any quality assurance reviews that would be in addition to or of a different nature than those the Ministry would typically perform on the original design.
- (f) an itemized cost analysis of the Net Cost Savings.
- (g) the date, which must be not less than two weeks after the date of the VEP, by which the VEP must be implemented in order to maximize the cost reduction during the remainder of the Agreement.

125.04 Implementation of a VEP

125.04.01 For those VEPs approved by the Ministry, the Ministry Representative will prepare a Supplemental Agreement or a Work Order which will set out the Net Cost Savings, the Change to Work required realizing the Net Cost Savings, and the adjustment to the Contract Price(s) and payment terms resulting from the VEP, in accordance with these guidelines.

125.04.02 The Ministry will retain the right to utilize a VEP, or part thereof that has been accepted for the Project, on any other or subsequent projects without any obligation to the Contractor, unless the Contractor has provided written notice in the Summary VEP and the VEP regarding patented materials or processes involved in the VEP.

125.04.03 The Contractor shall provide and be responsible for all investigations and studies required for the development and implementation of the VEP.

125.04.04 Unless the parties agree otherwise, all Works associated with the VEP shall be executed in accordance with the terms and conditions of the Ministry's [Design Build Minor General Conditions](#) and the [Design Build Standard Specifications for Highway Construction \(inclusive of any amendments\)](#). The Contractor shall also provide additional insurance in accordance with [INS-172](#).

VALUE ENGINEERING - PROPOSAL GUIDELINES

including without limitation Professional Liability Insurance and Wrap-Up CGL insurance, for the Works associated with the VEP.

125.05 Associated Technical Circulars

125.05.01 The Contractor will, for all engineering works required for each implemented VEP:

- (a) cause their engineer(s) or other registered professional(s) to be responsible for all engineering and field reviews required for the Work included in, or affected by, the VEP;
- (b) comply with the requirements of Ministry Technical Circular T-06/09 "Engineer of Record and Field Review Guidelines", or any update or replacement thereof, including, without limitation, identifying Engineers of Record, Professionals of Record, Field Reviewers, and, if appropriate, a Coordinating Professional Engineer or Coordinating Registered Professional, and submitting all related letters of assurance to the Ministry Representative;
<https://www2.gov.bc.ca/assets/gov/driving-and-transportation/transportation-infrastructure/engineering-standards-and-guidelines/technical-circulars/2009/t06-09.pdf>
- (c) coordinate with the Coordinating Professional Engineer or Coordinating Registered Professional responsible for the original design as tendered by the Ministry to ensure that all aspects of engineering between the original design and the VEP are covered by the Contractor's engineers; and
- (d) provide to the Ministry Representative complete signed and sealed record drawings, electronically and in original hard copy, prepared in accordance with Ministry Technical Circular T-07/09 "Record Drawings Amended April 2010" or any update or replacement thereof.

<https://www2.gov.bc.ca/assets/gov/driving-and-transportation/transportation-infrastructure/engineering-standards-and-guidelines/technical-circulars/2009/t07-09b.pdf>

SECTION 135

CONSTRUCTION SITE SAFETY

135.01 General – This Section describes the responsibilities of the Contractor for safety on the Site.

Virtually every Ministry project will occur on a “multiple employer worksite”, by virtue of the Contractor retaining Subcontractors, by the Ministry having retained other independent contractors to work on or near the Site, by the Ministry Representative having forces on-Site, or through utilities, municipalities or other entities performing work on the Site.

Under the Workers Compensation Act and Regulation, each of these parties has responsibilities as an “employer” and may have responsibilities as a “prime contractor” for other employers working on the Site.

In any area where the operations of the various parties overlap or otherwise affects the others, one of those parties will be designated by the Ministry as the Prime Contractor, and, as laid out in this Section and the Workers Compensation Act and Regulation, will have responsibility for the overall co-ordination of safety in assigned Area(s) of Responsibility.

Where the Contractor takes on the role of Prime Contractor, it continues to have the responsibilities of the Contractor and adds those of the Prime Contractor, each as described in this Section.

Where the Contractor is not the Prime Contractor, but is working on a Site which is under the co-ordination of a Prime Contractor designated by the Ministry, they will be responsible for co-operating with the Prime Contractor as described in this Section.

135.02 The distinction between the geographic areas of the “Site” (where the Contractor is to build something under the terms of the Contract) and the “Area of Responsibility” (where the Contractor or another party is responsible for health and safety issues) is important.

135.03 Definitions – In this Section, except as otherwise expressly provided herein:

- (a) **Adjacent or Nearby Worksite** is a multiple employer workplace described by another Ministry contract, or in a written notice that may be issued by a Ministry Representative, and/or as described in a Notice of Project, for which another party has been engaged as prime contractor but which may be entered for the purposes of the Contract by the Workers, Suppliers and Subcontractors of the Contractor.
- (b) **Area(s) of Responsibility (“AoR”)** means one or more Multiple Employer Workplaces so designated in writing by the Ministry, comprised of the Site including additional area(s) outside the Site or excluding area(s)

within the Site, which is to be reflected in the Notice of Project.

- (c) **AoR-Specific Safety Plan** is a companion document to a Base Safety Plan which identifies specific hazards or conditions affecting an Area of Responsibility and designates protocols for mitigation and response for a specific project, consistent with the requirements of the Workers Compensation Act and the Regulation
- (d) **Base Safety Program** means the occupational health and safety program that the Prime Contractor will initiate and maintain within its assigned Area(s) of Responsibility, all as described in this Section.
- (e) **Employer** means an employer as defined in the Workers Compensation Act, and includes the Contractor.
- (f) **Incident** means an accident, Near Miss, or other occurrence which resulted in or had the potential for causing an injury, occupational disease, or property damage;
- (g) **Joint Committee** means the joint health and safety committee that shall be established and maintained by the Prime Contractor in compliance with the Workers Compensation Act and the Regulation (see also “Other Joint Committees”).
- (h) **Multiple Employer Workplace** means, in accordance with the Workers Compensation Act and the Regulation, a workplace where workers of 2 (two) or more Employers are working at the same time.
- (i) **Near Miss** means any event that occurred which had the potential to cause injury or property damage but did not.
- (j) **Notice of Project** means the notice of project described in Part 20 of the Regulation.
- (k) **Notice of Unsafe or Harmful Condition or Act** means the written notice that the Contractor will deliver to any Employer, or other persons, pursuant to SS 135.07, in the form set out in Appendix 135-A.
- (l) **Other Authorized Party** means any entity, other than the Prime Contractor or Contractor, who has a direct agreement with the Ministry and may therefore have contractual obligations to the Ministry and/or the Prime Contractor with respect to health and safety issues on the Site or Area of Responsibility. Such Other Authorized Parties may include but not be limited to, another construction contractor, a Ministry maintenance contractor, a utility company, a permittee or a licensee.
- (m) **Other Joint Committees** means additional committees that may be established by the Contractor in compliance with the Workers Compensation Act and the Regulation.

- (n) **Other Persons** means any entity, who is not the Prime Contractor, Contractor or an Other Authorized Party, who, for whatever reason or purpose, are on the Site or Area of Responsibility and do not have contractual responsibilities to the Ministry or the Prime Contractor. As examples, such Other Persons would include, but not be limited to, the public, the RCMP, and WorkSafe BC.
- (o) **Prime Contractor** means a prime contractor as defined in Section 13 of the Workers Compensation Act, and Part 20 of the Regulation.
- (p) **Qualified Coordinator** means the person, who meets the qualifications of a qualified coordinator described in the Regulation, and who is appointed by the Prime Contractor to discharge for the Area of Responsibility, the responsibilities of a qualified coordinator as described in the Workers Compensation Act and the Regulation.
- (q) **Qualified Person** means the person who meets the qualifications of a qualified person described in the Workers Compensation Act and the Regulation, and who is designated by the Contractor to discharge for the Site the responsibilities of a qualified person as described in the Workers Compensation Act and the Regulation.
- (r) **Regulation** means the British Columbia Regulation 296/97 entitled Occupational Health and Safety Regulation prescribed under the Workers Compensation Act, as amended from time to time.
- (s) **Safety Requirements** means the compendium of all requirements related to health and safety applicable to the Work, including without limitation, the Workers Compensation Act, the Occupational Health and Safety Regulation, the Mines Act, the Health, Safety and Reclamation Code for Mines in British Columbia, the Base Safety Program, the AoR-Specific Safety Plan(s), the Traffic Management Manual, the Contract, and this SS 135.
- (t) **Traffic Management Manual** ["TMM"] means the current edition of the Traffic Management Manual for Work on Roadways, as published by the Ministry and including all amendments, available for download from <https://www2.gov.bc.ca/gov/content/transportation/transportation-infrastructure/engineering-standards-guidelines/traffic-engineering-safety/trafficmanagementmanual/2020trafficmanagementmanual>
- (u) **Workers** mean the employees, workers, servants, agents or invitees of Employers, including of the Contractor, at the Site.
- (v) **Workers Compensation Act** means the Workers Compensation Act, R.S.B.C. 2019, c.1 as amended from time to time.

- (w) **Workers' Compensation Board** ("WCB" or "WorkSafeBC") means the Workers' Compensation Board constituted under the Workers Compensation Act.

135.04 Ministry

135.04.01 The Ministry may designate one or more Areas of Responsibility, each with a single Prime Contractor.

135.04.02 The Ministry may, in its sole discretion, appoint in writing:

- (a) the Contractor,
- (b) a party other than the Contractor, or
- (c) the Ministry

to be the Prime Contractor for one or more Areas of Responsibility and will notify the Contractor in writing of that appointment.

135.04.03 The Ministry may at any time, in its sole discretion and in writing, change or revoke any appointment of the Prime Contractor or designation of any Area of Responsibility.

135.04.04 Any designation or appointment made pursuant to SS 135.04.01 to SS 135.04.03 will include:

- (a) a description of the geographic Area of Responsibility, and
- (b) the time and date, or condition, that the designation or appointment comes into effect.

135.04.05 Receipt and retention by the Ministry of the information and documentation listed in this Section in no way constitutes approval of or authorization of the form, content or adequacy of such information and documentation on the part of the Ministry.

135.05 Prime Contractor

135.05.01 Acknowledgement of Assignment as Prime Contractor – Upon notification of such appointment, the Contractor shall deliver written confirmation to the Ministry of its acknowledgement to being the Prime Contractor for the Area of Responsibility.

135.05.02 Prime Contractor Responsibilities – The Prime Contractor shall, for each assigned Area of Responsibility:

- (a) ensure that all required Notices of Project have been prepared, accurately reflect the Area of Responsibility and Prime Contractor assignment, are submitted to the Workers' Compensation Board and are posted within the Area of Responsibility,
- (b) at the sole cost of the Prime Contractor, provide, post and maintain 2 (two) signs, in accordance with the specifications, which contain the informational messaging stipulated in Drawing SP135-01 of this Section, at locations satisfactory to the Ministry Representative,

(c) appoint and maintain a Qualified Coordinator, meeting the qualifications of a “qualified coordinator” as described in the Workers Compensation Act and the Regulation, and notify the Ministry Representative and all Employers working within the Area of Responsibility of any changes to the appointment. The duties of the Qualified Coordinator include:

- (i) to perform the functions of the “qualified coordinator” as described in the Regulation,
- (ii) to ensure the coordination of health and safety activities for the Area of Responsibility, including establishing Joint Committees and holding meetings and maintaining records thereof,
- (iii) ensure all Employers and Workers are informed of hazards created, and
- (iv) ensure that the hazards are addressed throughout the duration of the work activities,

(d) ensure all other Employers designate and at all times maintain not less than one Qualified Person to be responsible for the Employer’s health and safety activities connection with the Work, including administering the Safety Requirements,

(e) ensure periodic crew safety meetings are held, and minutes kept, to inform the Workers of any known or reasonably foreseeable risks on the Area of Responsibility and the actions to be taken to eliminate or minimize those risks,

(f) ensure appropriate first aid resources meeting Regulatory requirements are always present at the Area of Responsibility.

(g) ensure the availability of first aid attendants and that they are only assigned duties that allow them to respond immediately without any resulting risk to the workers or the public.

(h) ensure safe access for delivery of equipment and material.

(i) create, implement and maintain an AoR-Specific Safety Plan, and submit that Plan to the Ministry Representative for review and acceptance, a minimum of seven (7) days prior to Work commencing. The submission shall include a Quality Control check sheet, identifying that all items in this SS 135.05.02(i) have been addressed and are compliant with all Safety Requirements

The AoR-Specific Safety Plan must be provided to all contractors and Employers within the Area of Responsibility, and must explain and demonstrate how safety will be managed within that Area, without

limiting the generality of the foregoing, the Plan shall provide:

(i) the name of the person(s) designated to supervise the Works,

(ii) the name of the Qualified Coordinator,

(iii) the name(s) and certification level(s) of first aid attendant(s) and a means of summoning an attendant,

(iv) a system for identifying foreseeable and known hazards that may affect Workers and Employers,

(v) a system for communicating hazards to Employers and Workers and ensuring Employers notify the Prime Contractor of undertakings likely to create a hazard,

(vi) that the quality management systems of all Employers operating within the Area of Responsibility include provisions for the review of the Safety Requirements and the Contractor’s compliance with those requirements,

(vii) a set of construction procedures designed to address hazards and protect the health and safety of Workers,

(viii) emergency response procedures which address potential hazards, or impacts to the AoR, during regular and after hours work activities, including but not limited to:

- first aid response and coordination
- the provision of first aid supplies, equipment and facilities
- identification of, and directions to, medical aid, including the means of transportation of injured workers
- the method of notifying workers, Employers and others the notification methods for evacuating the AoR and identification of appropriate muster stations,

(ix) a drawing showing the project layout, first aid locations, emergency transportation provisions and the evacuation marshalling station,

(x) measures to ensure:

- regular inspections are carried out,
- all Workers and visitors receive an occupational health and safety orientation,
- regular site safety and toolbox meetings are held, including by other Employers,
- any Incidents are reported, investigated, and reviewed in accordance with SS 135.05.02(j) and SS 135.05.02(k)

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- (j) immediately notify the Ministry Representative of any Incident that the Prime Contractor becomes aware of that:
- (i) resulted in serious injury to or the death of a worker,
 - (ii) involved a major structural failure or collapse of a building, bridge, tower, crane, hoist, temporary construction support system or excavation,
 - (iii) involved the major release of a hazardous substance, or
 - (iv) was an incident required by the Regulation to be reported, or
 - (v) materially impacts the movement of traffic or presents a safety concern for the travelling public.
- (k) ensure any of the above Incidents, and any others resulting in medical aid, property damage in excess of five thousand dollars (\$5,000.00), or involving members of the public are reviewed and investigated, and that incident reports are provided to the Ministry Representative within 72 hours from the time of occurrence, and
- (l) when requested by the Ministry Representative, shall prepare and provide a statistical summary, on Ministry form H-1024 (link below), of all health and safety records applicable to the Area of Responsibility and covering the period requested by the Ministry Representative.
- <https://www.th.gov.bc.ca/forms/getForm.aspx?formId=783>
- (m) follow the Notice Process specified in SS 135.08 whenever an unsafe or harmful condition or act occurs in the Area of Responsibility or on an Adjacent or Nearby Worksite.

135.05.03 Base Safety Program – Notwithstanding the limitation relating to the requirement for initiating and maintaining an occupational health and safety program set out in the Regulation, the Prime Contractor will and submit to the Ministry Representative, a Base Safety Program a minimum of seven (7) days prior to the commencement of work, or as requested by the Ministry Representative.

The Prime Contractor will initiate and at all times maintain, at the Area of Responsibility, the Base Safety Program which will be designed to prevent injuries and occupational diseases within the contemplation of the Safety Requirements, describe how the Prime Contractor will meet the requirements of SS 135.05.02 and, without limiting the generality of the foregoing, will describe how the Prime Contractor will ensure that:

- (a) the requirements of an occupational health and safety program as stipulated in the Safety Requirements will

be satisfied, including the establishment and maintenance of a Joint Committee,

- (b) the activities of all Employers, Workers and Other Persons at the Area of Responsibility and relating to occupational health and safety will be coordinated,
- (c) reasonably practicable measures are taken to establish and maintain a system or process to ensure compliance with the Safety Requirements in respect of the Area of Responsibility, including ensuring regular health and safety inspections are carried out by each Employer,
- (d) the Prime Contractor, all Other Authorized Parties, all Workers, Employers, and Other Persons
 - (i) are made aware of all known or reasonably foreseeable health and safety hazards to which they may be exposed on the Area of Responsibility,₂
 - (ii) notify the Prime Contractor of any operations they plan to undertake that may create new hazards, and
 - (iii) are instructed in the terms of the Safety Requirements,
- (e) complete and accurate reports, records and documents relating to occupational health and safety activities at the Area of Responsibility, or related to the Work, will be established, maintained, reviewed and assessed on an ongoing basis,
- (f) safety equipment and apparel are properly used and worn by Workers and Other Persons, as required pursuant to the Safety Requirements, and
- (g) a first aid program in compliance with the Safety Requirements is established, implemented and maintained at all times on the Area of Responsibility.

135.05.04 The Prime Contractor will determine and control the manner in which the Base Safety Program is established, initiated, and maintained and the manner in which the instructions, directions, rules, policies and procedures contained therein are carried out.

135.05.05 Prime Contractor Obligations to Cooperate with Adjacent or Nearby Sites – If another party is the prime contractor in connection with another Adjacent or Nearby Worksite or Area of Responsibility, the Prime Contractor:

- (a) will continue to undertake and perform the obligations of a Prime Contractor in the Area of Responsibility at all times in accordance with the terms of the Safety Requirements,
- (b) will advise the other party in writing of the name of the Qualified Coordinator, and of any change in the Qualified Coordinator, designated to be responsible for the health and safety activities on or in connection with the Area of Responsibility, and any activities in the Adjacent or Nearby Worksite,

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- (c) will co-operate with the other party so as to accommodate any requests that may be issued by the other party to cooperate the health and safety activities on the Adjacent or Nearby Worksite, and
- (d) may request the cooperation and accommodation of prime contractors on Adjacent or Nearby Worksites to ensure the health and safety activities in the Area of Responsibility are compliant with the Safety Requirements.

135.05.06 When requested, the Prime Contractor shall provide to the Ministry Representative copies of any records mentioned in SS 135.05.

135.06 Contractor

135.06.01 The Contractor, by entering into the Contract, agrees to, for the purposes of the Workers Compensation Act and the Regulation, the designation of the Prime Contractor and the Area(s) of Responsibility made by the Ministry pursuant to SS 135.01.01 to SS 135.04.03.

135.06.02 The Contractor will, prior to commencement of the Work,

- (a) deliver to the Prime Contractor, in writing: the name of each of the Qualified Persons in connection with the Site, and
- (b) a description of the works the Contractor will be undertaking in the Prime Contractor's Area of Responsibility.
- (c) a copy of all Notices of Project.
- (d) the Contractor will, in connection with the Work and the Site:
- (e) observe, abide by and comply with the Safety Requirements, including providing any first aid resources, necessary for the Contractor's operations, which are beyond those provided through the Prime Contractor.
- (f) ensure all required Notices of Project have been submitted to the Workers' Compensation Board in accordance with the Workers Compensation Act and the Regulation, and that other agencies are notified as specified in the Safety Requirements
- (g) prominently post and at all times maintain on the AoR all Notices of Project in accordance with the Workers Compensation Act and the Regulation,
- (h) ensure that the person or persons to be appointed Qualified Person meet the qualifications of a "qualified person" as described in the Workers Compensation Act and the Regulation,
- (i) designate and at all times maintain not less than one Qualified Person to be responsible for health and safety activities of the Contractor in connection with the Work, including, without limiting the generality of the

foregoing, administering the Safety Requirements, as they applies to the Contractor,

- (j) deliver written notice to the Prime Contractor of any change in the appointed Qualified Person referenced in SS 135.06.02(a),
- (k) participate in the Joint Committee and ensure that the Contractor's Workers attend periodic safety meetings established by the Prime Contractor including the Prime Contractor's site safety orientation,
- (l) establish and participate in Other Joint Committees and periodic crew safety meetings to inform the workers of any known or reasonably foreseeable risks on the Site and the actions to be taken to eliminate or minimize those risks, and maintain minutes of those meetings,
- (m) establish and maintain at a location on the Site, or at a location off the Site that is satisfactory to the Prime Contractor, accurate records, including supporting documents, relating to the Contractor's implementation, operation, and maintenance of the Safety Requirements,
- (n) make copies of the Safety Requirements readily available at the Site,
- (o) provide and maintain the Site in a manner that ensures the health and safety of persons at or near the Site,
- (p) ensure that the Prime Contractor is given the information known to the Contractor that is necessary to identify and eliminate or control hazards to the health or safety of Workers and Other Persons at the Site,
- (q) ensure that the Prime Contractor, Workers, Employers and Other Persons are made aware of all known or reasonably foreseeable health and safety hazards to which they are likely to be exposed by the Work on the Site,
- (r) do everything that is reasonably practicable to establish and maintain a system or process that will ensure compliance with the Safety Requirements with respect to the Site,
- (s) immediately notify the Prime Contractor, Employers, Workers, Suppliers and Subcontractors of the Contractor, Other Authorized Parties, and Other Persons of any hazard created by virtue of overlapping or adjoining work activities of two (2) or more Employers and ensure that the hazards are addressed throughout the duration of such activities,
- (t) immediately notify the Prime Contractor and Ministry Representative of the occurrence of any Incident that:
 - (i) resulted in serious injury to or the death of a worker,
 - (ii) involved a major structural failure or collapse of a building, bridge, tower, crane, hoist, temporary construction support system or excavation,

- (iii) involved the major release of a hazardous substance, or
 - (iv) was an incident required by the Regulation to be reported, or
 - (v) materially impacts the movement of traffic or presents a safety concern for the travelling public.
- (u) ensure any of the above Incidents, and any others resulting in medical aid, property damage in excess of five thousand dollars (\$5,000.00), or involving members of the public are, reviewed, investigated and that incident reports are provided to the Prime Contractor within seventy-two (72) hours from the time of occurrence.
- (v) comply with any directive related to occupational health and safety matters issued by the Prime Contractor, and
- (w) provide to the Prime Contractor:
- (i) the name of the person designated to supervise the Contractor's Workers,
 - (ii) the name of the Qualified Person(s),
 - (iii) copies of safety meeting records and toolbox talks,
 - (iv) copies of worksite inspections,
 - (v) any and all notices, directives and inspection reports which have been delivered to the Workers' Compensation Board by the Contractor or which the Contractor has received from the Workers' Compensation Board, Ministry of Energy, Mines and Petroleum Resources, or other regulatory body.
 - (vi) a list of known site hazards and of any operations the Contractor is planning on undertaking that are likely to create a hazard, and updates as hazards change,
 - (vii) immediate notification of any incidents, including near misses,
 - (viii) incident investigation reports,
 - (ix) a written summary of the actions taken by the Contractor to coordinate, in conjunction with the Prime Contractor, the occupational health and safety activities of the Employers, the Workers, and the Other Persons within the Site,
 - (x) statistical summaries, on Ministry form H-1024 (link below), as necessary to respond to a request initiated by the Ministry Representative,
<https://www.th.gov.bc.ca/forms/getForm.aspx?formId=783>
- and

- (xi) such other information as the Prime Contractor may request.

135.06.03 Payment Conditional Upon Documentation – The delivery of any payments by the Ministry to the Contractor under the Contract is conditional on the prior submission to the Prime Contractor by the Contractor, of the documentation required under this Section.

135.07 Other Authorized Parties

135.07.01 All Other Authorized Parties shall comply with the health and safety related terms and conditions of their respective agreements with the Ministry, and upon request shall provide a copy of those terms and conditions to the Ministry Representative and the Prime Contractor.

135.07.02 Where the terms of their specific agreement appear to be inconsistent with the requirements of the Prime Contractor for any Area of Responsibility, the Other Authorized Party shall cooperate with the Prime Contractor and the Ministry in resolving any such inconsistency.

135.07.03 Should the Other Authorized Party's agreement with the Ministry not contain any health and safety coordination responsibilities, the Other Authorized Party shall be subject to any health and safety requirements established by the Prime Contractor.

135.08 Notice Process

135.08.01 Any Employer who becomes aware of an unsafe or harmful condition or act shall provide notice to the Prime Contractor in that Area of Responsibility.

135.08.02 If the Prime Contractor, in connection with the Area of Responsibility, determines, in its reasonable discretion, that:

- (a) any Employer or its Workers has failed to comply with the Safety Requirements, and
- (b) the failure creates or may create an unsafe or harmful condition in the Area of Responsibility that poses a danger of injury or death to Workers,

the Prime Contractor, may issue a Notice of Unsafe or Harmful Condition or Act to the applicable Employer, stipulating in reasonable detail the basis for the issuance of the Notice of Unsafe or Harmful Condition or Act, and the Prime Contractor, will deliver a copy of the Notice of Unsafe or Harmful Condition or Act to the Ministry Representative.

135.08.03 For the purposes of this SS 135.08, the terms "Employer" and "Workers" will not include the Prime Contractor's Subcontractors or Suppliers, or their respective employees, workers, servants, agents or invitees, as the Prime Contractor is directly responsible for the actions of those parties.

135.08.04 Upon receipt of the Notice of Unsafe or Harmful Condition or Act, the Ministry Representative will issue to the applicable Employer a Stop Work Order at the location

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CONSTRUCTION SITE SAFETY

in the Area of Responsibility that is specified in the Notice of Unsafe or Harmful Condition or Act.

135.08.05 The Ministry will be deemed to have relied on the terms of and reasons for the issuance of the Notice of Unsafe or Harmful Condition or Act, notwithstanding any subsequent investigation or inquiry of the matter by the Ministry which the Ministry may but will not be obligated to undertake.

135.08.06 If the Prime Contractor and the Employer referenced in SS 135.08.02 fail to agree on the reasons for the issuance of the Notice of Unsafe or Harmful Condition or Act, then within one day of the issuance of the Notice of Unsafe or Harmful Condition or Act, the directive of the Prime Contractor to rectify the failure to comply with Safety Requirements, will be immediately implemented.

135.08.07 When the failure to comply with the Safety Requirements stipulated in the Notice of Unsafe or Harmful Condition or Act has been rectified, to the reasonable satisfaction of the Prime Contractor, by the Employer to whom it was given to, the Prime Contractor, will immediately:

- (a) withdraw the Notice of Unsafe or Harmful Condition or Act by endorsing on a copy thereof confirmation that the failure to comply with the Safety Requirements has been rectified.
- (b) deliver the endorsed copy of the Notice of Unsafe or Harmful Condition or Act confirming the cancellation

thereof to the applicable Employer and to the Ministry Representative.

135.08.08 The Ministry Representative will issue a Resume Work Order on receipt of the endorsed copy cancelling the Notice of Unsafe or Harmful Condition or Act.

135.08.09 If the Prime Contractor is delayed in performing the Work as a result of being required to issue the Notice of Unsafe or Harmful Condition or Act and the Work is stopped pursuant to an Stop Work Order under SS 135.07, then the Completion Date will be extended by the number of days equal to the delay, as determined in the sole discretion of the Ministry Representative, and the Prime Contractor will not be entitled to any compensation, in damages or otherwise, including without limiting the generality of the foregoing, any claim for lost profits or other consequential loss, including business loss, as a result of or in connection with the issuance and operation of any Notice of Unsafe or Harmful Condition or Act or the issuance and operation of any Stop Work Order pursuant to this Section.

135.08.10 Notwithstanding any other term of this Section or of the Contract, the Ministry may withhold any payment due to the Contractor under the Contract if the Prime Contractor has issued, in connection with the Site, a Notice of Unsafe or Harmful Condition or Act in accordance with this Section, and the Ministry may continue to withhold such payment until such time as the Notice of Unsafe or Harmful Condition or Act has been cancelled and the Resume Work Order has been issued.

APPENDIX 135-A

PRIME CONTRACTOR'S NOTICE OF UNSAFE OR HARMFUL CONDITION OR ACT (the "Notice")

This Notice has been issued by the Prime Contractor to:

Name of Contractor: _____

Address: _____

The following unsafe or harmful condition or act was observed at the following location and time:

Description of unsafe or harmful condition or act: _____

Location: _____ Date: _____ Time: _____ AM/PM

The Contractor named in this Notice is required to correct the above referenced unsafe or harmful condition or act prior to performing any more work activity at the location identified above.

Prime Contractor: _____

Signature: _____ Date: _____ Time: _____ AM/PM

Receipt acknowledged by the Contractor:

Signature: _____ Date: _____ Time: _____ AM/PM

The Ministry Representative received a copy of this Notice on: Date: _____ Time: _____ AM/PM

On the basis of this Notice, the Ministry Representative will issue a "Stop Work Order" in connection with the unsafe or harmful condition or act as identified on this Notice.

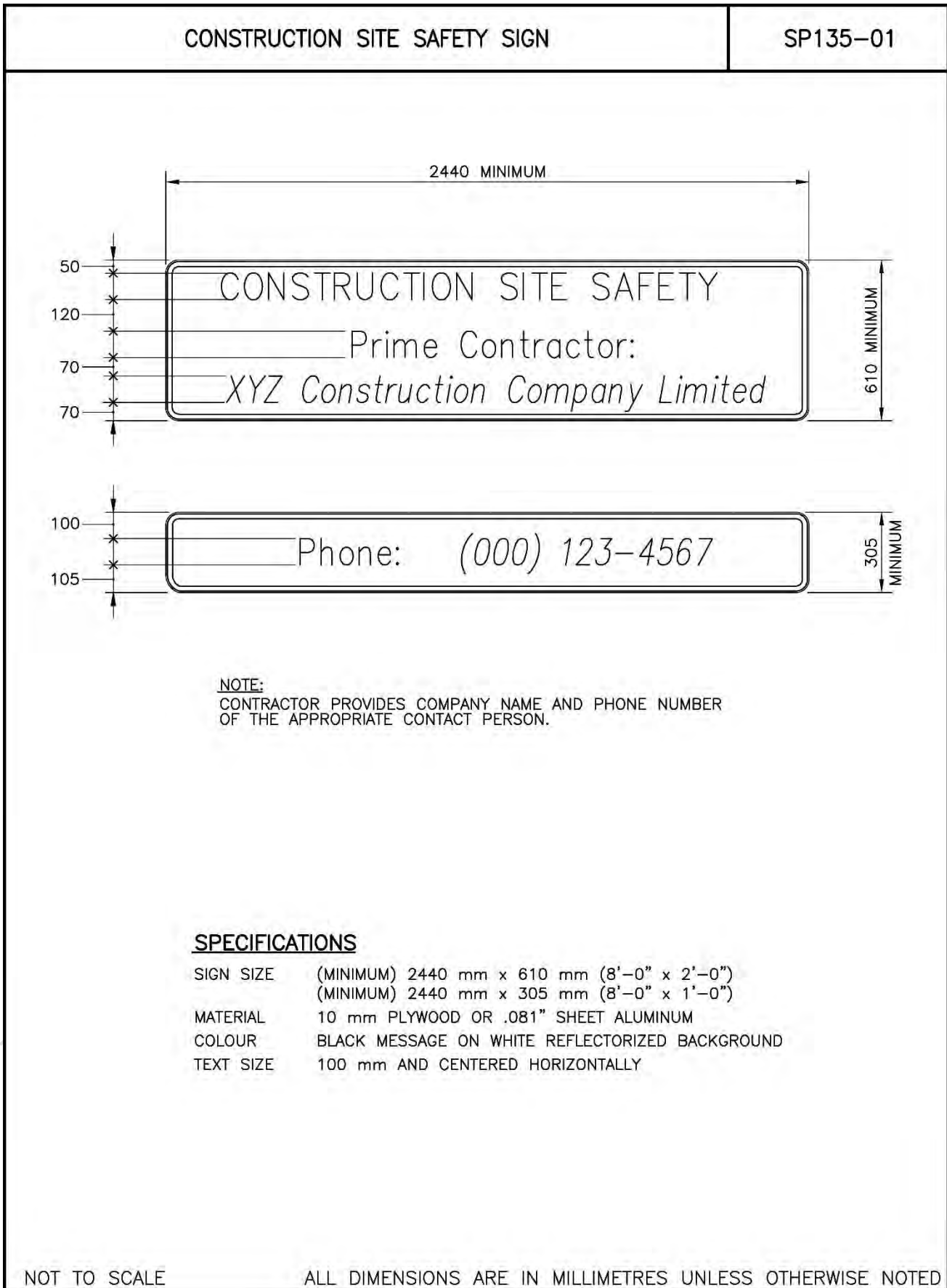
The Prime Contractor will, if satisfied that the unsafe or harmful condition or act has been rectified cancel and withdraw this Notice.

Date of cancellation/withdrawal: _____ Time: _____ AM/PM

Prime Contractor - Signature: _____

The Ministry Representative will issue an "Resume Work Order" when in receipt from the Prime Contractor of this Notice endorsed to indicate that the unsafe or harmful condition or act has be rectified.

DISTRIBUTION: Original: Contractor Copy: Ministry Representative File Copy: Prime Contractor



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GENERAL REQUIREMENTS FOR HIGHWAY, BRIDGE, AND MARINE CONSTRUCTION

145.01 General Requirements – This Section covers the general Work requirements for highway, bridge, and marine construction.

The standards and materials specifications contained in the Standard Specifications for Highway Construction govern the specific requirements for the Work. Special Provisions provide additional specific requirements for the Work and supersede the Standard Specifications where there is conflict.

Specifications, standards, regulations and codes of other authorities named in full or by acknowledged abbreviations in the Glossary of Terms of the Standard Specifications or the Contract Document Package shall be adopted by reference.

Such reference in whole or in part by the necessary alphanumeric designation from the latest edition or revision is intended to fully detail the requirements of the Work.

145.02 Reference Information – Reference information for the Work, in the form of General Reference Documents and Specific Reference Documents, is identified in the Tender and the Contract, and is made available under the conditions specified therein.

145.03 Most Recent Metric Edition – Unless otherwise specified, any standard, specification, publication or similar document shall be the most recent edition, using metric units of measure, published on or before the date the Work is tendered or otherwise first offered.

Where an earlier edition is specified in these Standard Specifications, the reference will be in a form that includes the year of the specific edition that applies.

As an example, ASTM C136 would mean the current metric edition (i.e. as of 2020-11-01: ASTM C136M-19); ASTM C136-14 would specifically mean the 2014 edition.

Unless otherwise specified or implied in the context, where a standard or specification has both metric and imperial editions, the metric edition shall apply to the Work.

145.04 Scope of Work – The scope of Work is described in the Contract. No warranty is given or implied that any or all of this Work will be required or supplied.

145.05 Quantities and Dimensions – Quantities, dimensions, lines and elevations as shown on the plans and Drawings are designed neat lines. Approximate quantities entered in Schedule 7 – Approximate Quantities and Unit Prices are quantities approximated using the lines and dimensions shown in the Contract Document Package and other relevant factors, and may vary from quantities derived

from the Drawings. Actual payment quantities will be measured, calculated, or otherwise determined in accordance with the Contract.

145.06 Unit Prices – Unit Prices in the Schedule of Approximate Quantities and Unit Prices shall be complete and contain all the elements of cost involved, provision of all things, superintendence and profits. The price shall stand independent of any other Unit Price Item on the Schedule of Approximate Quantities and Unit Prices.

145.07 Pre-construction (Post-Award) Meeting – The Contractor shall prepare the required documentation in accordance with the Contract, then advise the Ministry Representative, who will determine the pre-construction meeting's location and time and notify the Contractor. The meeting shall be held reasonably soon after Award and is intended to afford both parties an opportunity to establish relationships and to gain an enhanced understanding of the project and each party's plans and expectations.

145.08 Mobilization – Mobilization consists of the necessary Work and operations including, but not limited to, the movement of personnel, equipment, supplies, and incidentals to the Site, the establishment of offices, camps, and other facilities necessary to undertake the Work and all other Work Items and operations which must be initiated and finished as part of completion of the Work.

145.08.01 Payment for Mobilization – Where the Contract includes a payment Item for mobilization, the Lump Sum Price for mobilization includes any or all of the related expenses incurred for mobilization, demobilization and any re-mobilization not covered under the Work Items which must be initiated and finished as part of the Work. The Lump Sum Price for mobilization will be paid in increments as the Work progresses.

If the Contract does not contain a payment Item for "Mobilization", all costs associated with mobilization, demobilization and re-mobilization will be included in payment for Items having such costs.

Payment for mobilization, if applicable, will be made at the tendered Lump Sum Price as follows:

- (a) The Contractor will be paid any portion of the mobilization Lump Sum Price exceeding five percent (5%) of the Tender Price within sixty (60) days after the Actual Completion Date or on the final progress estimate (whichever occurs first).
- (b) That portion of the mobilization Lump Sum Price not exceeding five percent (5%) of the total Tender Price,

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will be paid to the Contractor according to the following breakdown:

- (i) Seventy-five percent (75%) of this portion when the Contractor has submitted, in an acceptable form, to the Ministry Representative each of the following that is required under the Contract
 - Construction Schedule,
 - Cash Flow Projection Schedule,
 - Traffic Management Plan,
 - Quality Control Plan,
 - Construction Environmental Management Plans,
 - Sediment and Drainage Management Plan,
 - Base Safety Plan and Area of Responsibility Safety Plans, as applicable,

and the value of the Work completed on bid Items other than mobilization exceeds two percent (2%) of the Tender Price.

- (ii) fifteen percent (15%) of this portion of the mobilization Lump Sum Price will be paid prorated on a monthly basis for the percentage of Work completed as determined by the Ministry Representative, but contingent upon the Ministry Representative receiving acceptable construction schedule and cash flow updates and all required or requested quality control records or reports, and
- (iii) the final ten percent (10%) of this portion of the mobilization Lump Sum Price when all completion documentation has been submitted by the Contractor as required by the Ministry, all equipment has been removed from the Site, and the Site has been cleaned up to the satisfaction of the Ministry Representative.

The payments from the Lump Sum Price, as set out above, will be full compensation for mobilization, demobilization and re-mobilization, regardless of the number of times the Contractor mobilizes.

145.09 Authority of the Ministry Representative – The Ministry Representative has the authority to accept or reject the Contractor’s Work in accordance with the conditions set out in the Contract. Until the Work meets the requirements of the Contract and has been accepted by the Ministry Representative, the Ministry is not obligated to make payment for Work done by the Contractor. The Ministry Representative shall take all necessary measurements and calculations to determine the final quantities for final payment.

145.10 Power, Telephone Lines and Other Utilities – Moving of existing power and telephone lines, poles and

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other utilities from their current to their final design locations, unless specified as being part of the Work, shall be performed by third parties at the Ministry’s expense.

If the Contractor’s operations require the temporary removal or relocation of lines or poles, such removal or relocation shall be entirely the responsibility of the Contractor and costs of any interim utility relocations shall be to the Contractor’s account. Utility owner(s) must be contacted prior to doing any utility work.

The Contractor shall be responsible for any damage to poles, lines or utilities caused by the Contractor’s construction operations, and shall repair and make good the same or bear the expense of making good and repairing the same.

The Contractor shall take the necessary precautions to protect existing utility services during the performance of the Work and shall be responsible for any losses or damages caused by the Contractor.

Locations of the utilities shown on the Drawings are approximate and may not be complete. The Contractor shall be responsible to ensure the locations of the utilities are complete and shall notify the Ministry Representative of any discrepancies between the Drawings and the Contractor’s findings.

145.11 Permits – The Contractor shall give all requisite notices in connection with the Work to the proper authorities and shall procure at the Contractor’s expense all permits, licences, etc., of every description necessary for the construction and completion of the Work.

The Contractor shall deliver or make available all original certificates to the Ministry Representative for all or any part of the Work for which such certificates may be required in connection with performing the Work.

145.12 Ministry Inspection of Work

145.12.01 General – All materials and each part or detail of the Work shall be subject to inspection by the Ministry Representative or designated inspector.

The Contractor shall be held strictly to the true intent of the Contract in regard to quality of materials, work practice and the diligent execution of the Contract.

The Ministry Representative shall be given access to all or any part of the Work, and to the preparation, fabrication or manufacture of the materials to be used. The Contractor shall furnish all information and assistance as is required to make a complete, true and detailed inspection.

The Ministry Representative may reject any part or parts of the Work which do not comply with the requirements of the Contract. The Ministry’s decision will be accepted as final.

Inspection may extend to all or any part of the Work and to the preparation, fabrication or manufacture of the materials

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to be used. The Ministry may station others on the Site to report on the progress of the Work and the manner in which it is being performed.

The Ministry Representative will report to the Contractor on materials furnished or Work performed that fails to fulfil the requirements of the Contract. Ministry costs for re-inspection and re-testing for repaired and replaced Work items that originally did not meet Contract requirements shall be at the Contractor's expense.

Inspection or its lack shall not relieve the Contractor from any obligations to furnish acceptable materials or to provide completed construction Work that complies with the Contract.

The Contractor shall furnish for the approval of the Ministry Representative, at proper times, all shop and installation drawings as requested by the Contract or which the Ministry Representative may deem necessary.

In accepting drawings submitted by the Contractor, the Ministry Representative will assume no responsibility as to the correctness of such drawings and the Ministry Representative's examination is only to ensure general compliance with the Contract.

145.12.02 Hold Points and Witness Points – Hold Points and Witness Points (as defined below) are required at critical times for the Ministry Representative and the Engineer(s) of Record (EOR)/Professional(s) of Record (POR) to review crucial work processes.

(a) Definitions

(i) Hold Points are verification points identified by the Ministry Representative beyond which Work may not proceed until:

(A) the Contractor notifies the Ministry Representative,

(B) the Ministry Representative, or the Professional of Record, performs mandatory verification; and

(C) the Ministry Representative gives notice to the Contractor to proceed.

(ii) Witness Points are identified points in the process where the Ministry Representative may review, witness, inspect methods or processes of Work. The activities however may proceed.

(b) Hold Points – Hold Points may be specified throughout these Special Provisions, the SP Appendices, the Drawings, and in the Standard Specifications.

The Contractor shall provide the Ministry Representative seven (7) days' notice in advance of a Hold Point to allow the Ministry Representative to provide adequate notice to the EOR.

GENERAL REQUIREMENTS FOR HIGHWAY, BRIDGE, AND MARINE CONSTRUCTION

Additional Hold Points may be added by the Ministry Representative at any time during the Contract, by written notice to the Contractor.

(c) Witness Points – Witness Points may be specified throughout these Special Provisions, the SP Appendices, the Drawings, and in the Standard Specifications.

The Ministry Representative will provide the Contractor with notice of any additional Witness Points as the Contract progresses. The Contractor will provide the Ministry Representative with one (1) clear days' notice of the commencement of the work requiring witnessing.

145.13 “Ministry's Instruction” and “Contractor's Declaration”

145.13.01 “Ministry's Instruction” – [“Ministry's Instruction” form \(H0202\)](#) will be issued by the Ministry Representative to the Contractor in order to:

- (a)** describe a portion of the Work which may not be covered specifically in the Contract.
- (b)** confirm instructions which may affect the method of carrying out the Work or payment.
- (c)** confirm verbal agreements made with the Contractor's representative.
- (d)** respond to the Contractor's representation on [“Contractor's Declaration” form \(H0203\)](#).

145.13.02 “Contractor's Declaration” – The Contractor is to issue the [“Contractor's Declaration” form \(H0203\)](#) to the Ministry Representative in order to:

- (a)** make a declaration of acceptance or objection to contents of the “Ministry Representative's Instruction”.
- (b)** formally present the Contractor's opinions, problems or requirements for clarification.

145.14 Protection and Restoration of the Work – The Contractor shall secure and protect the Site, all Work, and the surrounding infrastructure from loss, injury or damage until the issuance of the Completion Certificate.

If the Contractor fails to secure and prevent any loss, injury, or damage to the Work, the Contractor shall rebuild, repair, replace, and restore at its expense any portion of the Work so lost, injured or damaged.

The Ministry may reimburse the Contractor for any injury or damage caused by an event not foreseen and not reasonably foreseeable by the Contractor and beyond the control and not resulting from any fault or negligence of the Contractor including but not restricted to acts of God, acts of a Public Enemy, acts of the Ministry, extraordinary action of the elements and unavoidable slides.

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145.15 Materials Supplied by the Contractor – The Contractor shall provide all materials necessary for the orderly completion of the Work with the exception of the materials supplied by the Ministry as listed in the Special Provisions.

Further requirements for asphalt materials are contained in SS 502.

All materials supplied under the Contract shall:

- be new,
- conform to the requirements of the Contract, and
- be subject to inspection and approval by the Ministry prior to incorporation in the Work.

145.15.01 Ministry Inspection of Materials – Includes material certificates review, fabrication inspection, sampling and testing of materials.

The Contractor shall provide to the Ministry, material or mill certificates two weeks prior to shipping of the materials supplied by the Contractor. Such material or mill certificates shall include the material properties that demonstrate the material's compliance with the requirements of the Contract.

At the request of the Ministry Representative, the Contractor shall submit representative samples of materials at no charge to the Ministry.

The Contractor shall notify the Ministry Representative of the sources of materials supplied by the Contractor, and obtain the right for the Ministry to enter upon the premises of the material supplier or fabricator to carry out such inspection. Such notification shall be given sufficiently far in advance of materials delivery dates to enable the Ministry Representative to make inspection of the material at the source. Insufficient notice will be cause for the Ministry to reject the materials without compensation.

The Ministry will not be responsible for any delays to the Contractor's operations where the Contractor fails to give sufficient advance notification to the Ministry Representative to enable the Ministry to carry out the inspection before the scheduled shipping dates.

145.15.02 Materials or Components Sourced from Outside British Columbia – If materials or components to be incorporated into the Work are sourced from outside the Province then:

- (a) the Contractor will be charged for all travel time, transportation, and board and lodging costs incurred by the Ministry or its designate for out-of-province quality assurance and inspection of such materials and components,
- (b) the Contractor will be charged for the costs incurred by the Ministry in inspecting replacement materials and components or re-inspecting reworked materials and

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components which did not meet the requirements of the Contract, and

- (c) the Ministry will offset the costs incurred by the Ministry from any payments due to the Contractor

145.15.03 Not Used

145.15.04 Alternative Materials – Brands, species and manufacturer's names specified in the Contract are approved for use in the performance of the Work. No alternative materials will be considered unless approved in advance by the Ministry Representative in writing.

Savings as a result of using alternate materials will be evaluated using SS 125 Value Engineering Proposals.

145.15.05 Recognized Products List – The Ministry has reviewed a number of products for use in highway applications and publishes a "Recognized Products List", indicating which products are considered generally acceptable for particular purposes. Contractors are encouraged to review the list to aid in determining which products they will supply, but are cautioned that the products listed, although generally acceptable, may not meet Contract specifications.

The Categories "Proven," "Tentative" or "Approved" in the Recognized Products List are an indication of the product's record of use and purpose. Products in any category are "recognized" and may be used as long as the product meets Contract specific specifications.

Unless so specified in the Contract, Materials provided are not required to be amongst those on the Recognized Products List.

The Recognized Products list is available on-line at:

<https://www2.gov.bc.ca/gov/content/transportation/tr-ansportation-infrastructure/engineering-standards-guidelines/recognized-products-list>

145.16 Materials Supplied by the Ministry – Ministry supplied materials will be procured and paid for by the Ministry by purchase order to the Purchasing Commission, who on instruction from the Ministry Representative notifying of the Contractor's requirement for the material, shall arrange for release of the material to the Contractor in accordance with the purchase order.

Separate purchase orders will be issued for each class of material.

Materials ordered for a specific Site shall not be transferred to any other Site without the written consent of the Ministry Representative.

Ministry supplied and owned materials will be used only for Ministry Work.

145.16.01 Delivery Points – Each purchase order will specify points of delivery. The Contractor shall be

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responsible for all costs that may arise such as unloading from truck or rail car, hauling to the Site, stockpiling, dunnage, and reloading.

The costs to transport all materials to the Site will be incurred by the Contractor at its own cost and will be considered incidental and no separate payment will be made for this portion of the Works.

Should the Contractor request delivery be made to an alternative point other than the nearest specified point of delivery or in any manner of delivery other than specified by the Purchasing Commission, the additional cost for this delivery shall be charged to the Contractor's account.

145.16.02 Delivery and Storage of Materials – When the Ministry Representative advises the Contractor of the name of the supplier(s), the Contractor shall be responsible at no cost to the Ministry for:

- (a) determining requirements and ordering far enough in advance that work is not delayed.
- (b) acceptance of all materials; careful unloading and handling; hauling and delivery from delivery point to an interim storage facility and to the Site; storage and security; prompt turn-around of tank or shipping cars, trucks or delivery containers.
- (c) prompt payment of all demurrage or rental charges on cars, freight, handling, delivery, storage or any other charges arising after the initial receipt of material at the delivery point.
- (d) ensuring that proper accounting is kept of all materials received.

The Contractor shall notify the Ministry Representative of the materials delivery schedule and allow sufficient time for the Ministry Representative to check quantity and quality before the Contractor takes delivery. The Contractor shall not take delivery of any material without the presence of the Ministry Representative.

The Contractor shall report to the Ministry Representative any materials damage or discrepancies between the quantities received and the quantities shown on the bills of lading immediately upon materials arriving at the delivery point. If damage or deficiencies are not so reported, it will be assumed that the materials arrived in good order.

The Contractor shall provide a secure storage acceptable to the Ministry Representative for all materials supplied by the Ministry. The Contractor shall, in advance of receipt of the shipment of the Materials, notify the Ministry Representative of the storage facilities locations for their inspection and acceptance.

The materials shall be kept in the designated storage immediately upon receipt of the materials. The Contractor shall not remove any materials from the storage locations,

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except for incorporation in the Work, without authorization from the Ministry Representative.

The Contractor shall be solely responsible for replacement of any materials due to loss, damage, or improper use prior to acceptance into the final Works.

If asphalt materials are hauled directly from the delivery point for immediate application to the roadway, the material will be measured by mass or volumetrically by the Ministry Representative at the time of delivery. The Contractor shall ensure that an acceptable means of measurement is available.

145.16.03 Distribution of Materials – To control distribution of Ministry supplied materials and to facilitate accounting:

- (a) the Contractor shall arrange for delivery of the materials only after applying for and receiving authority to do so on "Authority to Arrange for Delivery of Construction Materials" form (H0313) which will be provided by the Ministry Representative. The requirements of the Contractor shall be indicated on Part "A" of the form. Approval from the Ministry Representative shall be indicated on Part "B" of the form. The Contractor may then arrange for delivery from the supplier. As delivery will be made only on receipt of the purchase order number and the serial number of the "Authority" form, the Contractor shall arrange with the Ministry Representative to immediately distribute appropriate copies of the "Authority" form.

Material suppliers will be required to notify both the Ministry Representative and the Contractor when materials are shipped, giving sufficient information to identify each and every shipment against an appropriate "Authority" form serial number and purchase order number.

- (b) The use of the "Authority" form shall not in any way relieve the Contractor of any responsibility imposed under SS 145.16.02.
- (c) If the Ministry Representative refuses to authorize the delivery of materials to the job, the Ministry Representative will send the refusal, with reasons in writing, to the Contractor.
- (d) The Ministry may, at the completion of the Work, if materials are delivered in excess of job requirements through conditions beyond the control of the Contractor, take over the surplus material or grant the Contractor the option of purchasing the material on Site. The transaction will be completed only after the Contractor obtains for the Ministry Representative, a supplier's credit note for the total cost to the Ministry of the excess material purchased f.o.b. at the delivery point.

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Upon completion of the Work, the Contractor will deliver all unused materials, which have been purchased by the Ministry, to a location as specified by the Ministry Representative and shall dispose of all empty materials packaging that are not required for their original purpose at the disposal site. No payment will be made for this portion of the Work.

145.17 Operation of Pavement Brooms on Highway – No mechanical pavement broom shall be operated on any highway without a rotating amber flashing light in operation. The light shall be mounted on self-propelled brooms and upon the towing vehicle of trailer type brooms.

Traffic Control is to be in accordance with the Traffic Control manual for Work on Roadways. In addition, on extremely winding narrow roads an additional shadow vehicle shall be used ahead of the broom with a C-076 “Sweeper Working” sign on the front of the vehicle and a flashing arrow in bar mode.

145.18 Supply and Maintain Facilities

145.18.01 Working Office – When specifically required under the Special Provisions, the Contractor shall provide a suitable working office with lock-up for the sole use of the Ministry. The office shall meet the requirements of Appendix A and be weatherproof, reasonably soundproof, and have ample window area and ventilation. Location of the office shall be determined by the Ministry Representative.

The Contractor shall regularly collect refuse and keep the office clean and properly maintained with heat and light.

The office and contents shall be for the use of the Ministry for the duration of the Work and may if necessary be used concurrently with the other inspection agencies.

Where service is available on-Site, through land-line or by satellite, the Contractor shall provide high-speed wireless internet for the Ministry office. If cellular coverage is not available or is poor at the office site, but land-line services are available nearby, the Contractor shall supply a private telephone line for the sole use of the Ministry Representative. Ministry’s long distance calls will be paid for by the Ministry. Where cell coverage is poor, the Contractor may provide a cell signal booster, instead of a land-line, if it provides a reliable signal.

Payment for the Ministry’s Site Office (when required) will be made at the Lump Sum price in the Contract.

Payment will include the provision of the office, furnishings, communications systems, maintenance throughout the construction period, and removal in its entirety.

Payment of the Lump Sum will be made as follows:

(a) 50% when the office is ready for occupancy;

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(b) 40% paid monthly, prorated to the percentage complete of the Work, and

(c) the remaining balance after the office has been removed and the Site cleaned up to the satisfaction of the Ministry Representative.

145.18.02 Testing Facility – When specifically required by the Contract, the Contractor shall provide the Ministry with a material testing facility with a minimum of 12 m² floor space either in the form of a small trailer or the portion of a large trailer. The testing facility shall include power, light, heat, ventilation, plus basic furniture to permit testing and preparation of test records. The facility shall be stable, reasonably quiet and lockable.

Payment for the Ministry’s Material Testing facility (when required) will be made at the Lump Sum price in the Contract

Payment will include the provision of the facility, furnishings, maintenance throughout the construction period, and removal in its entirety.

Payment of 50% of the Lump Sum will be made when the facility is ready for occupancy and the remaining 50% after it has been removed and the Site cleaned up to the satisfaction of the Ministry Representative.

145.18.03 Service for Ministry Facilities – Where the Ministry provides its own facilities on Site, the Contractor shall, for the duration of the Contract, provide light and heat and any other services requested by the Ministry Representative for same, and the Contractor will be compensated on a Force Account Basis.

145.18.04 Weigh Scale

(a) Except as provided in SS 145.18.04(b), the Contractor shall provide a truck weigh scale wherever measurement of materials by mass is required.

The weigh scale shall be an approved electronic weigh scale or platform type with remote balancing beam. The scale shall be of sufficient size and capacity to weigh, at a single pass, the largest, rigid-framed haulage vehicle to be used on the job and in any case shall be of not less than 13 t (tonne) capacity. The scale shall be capable of weighing all loads to within 20 kg of the true mass.

At the start of the Work, and as frequently as the Ministry Representative may deem necessary in order to ensure the accuracy, the scale shall be calibrated and checked at the Contractor's expense by an Inspector of Weights and Measures.

The remote balancing beam shall be enclosed in a sound, watertight, dust-free scale house, properly ventilated and insulated. Adequate heating must be provided when the shade temperature is below 16°C.

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The scale house shall be provided with full length windows facing the scale platform, and windows shall also be provided at either end, so that both the Contractor's scaleperson and the Ministry Representative's scale checker can at all times see vehicles to be weighed, not only on the scale but also approaching and leaving.

The front window shall be sufficiently large and low that the whole scale platform is readily visible from the scale house. Immediately behind the balancing beam, a bench or table of sufficient length shall be provided to permit both scaleperson and checker to sit abreast in a position in which they can both write on the table and view the electronic scale or weigh beam and scale platform. SS Drawing SP145-01 shows these requirements.

The Contractor shall supply and maintain, at no additional expense to the Ministry, portable washroom facilities at the weigh scale site.

Payment for the weigh scale and ancillary facilities shall be incidental to the Unit Prices of the materials being weighed.

(b) Where any aggregate is being paid by weight (i.e. per tonne) and the required quantity for the Item is less than 5,000 t, the Contractor will be permitted to use any scale that is certified and able to provide a printed ticket (e.g. loader bucket scale) in lieu of a stationary weigh scale.

Such alternative scales shall not be used to weigh aggregates into either concrete or asphalt mixes.

145.19 Weighing Materials – The Contractor shall supply, at no additional expense to the Ministry, a qualified scaleperson to carry out the necessary weighing which may be monitored by the Ministry. A weighing certificate prepared in triplicate, certified by the scaleperson, will be issued for each load. The Contractor will be paid only for such material as has been received at the point of delivery designated by the Ministry Representative.

The delivery receipts will bear the signature, initials or other approved marking of the issuing scaleperson and receiving checker.

If the Contractor has an automated scale and the following conditions are being met, the Contractor shall not be required to supply a scaleperson:

- (a) All weighed material is being delivered to a single job site. If the Contractor is delivering to two or more job sites at the same time, the Contractor shall supply a scaleperson as detailed in the first paragraph.
- (b) Truck operators do not require instruction.
- (c) There are no overloads, tarp covering, spillage difficulties, or any safety problems.

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- (d) The scale platform is maintained by the Contractor to be clean and free from loose material at all times.
- (e) All vehicles being scaled must be tared at least once a day.

In the event that any of the above conditions are not being met, in the opinion of the Ministry Representative, then the Contractor shall supply a qualified scaleperson as detailed in the first paragraph to carry out the necessary weighing and issuing of a weighing certificate, prepared in triplicate, for each load.

The Contractor will be paid only for such material as having been received at the point of delivery designated by the Ministry Representative.

The delivery receipts will bear the signature, initials or other approved marking of the issuing scaleperson and receiving checker and shall be submitted to the Ministry Representative within forty-eight (48) working hours from each load delivery for approval.

In the event the Ministry Representative determines a scaleperson is not required, all weighed vehicles must be tared by both the Ministry Representative and the Contractor together once a day. The Contractor must provide certified automated scales that will generate weigh tickets. The Contractor's scaleperson will be required to scale and record the weights of the material required for the project according to the requirements of the SS_502.04.04. The Contractor shall supply the Ministry Representative with the calibration record of the balance. The calibration record shall be done once a year or when needed.

At any time the Ministry Representative may direct that any loaded truck return to the scale and be reweighed in the presence of the Ministry Representative.

145.20 Not Used

145.21 Survey/Layout and Calculation of Quantities – In general and as detailed below, the Ministry will establish control line points for the Site, survey the original and excavated ground lines, and calculate quantities for payment. The Contractor shall lay out the Work, and assume full responsibility for the alignment, dimensions and elevations of each and every part of the work and their mutual agreement.

145.21.01 Ministry Responsibilities – The Ministry will provide a listing of any existing survey control line (primary) reference points to the Contractor.

The Ministry will survey the original ground line, and any subsequent material horizons, for determination of appropriate pay item quantities.

Where the Ministry has a pre-existing survey of the Site, the Ministry may use that survey as the original ground. If the Ministry elects to use such a survey, the Ministry Representative will notify the Contractor of such intent and

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give the Contractor an opportunity to view the cross-sections, contours, or Digital Terrain Model. Any areas of the survey or model that do not adequately reflect the Site at the start of construction will be identified by the Contractor or the Ministry and will be resurveyed by the Ministry prior to the Contractor starting work in any affected area of the Site.

Unless otherwise specified in the Contract, calculation of quantities will be on a neat line basis, as calculated from the design lines shown on the Drawings, adjusted to incorporate design changes and field-fits authorized by the Ministry Representative. Any material placed or excavated beyond the design lines is considered Unauthorized Work.

For items such as Type D excavation where the upper boundary of a cut cannot be determined until the completion of stripping, the design line will be revised to reflect the actual bottom of stripping line as determined by the Ministry's survey.

Quantities calculated from digital terrain models will be done using average-end-areas based on cross-sections extracted from the model or, at the Ministry Representative's option, by using prismatic volumes between surfaces in the model.

Where agreed in writing between the Ministry and the Contractor, alternate methods of determining payment quantities may be employed.

The Contractor may be required to adjust its construction activities from time to time to allow the Ministry's measurement work to proceed unencumbered. The Contractor shall not proceed with the next stage of any construction until the Ministry has completed measurements of the previous work.

Quality Assurance and checking of layout may be performed by the Ministry Representative.

145.21.02 Contractor's Responsibilities – The Contractor shall perform all layout of the work and all other survey work required to execute the Contract and construct the works in accordance with the Contract.

The Contractor shall be satisfied, before commencing any Work, as to the meaning, intent and accuracy of any control points, control lines and benchmarks established by the Ministry.

Should the Contractor discover or suspect any errors in any control points, control lines, benchmarks, and data provided by the Ministry, the Contractor shall at once discontinue the affected work until such errors are investigated by the Ministry Representative and, if necessary, rectified.

(a) Conventional Survey Layout – The Contractor shall be responsible for establishing all secondary control points and/or lines, all slope stakes, the establishment of line and grades for subgrade and the various granular

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aggregate layers, layout by line and grade of all structures, culverts, and underground utilities, and shall perform all other layout and measurement necessary for the proper execution of the Contract. Such layout shall include making incidental field adjustments, such as staking of embankments and culverts to match post-stripping ground lines and actual field drainage patterns.

The Contractor's survey layout shall conform, as a minimum, to the requirements outlined in Appendix 145-B – Survey Layout Services and Products.

(b) Machine Controlled Grading – The Contractor may utilize machine-controlled grading as a substitute for conventional grade staking under the following conditions:

(i) The equipment utilized shall be capable of meeting the Design vertical and horizontal tolerances.

(ii) The Ministry Representative may require the Contractor to revert to conventional staking methods at any point during construction if the machine-controlled grading is producing Unacceptable Work.

(iii) The Ministry will provide the Contractor the available electronic files of Design information in accordance with the terms specified in the Special Provisions without warranty with respect to the suitability for the purposes intended by the Contractor.

(iv) As a minimum the Contractor shall provide an orientation stake every 100 metres showing station, offset and grade.

(c) Calculations – All calculations necessary shall be performed by the Contractor and provided to the Ministry Representative at any time upon request. Upon request of the Ministry Representative the Contractor shall provide the Ministry Representative a copy of calculations prior to the start of earthwork operations.

(d) Accuracy of Layout and Construction – The staking of all works shall be of a sufficient accuracy and frequency for the Ministry to carry out its quantity measurements and quality assurance program.

On request of the Ministry Representative, the Contractor shall submit documentation to verify the accuracy of the layout work and the construction of the Work.

Unless tighter tolerance are required for the satisfactory performance of the Work, the Work shall be so laid out and constructed that the as-built locations of the Work

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shall not differ from the design positions by more than the following amounts:

- Elements and their tolerances as specified in Appendix 145-B
- Substructure elements – 12 mm horizontally and vertically
- Anchor groups – 6 mm in plan.
- The elevation of bearing seats shall not differ from true elevations by more than 3 mm.

Any deviation beyond the above-mentioned tolerances shall be satisfactorily corrected by the Contractor at the Contractor's expense.

145.21.03 Payment for Survey/Layout – Except as noted below, survey layout is incidental to the Work and no separate payment will be made.

The Contractor shall provide any additional or revised survey layout for any design change directed by the Ministry Representative and will be paid for such additional survey layout. Compensation will be determined in accordance with GC 38.00 and any approved payments will be made from the Provisional Sum for Site Modifications.

145.22 Access to Site – The Contractor shall provide access to the Site as may be necessary, unless otherwise specified in the Special Provisions. No separate payment will be made for the provision of any access roads or incidental works in connection therewith.

The Contractor shall make arrangements with the property owners adjacent to and around the terminal site for the use of access and for the use of any storage areas the Contractor may require. The Contractor shall acquire such access and storage rights at the Contractor's expense.

In the event of roads being temporarily closed to the travelling public, the Contractor shall, at the Contractor's expense, provide, erect and maintain all requisite barriers, fences or other proper protection and must provide, keep and maintain such patrollers, lights and danger signals as may be necessary, or as may be required by the Ministry Representative to ensure safety to the public and to the workers.

The Contractor, during the Contractor's operations, must also provide and maintain reasonable road access and egress to property fronting along or in the vicinity of the work under contract, unless other means of road access exists. The Ministry Representative will be the sole judge as to what may be deemed reasonable road access.

145.23 Accommodation of Traffic – The Contractor shall cause as little inconvenience as possible to the travelling public during the Contractor's operations and shall erect and maintain proper and adequate barricades, traffic signs, lights and other traffic control devices as may be considered

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necessary, in the opinion of the Ministry Representative, for the safety of both workers and public traffic.

All barricades, traffic signs, lights and other control devices shall be provided at the Contractor's expense, and shall be erected in accordance with, and otherwise conform to the standards as set out in the Ministry's Traffic Management Manual for Work on Roadways, Special Provisions, and Standard Specifications.

145.23.01 Maintenance of Traffic – The Contractor shall comply with the requirements of the Transportation Act and amendments thereto and such other Acts, By-Laws, or Regulations as are in force for the governing and regulating of traffic or use of any road upon or over which it is necessary to do work or transport materials. The Contractor must arrange to keep roads open to traffic at all times and must cause as little inconvenience as possible to the travelling public.

145.23.02 Road Closure Notification – Road Closures will be in accordance with SS 194 and the Special Provisions.

All costs related to the above shall be for the Contractor's account.

145.24 Alterations to Drawings and Plans – It shall be understood that the Drawings and Plans represent the nature and general layout of the work to be executed and not necessarily the works exactly as they will be carried out. The Ministry Representative will, without invalidating the Contract, be at liberty to make any reasonable alteration or to furnish any additional or amended Drawings which do not change the type of construction.

Payment for any such altered Work will be at the rates set forth in the Schedule 7 Approximate Quantities and Unit Prices.

145.24.01 Adherence to Drawings – The various parts of the Work shall be built in strict accordance with the furnished Drawings and with any supplementary Drawings which may be furnished during the progress of the Work as occasion requires.

Special Provisions, special specifications, Standard Specifications and the Drawings/Plans are to be considered as explanatory of each other, but should anything appear in one that is not described in the other, or should any discrepancy appear, no advantage shall be taken of such omission or discrepancy and the decision of the Ministry Representative as to the true intent and meaning of the Special Provisions, special specifications, Standard Specifications and the Drawings in such instances shall be final and binding, it being clearly understood that the Contractor is to execute all the works requisite for the perfect completion of each and all of the several parts of the Work according to the true intent and meaning of the Contract.

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On all Drawings, in case of any discrepancy between the figured dimensions and the scaled dimensions, the figured dimensions shall govern.

145.25 Use of Explosives – In using explosives the Contractor shall exercise the utmost care so as not to endanger life or property and, whenever directed, the number and size of the charges shall be reduced. Before doing any blasting the Contractor shall ascertain if there are in the neighbourhood any railway, power or other lines, water or other pipes, etc., which are likely to be in danger of being damaged. The Contractor shall give to the owner of such lines, pipes or other works timely notice of the intention to blast.

Whenever possible electric batteries shall be employed for simultaneous blasting.

The storage of explosives shall be in accordance with the [Explosives Regulations](#) of the Canada [Explosives Act](#) and with any requirements of the representative of the Explosives Regulatory Division within the Mineral Technology Branch of the Minerals and Metals Sector of the Federal Department of the Natural Resources Canada.

145.26 Work in Pits or Quarries – The Contractor shall be responsible for compliance with appropriate safety, environmental, forestry, agricultural and other applicable regulatory agencies for all work related to development, operation and reclamation of pit or quarry.

145.26.01 Ministry Pit or Quarry – When operating in a Ministry pit or quarry the Contractor shall comply with all provisions of the [Mines Act](#), the [Health, Safety and Reclamation Code for Mines in British Columbia](#), and/or the WCB [Occupational Health & Safety Regulation](#) which are applicable to the work undertaken.

Prior to entering any Ministry Pit or Quarry the Contractor shall provide a completed [Sand and Gravel/Quarry Notice of Work \(Form H1258\)](#) to the Ministry Representative for each Ministry Pit or Quarry the Contractor intends to enter in, at, or about to mine fourteen (14) days before, including any seasonal activation.

145.26.02 Contractor Pit Manager – The Ministry may, at its sole discretion, require that the Contractor, or a party other than the Contractor, take responsibility to ensure that the work in the pit or quarry is done in compliance with the requirements of the [Mines Act \(RSBC 1996\)](#) and the [Health, Safety and Reclamation Code for Mines in British Columbia \(B.C. Reg. 126/94\)](#). When the Contractor is so required, the Contractor must provide the name of a person, or persons, who will act as the Pit Manager to the Ministry Representative prior to the commencement of work. This person must possess the qualifications established by the regulations or code, and attend daily at the pit or quarry when it is in operation.

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When the role of Pit Manager is not the responsibility of the Contractor, the Contractor will co-operate with, and follow the direction of the acting or appointed Pit Manager.

145.26.03 Pit Supervisor – The Contractor shall appoint a properly qualified supervisor or shift boss in any Ministry pit or quarry where work is undertaken, in compliance with [Section 26 of the Mines Act](#), and ensure that the supervisor or shift boss is certified in accordance with Section 1.13 of the [Health, Safety and Reclamation Code for Mines in British Columbia](#).

145.26.04 Notice of Unsafe Condition – If the Contractor, in connection with the pit or quarry, determines, in its reasonable discretion, that any person, employer or worker has failed to comply with any safety regulation, Act, or code, or any applicable site practice or procedure that creates or may create an unsafe or harmful condition that poses a danger of injury or death, the Contractor will deliver a written notification of this failure to the Ministry Representative.

145.26.05 Use of the Pit or Quarry – When the Contractor is operating in a Ministry pit or quarry, the Contractor shall utilize the pit or quarry in accordance with the Ministry Pit Development Plan or obtain written approval from the Ministry Representative prior to deviating from the plan. Upon completion of the work, the Contractor shall re-grade all faces in the pit in accordance with the Ministry Pit Development Plan and/or an approved Pit Reclamation Plan.

Ministry pits or quarries may be used without charge, provided they are utilized, worked and maintained in accordance with the Standard Specifications and the Ministry's development plans. No warranty is given or implied that the source of aggregate material from the Ministry pits or quarries will meet requirements or requisite quantities.

The Contractor shall contact the Road and Bridge Maintenance Contractor a minimum of two (2) weeks before the date the Contractor anticipates the use of any Ministry pits or quarries.

The Contractor and the Road and Bridge Maintenance Contractor shall co-ordinate their activities and schedule usage of the pit among themselves and any other affected parties.

The Contractor, by entering an available source, will be deemed to have accepted all obligations, risks and costs associated with its use and security, just as if the pit or quarry was private.

No warranty is given that the source will meet the quality and quantity requirements. The Contractor, without any additional compensation, shall provide whatever selection, screening, crushing, mixing, blending or other processing of any kind may be required to produce aggregate meeting all

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requirements for its intended use, including but not limited to:

- specified aggregate gradation,
- fracture count,
- aggregate quality requirements such as but not limited to those set out in SS 202.04 and Table 501-D of SS 501 as applicable, and
- the specified properties of any products into which the aggregate is to be incorporated.

The Contractor shall comply with the Ministry pit development plan and the Ministry Representative's directions regarding the location of material extraction within the pit by the Contractor.

The Contractor's use of Ministry pits is limited to the extraction of previously unexcavated material from the bank, for the supply of materials for use on this Contract.

The Ministry's Road and Bridge Maintenance Contractor has certain rights and obligations with respect to some Ministry pits, and the Contractor shall coordinate its activities with those of that contractor, where applicable, and with any other users, to the satisfaction of the Ministry Representative and the Ministry's District Manager, Transportation.

No separate payment will be made for clearing, grubbing, disposal or relocation of stockpiles, debris or contaminated materials, or for any other costs of site preparation, pit development, or access, or for any delay or other cost arising from the use of Ministry pits by others, and all costs thereof shall be covered in the prices for the Items under which payment is provided for the applicable materials.

145.26.06 Materials Sourced from Ministry Lands but Processed Elsewhere – Where the Contractor elects to source raw materials from a Ministry pit or quarry, but process those materials on private lands:

- (a) The Contractor shall extract only as much raw material from the Ministry source as is necessary to produce the quantity of product required to be incorporated in the final Project, inclusive of staging and maintenance requirements, with a small additional contingency amount as approved by the Ministry Representative.**
- (b) Ministry-sourced materials shall not be used in any way to improve the private lands, including without limitation for such purposes as access development, maintenance, or stockpile site preparation, unless approved in advance by the Ministry Representative under any conditions the Ministry Representative may require.**
- (c) Any surplus raw or processed material remaining on the private lands at the end of construction shall:**

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(i) be returned by the Contractor to the Ministry source and stockpiled at a location determined by the Ministry Representative, all at the Contractor's expense; or

(ii) upon request by the Contractor and at the Ministry Representative's sole discretion, the Contractor may be offered the options of:

(A) of leaving the materials on the private land and providing a written agreement with the owner of the property which shall state that the Ministry will have free access to and exclusive use of the remaining materials for a period of twenty-four (24) months after the completion of the Contract; or

(B) purchasing the remaining material at a negotiated amount to be back-charged to the Contractor.

(iii) Where the volume of material remaining is less than the additional contingency amount determined pursuant to SS 145.26.06(a), the Ministry Representative will make the offers in SS 145.26.06(c)(ii). If agreement is not reached, SS 145.26.06(c)(i) will apply.

145.26.07 Private Pit or Quarry – Prior to supply of any material to the Site, the Contractor shall obtain and provide to the Ministry Representative written approval from the titled land owner, for the right to occupy and operate private pit or quarry, including appropriate permits.

Availability and use of the listed reserves or any other private aggregate source must be arranged by the Contractor. Prior to the removal of any aggregate material from an aggregate source, the Contractor shall provide a written notice from the owner authorizing the Contractor to remove aggregate material for use in the work.

145.27 Highway Right-of-Way – The Contractor shall confine its operations to those portions of the highway right-of-way and Licences to Construct indicated in the Contract as being available to the Contractor. Any land, property or water lot outside these boundaries required for construction yards, access roads, disposal or storage areas, or other operations of the Contractor, shall be acquired by the Contractor at no additional expense to the Ministry. The Contractor shall make arrangements for the use and compensation of such land with the titled land owner.

Work within the Site shall be confined to that necessary to construct the Project to the lines and grades specified. Excavation outside the design lines and grades to extract aggregates or for other purposes is prohibited unless authorized in advance by the Ministry Representative.

145.27.01 Not Used

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145.27.02 Disposal Site Outside the Right-of-Way – Waste, or other materials specified or designated by the Ministry's Representative to be disposed of, such as, but not limited to clearing and grubbing materials, fences, old culverts, old concrete and asphalt, etc., shall be removed from the Site to a disposal site provided by the Contractor. All disposal sites shall be the responsibility of the Contractor.

The Contractor shall ensure that the disposal site complies with all Federal, Provincial, Municipal and Regional by-laws and regulations prior to disposing of the material. Any fees incurred shall be paid by the Contractor. The Contractor must provide documented proof to the Ministry Representative for the use of the selected disposal site.

In addition, the Contractor shall ensure that the disposal site complies with all municipal and regional government by-laws and regulations and, when relying upon any exemption permitted under section 2(2)(d) of the Agricultural Land Reserve Use, Subdivision and Procedure Regulation, shall provide a written confirmation from the Regional District of their agreement that no permit is required.

145.27.03 Access Roads to Private and Public Lands – The Contractor shall not block or impede the use of any private access roads or driveways during the Work of this Contract without the consent of the Ministry Representative.

The Contractor shall schedule any anticipated closure of an access with the property owner to minimize impact on the owner.

145.28 Protection of Land Subdivision Monuments, Roads and Property

145.28.01 Protection of Subdivision Monuments – The Contractor shall protect, preserve, and keep uncovered all land subdivision monuments or property marks during its construction activities which may cause disturbance to them. At points where construction activities will cover or destroy any land subdivision monuments or property marks, the Contractor shall restore them afterwards to the satisfaction of the Ministry Representative.

145.28.02 Protection of Survey Monuments – The Contractor shall be responsible for the preservation during construction of all geodetic benchmarks, survey monuments and property markers on the right-of-way. The Contractor shall use, at no additional expense to the Ministry, a British Columbia Land Surveyor to replace any survey monuments destroyed or damaged as a result of the Contractor's negligence. At locations where construction work will cover or destroy such markers, the Contractor shall not move or remove them until written direction is received from the Ministry Representative.

145.28.03 Protection of Roads – The Contractor shall ensure all equipment working within, or hauling material to

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and from, the Site does not damage or deposit material onto any part of an existing roadway. Materials spilled onto the public roadways or driveways opened to public traffic shall be cleaned up immediately. The Contractor has the full responsibility to repair any damage to existing highways, local roads and driveways caused by its construction equipment and/or operations.

The Ministry manages a Province-wide seasonal strength loss program that, in conjunction with engineering assessments, determines the timing and degree of axle load restrictions which will apply to Ministry roads outside the Limits of Construction. Such restrictions do not apply within the Limits of Construction, but the Contractor is responsible for any damage caused by its operations within that area.

145.28.04 Protection of Railway Property – The Contractor shall be wholly responsible for any damage to railway trackage or property on the railway company's right-of-way resulting from highway construction operations. It shall be the Contractor's responsibility that all workers and persons employed by the Contractor or the Contractor's agents, or under the Contractor's control, shall use due care that no person or property is injured, and that no rights are infringed in performing the work. The Contractor shall compensate injured parties.

The Contractor shall be responsible for traffic control persons or other personnel, equipment or procedures required for the protection of the railway trackage or property. Where it is found necessary to divert, relocate or re-route utilities or other facilities and infrastructure during the course of operations, or where it is found necessary to make repairs to such installations as a result of contract operations, such re-routing or repair work shall be the responsibility of the Contractor, and shall be for the Contractor's account.

Any costs borne directly by or billed directly to the Ministry which relate to the above responsibilities will be charged to the Contractor without any surcharge.

The Contractor shall restore and clean all slopes between the highway and the railway track together with any fences, ditches and drains disturbed by the work to the satisfaction of the Ministry Representative.

The Contractor shall give a minimum of thirty (30) days advance notice to the railway company, or other timelines specified by the railway, for any work being done adjacent to, on or above the rail right-of-way. Notice will include submission of detailed plans and procedures, as specified in the Contract, for review and approval, as required, by the railway company. No Work will be done (construction or demolition) without approval from the railway and consent from the Ministry Representative.

145.28.05 Construction Stakes, Monuments, Benchmarks and Tide Gauge – The Contractor shall set,

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as the Ministry Representative may direct and as is necessary for the proper layout of the Work, monuments and bench marks of concrete or other material sufficiently permanent and rigid to remain intact during the duration of the Work. The Contractor shall particularly avoid destroying or disturbing such monuments or construction stakes and, should such destruction be unavoidable, shall notify the Ministry Representative in advance so that the points may be properly referenced and re-established.

145.28.06 Protection and Restoration of Property and Environment – The Contractor shall not enter upon private property for any purpose without first obtaining permission of the owner of that property and shall be responsible for the preservation of public property along and adjacent to the Work, and shall use every reasonable precaution necessary to prevent damage or injury thereto.

The Contractor shall be liable for all damages caused by fire started by the Contractor and shall, under no circumstances start fires without first securing the required permits and approval of authorities having jurisdiction even though it may be ordered or required to do such burning.

145.29 Haul Routes and Overloads – The Contractor shall be responsible for ascertaining that the Contractor's haul routes are acceptable to the authority having jurisdiction for highway loading.

Overweight or oversize vehicles are not permitted to use existing highway as a haul road for the hauling of construction materials, notwithstanding [Section 7.04 of the Commercial Transport Regulation](#), as amended from time to time. Where a portion of the existing highway is to be later reconstructed as part of the Work, the Ministry Representative may authorize overweight or oversize haul on that portion of the highway, under such conditions as the Contract or the Ministry Representative may establish.

Off-highway equipment may be permitted to cross the existing highway only at specific locations as approved by the Ministry Representative.

Once the crushed granular base, surfacing or pavement work has commenced on any portion of the new, reconstructed or relocated highway, no overloads will be permitted on that portion unless written approval is given by the Ministry Representative.

145.30 Health and Safety – See also SS 135 Construction Site Safety.

145.30.01 Occupational Health and Safety Program – The Contractor shall provide an Occupational Health and Safety Program in accordance with SS 135.

145.30.02 Worksite Hazards – The Contractor shall erect and maintain signs, fences and barricades at the ends of the bridges before they are opened to traffic and at other dangerous areas of the Project, as necessary to prevent

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access of vehicles and pedestrians to the unfinished bridge and other dangerous areas.

145.30.03 Equipment – All trucks and other equipment used by the Contractor whether owned or rented for use on the Project shall meet manufacture's specifications for its purpose.

All trucks used on public roads shall have Motor Carrier approval.

Hire of trucks and other equipment paid on a production rate basis constitutes a Subcontract.

145.30.04 Audible Back Up Warning Device – All haul vehicles shall be equipped with an automatic audible warning device which will activate whenever the vehicle is backing up. The warning device shall be clearly audible above the ambient noise level at a minimum distance of 6 m from the back of the vehicle

145.31 Unacceptable Work – The Contractor shall correct promptly any Unacceptable Work to the satisfaction of the Ministry Representative.

145.32 Temporary Excavation – Where temporary excavations are undertaken in the course of the Work, the Contractor shall be responsible for maintaining the stability of adjacent ground and all structures and other works thereon, and for the cost of any temporary works which it provides for this purpose for which payment is not expressly provided in the Contract.

145.33 General Sign Requirements

145.33.01 Contractor Identification Signs – The Contractor may supply and install contractor identification signs from the time of occupying the Site until the Actual Completion Date. The Contractor shall maintain and remove such signs and all costs of supply, installation, maintenance and removal shall be the responsibility of the Contractor.

The contractor identification sign shall display only the Contractor's company name and beneath it the word "Information" followed by a telephone number which directly connects any caller to the Contractor's personnel who are familiar with, and are currently active on the Project.

The sign will not be reflectorized, and the background colour will not be either red, yellow or orange. The sign dimensions will not exceed 2,400 mm horizontal and 1,200 mm vertical, and the text size (height) will not be less than 150 mm.

The contractor identification sign will be erected only in conjunction with the C-35 project identification sign, and will not be in advance of or at the same location as the C-35 sign. Where there is no C-35 sign, no contractor identification sign will be permitted.

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No contractor identification sign may be installed without prior approval of both the sign and the intended installation and location by the Ministry Representative who, in determining whether or not to grant such approval, will not necessarily be limited to the requirements set out above.

145.33.02 Prime Contractor Sign – When designated as the Prime Contractor, the Contractor shall provide “Prime Contractor” signs as specified in SS 135 Construction Site Safety.

145.33.03 Traffic Control Signs – See SS 194 Traffic Management for Work Zones.

145.34 Timelines for Review of Contractor Submissions – Timelines specified in the Contract for the Ministry to review Contractor submissions (for example: Quality Management Plans, Construction Environmental Management Plans, Bridge Erection and Demolition Plans) are, unless explicitly specified otherwise, target times.

The timelines specified are considered by the Ministry to be reasonable under most circumstances, and typically allow for one full review by the Ministry and third-party approval

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agencies (such as environmental Ministries and utilities), plus a short confirmatory review of a revised submission to ensure all issues raised have been adequately addressed.

However, actual time required for the Ministry and agencies to complete the review can vary significantly based on any number of factors, including without limitation:

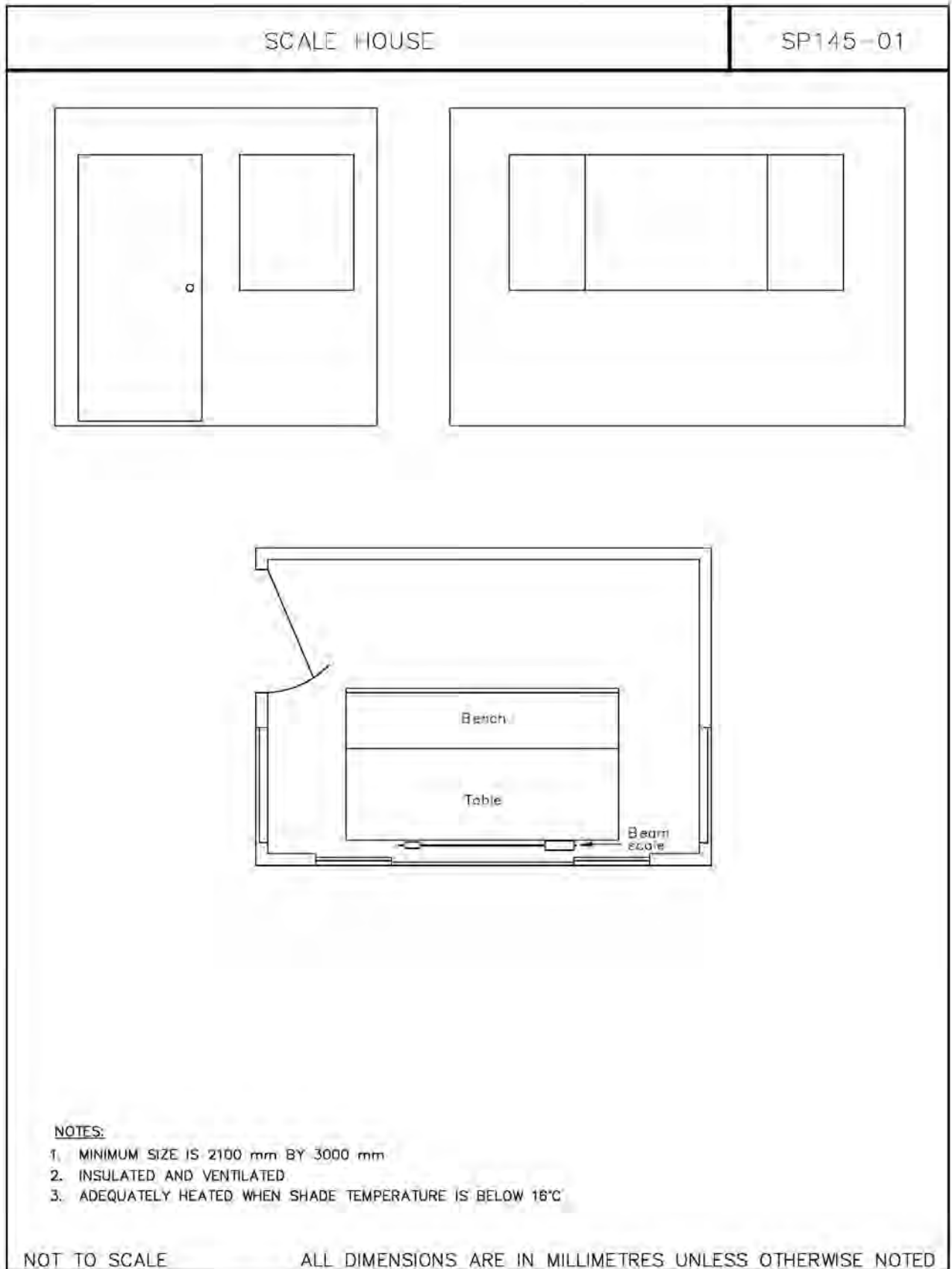
- (a) the quality of the original submission;
- (b) the number of times a document must be resubmitted before it is judged compliant;
- (c) the time taken by the Contractor while revising a submission;
- (d) the number of working days within the review period; and
- (e) the effect of any reduced staff availability during holiday and vacation periods.

The Contractor is encouraged to consult with the Ministry Representative, based on the anticipated schedule of the Contractor’s submissions, and plan its actual submission times accordingly.

APPENDIX 145-A

Ministry Office Requirements

ELEMENT	OFFICE TYPE		
	A	B	C
Lockable desk/chair	1	2	3
Typist desk/chair	-	-	1
Drafting table 1.2 m x 2.4 m /stool	1	1	1
Reference table 1.2 m ²	-	1	3
Conference table 1.2 m x 2.4 m	-	-	1
Additional chairs	2	2	9
Plan case or drawing storage unit	1	1	2
Filing cabinet (# drawers)	1 – 2 drawers	1 – 4 drawers	3 – 4 drawers
Shelving bookcase m ²	2	5	20
Fire Extinguisher for Class A, B and C fires	1 – 5 kg	1 – 5 kg	2 – 5 kg
Supply Activated Telephones and Lines (if cellular cover is unavailable or poor)	One (1)	One (1)	Three (3) on two (2) lines with external bell and switch
High Speed Internet	<u>Wireless covering entire office or one drop at each desk and drafting table</u>		
Toilet	On site	On site	In washroom, hot and cold running potable water and wash basin
First Aid Kit	On site	#1 Unit Kit	#1 Unit Kit
HVAC	As needed to maintain an indoor temperature of 20°C to 27°C year-round.		
Floor Areas – m²			
a) Entrance	-	-	5
b) Ministry Representative's office	12	12	12
c) Additional Staff office	-	8	20
d) Wash room	-	-	As required
e) Store room	-	-	5



Appendix 145-B

Survey Layout Services and Products¹

<u>Survey Layout</u>	<u>Maximum Interval</u>	<u>Product</u>	<u>Tolerances²</u>
<u>Right-of-way³</u>	<u>At each point of deflection and at sufficient points between as to be continuously visible.</u>	<u>Stake showing station and offset, or flagging.</u>	<u>Sufficient accuracy to prohibit encroachment into adjoining properties.</u>
<u>Clearing and Grubbing</u>	<u>Same as Right-of-Way.</u>	<u>Same as Right-of-Way.</u>	<u>Sufficient accuracy to prohibit encroachment into adjoining properties.</u>
<u>Grading - Slope Stakes</u>	<u>10 m in rock cuts; 20 m in all other cases. For Machine Controlled Grading Projects – only an orientation stake is required every 100 m</u>	<u>One slope stake each side, at top of cut or bottom of fill, showing station, offset, vertical dimension to subgrade, and slope, plus cut/fill transition stake. Non-standard ditches will be staked separately. An additional slope stake, where applicable, at the top of a rock cut after the removal of overburden.</u>	<u>Up or down chainage: ±0.30 m Offset from \mathcal{C}: ±30 mm Vertical: ±25 mm</u>
<u>Grading – Subgrade</u>	<u>20 m. For Machine Controlled Grading Projects – only an orientation stake is required every 100 m</u>	<u>One stake at each side of the subgrade, showing station, offset and grade at the stake location, one at each break point, and one at centreline.</u>	<u>Up or down chainage: ±0.30 m Offset from \mathcal{C}: ±30 mm Vertical accuracy per SS 201.47</u>
<u>Top of Sub-base</u>	<u>20 m. For Machine Controlled Grading Projects – only an orientation stake is required every 100 m</u>	<u>One stake at each side of the sub-base course, showing station, offset and grade at the stake location, one at each break point, and one at centreline.</u>	<u>Up or down chainage: ±0.30 m Offset from \mathcal{C}: ±30 mm Vertical accuracy SS 202.25.04</u>
<u>Each Base Course</u>	<u>20 m. For Machine Controlled Grading Projects – only an orientation stake is required every 100 m</u>	<u>One stake at each side of the base course, showing station, offset and grade at the stake location, one at each break point, and one at centreline.</u>	<u>Up or down chainage: ±0.30 m Offset from \mathcal{C}: ±30 mm Vertical accuracy per SS 202.26.04</u>

¹ This table shows layout details for general situations; particular circumstances may require more or less staking.

² Staking tolerances also represent the tolerances for construction of the Work. Any Work constructed outside of these tolerances constitute a “change” and must be approved by the Ministry Representative and shown on the Record Drawings. See Special Provision clause titled “Marked Up Drawings”.

³ The right-of-way limit will be laid out only where there is work, including utility relocation, to be performed within three metres of it.

<u>Survey Layout</u>	<u>Maximum Interval</u>	<u>Product</u>	<u>Tolerances²</u>
<u>Final Base Course only</u>	<u>10 m.</u> <u>For Machine Controlled Grading Projects – only an orientation stake is required every 100 m</u>	<u>"Blue tops"⁴ at each break point across the base course surface, or final grade stakes.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u> <u>Vertical accuracy per SS 202.26.04</u>
<u>Culverts</u>	<u>Inlet and outlet.</u>	<u>One stake at each end of the culvert, plus an offset line, showing invert elevation.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u> <u>Vertical: ±20 mm</u>
<u>Storm Drainage, Subdrain, Watermain or Sanitary Sewer</u>		<u>Stakes showing locations of manholes, catch basins and other structures, and invert locations of pipe inlets and outlets.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u> <u>Vertical: ±20 mm</u>
<u>Retaining Walls</u>	<u>Not more than 10 m, and at alignment changes.</u>	<u>One stake showing control line location and either the elevation at the top of wall or the elevation at the bottom of footing excavation.</u>	<u>As defined in the Special Provisions.</u>
<u>Paving</u>	<u>100 m on tangent, 20 m on curves, and at each deflection point.</u>	<u>Reference points.</u>	<u>N/A</u>
<u>Concrete Barriers</u>	<u>Same as paving.</u>	<u>Same as paving.</u>	<u>Offset from \mathcal{C}: ±30 mm</u>
<u>Signs</u>	<u>Stake at each sign location.</u>		<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u>
<u>Curb and Gutter</u>	<u>10 m and at alignment changes.</u> <u>Curb returns: 5 m or at quarter points, whichever is less.</u>	<u>Offset hub and nail with cut/fill to gutter grade.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u> <u>Vertical: ±10 mm</u>
<u>Median/Island Curb</u>	<u>Continuous.</u>	<u>Paint line at face/edge of curb.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u>
<u>Sidewalk or Path</u>	<u>5 m and at alignment changes.</u> <u>No layout required if parallel to curb.</u>	<u>Offset stake or nail with cut/fill to grade.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u> <u>Vertical: ±15 mm</u>

⁴ "Blue top" - a stake driven so that its top surface is at the required elevation for the finished surface of the work.

<u>Survey Layout</u>	<u>Maximum Interval</u>	<u>Product</u>	<u>Tolerances²</u>
<u>Electrical Pedestal, Junction Box, Sign Structure, and Detection Loops</u>	<u>Per installation.</u>	<u>Offset stake or nail with cut/fill to top of pedestal, or junction box; with additional stake to show orientation.</u> <u>Survey crew will lay out locations of cross walks, lane lines and stop bars; Contractor to derive location of loop.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u> <u>Vertical: ±15 mm</u>
<u>Irrigation Preducting</u>	<u>Per installation</u>	<u>One stake at each end of the duct, and offset line showing lowest elevation.</u>	
<u>Pavement Marking (temporary and permanent)</u>	<u>At all alignment break points;</u> <u>100 m maximum on tangent;</u> <u>20 m maximum on curves;</u> <u>10 m maximum on gores.</u>	<u>Reference points.</u>	<u>Up or down chainage: ±0.30 m</u> <u>Offset from \mathcal{C}: ±30 mm</u>

SECTION 165

PROTECTION OF THE ENVIRONMENT

165.01 General – This Section covers the general and specific provisions for the protection of the environment under the direction of the Ministry Representative, in cooperation with any Federal and/or Provincial Environmental Agency representative, Ministry environmental staff, or an Environmental Monitor, as designated in the Special Provisions.

165.01.01 Intent of Specifications – These Specifications give the Ministry or its Agent the right to exercise control over environmental aspects of the Work. The Contractor shall adhere to specific instructions if the work may result in an adverse impact on the environment.

The Ministry will determine the significance of environmental impacts in consultation with Environmental Agencies and Appropriately Qualified Professionals. The Ministry reserves the right of approval over the general methods employed by the Contractor in the performance of the Work, but only insofar as they may affect compliance with Environmental Approvals and the Special Provisions, and the protection of aquatic and terrestrial resources, the health and safety of public, and protection of socio-community resources and features.

These specifications are for the protection of the environment and shall be given such interpretation as will secure this intent.

165.01.02 Definitions – For the purposes of this Section, the following general terms are defined as follows:

- (a) **“Appropriately Qualified Professional”** (AQP) means an applied scientist or technologist specializing in a relevant applied science or technology including, but not necessarily limited to, archaeology, agrology, forestry, biology, engineering, erosion and sediment control, geomorphology, geology, hydrology, hydrogeology or landscape architecture. An appropriately qualified professional must be recognized in British Columbia with the appropriate professional organization, registered and in good standing, and acting under that organization’s Code of Ethics and subject to disciplinary action by that organization. He or she must also be someone who, through demonstrated suitable education, experience, accreditation and knowledge directly related and relevant to the level and responsibilities of the particular matter, may be reasonably relied on to provide advice within his or her area of expertise and experience.
- (b) **“Construction Environmental Management Plan”** (CEMP) means a plan, written by an AQP, that describes the environmental requirements of the Work and outlines the approach the Contractor will undertake to mitigate impacts. The CEMP describes the specific

commitments that the Contractor will adhere to, with respect to environmental protection, throughout the duration of the Work. It encompasses all the elements that may be addressed separately in mitigation, monitoring and action plans.

- (c) **“Deleterious Substance or Material”** means the same as ‘deleterious substance’ as defined in [Section 34\(1\) of the Canada Fisheries Act](#). In brief, it means any substance harmful to fish or fish habitat and/or Environmentally Sensitive Areas, and includes sediment and other toxic substances.
- (d) **“Designated”** shall mean designated in the Special Provisions or by the Ministry Representative.
- (e) **“Environment”** refers to the physical, biological, social, spiritual and cultural components that are interrelated and affect the growth and development of living organisms. The term “environment” in these specifications shall include socio-community issues and resources.
- (f) **“Environmental Agencies”** shall mean the current, seceding or successional appropriate regulating branches of the Federal and Provincial government agencies responsible for the management and protection of the Environment and human resources and any issuance of environmental permits, approvals or licences.
- (g) **“Environmental Approval”** is defined as the written authority issued by a government agency that allows Work that otherwise may not be permitted by law or which is not defined in law. An Environmental Approval is a broad, generic term referring to informal or formal authorization for actions that may have an adverse effect on the environment.
- The term “approval” can include related forms of authorization such as permits and licenses.
- (h) **“Environmental Monitor”** shall mean an Appropriately Qualified Professional hired by the Contractor to ensure the Contractor's compliance with the environmental protection aspects of Standard Specifications, Special Provisions, legislation, permits and approvals, and to advise the Contractor and Ministry Representative on environmental issues or concerns.
- (i) **“Environmental Procedures”** are Work specific written procedures documenting environmental protection measures to be used to mitigate potential environmental impacts from activities in Environmentally Sensitive Areas.

- (j) **“Environmentally Sensitive Areas” (ESAs)** shall mean area(s) requiring special management and attention to protect resources, habitat or species (which includes and is not limited to watercourses, Designated sensitive areas and rare and endangered ecosystems, fish and fish habitat, vegetated areas containing rare and endangered flora/fauna, vulnerable aquifers and archaeological, heritage and cultural resources). ESAs are Designated in the Special Provisions, Environmental Approvals, legislation, and/or by the Ministry Representative.
- (k) **“Environmental Timing Window”** shall mean the period that the natural or human environment is likely less susceptible to adverse impacts. Works may be restricted outside of these timing windows. Environmental Timing Window is synonymous with least risk timing window. Environmental Timing Windows are recommended by the Contractor’s AQP based on professional experience, federal, provincial, and regional guidelines, legislation, and input from Environmental Agencies, unless otherwise specified in the Special Provisions.
- (l) **“Fisheries Sensitive Zone”** is defined as the instream aquatic habitats, as well as out-of-stream supporting habitat features such as side channels, wetlands, and vegetated riparian areas adjacent to these features, that support fish.
- (m) **“Fishery Timing Window”** refers to the time period(s) of reduced risk for important commercial, sport, and resident fish species, based on their life histories. The Fishery Timing Window is the time of year during which there are no fish eggs or alevins present in the substrates of local Watercourses, and the period when fish migration (juvenile out-migration and adult spawning in-migration) is not occurring. This is generally the preferred period for (i) instream work or (ii) work adjacent to or over top of fish-bearing streams with the potential to create adverse impacts on fish or fish habitat. The Fishery Timing Window is that period of any given year as identified by the Environmental Agencies or as Designated in the Special Provisions. “Fishery Timing Window” and “Instream Work Window” are synonymous.
- (n) **“Habitats”** are defined as those parts of the environment on which terrestrial and/or aquatic species depend, directly or indirectly, in order to carry out their life processes.
- (o) **“High Water Mark”** is defined as the visible high water mark of any lake, stream, wetland or other body of water where the presence and action of the water are so common and usual and so long continued in all ordinary years as to mark upon the soil of the bed of the lake, river stream, or other body of water a character distinct from that of the banks, both in vegetation and in the nature of the soil itself. Typical features may include, a natural line or "mark" impressed on the bank or shore, indicated by erosion, shelving, changes in soil characteristics, destruction of terrestrial vegetation, or other distinctive physical characteristics. The area below the high water mark includes the active floodplain.
- (p) **“Impact”** is defined as an alteration, either positive or negative, to the environment brought about as a direct or indirect result of a highway project, including construction, operation and maintenance work (e.g., the consequence of a highway-related activity interacting with its surroundings).
- (q) **“Invasive Plant(s)”** are defined as a non-native plant species that has the potential to pose undesirable or detrimental impacts on people, animals, infrastructure, or the ecosystem. All Noxious Weeds are invasive plants, but not all invasive plants are Noxious Weeds.
- (r) **“Instream Work Window”** See “Fishery Timing Window”.
- (s) **“Noxious Weed(s)”** refers to plants as listed in the BC Weed Control Act and Regulation.
- (t) **“Mitigation”** refers to a procedure or an action designed to avoid, reduce or control the severity, magnitude, duration and/or frequency of environmental impacts of a project through design alternatives, scheduling or other means.
- (u) **“Riparian Area”** is defined as the land adjacent to the normal High Water Mark in a stream, river, lake or pond and extending to the portion of land that is directly influenced by the presence of adjacent ponded or channeled water, or a groundwater zone fed by surface water bodies (e.g., zone in which rooted vegetation is influenced). Riparian areas typically contain important vegetation resources which provide several critical functions for the survival of fish (e.g., protection from predators, shade for temperature regulation of the Watercourse, and sources of food)
- (v) **“Stream”** is defined by the Water Sustainability Act and may be designated in the Special Provisions, Environmental Approvals, legislation, and/or by Ministry Representative. Stream is synonymous with Watercourse.
- (w) **“Watercourses”** shall apply to all bodies of water including streams, rivers, canals, ditches, lakes, ponds, and wetlands. Watercourses may be ESAs. Watercourse is synonymous with Stream.

165.01.03 General Constraints for Protection of the Environment – The following general constraints in regard to the protection of any Watercourse shall apply and are

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considered incidental to the price bid for work under Contract and no other compensation will be made:

- (a) The Contractor shall place and/or dispose of all organic material, refuse, ash, petroleum products and other Deleterious Materials so as not to directly or indirectly pollute. The placement and disposal of all such products and materials shall be done in an environmentally acceptable manner.
- (b) Except as required by the Contract Documents, all inorganic material shall be placed and/or disposed of in a manner that does not obstruct or unduly disturb any permanent or seasonal Watercourse. Any such obstruction or disturbance shall be restored to the original drainage pattern as reasonably required by the Ministry Representative. Any removal of inorganic material from a Watercourse shall be done in a manner that minimizes adverse impacts and shall be carried out as directed by the Ministry Representative.
- (c) All activities below the High Water Mark of any Watercourse shall be kept to an absolute minimum. Machinery and equipment shall not be operated below the High Water Mark of any Watercourse other than the terms of the Authorisation and permits issued by the Environmental Agencies. An Environmental Monitor shall be present at the direction of the Ministry Representative during any work below the High Water Mark of any Watercourse during the Works.

165.01.04 Designated Streams and Designated Environmentally Sensitive Areas – Any Watercourse or Environmentally Sensitive Area Designated in the Special Provisions or by the Ministry Representative is subject to all of the constraints of this Section.

165.02 Environmental Monitor – The Contractor shall retain the services of an Environmental Monitor.

The Environmental Monitor shall be suitably experienced in, and responsible for:

- The preparation and implementation of environmental protection plans, including the CEMP;
- Preparing environmental monitoring reports;
- Compliance reporting to Environmental Agencies, if required by Environmental Approval(s).
- liaising with Environmental Agencies;
- applying for Environmental Approvals;
- fish and wildlife salvage operations, including obtaining necessary permits;
- proactive sediment and erosion control; and
- sampling, analysis and monitoring of water quality.

The Environmental Monitor shall be available throughout the duration of the Work to represent the Contractor in all matters related to the protection of the environment and will

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attend all key meetings at which environmental protection measures are to be discussed.

The Environmental Monitor and the Contractor shall work together on the development and implementation of the Construction Environmental Management Plan, including Environmental Procedures, as well as any applications for Environmental Approvals.

Unless otherwise approved by the Ministry Representative, the Environmental Monitor shall be on-site during work in Environmentally Sensitive Areas including, but not limited to the following activities:

- installation and decommission of temporary stream crossings, bridge works, construction accesses, and in-stream sediment control and isolation measures;
- fish and wildlife salvage operations;
- excavations, or placement of riprap, below the High Water Mark of streams;
- construction activities in and around listed species or ecosystems; and
- concrete pours and grouting operations in and around Designated Watercourses.

At a minimum, the Environmental Monitor shall attend at the Site on a monthly basis to perform a general compliance review of all the Contractor's environmental responsibilities, and provide a report of the observations and findings within seven (7) days of that review to the Contractor and the Ministry Representative.

165.02.01 Planning and Scheduling – The Contractor shall carefully plan and schedule construction activities in a manner that ensures the avoidance or absolute minimization of environmental damage. The Contractor shall be familiar with and be able to identify those areas and times which present environmental problems and shall prepare schedules and work methods accordingly. The Contractor shall forward a copy of each of the schedules to the Ministry Representative at least one week in advance of commencement of each of these operations, unless otherwise specified in the contract.

165.02.02 Construction Environmental Management Plan

The Contractor shall provide to the Ministry a Construction Environmental Management Plan (CEMP), prepared and/or signed and accepted by an Appropriately Qualified Professional.

The following time periods are required for plan review and acceptance unless otherwise specified in the Special Provisions or by the Ministry Representative. No review or change requested by the Ministry Representative in any way relieves the Contractor of any of its responsibilities for protection of the environment.

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- CEMP shall be submitted to the Ministry Representative for review at least fifteen (15) work days prior to mobilization to the site. The following conditions shall apply:
 - Any plan modifications shall be submitted to the Ministry Representative for review at least ten (10) working days prior to any changes being made in the field__.
 - A staged plan shall be submitted to the Ministry Representative for review at least ten (10) working days prior to implementation of a new stage.
 - Where a plan modification or staged plan requires environmental agency review, the changes must be approved by the agency prior to implementation.

Following review, the CEMP shall be revised as necessary and a final version provided to the Ministry Representative.

The Contractor shall make copies of the CEMP accessible to personnel on-site.

The CEMP must be inclusive of all elements relevant to the complete scope and duration of the work being undertaken and, including, but not limited to, the following:

- a clear description of how the work will comply with the environmental protection requirements of the Contract, including, but not limited to, the Standard Specifications, Special Provisions, and Environmental Approvals;
- a summary that clearly demonstrates the Contractor's understanding of the specific environmental issues involved with the Work, including Environmental Approvals;
- a description that demonstrates the Contractor's understanding of Ministry/Contractor responsibilities with regards to protection of the environment;
- clear identification of the process, including duration and sequence of each task, leading to the receipt of agency approvals, and the linkage between the process and the project schedule;
- Linkage between project schedule and Environmental Timing Windows
- Environmental Procedures;
- Contact names, positions and telephone numbers of individuals responsible for elements of the plan and Environmental Agency contacts.

And some or all of the following plans that are relevant to the Works:

- Air Quality and Dust Control Plan

PROTECTION OF THE ENVIRONMENT

- Archaeology Management Plan
- Clearing and Grubbing Plan;
- Concrete Waste Management Plan
- Construction and Waste Management Plan;
- Environmental Incident Reporting Plan;
- Environmental Monitoring Plan identifying the Work activities during which the Environmental Monitor will be on-site;
- Erosion and Sediment Control Plan;
- Invasive Plant Management Plan
- Reclamation Plan; and
- Spill Contingency Plan.

The CEMP shall clearly indicate how the Works will be undertaken to avoid negative impacts pertaining to, but not limited to, the following resources:

- air quality;
- archaeological, heritage, and cultural resources;
- fish and fish habitat; including Fisheries Sensitive Zones, and designated streams;
- identified sensitive areas, including designated Environmentally Sensitive Areas;
- landscaping and visual aesthetics;
- noise during construction;
- rare and endangered flora/fauna;
- soil conservation (including site stability, dust control);
- vegetation (including riparian plant communities);
- water quantity and quality (including all surface and sub-surface sources); and
- wildlife.

The Contractor, in consultation with the Ministry Representative, Environmental Monitor and the Environmental Agencies, shall be responsible for determining the conditions under which the work must be carried out in Environmentally Sensitive Areas, and for making all required on-site examinations and examinations of documents supplied and referenced by the Ministry in order to fully comprehend the environmental aspects of the work required.

165.02.03 Environmental Procedures – The Ministry Representative or the Environmental Agencies may require one or more detailed, task specific, Environmental Procedures for any work in and around Environmentally Sensitive Areas, or for works outside of Environmental Timing Windows that have the potential to impact the Environment. When an Environmental Procedure is

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required, the Contractor and the Environmental Monitor shall work together to prepare and submit the procedures for acceptance by the Ministry Representative and the Environmental Agencies prior to undertaking work in these areas. These detailed procedures shall be an element of the CEMP. The Contractor shall forward a copy of the final approved set of procedures to the Environmental Agencies and Ministry Representative, prior to undertaking the work. The Environmental Procedures shall contain the following items:

- (a) Existing environmental conditions. Identification of the environmental resources in the area of the proposed work. This includes baseline conditions to monitor against (e.g. water quality).
- (b) Description of work proposed in the Environmentally Sensitive Area. Details of the proposed work, staging, equipment to be used, schedule of activities, and location.
- (c) Environmental protection measures. Detailed description of the protective measures that will be used to protect environmental resources from each anticipated adverse impact. This includes scheduling works to coincide with Environmental Timing Windows. If works cannot occur within the Environmental Timing Window the Environmental Procedure will include detailed site-specific mitigation measures to address working outside the least risk timing window(s).
- (d) Contingency plan. Description of alternative or backup plan in the event of an environmental emergency or failure of any of the mitigation or protective measures.
- (e) Environmental monitoring requirements. Indication of any specific or unique environmental monitoring requirements to ensure compliance with environmental specifications and proper implementation of the procedure.
- (f) Environmental Procedures will be required for, but not limited to, any of the following activities taking place within Environmentally Sensitive Areas;
 - (i) Clearing or grubbing;
 - (ii) Streambed or streambank excavation, or riprap placement in a Watercourse;
 - (iii) Installation or decommissioning of temporary features such as work bridges, cofferdams, instream shoring, tote roads, or construction accesses;
 - (iv) Fish and wildlife salvage operations;
 - (v) Works below the High Water Mark of a Watercourse;
 - (vi) Concrete pours and grouting in and around watercourses;

PROTECTION OF THE ENVIRONMENT

- (vii) Demolition of existing structures; and
- (viii) Other activities as directed by the Ministry Representative or Environmental Agencies.

165.02.04 Project Orientation and Meetings – The Contractor and Environmental Monitor shall meet regularly with the Ministry Representative. The purpose of such regular meetings will be to outline the schedule of upcoming construction and proposed activities, and to review the activities of the previous week. Such regular meetings do not relieve the Contractor from attending or arranging other types of meetings as required due to the nature or extent of the work being done.

The Contractor shall arrange and conduct such regular meetings during construction, subject to the following conditions:

- (a) Meetings shall be held on a weekly basis when construction activities are to proceed during an Environmental Timing Window, during a period when work is to proceed in an Environmentally Sensitive Area, or during a period when the proposed construction activity elsewhere on a project is to proceed for a duration greater than two weeks and has the potential to cause adverse environmental impacts.
- (b) During periods other than that specified above, regular environmental meetings shall be held every two weeks, or as otherwise agreed to by the Ministry Representative.
- (c) In the event that the Environmental Monitor or the Ministry Representative deem the Contractor's compliance regarding environmental requirements to be inadequate at any time during the project, the frequency of the weekly or biweekly regular meetings shall be increased accordingly until the Environmental Monitor and the Ministry Representative are satisfied with the Contractor's compliance rate.
- (d) The Environmental Monitor shall record highlights from the meetings (e.g., summary of major discussion items and key action items) and forward a copy to the Contractor, as well as include this material in the monitoring report

The Contractor is responsible for ensuring that the forepersons, operators, and work crews (including any subcontractors) understand the specific environmental issues on the work site and their responsibilities under this Section. The Environmental Monitor shall give all construction personnel a brief environmental orientation prior to such personnel commencing work in any Environmentally Sensitive Area. The Environmental Monitor shall ensure that construction personnel are familiar with the environmental requirements and acceptable construction practices during the Works. The Contractor shall support the Environmental Monitor's role in this regard.

165.02.05 Activity Within Designated Watercourses and Environmentally Sensitive Areas – All activities that are conducted below the High Water Mark of any Designated Watercourse or within other Designated Environmentally Sensitive Areas must conform to the timing restrictions and Environmental Timing Windows stated in the Special Provisions, Environmental Approvals, or Best Management Practices. Construction work must be scheduled so that all operations affecting the Fisheries Sensitive Zone or other Designated Environmentally Sensitive Areas can be completed within these time periods. Prior to the commencement of any construction activities, all work in these areas must be approved by the Environmental Agencies and the Ministry Representative.

The Contractor must give at least 48 hours notice to the Ministry Representative in advance of any construction-related activity in Designated areas and ESAs. Construction activities within Designated Watercourses or other ESAs shall not commence without the approval and attendance of the Environmental Monitor.

165.02.06 Inclement Weather – The Contractor shall cease operations, modify construction methods, or relocate to an alternative site within the project area during periods of inclement weather to avoid siltation of Designated Watercourses. If the Ministry Representative judges that the Contractor is not complying with the intent of this Section, the Ministry Representative may direct the Contractor as to measures required without additional compensation for delays or alterations in the Contractor's work.

165.02.07 Work Stoppage – Prior to initiating work stoppages, the Contractor shall initiate and complete any mitigative and environmental protection measures required to safeguard the environment and the Project during work stoppages.

The Contractor shall give the Ministry Representative sufficient notice of impending shutdowns to enable the Ministry Representative, Contractor, and, if employed, the Environmental Monitor to examine the project. This notice shall be sufficient to permit the Contractor adequate time to install all necessary additional environmental mitigation measures as may be directed by the Ministry Representative.

The Contractor shall ensure the Environmental Monitor and construction personnel inspect Environmentally Sensitive Areas on the Project on a regular basis during any extended work stoppages (e.g., weekends, statutory holidays, Christmas vacation periods) to prevent environmental problems. If potential adverse environmental impacts are observed during work stoppage periods, the Contractor shall initiate and undertake any required environmental measures to avoid or minimize impacts in a timely manner to the satisfaction of the Ministry Representative.

165.03 Compliance, Enforcement and Payment

165.03.01 Compliance with Specifications – The Contractor is responsible for ensuring that all subcontractors and employees are in compliance with these Specifications and all applicable environmental legislation and regulatory requirements at all times, and shall take immediate action to rectify problems in this regard. The Contractor shall initiate action to rectify environmental problems within the time period as specified by the Ministry Representative. The Contractor shall have available such additional safeguards, safety devices and protective equipment as are necessary to protect the environment. The Contractor shall be responsible for ensuring sufficient safety devices and protective equipment (e.g., pumps, silt fence, armouring, tarps, fuel spill cleanup kits, etc.) are readily available at all times during construction of the Project. The location and inventory of safety devices and protective equipment shall be documented and placed in a prominent location in the Contractor's field office. A copy of this information shall be updated as necessary and provided to the Ministry Representative.

The Environmental Monitor shall inspect and monitor conditions at, and in the vicinity of, the Works to ensure compliance with the environmental specifications contained herein and in the Special Provisions.

The frequency of such inspection and monitoring efforts shall correspond to the sensitivity and location of construction activities, as well as to the environmental conditions (e.g., increased monitoring and inspection will be conducted during periods of inclement weather). In the event that the Ministry Representative is not satisfied with the frequency and duration of such inspection and monitoring, the Contractor and/or the Environmental Monitor will be required to increase their efforts in this regard to a level satisfactory to the Ministry.

165.03.02 Compliance with Environmental Legislation and Regulatory Requirements – The Contractor shall observe and comply with all federal, provincial, municipal and local laws and regulations which seek to ensure that construction work does not adversely affect the environment or social-community resources.

Environmental legislation relevant to most Works includes, but is not limited to:

Federal Legislation;

- Fisheries Act
- Migratory Bird Convention Act
- Canadian Navigable Water Act
- Seeds Act
- Species at Risk Act

Provincial Legislation:

- Environmental Management Act
- Heritage Conservation Act
- Integrated Pest Management Act
- Water Sustainability Act
- Weed Control Act and Regulation
- Wildlife Act

A non-inclusive listing of Best Management Practices and Environmental Agencies' areas of concern are provided in the current versions of the following:

- The joint publication “Land Development Guidelines for the Protection of Aquatic Habitat”, Fisheries and Oceans Canada and the Ministry of Environment, Lands and Parks (1992), hereafter referred to as the “Land Development Guidelines”;
- Standards and codes of practice, Fisheries and Oceans Canada <https://www.dfo-mpo.gc.ca/pnw-ppe/practice-pratique-eng.html>
- “Develop with Care 2014: Environmental Guidelines for Urban and Rural Land Development”, Ministry of Environment (2014)
- The “Standards and Best Practices for Instream Works”, Ministry of Water, Land and Air Protection (2004);
- “A User’s Guide to Working In and Around Water” Ministry of Environment (2009 update);
- “National Guide to Erosion and Sediment Control on Roadway Projects”, available for purchase from the Transportation Association of Canada (2005);
- Ministry publication “Reclamation and Environmental Protection Handbook for Sand, Gravel and Quarry Operations in British Columbia” (1995);
- Ministry publication “Manual of Control of Erosion and Shallow Slope Movement” (1997) (being updated as the “Erosion and Sediment Control Manual” (2020))
- Federal “Transport of Dangerous Goods Regulations”;
- “A field guide to Fuel Handling, Transportation and Storage” Ministry of Water, Land and Air Protection (2002);
- Confirmed Pest Management Plans
- “Freshwater Intake End-of-Pipe Fish Screen Guideline” Fisheries and Oceans Canada (1995)
- “Measures to Avoid Causing Harm to Fish and Fish Habitat” Fisheries and Oceans Canada, Available Online; and

- A joint publication between the Ministry and the Invasive Plant Council of BC “Best Practices for Managing Invasive Plants on Roadsides” (2010).

In the event of conflict between the requirements set out in these Specifications and quality control laws, statutes, regulations and ordinances of federal, provincial, municipal or local agencies, the more restrictive requirements or regulations shall apply. A violation of the environmental laws and regulations reported to the Ministry by the responsible agencies may result in the issuance of a non-conformance report. Should the situation warrant more stringent measures, it may also result in the issuance of a stop-work order until the violation is corrected. The Contractor shall have no recourse for reimbursement due to delays or alterations to construction activities arising from such violations or the correction of such violations.

The Contractor shall observe and comply with all terms and conditions of Environmental Approvals. Unless otherwise indicated, the Contractor is responsible for all work needed to comply with the terms and conditions of Environmental Approvals. The Contractor shall make copies of all Environmental Approvals accessible to personnel on-site.

165.03.03 Temporary Pollution Control – The Contractor shall be responsible for implementation of any temporary environmental protection measures, such as pollution control measures. If the Contractor fails to respond to this requirement or to the instructions in this regard from the Ministry Representative or the Environmental Monitor within directed time span, the Ministry Representative may take whatever action is necessary to provide the proper corrective measures. In the event the Contractor fails to take reasonable action to implement temporary environmental control measures, the Ministry's Representative and/or the Environmental Agencies shall determine the need for additional corrective action. The Ministry's Representative may deduct incurred costs from any money due or to become due to the Contractor for any corrective action taken in this regard.

165.03.04 Responsibility for Damage to Environment – The Contractor shall bear sole responsibility for any direct or indirect damage to the environment which occurs as a result of failure to comply with environmental legislation, Environmental Approvals, these Specifications, the Special Provisions, and/or the directions of the Ministry Representative or recommendations of the Environmental Monitor.

165.03.05 Stop Work Orders – In the event that the Ministry Representative or the Environmental Monitor determines that some aspect of the construction is creating or will result in a substantial adverse effect on environmental values or resources on or adjacent to the project site, the Ministry Representative or the Environmental Monitor may issue a "stop work order" on behalf of the Ministry for that aspect of work potentially having an adverse effect. The Contractor shall subsequently

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be responsible for advising the Ministry Representative of the intended remedial action. The Ministry Representative or the Environmental Monitor may direct the Contractor to cease construction work on an unaffected portion of the site so that any necessary equipment can be immediately diverted to address the environmental emergency. In such an event the Contractor shall not be entitled to any claim for compensation from the Ministry.

In the event that the Ministry Representative cannot be contacted, the Environmental Monitor shall assume the Ministry Representative's role regarding Stop Work Orders, as detailed above, until such time as the Ministry Representative can be contacted. The Environmental Monitor shall fully document all such instances. In such an event, the Contractor will not be entitled to any legal recourse or claim for compensation from the Ministry.

The Contractor shall be required to modify or halt any aspect of construction or maintenance that the Environmental Agencies determine poses or will pose an environmental concern. The Environmental Agencies may verbally direct the Contractor to modify or halt any such activity or submit formal written instructions to that effect. In either case, the Contractor's immediate compliance with their instructions will be mandatory. The Contractor shall document any such requests by the Environmental Agencies and forward a copy of this documentation to the Ministry Representative within five working days of the initial notification by the Agencies.

165.03.06 Payment – All requirements for compliance with these environmental specifications shall be considered incidental to the price bid for the Project under contract and no other compensation shall be made to the Contractor. However, the Contractor may be eligible for reimbursement for the installation of any additional environmental works (e.g., enhancement measures) where directed and approved in writing by the Ministry Representative and not covered in this Section or elsewhere in the Special Provisions or Schedule of Quantities and Unit Prices. The Ministry Representative will be responsible for determining the Contractor's eligibility in this regard and for identifying what environmental work is beyond the scope of the Contract and these specifications.

165.04 Erosion, Sediment, and Drainage Control – All works shall be undertaken in a manner that avoids or absolutely minimizes erosion problems and the discharge of siltation or other deleterious substances into any Watercourse. The Contractor shall not disturb Designated Watercourses, unless the express written consent of the Environmental Agencies is obtained prior to initiating construction activities in such areas. No obstruction or debris shall be placed in any Watercourse during any operations, unless specified in the Special Provisions or by the Ministry Representative, or for Designated Watercourses approved by the Environmental Agencies. Should any material be inadvertently placed below the normal High Water Mark of a Watercourse, the Ministry

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Representative shall be notified immediately and the Contractor shall remove the material immediately, using environmentally acceptable construction procedures and under the direction of the Environmental Monitor.

165.04.01 Erosion and Sediment Control Plan (ESCP) – The Contractor is responsible for preparing and implementing an ESCP for the project in accordance with the methodology of the “National Guide to Erosion and Sediment Control on Roadway Projects”, Transportation Association of Canada 2005. The ESCP shall be updated to reflect changing conditions on Site, at the direction of the Environmental Monitor or the Ministry Representative. The Contractor shall incorporate all temporary and permanent soil erosion and sediment control features into the Works, as outlined in the accepted work schedule and ESCP, and shall be responsible for daily inspection of the integrity of such measures during adverse weather conditions or when construction operations are proceeding in Environmentally Sensitive Areas. Erosion and sediment control measures shall be capable of continuous operation during working and non-working hours, and are subject to acceptance by the Ministry Representative. Any deficiencies which are observed in erosion control measures shall be immediately corrected.

ESC measures include, but are not limited to:

- interceptor ditches or berms to direct runoff away from erodible areas;
- slope protection measures such as mulches, hydroseeding, erosion mats, geotextiles, filter fabric, polyethylene covers, or riprap;
- silt fences;
- ditch blocks to reduce flow velocities;
- sediment control measures, such as settling ponds, and
- check dams.

The Contractor shall have sufficient materials, such as clean rock, granular material, and filter fabric available on-site for emergency protection measures when required.

The Contractor shall regularly maintain sediment and drainage control measures, such that they function as designed. Immediate action shall be taken by the Contractor to correct any deficiency observed in the operation of erosion and sediment control measures. In the event that a deficiency in any sediment or drainage control measures is directly or indirectly creating an adverse environmental impact, the Contractor shall initiate the necessary action to correct the problem within one hour of observing or being informed of the situation. In the event that a deficiency in any sediment or drainage control measures poses the potential to create an adverse environmental impact, the Contractor shall take action within two days of observing or being informed of the situation. In the event that environmental conditions or the status of the situation

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changes in regards to a potential deficiency, the Contractor shall accelerate any proposed correction measure(s) by taking immediate action (i.e., within one hour). The judgment of the need and timing for corrective action in regard to sediment or drainage control measures shall be at the discretion of the Ministry Representative, Environmental Monitor, and the Environmental Agencies.

Sediment and debris accumulations which compromise the functioning of the erosion and sediment control measures shall be removed by the Contractor and disposed of in an environmentally acceptable location and manner. At a minimum, silt fence structures shall be cleaned when sediment accumulation heights exceed 30 cm along any portion of the structure.

In the absence of site-specific requirements from Fisheries and Oceans Canada or the British Columbia Ministry of Environment, the Contractor shall comply with criteria for suspended solids in runoff water from the site as agreed upon prior to construction by the Ministry Representative and Environmental Agencies.

Erosion, sediment and drainage control methods are detailed in the "National Guide to Erosion and Sediment Control on Roadway Projects" and the "Land Development Guidelines", the "Standards and Best Practices for Instream Works" and the Ministry publication "Control of Erosion and Shallow Slope Movement" (to be updated in 2020 as the "Erosion and Sediment Control Manual"). Compliance with the spirit and intent of these guidelines is mandatory. The determination of the Contractor's compliance with these guidelines shall be at the discretion of the Ministry Representative and Environmental Agencies.

The Contractor, in consultation with the Environmental Monitor, shall be responsible for the preparation and implementation of the ESCP, which forms part of the CEMP, required for the Project. The ESCP(s) will be subject to, but not limited to the following requirements:

- (a) Prior to commencement of construction activities on the Project and thereafter when deemed necessary, the Contractor and the Environmental Monitor shall identify areas and major construction activities for which the Contractor will be required to prepare one or more Erosion and Sediment Control Plans. Designation of these areas and activities will be done in consultation with the Environmental Agencies and the Ministry Representative.
- (b) In addition to activities within the Project area, a Erosion and Sediment Control Plan shall be submitted for activities related to the Contractor's work in areas away from the project that have the potential to create adverse environmental impacts, which shall be determined by the Contractor in consultation with the Environmental Monitor, the Environmental Agencies and the Ministry. These activities are not confined to the areas immediately adjacent to the right-of-way, but also in other locations in which the work or associated

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activities will be conducted, including but not limited to haul or access roads, temporary structures, borrow and granular excavations, staging/laydown areas, material storage sites and disposal sites.

- (c) The Contractor shall not commence work in any area so identified until an Erosion and Sediment Control Plan has been submitted and accepted by the Ministry Representative and involved Environmental Agencies.
- (d) The acceptance of the Contractor's Erosion and Sediment Control Plan or updated Plan shall be valid only so long as the conditions and anticipated conditions upon which the Plan and its acceptance were based continue to be applicable.
- (e) The Contractor shall at all times be responsible for ensuring that work is carried out in accordance with a Erosion and Sediment Control Plan or updated Plan, which has been accepted by the Ministry Representative and Environmental Agencies.
- (f) The Ministry Representative or the Environmental Agencies may request the Contractor to update and resubmit its plan(s) at any time, and may require the Contractor to cease work until the update(s) has been submitted and accepted. Whether or not the Ministry Representative or the Environmental Agencies so request, the responsibility for carrying out the work in accordance with a validly accepted Plan lies entirely with the Contractor.
- (g) The Ministry Representative or the Environmental Agencies may order the Contractor to stop work or to take other precautionary or remedial measures whenever the Contractor is carrying out any work that is not in accordance with an Erosion and Sediment Control Plan or updated Plan which has been accepted by the Ministry Representative or the Environmental Agencies, and for which that acceptance is still valid. All costs of any resulting delay shall be borne entirely by the Contractor.
- (h) At a minimum, an Erosion and Sediment Control Plan shall include the following information and shall be prepared in a format satisfactory to the Ministry Representative and the Ministry:
 - (i) A schedule for the proposed activities, as they pertain to the Erosion and Sediment Control Plan and construction staging, including anticipated duration of construction.
 - (ii) A description of the construction procedures that will be used to limit the potential for erosion and sediment production, including estimates of work areas where applicable (e.g., volume of material in a proposed stockpile site), description of construction equipment to be used, and staging of operations.

- (iii) A detailed description of the site-specific measures for runoff and drainage management.
- (iv) A detailed description of the site-specific mitigation measures and design information for erosion prevention and control (e.g., sizing of culverts, ditches or sediment/detention ponds, silt fence specifications, and description of ditching and berming).
- (v) A schedule that identifies the various sloped areas by station and the dates in which they will be protected using temporary and permanent revegetation measures, such as hydroseeding.
- (vi) A detailed description of mitigative measures for ensuring acceptable water quality and quantity at points of discharge to Watercourses, including sizes and specifications for any proposed water treatment facilities.
- (vii) A key plan and drawings, in suitable scale and detail, of the site and proposed mitigative measures and applicable construction procedures.
- (viii) A written commitment and list of on-site equipment (e.g., water pumps) and materials (e.g., silt fence, hay bales, rock armou~~r~~ing, and ditch breakers) for erosion, sediment and drainage control to deal with emergency situations that may arise.
- (ix) A written commitment of on-site equipment and staff to handle any fish salvage and transfer operations, if required.
- (x) A description of the specific monitoring procedures and inspection frequencies prior to, during and after completion of construction activities at this site. This shall include a description of water quality testing locations and testing frequencies.

165.04.02 Not used.

165.04.03 Sediment Control Ponds – The Contractor shall construct sediment control ponds where necessary to prevent the release of unavoidably entrained sediments in runoff from the construction site, and shall construct stormwater detention ponds for the temporary and/or permanent control and discharge of stormwater runoff. The Contractor shall provide designs for and construct sediment control ponds, as well as stormwater detention ponds, on a site-specific basis in a manner that conforms to the “National Guide to Erosion and Sediment Control on Roadway Projects” or “Land Development Guidelines”, other than for the following exception(s):

- Sediment control ponds and dry or wet stormwater detention ponds shall be designed and constructed to accommodate the 10-year return period storm under

developed conditions with a minimum 0.60 metre freeboard to the top of the berm.

The Contractor is referred to Section 3 of the “Land Development Guidelines” for other pond design criteria and Section 8 of that publication, for sample calculations for water flows and pond sizes.

If during construction the settlement pond is filled to capacity and is still required for sediment control, accumulated settled sediments shall be removed from sediment control ponds and stormwater detention ponds. Such sediments shall be disposed of in an environmentally acceptable location and manner.

165.04.04 Maintenance of Drainage Patterns – Except where interceptor ditches or berms are required to divert sediment laden runoff from the site to a sediment control pond or filtration area, original drainage patterns shall be maintained throughout construction operations. Interceptor ditches or berms shall be constructed to divert water entering the site away from erosion prone areas. The Contractor shall not rely on drainage courses or conduits being shown on the Drawings, and shall make whatever investigation is necessary. Where a natural Watercourse traversing the construction site crosses this interceptor ditch or berm, an armoured or lined ditch or a culvert shall be installed in order to pass accumulated flows through or around the construction site in a manner that maintains the natural runoff pattern, unless otherwise directed by the Ministry Representative or Environmental Agencies.

All ditches constructed for interception of clean water outside the work site(s) to divert it around the work site(s) and for collection of treated water from the work site(s) shall be designed and constructed to accommodate the 10-year return period storm with a 0.3 m freeboard under developed conditions. Temporary and/or permanent drainage ditches, including those constructed for erosion and sediment control shall be designed to prevent high water velocities and erosion by including measures such as check dams, drop structures or erosion-resistant liners or armou~~r~~ing.

165.04.05 Storm Drainage Systems – In order to minimize siltation, the Contractor may be required to block storm drain inlets, or to activate inlets by means of sandbags, berms or swales, as circumstances require, or at the direction of the Ministry Representative. Berms shall be constructed of clean, non-erodible granular material. The Contractor shall maintain, on a regular basis, any inlets activated during the course of construction. Any deficiencies noted in these works shall be corrected immediately in the event that the inlet is plugged and within the same day if the site is relatively dry.

The Contractor shall ensure adequate inspection and correction of any storm drainage system deficiencies for sites within the influence of the Project. During periods of inclement weather, the Contractor shall ensure project staff inspect storm drainage systems throughout the day, and if necessary, make arrangements for inspection and

maintenance during regular work stoppage periods, such as evenings or weekends. At a minimum, such inspections shall be carried out at the start of the regular workday, at mid-day in the workday, and one hour prior to end of the regular workday. Furthermore, in the event of flooding problems, the Contractor shall be responsible for any damages or compensation resulting from impacts on residences or businesses caused by a failure or deficiency in the Contractor's design and maintenance of the storm drainage system.

During concrete curb and gutter construction, road surface runoff shall be directed by berms or swales away from concrete which has been poured within the previous 48 hours.

165.05 Clearing and Grubbing Plan (CGP) – The Contractor shall obtain acceptance for this plan from the Ministry Representative. The Contractor shall also obtain any necessary tree removal permits from the Ministry of Forests, Lands and Natural Resource Operations prior to initiating any clearing.

If clearing and grubbing is scheduled to occur outside of an Environmental Timing Window, the Contractor's AQP shall develop a site-specific mitigation plan for acceptance by the Ministry Representative.

165.05.01 Limits for Clearing and Grubbing – Clearing and/or grubbing shall extend only to the Designated limits, as defined in the Contract Drawings and marked on site. Prior to clearing or grubbing, marked limits will be inspected by the Ministry Representative, who may amend them. The Contractor may be required to use close-cut, no grub practices (i.e., cutting trees at ground level and not removing root system) or undertake hand clearing at environmentally sensitive sites, as Designated in the Special Provisions or by the Ministry Representative, or Environmental Agencies.

The Contractor shall not proceed with clearing and grubbing closer than 50 m from a Designated Watercourse or Environmentally Sensitive Area prior to marking the limits of the Environmentally Sensitive Area. The width of the ESA, if not specified in the Special Provisions, will be determined by the Ministry Representative in consultation with the Environmental Agencies and shall be visibly marked in the field.

All clearing and grubbing activities in the ESA must be granted prior approval by the Environmental Agencies. The Contractor shall not conduct approved grubbing operations within the ESA until ready to proceed with earthwork and stabilization.

In areas near Designated Watercourses or other ESAs determined by the Ministry Representative or Environmental Agencies to be of high erosion or siltation potential, the surface area of erodible soil exposed at one time may be limited by the Ministry Representative. This will supersede SS 200.03 and SS 200.04. Where exposed

materials are sensitive to erosion, the Ministry Representative may confine grubbing to an area where excavation and earthwork is to be actively conducted within approximately 30 days following the completion of grubbing operations. In the event that a highway project consists only of clearing and grubbing operations, the cleared ground shall be protected and able to withstand specified periods of inclement weather.

165.05.02 Protection of Vegetation – The Contractor shall protect all vegetation growing outside of the Designated and marked areas for clearing and grubbing, as specified in SS 769 "Protection and Retention of Vegetation", with the exception of danger trees, which must be hand-felled and removed with minimum disturbance to retained vegetation.

165.05.03 Clearing Activities – Any trees or large pieces of woody debris that accidentally fall into a Watercourse and require removal shall be removed in a manner that minimizes the disturbance of the Watercourse and adjacent banks, and is approved by the Ministry Representative. Skidding of logs across Watercourses will not be permitted. The Environmental Monitor must be present during removal of any large woody debris or trees from Watercourses.

Riparian trees that require falling or full or partial removal along a fish-bearing Watercourse shall have first right-of-refusal for use as woody debris habitat complexing as approved by the Ministry Representative.

165.05.04 Disposal of Combustible Materials – The Contractor shall comply with the Forest Act and the Open Burning Smoke Control Regulation under the British Columbia Environmental Management Act. For additional information regarding the regulation, the Contractor should contact regional staff in the British Columbia Ministry of Environment office and the Fire Centre staff in the British Columbia Ministry of Forests, Lands and Natural Resource Operations. The Contractor is responsible for submitting any necessary notifications, or obtaining any necessary permits, for burning from the Ministry of Forests, Lands and Natural Resource Operations.

The Contractor shall prevent heat or smoke damage to all vegetation that has been Designated for preservation. The use of waste oil and/or tires as fire accelerators shall not be permitted. The Contractor may be required to use a forced air method of burning. Burning piles shall not be located within 50 m of any Watercourse, wetland or other Environmentally Sensitive Area, or in areas where ditches are to be constructed without written approval of the Ministry Representative.

In situations in which the material generated as a result of clearing and/or grubbing may not be burned on site, the Contractor shall dispose of it in a manner approved by the Ministry Representative and/or the Environmental Agencies.

165.06 Stripping Operations

165.06.01 Exposure of Erodible Earth – In areas where erosion or siltation is anticipated, the duration of exposure of erodible earth material shall be minimized. In such cases the surface area of erodible earth material exposed at one time shall be determined and approved in writing by the Ministry Representative. (See SS 165.05.01 Limits for Clearing and Grubbing)

165.06.02 Placement of Stripped Material – The Contractor shall avoid placement of stripped materials on lands within the Agricultural Land Reserve and in areas adjacent to Watercourses or other Environmentally Sensitive Areas. Erosion and sediment control measures must be taken prior to and after placement of stockpiles of stripped material in areas where natural drainage or storm water could erode the stockpile and thereby transport pollutants to surface waters. The location of stockpiles and erosion control measures must be approved by the Ministry Representative prior to placement of stripped materials and may be specified in the Special Provisions. The Contractor shall ensure that all stockpiles are stable.

Polyethylene sheeting, other suitable tarp material or mulch covering shall be used to cover temporarily exposed steep surfaces or stockpiles of erodible materials, such as topsoil, sand, gravel or roadbase fill. The Contractor shall ensure such erodible materials are properly covered with sheeting, suitable tarp materials or mulch covering immediately after creation of any temporary stockpiles. Such sheeting, tarp materials or mulch covering shall be examined and maintained on a regular basis. The sheeting, tarp materials or mulch covering shall be sufficiently anchored or applied to prevent displacement by winds or runoff.

See also SS 751.31.01 Stripping of Topsoil.

165.06.03 Slope Protection – Not used.

165.07 Earthwork Operations (Subgrade Construction)

165.07.01 Excavation and Disposal of Waste or Surplus Material – The creation and use of any site on Crown or private lands for the placement and disposal of waste or surplus material requires prior approval from the Ministry Representative and may require the approval of the Agricultural Land Commission or representatives of other provincial Environmental Agencies. Unless another time period is specified in the Special Provisions or by the Ministry Representative, not less than fifteen days prior to disposing of any waste or surplus material, the Contractor shall submit to the Ministry Representative for approval, a proposal delineating the locations and extent of the areas in which the Contractor intends to dispose of such material. The proposal shall describe the nature of the material and the methods to be employed in material placement, stabilizing and site revegetation. If the Contractor is aware that the material contains any noxious weeds, the Contractor shall note this in the proposal. If a disposal site is required off the right-of-way, the proposal shall also indicate the

procedures for any required land clearing activity. The proposal will be reviewed by the Ministry Representative who may consult with Environmental Agencies. No waste or surplus material shall be disposed of until the proposal has been approved. All work shall be done in accordance with the approved proposal.

All waste disposal sites shall be selected such that spoil is prevented from entering any Watercourse. The excavated material shall be properly drained, spread and trimmed to a stable slope not exceeding 1.5 to 1, in a manner which minimizes disturbance of Watercourses and vegetated areas. All waste sites shall be revegetated immediately after creation of the waste disposal site, or else suitable temporary erosion control measures, such as application of tarps or mulch, shall be used until revegetation is undertaken. The Contractor is responsible for ensuring that revegetation is successfully accomplished unless otherwise specified in the Special Provisions.

165.07.02 Work in Areas of High Water Table – Work in areas of high water table shall be scheduled to proceed during dry weather periods unless otherwise specified in the Special Provisions. The Contractor shall adopt all necessary mitigation measures to avoid or minimize adverse impacts in such areas.

165.07.03 Drilling and Blasting – Blasting operations shall be conducted in a manner that minimizes the disturbance of residences, businesses and public infrastructure (e.g., schools, churches, libraries and hospitals) and to aquatic and terrestrial habitats and organisms due to shock waves, noise and vibration.

The Contractor shall not conduct blasting without the acceptance of a blasting plan by the Ministry Representative. The Contractor is responsible for preparing the blasting plan.

Use of explosives in or near water produces shock waves that can damage a fish bladder and rupture internal organs. Blasting vibrations may also kill or damage fish eggs or larvae. For the most current Fisheries and Oceans requirements for blasting in fish waters please refer to their main website under “Projects Near Water, Fish Protection” (available online).

Blasting operations shall be controlled to minimize the entry of fly rock into Watercourses or adjacent to the right-of-way, particularly in an urban setting. If required by the Environmental Agencies, the Contractor shall comply with the Agencies' directions in removal of fly rock from Watercourses and Environmentally Sensitive Areas at no cost to the Ministry.

When blasting is to be conducted adjacent to or in a Designated Environmentally Sensitive Area, the Contractor shall provide at least 24 hours notice of each blast to appropriate Agencies and the Ministry Representative. The Contractor shall provide at least 24 hours written notice to

those individuals or groups potentially affected by shock waves, noise and vibration from blasting operations.

165.07.04 Pile Driving – Pile Driving operations shall be conducted in a manner that minimizes the disturbance of residences, businesses and public infrastructure (e.g., schools and churches) and to aquatic and terrestrial habitats and organisms due to shock waves, noise and vibration.

The Contractor shall not conduct pile driving within or near fish habitat without the acceptance of the pile driving procedure by Fisheries and Oceans Canada and/or provincial Environmental Agencies, and the Ministry Representative. Vibrations and shock waves in fisheries habitat resulting from pile driving shall comply with the Fisheries and Oceans Canada and BC Marine and Pile Driving Contractor's Association "Best Management Practices for Pile Driving".

The Contractor shall provide at least 24 hours notice of pile driving activities to the Ministry Representative. The Contractor shall provide at least 24 hours written notice to those individuals or groups potentially affected by shock waves, noise and vibration from pile driving operations.

165.08 Borrow and Sand / Gravel Pits – All Borrow and Sand/Gravel pit locations, operations, reclamation and revegetation shall comply with the Ministry of Energy and Mines / the Ministry of Transportation and Infrastructure / Natural Resources Canada joint publication "Reclamation and Environmental Protection Handbook for Sand, Gravel and Quarry Operations in British Columbia".

165.08.01 Control of Drainage – Prior to borrow excavation, or quarry, sand or gravel pit development, the Contractor shall prepare an Erosion and Sediment Control Plan (see SS 165.04.01) prior to the creation and/or operation of any borrow excavations or quarry, sand or gravel pit development sites. The Contractor shall also prepare a Reclamation Plan.

165.08.02 Location and Development of Borrow and Sand/Gravel Pits – Clearing for borrow and sand/gravel pits shall comply with SS 165.05 – Clearing and Grubbing. All organic strippings topsoil shall be stockpiled for future reclamation.

Borrow excavations shall not be located below the High Water Mark of a Watercourse or on a stream floodplain at a location likely to create a new channel to the stream at a time of flooding, without the approval of the Environmental Agencies. Borrow sources located within 100 metres of the High Water Mark of a Watercourse or at distances less than 100 m from any water-impounding dyke or dam embankment will require the design of a Professional Engineer or Professional Geoscientist. Final floor elevations shall be sufficiently high that they will not be subject to fluctuations in the groundwater table.

In erosion-prone areas, overburden removal shall be restricted to an area that will be excavated within one year. The open face(s) of the borrow pit shall be no larger than

necessary for efficient pit operation. Material shall be exhausted at one location in a pit prior to the opening of a new face to remove a similar type and grade of material.

The Contractor shall be responsible for maintaining the functionality of erosion, sediment and drainage control measures within and around borrow and sand/gravel pits throughout all phases of overburden and material removal, as well as during periods of work stoppages. Borrow and/or sand/gravel pits shall be rehabilitated and decommissioned in an environmentally acceptable manner and to the satisfaction of the Ministry Representative and the Environmental Agencies.

To minimize the spread of noxious weed seeds or reproductive plant parts between construction sites and gravel pits/quarries, all crushing equipment and mining vehicles must be cleaned with a power/high pressure washer (manually cleaned in freezing temperatures), before entering any MoT pit or quarry. If the pit contains noxious weeds the crushing equipment and mining vehicles must also be cleaned with a power/high pressure washer (manually cleaned in freezing temperatures) before leaving the pit/quarry. Prior to the production of aggregates the pit floor, crusher/plant location, equipment-parking area, stockpile site(s), pit face(s) and area to be mined shall be inspected by an Appropriately Qualified Professional for the presence of noxious weeds. If noxious weeds are present they must be removed mechanically or treated chemically under the relevant confirmed Pest Management Plan by the Contractor prior to conducting work in the pit/quarry, and shall be disposed of as directed by the Ministry Representative or the Environmental Monitor. All noxious weeds found must be reported to the Ministry Representative. Noxious weeds are defined as non-native plants species listed in the Weed Control Act. All treatment and disposal of Noxious Weeds shall be as per the Contractor's accepted Invasive Plant Management Plan, a component of the CEMP, which shall be consistent with the requirements of the confirmed Pest Management Plan.

165.08.03 Washing Operations – In locations where siltation of a fish-bearing Watercourse may occur, approval shall be obtained from the Environmental Agencies prior to the establishment of aggregate washing operations, and all wash water shall be subject to SS 165.04. Where possible the Contractor may be directed by the Ministry Representative to use water from pit dewatering or clarified water from the sediment pond(s) in the wash plant.

Waste or surplus material shall be disposed of at a waste disposal site approved by the Ministry Representative and Environmental Agencies. The Contractor shall create and designate, with appropriate signing, suitable sites and facilities for disposal of waste or surplus material. In particular, the disposal of deleterious materials, such as surplus concrete or concrete wash water in sites other than those approved by the Ministry Representative and the Environmental Agencies is prohibited. The Contractor and the Environmental Monitor shall be responsible for ensuring

all construction personnel, including subcontractors, are made aware of this requirement. In the event that it is not feasible to install a suitable concrete waste disposal site immediately adjacent to the work site, such as during a concrete pour on a long bridge deck, then the Contractor shall take the necessary precautions to isolate the work site from any Watercourse or other Environmentally Sensitive Area and to temporarily collect the concrete waste or wash water and transport it to a nearby suitable disposal facility. The Contractor shall notify and seek approval from the Ministry and the Environmental Agencies for any changes in the location of waste disposal sites during construction operations.

165.09 Detour, Access and Haul Roads

165.09.01 Design, Construction and Operation of Access Roads – Any temporary access, detour and/or haul roads, including associated lay-down or staging areas, associated with the project shall be constructed to accommodate all required uses and maintained throughout the course of construction operations in a safe, environmentally sound manner.

The location, alignment, design and construction of all detour, access and haul roads shall be subject to the approval of the Ministry Representative, and the appropriate Environmental Agencies. The Contractor shall be responsible for applying for and acquiring Environmental Approvals for access roads not previously identified in Environmental Approvals acquired by the Ministry for access.

Clearing shall extend no further than safe operation requires, or to the edge of cut or toe of fill. Root systems of cleared vegetation shall not be removed except in the area of cuts or actual road surface.

The Contractor shall employ suitable measures to maintain air quality, visibility, and safe conditions in the use of access, detour and/or haul roads associated with the Project.

165.09.02 Drainage and Erosion Control – Drainage structures shall be incorporated into and maintained for the duration of the project along all detour, access and haul roads to minimize erosion and maintain drainage patterns.

165.09.03 Abandonment – The Contractor shall winterize haul and access roads at the end of the construction season and shall ensure proper drainage control measures are in place. Culvert inlets and outlets shall be flagged. During periods of work stoppages, the Contractor shall ensure sufficient inspection and maintenance of the access, detour and/or haul roads such that adverse environmental impacts are avoided or minimized. In the event that potential or existing environmental problems are identified by the Contractor, its staff, the Ministry, the Environmental Agencies or the public, the Contractor shall take immediate steps to rectify the problem and ensure corrective action is taken to prevent future recurrence of similar problems.

When no longer required by the Contractor or the Ministry, construction roads shall be properly decommissioned, including measures such as blocking such roads off from vehicular traffic, scarifying the compacted surfaces and revegetating the area as stipulated in Environmental Approvals.. All drainage and crossing structures shall be removed, and sufficient cross ditches and ditch blocks constructed and stabilized to restore the original drainage patterns and prevent erosion. The Ministry Representative may require that windrows along the side of the road be removed, and the road surface sloped to drain in the direction of the surrounding contours. Fill may be required to be pulled back onto the road if it is judged to be unstable. Remaining cut slopes shall not exceed the natural angle of repose. Reclamation works required by the Ministry Representative may include, but may not be limited to decompaction of the road surface, revegetation of exposed soil surfaces, and reforestation.

165.10 Instream Works – Instream works shall occur during the Fishery Timing Window as outlined by the Ministry of Environment. The Contractor, in consultation with the Environmental Monitor, shall prepare a detailed set of Environmental Procedures for any work within a Fisheries Sensitive Zone, below the High Water Mark of any fish-bearing Watercourse (or any Watercourse that flows directly into a fish-bearing stream), or for any work with the potential to cause major adverse impacts on a fish-bearing Watercourse (e.g., concrete pour for a bridge deck over a fish-bearing Watercourse, hydro-blasting of bridge substructures over a fish-bearing Watercourse, etc.). Environmental Procedures shall be prepared by the Environmental Monitor, for acceptance by the Ministry Representative, for the following works including, but not limited to:

- Bridge Construction
- Bridge Maintenance
- Ditch construction or maintenance
- Instream equipment crossing
- Watercourse diversions
- Culvert installations, and
- Temporary instream crossings.

Waste material generated during instream works shall not be stored or dumped within the floodplain unless otherwise approved by the Ministry Representative.

Drainage of waterbodies, such as wetlands, swamps or beaver ponds, shall be subject to the approval of the Ministry Representative who will consult the Environmental Agencies, unless drainage of these areas is specifically required as part of the Contract and has been approved by Fisheries and Oceans Canada and any provincial Environmental Agencies.

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165.10.01 Fish Passage and Fish Salvage – Unless the Contractor has written approval from the Ministry Representative and the Environmental Agencies, any instream works in fish bearing streams shall provide for fish passage.

The Contractor and the Environmental Monitor shall be responsible for fish salvage operations, including the timely acquisition of fish collection permits from Fisheries and Oceans Canada and any provincial Environmental Agencies.

165.10.02 Management of Water Discharges – During preparation of stream diversions, culvert installations and other operations involving dewatering where drainage could readily reach a Designated Watercourse, all effluent and silt-laden water shall be discharged to a sediment control pond or a vegetated area acceptable to the Ministry Representative and the Environmental Monitor for removal of silt prior to its release into that Watercourse. The direct discharge or discharge via seepage of untreated, silt-laden water or other deleterious substance into any Watercourse is prohibited.

165.10.03 Encroachment of Fill into Watercourses – Embankments which encroach on areas below the High Water Mark of fish-bearing Watercourses shall either be isolated and dewatered, by means of a cofferdam, until surfaced with riprap, or constructed of clean granular material and riprap or shot rock which is free of fine-grained material or other potential contaminants. Riprap composition and placement may be further detailed in the Special Provisions. The method of placement of this material shall be such that disturbance and/or alteration of aquatic habitats is absolutely minimized. Fill material must be entirely contained within the proposed fill cross section.

165.10.04 Temporary Stream Crossings – Prior to construction, the Contractor shall provide the Ministry Representative with an Environmental Procedure for acceptance, describing the proposed locations and types of stream crossings, complete with construction procedures and timing of construction. The Contractor shall forward a copy of the final Environmental Procedure to the Ministry Representative and obtain Environmental Approvals as needed, unless Environmental Approvals are already in place. Temporary stream crossings shall be subject to the same environmental constraints as permanent crossings, and shall be built to pass, at least, the 10 year return period flood for the time of year during which the structure will be in place. Temporary stream crossings that have been constructed during periods of low precipitation (i.e. summer and late winter) shall be completely removed prior to periods of increased precipitation (i.e. fall and spring freshet), unless otherwise approved by the Ministry Representative and Environmental Agencies.

165.10.05 Instream Equipment Operation or Crossings – The entering or crossing of any Designated Watercourse by construction equipment, when not specified in the

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Special Provisions or Environmental Approvals, shall require the Contractor to receive written approval of the Ministry Representative and to submit applications or notifications to the Environmental Agencies, as needed. The Contractor shall notify the Ministry Representative 48 hours prior to allowing equipment to cross or enter Designated Watercourses.

Upon completion, the banks shall be restored, stabilized and revegetated to prevent erosion. This work shall be completed to the satisfaction of the Ministry Representative and Environmental Agencies.

165.10.06 Culvert Installations – Unless otherwise approved by the Ministry Representative, culvert installation in Watercourses shall involve an appropriate method of isolating the work site from the stream, such as the diversion of the stream around the culvert site, and the placement of the culvert in the "dry".

Installation of riprap and other protective works shall be carried out at the earliest possible time following culvert installation in order to prevent erosion and siltation.

165.10.07 Culvert Obstructions – Unless the Watercourse is dry, an obstructed culvert shall be cleared by mechanical means. Any variance to using mechanical means to clear wet obstructed culverts must have the prior approval of the WorkSafeBC.

165.10.08 Channel Diversions – Channel diversions shall be performed in accordance with the Special Provisions. If not specified in the Special Provisions, methods shall be approved by the Ministry Representative, and shall comply with the "Standards and Best Practices for Instream Works", "A Users' Guide to Working in and Around Water", and the "Land Development Guidelines".

165.10.09 Bridge Construction – Bridge End-fills shall be constructed implementing fully the applicable subsections of SS 165, and SS 769, Protection and Retention of Vegetation. The Fisheries Sensitive Zone shall not be disturbed until construction of the end-fill or crossing structure is ready to proceed. The Contractor may be required to hand fall trees and leave the root systems in place using close-cut no-grub techniques.

165.10.10 Watercourse Clean-up – Final Watercourse clean-up shall consist of, but not be restricted to, the removal of temporary haul roads and temporary crossings, the reshaping of the stream to its original configuration, width and depth, the protection of stream banks, revegetation, and the removal of all construction related material and debris, including any material and debris deposited upstream or downstream of the site as a result of operations, under the direction of the Environmental Monitor and to the satisfaction of the Ministry Representative. All abandoned temporary diversions shall be plugged and stabilized. In the event that erosion has occurred during the course of construction, the Contractor

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will be required to recontour, stabilize and revegetate the affected area.

165.11 Ditch Maintenance – To reduce the risk of damage to aquatic habitat and sensitive life stages of fish and other aquatic organisms, the Contractor shall ensure that the excavation and maintenance of ditches is conducted in an environmentally sound manner.

Excavation for ditch cleaning of any ditch that contains or directly drains into fish-bearing waters shall be performed only within Timing Windows as detailed in the "Land Development Guidelines, Appendix III", Ministry of Environment reduced risk timing windows documents, or as specified in the Special Provisions.

The Contractor shall consult local Environmental Agencies for the locations of sensitive areas and local Timing Windows prior to commencement of ditch maintenance. Areas identified as Fisheries Sensitive Zones shall be revegetated by the Contractor prior to the close of the timing window.

165.12 Bridge Maintenance – In performing any bridge rehabilitation or maintenance, the Contractor shall follow best management practices and shall prepare Environmental Procedures for acceptance by the Ministry Representative.

The Contractor shall take measures to ensure absolute minimal loss of paint and abrasive material into Watercourses during bridge cleaning. Mitigation measures may include placement of ground covers, lined nets or tarpaulins to capture falling debris, blast enclosures to encase the section of the bridge to be abrasively blasted, use of vacuum-shrouded power tools, and use of floating booms and barges to trap floating debris. Similar measures shall also be employed to assist in mitigating paint spray drift.

165.13 Servicing and Operation of Equipment

165.13.01 Transport of Materials – The Contractor shall use equipment and containers that are capable of safely transporting petroleum products and/or hazardous materials in compliance with Section 7.33.1 of the Federal "Transport of Dangerous Goods Regulations" for bulk containers, and Sections 7.21 and 7.23 of the same regulations for materials in packages or small containers. These regulations shall apply to both on-road and on-site transport. In addition to compliance with the Federal Transport of Dangerous Goods Regulations, the transportation of fuel trailers, fuel tanks on skids, and non-commercial fuel trucks shall comply with all the requirements of the Motor Vehicle Act and the British Columbia Fire Code.

The Contractor shall take the necessary precautions to prevent the loss of materials during transport on public highways, roads, access roads, and haul routes. Trucks carrying loose materials shall be covered and restraints shall be used to prevent materials from blowing or falling from vehicles. The Contractor shall be responsible for collection and removal of any and all litter deposited by vehicles or equipment along access routes during construction-related

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activities, including removal of dirt and mud deposited from truck tires on municipal roadways.

165.13.02 Location and Installation of Fuel Storage Facilities – The siting and installation of all fuel storage facilities shall be the responsibility of the Contractor. Fuel storage shall not be located within 30 m of a Watercourse, within a Watercourses flood plain, or where there is a potential for any spilled fuel to enter a Watercourse or groundwater. Fuel storage facilities shall be located on flat or gently sloping ground and shall be dyked to contain at least 125% of the total capacity of the storage containers. Dykes shall be constructed of impermeable material or lined to ensure that petroleum products cannot escape. If these specifications cannot be met due to site specific constraints, the Contractor shall submit a proposal for construction and operation of a fuel storage facility to the Ministry Representative for acceptance prior to construction of the facility.

All large fuel storage tanks must be locked and secured when not in use. Automatic shut-off nozzles shall be installed on all dispensing units over 250 L capacity. Fuel storage tanks must be drained within one week of completion of construction or within one month of a prolonged shutdown period.

All small fuel storage containers, such as 45 gallon (200 litre) drums, used as a fuel cache shall be installed on a stable storage rack, within an impermeable containment device capable of capturing at least 125% of the total capacity of the storage container(s). A cover, such as a tarp, must be placed over the top of the fuel cache to prevent accumulation of precipitation in the containment device. The small fuel storage container(s) shall contain a metal spigot with a padlock placed on the container when not in use.

In the transport, storage and/or dispensing of fuel and other petroleum-based products, the Contractor shall comply with the requirements outlined in the Ministry of Land and Air Protection /Ministry of Forests publication: "A field guide to Fuel Handling, Transportation and Storage" (Feb, 2002).

The Contractor shall be responsible for all costs associated with clean-up and disposal of any escaped toxic and hazardous substances.

165.13.03 Servicing of Equipment – The fuelling, refuelling, servicing or washing of machines or equipment shall not be undertaken within 30m of any Watercourse or surface water drainage. On-site fuelling and lubrication of equipment shall also be conducted as far as possible from detention and sediment control facilities. The Ministry Representative may require that servicing be conducted at Designated sites.

Refuse generated during the servicing of equipment (e.g., air and oil filters, hydraulic fluids, petroleum products) shall be collected and disposed of in an environmentally acceptable location and manner. Where possible, the

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Contractor is encouraged to recycle lubricants and other waste materials generated during the servicing of equipment and machinery, or alternatively dispose of such materials and refuse at nearby recycling depots. The dumping of oil or other deleterious materials on the ground or in any Watercourse is strictly prohibited. The Contractor shall provide a means of catching and retaining drained oil or other deleterious materials and shall properly dispose of these materials in a location approved by the Ministry Representative.

165.13.04 Equipment Operation in Environmentally Sensitive Areas – Construction equipment shall be operated only within the Designated construction site and access roads. Equipment operators shall not be allowed to damage or destroy vegetation or streambanks outside of this area.

The operation of equipment and machinery in a Fisheries Sensitive Zone must be:

- (a) authorized by the Ministry Representative;
- (b) kept to an absolute minimum; and
- (c) undertaken only during the period(s) specified in the Special Provisions.

When working in any Designated Watercourse, equipment shall be in good working condition free of leaks, use biodegradable hydraulic fluid and shall be steam cleaned of oil, grease and other contaminants deleterious to aquatic species, prior to commencing work. When working in or near any Watercourse, the Contractor shall ensure that all hydraulic systems, fuel systems and lubricating systems are in good repair. Equipment with fuel or fluid leaks shall not be permitted to enter areas below the High Water Mark of any Watercourse. Equipment developing such leaks shall be removed immediately and repaired.

165.14 Waste Disposal and Toxic/Hazardous Materials

165.14.01 General – All non-toxic or non-hazardous wastes shall be either recycled or disposed of in an approved sanitary landfill or other specialized area as indicated in the Special Provisions or as directed by the Ministry Representative. Any waste material that is inadvertently dumped in or adjacent to Watercourses or other ESAs shall be removed by the Contractor and disposed of in an approved manner at the Contractor's expense.

The Contractor shall be responsible for the regular collection, storage, and disposal of all waste material generated by employees and subcontractors. The Contractor shall take the necessary precautions to prevent loss of these materials during transport on public highways and roads, and shall be responsible for cleanup of all of these materials and all litter deposited by employees and subcontractors along access routes during construction-related activities, at no expense to the Ministry. Construction debris shall not be allowed to accumulate on the construction site but shall be collected promptly, placed

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and stored in suitable animal-proof containers and disposed of at an approved waste disposal site.

165.14.02 Spill Contingency Plans – Contingency plans for the clean-up of toxic or hazardous spills shall be prepared prior to construction and submitted, together with a list of spill abatement equipment to be stored on the job site, to the Ministry Representative for review. Spill contingency plans may be a sub-plan to the CEMP. The Ministry Representative may require the Contractor to have on site any additional equipment or materials deemed necessary to deal with a potential spill. The Contractor shall promptly replace any used spill abatement and clean-up materials and maintain a sufficient inventory of materials throughout construction operations. For operations being conducted adjacent to or below the High Water Mark of a Watercourse or other Environmentally Sensitive Area, the Contractor shall have an approved spill kit ready for use nearby.

The Contractor shall immediately report any spill of any toxic or hazardous material verbally to the Ministry Representative and Emergency Management BC (24 hour phone line: 1-800-663-3456). Written notification of the spill must follow within two weeks of this verbal report. The Contractor shall immediately take the necessary steps to abate the discharge and provide the necessary labour, equipment, materials and absorbents to contain and remove the spill, clean up the affected area, dispose of waste materials at an approved disposal site, and restore the area to the satisfaction of the Environmental Agencies, at the Contractor's expense. Any soil contaminated by spills shall be removed and replaced by comparable substitutes at the expense of the Contractor. Contaminated soil and vegetation removed shall be disposed of in an approved waste disposal site. If the Contractor fails to respond to the Ministry Representative's requirements for cleanup, the Ministry reserves the right to take whatever action is necessary to clean up the spill and deduct incurred costs from any money due or to become due to the Contractor.

The Environmental Monitor shall document any spills observed in the vicinity of the Project that are not the result of Project-related activities, and notify the Environmental Agencies of such incidents.

165.14.03 Contaminated Sites – If an area within or adjacent to the project site has been identified as a possible source of contaminated or hazardous material and the defined contaminated or hazardous material has not been removed prior to the start of the project, the Contractor shall notify the Ministry Representative at least two weeks prior to the time work is scheduled in or adjacent to that area.

In the event that the Contractor locates material on the project site believed to be contaminated or hazardous, and which has not been previously identified, the Contractor shall immediately cease work in that area and notify the Ministry Representative. The material is to be left in-situ or be segregated and stockpiled, and handled in accordance

with the Ministry's "Management of Suspect Contaminated Materials", in SS165 Appendix 165-A.

In general, the Guidelines require segregating, testing by a AQP retained by the Contractor, handling, hauling, and disposal of the excavated material at a legally acceptable disposal site or at a licensed disposal facility.

Prior to any disposal, the Contractor shall identify the disposal site(s) to the Ministry Representative.

Contaminated material hauled to off-Site disposal will be paid at applicable Unit Price for Contaminated Soil Disposal or, where there is no such bid Item, as a Change to Work per GC 38.00.

The cost of other work associated with the suspect contaminated materials, such as testing, AQP time, and additional handling, each as approved by the Ministry Representative, will be paid as a Change to Work.

165.14.04 Concrete Wastes – The Contractor shall isolate fresh concrete or cement from any Watercourse for 48 hours after placement. Containers or trucks carrying cement or fresh concrete shall be washed at a site approved by the Ministry Representative. Concrete wastes, including wastewater from batching or cleaning, or cutting cured concrete shall only be disposed of at approved and Designated disposal sites (i.e., location identified with proper signing). All cement-contaminated wastewater from cleaning or mixing is to be considered toxic, and must be prevented from entering any Watercourse for at least 48 hours to allow the water to reach pH 6.5-8.0. Any cement or concrete-contaminated wastewater shall be tested prior to release into a Watercourse. In the event that such wastewater pH levels are greater than 8.0 or unacceptable in terms of water quality for fish or other aquatic species, the Contractor shall take the necessary measures to contain and treat such wastewater until acceptable pH levels are achieved (i.e., pH levels are the same as the receiving waters or 6.5 to 8.0). If the Contractor is not able to adhere to these specifications due to project-specific constraints, a proposal for alternate specifications must be reviewed and accepted by the Ministry Representative.

The Contractor shall securely store on-site and use suitable equipment and materials for the mitigation of concrete spills into or in areas adjacent to Watercourses. For example, cylinders of gaseous carbon dioxide shall be kept on the Project site, which shall be used in the event of concrete or concrete leachate discharges into any Watercourse. During any concrete pour within 15 m of, or in work areas above the High Water Mark of any Watercourse, the Contractor shall ensure that carbon dioxide cylinders and suitable application devices (e.g., weighted soaker hoses) are available on-site and ready for use in the carbonation of water columns to neutralize any concrete leachate that is inadvertently discharged into the Watercourse. Where concrete leachate is allowed to enter settling ponds, such that pH levels become very high, bubbling with carbon dioxide may not be an effective buffering agent. In such

cases, it may be necessary to use other methods, such as acid buffers.

Aged, broken concrete shall only be used as riprap with the approval of the Ministry Representative and the Environmental Agencies. The Contractor shall remove broken concrete or concrete wastes that have been inadvertently placed in non-approved sites on the Project and dispose of them at an approved waste disposal site.

165.14.05 Petroleum Wastes – Solids, sludges and other pollutants generated as a result of construction or removed during the course of treatment or control of wastewaters shall be disposed of in a manner that prevents their direct or indirect discharge to any Watercourse or groundwaters. Effluent generated by the water scrubber of an asphalt mixing plant must be given retention time in suitably sized, impermeable settling ponds prior to release into an adjacent Watercourse. The required retention time will be determined by the Ministry Representative in consultation with representatives from the Environmental Agencies.

Asphalt pavement shall be stockpiled for recycling in a specified location, or disposed of in a location approved by the provincial Environmental Agencies, as directed by the Ministry Representative.

165.15 Noxious Weeds and Herbicides – The use of all herbicides is subject to pre-approval by the Ministry Representative. The use of herbicides for vegetation control, other than invasive plants, is not permitted. The Contractor shall comply with all Federal, Provincial and local regulations relative to the storage, use, and proper disposal of herbicides, and must follow all requirements in the confirmed Integrated Pest Management Plan or Pesticide Use Licence for the area. Relevant legislation includes the Federal Pest Control Products Act and the British Columbia Integrated Pest Management Act and associated Regulations. All herbicide wastes, washwaters, solvents, and containers shall be disposed of in compliance with the British Columbia Environmental Management Act and Hazardous Waste Regulation.

The Contractor is responsible for undertaking invasive plant management for invasive plants and noxious weeds that will be disturbed by the Works. Management methods, disposal sites and options, as well as best management practices, will be outlined in the Contractor's Invasive Plant Management Plan, a component of the CEMP.

165.16 Air and Noise Pollution

165.16.01 Noise and Emissions – All activities, equipment, processes and work operated or performed by the Contractor in accomplishing the specified construction shall be in strict accordance with Federal, Provincial and local regulations governing noise levels and air emission standards. The Ministry Representative may require the Contractor to have maintenance performed on equipment or alter practices which are judged to produce excessive noise or emissions.

165.16.02 Idle Reduction – The Ministry is taking initiatives to reduce greenhouse gas emissions from road building activities and has identified reduced idling of construction vehicles and equipment as a reduction strategy. Further to SS 165.02.02 the Construction Environmental Management Plan, Air Quality and Dust Control Plan shall include procedures outlining how drivers and equipment operators will be engaged in idle reduction practices.

The Contractor is encouraged to develop innovative and practical methods for potential idle reduction opportunities applicable to the Project.

Specific areas of opportunity could include:

- (a) Locating vehicle staging areas to minimize impact of emissions
- (b) Implementing vehicle idling time restrictions
- (c) Use of appropriate and practical idle reduction technologies
- (d) Effective communication and reinforcement of idle reduction opportunities that are undertaken

165.16.03 Dust Control – Application and handling of the any dust palliative, with the exception of water, shall be in compliance with the Ministry documents as given in “Dust Management Environmental Best Practice for Highway Maintenance Activities” and "Maintenance Guidelines for Dust Palliatives and Gravel Road Stabilization".

Dust control techniques may also be required by the Ministry Representative during other construction operations, including but not limited to demolition, drilling, sand blasting and concrete cutting.

165.17 Use of Water

165.17.01 Authorization for Use – For any source of water not specified in the Special Provisions, the Contractor is required to contact British Columbia Ministry of Forests, Lands and Natural Resource Operations Water Stewardship Division office before any water is diverted, impounded, pumped or used for any purpose, including dust control, compaction, or operation of a work camp. Authorization in the form of an approval for short term use of water may be required and may be obtained through an application to FrontCounter BC. The Contractor shall be aware that, if required, a water application approval may take at least 140 days to process.

Prior to commencing any work which may affect potable water supplies downstream of the construction area, the Contractor shall ensure that all owners of licensed water intakes have been notified.

Water sources must be approved by the Ministry Representative. In order to reduce the impact on local water supplies and Watercourses, the Contractor may be required

to use uncontaminated wastewater, such as that pumped from the surface of sedimentation basins.

165.17.02 Screened Intake Requirements – The intakes of all pumps or diversions used to withdraw water from fish-bearing Watercourses shall be screened in accordance with the Fisheries and Oceans Canada "Freshwater Intake End-of-Pipe Fish Screen Guideline".

The Contractor shall provide and maintain stable access routes to all water withdrawal sites. The location of all such routes shall be subject to the approval of the Ministry Representative and the Environmental Agencies.

165.18 Support Facilities

165.18.01 Location and Operation of Support Facilities – The preservation of trees, shrubs, ground cover, fish and wildlife must be considered in site selection and construction of all temporary support facilities. During site preparation, vegetation shall be removed using selective hand clearing (close-cut with no grubbing) in preference to blanket clearing with heavy machinery, retaining vegetative cover wherever possible. The Contractor shall locate temporary field offices, storage, plant and other facilities on pre-existing cleared portions of the job-site or on areas to be cleared during the course of routine construction unless otherwise authorized by the Ministry Representative.

Where possible, for aesthetic purposes, work camps and parking areas shall be located behind a vegetative screen buffer. The Ministry Representative may require that work vehicles and heavy equipment shall be parked in Designated locations only.

The Contractor shall abide by all pollution control practices, laws, ordinances and regulations applicable to the construction of work camps and support facilities. The Contractor shall obtain the approval of the British Columbia Ministry of Health, the British Columbia Ministry of Environment, and any applicable local authority prior to the installation and/or operation of sewage disposal and potable water distribution systems.

Work camps, field office facilities and work sites shall be kept clear of litter and garbage. The Contractor shall be responsible for providing and regularly servicing animal-proof refuse containers and for instructing all employees to dispose of food wastes accordingly. Sanitary landfill operations shall be located well away from the main camp in order to minimize conflicts with wildlife.

Solid wastes shall be recycled or disposed at an approved landfill facility where possible. Burning of combustible solid wastes shall be subject to all applicable emission guidelines. Solid waste containers shall be large enough to contain all of the wastes generated between collection periods. The containers shall be constructed such that spilled liquids are contained and access by insects and wildlife is prevented. Storage areas and containers shall be maintained in a sanitary condition and shall be covered to prevent spreading of wastes by water, wind or animals.

Transportation of solid waste shall be performed in a manner which prevents littering during transit to the disposal site.

Sanitary landfill sites shall be located a minimum of 450 m from Watercourses or campsites, and shall be situated such that contamination of any stream, lake or groundwater system is avoided. The location of all sanitary landfill sites must be approved by British Columbia Ministry of Environment, Municipal or local officials and the Ministry Representative. The bottom of the landfill site shall be located at least 2 m above the water table. Trees and other vegetation shall be removed progressively as the size of the landfill increases in order to minimize erosion by wind and water.

Sanitary wastes shall not be discharged into Watercourses or on the soil surface. All temporary toilets shall be equipped with approved septic tanks having safe drainage or with closed holding tanks which are emptied only into approved treatment plants or sewage tanker trucks. All temporary toilets used on-site shall be placed in environmentally acceptable areas, and shall be secured to avoid or minimize damage from vandalism.

165.18.02 Decommissioning of Sites – Upon completion of construction, all temporary support facilities and camp infrastructure, including buildings, equipment, lumber, refuse, surplus materials, fencing and other such items shall be removed. The original drainage pattern shall be re-established, and all disturbed areas shall be revegetated. All decommissioned temporary diversions shall be plugged and stabilised. In the event that erosion has occurred during the course of construction, the Contractor will be required to re-contour, stabilise, and revegetate the affected areas.

The Contractor shall ensure that all project-related sites are left in an environmentally acceptable manner, subject to the approval of the Ministry Representative and the Environmental Agencies. Decommissioned construction areas, material sources and access roads shall be reclaimed by the Contractor such that as much of the original forest, wildlife, agricultural productivity, recreational use, etc. as possible is restored or enhanced.

165.19 Protection of Livestock and Wildlife

165.19.01 Protection of Livestock and Wildlife – Harassment of livestock or wildlife in and adjacent to the project site is prohibited. The presence of livestock or wildlife in or adjacent to the project site, field office trailers or construction camp shall not be encouraged by feeding. The Contractor is required to inform work crews of the location of wildlife and livestock crossing sites situated within the boundaries of the construction area.

If it is necessary to arrange the removal or transfer of beavers, bears or other wildlife from the work-site or camp the Contractor shall contact the appropriate Environmental Agencies and acquire any permits required prior to taking action, and follow the approved course of action.

165.19.02 Disturbance of Fish and Wildlife – Rules regarding hunting, fishing and the discharge of firearms by the Contractor, employees and subcontractors, within the project area during the period of construction, shall be made in consultation with the Ministry Representative and provincial Environmental Agencies. The Contractor shall be responsible for ensuring compliance with these rules.

165.20 Archaeological and Paleontological Discoveries – In the event that any item of archaeological, heritage, historical, cultural or scientific interest is found on the project site, the following Chance Find Procedure shall apply:

Such item(s) shall remain the property of the Province and the Contractor shall, on making or being advised of such a find, immediately cease operations in the affected area, minimize activities which create ground disturbance in and adjacent to the affected area, and notify the Ministry Representative. The Archaeology Branch of the British Columbia Ministry of Forests, Lands and Natural Resource Operations will be notified as per the Heritage Conservation Act. Work shall not resume within 30 m of the discovery site until an appropriate directive has been received from that agency.

To protect archaeological and paleontological sites that are situated within or adjacent to a project site, the Contractor may be required to use a variety of mitigative measures, including but not limited to drainage or erosion control, slope stabilization measures, or erecting fences or other suitable barriers to protect archaeological or paleontological sites that are situated within or adjacent to a project site. These measures, with any negotiated extensions of time for completion of the contract they require, will be determined and adopted at the discretion of the Ministry Representative. The costs associated with such mitigative measures will be borne by the Ministry, unless otherwise specified in the Special Provisions.

A buffer zone, in which no land alteration or other activity is permitted, may be required to ensure adequate site protection. The width of this buffer zone shall be determined by the Ministry Representative in consultation with a representative of the Archaeology Branch of the British Columbia Ministry of Forests, Lands and Natural Resource Operations. The Contractor shall be responsible for the actions of employees and subcontractors with respect to site vandalism and the unlicensed collection of artifacts from Designated archaeological sites in and around the construction area.

165.21 Resolution of Disputes – In the event that a dispute arises between the Contractor and the Ministry, or the Environmental Monitor and the Ministry regarding environmental matters related to the project, the Ministry, or where appropriate, the Environmental Agencies, will have the final decision.

Appendix 165-AManaging Suspect Contaminated Material

165-A.01 Suspect Contaminated Material – If material is identified within the Site which is believed to be contaminated or hazardous and has not been previously identified, the Contractor shall immediately cease work in that area, notify the Ministry Representative, engage the Contractor’s AQP, and follow the process below.

165-A.02 Field Screening and Managing the Sampling of ‘Suspect Material’ – Areas of soil contamination or other material may be encountered during construction activities.

If suspect contaminated material is encountered, field screening of suspect soil or material shall be completed. Field screening may include visual and olfactory observations for evidence of contamination during all excavation activities. Examples of evidence of contamination may include the presence of free product, sheen, staining, debris and/or hydrocarbon odours.

If field observations of contamination are identified, the AQP shall be on-Site to further classify the material and direct appropriate activities, subject to review and approval by the Ministry Representative in conjunction with the Ministry Geoscientist. The AQP shall complete additional field screening which shall be based on the AQP’s professional judgement of actions needed to comply with the Environmental Management Act, the Contaminated Sites Regulation and the Hazardous Waste Regulation. These actions may include, but not be limited to:

- collection of Combustible Headspace Readings (CHRs),
- field screening for volatile petroleum hydrocarbons and volatile organic compounds
- directing segregating and stockpiling activities and
- completing soil sampling for characterization and disposal purposes.

165-A.03 Characterization of Suspect Contaminated Material – The suspect contaminated material shall be tested in situ or stockpiled under the supervision of the AQP. It is preferred that suspect contaminated soil is left in situ until material is confirmed contaminated or, if immediate excavation is required, the soil may be stockpiled and segregated until laboratory results are received.

Soil samples shall be collected by the AQP in accordance with the Ministry of Environment’s *Technical Guidance 1 and 2 (TG1 and TG2)*.

<https://www2.gov.bc.ca/assets/gov/environment/air-land-water/site-remediation/docs/technical-guidance/tg01.pdf>

<https://www2.gov.bc.ca/assets/gov/environment/air-land-water/site-remediation/docs/technical-guidance/tg02.pdf>

Laboratory analysis of potential contaminants of concern (PCOCs) from soil samples shall be determined by the AQP and depend on the nature of the suspect material encountered during construction, including visual and olfactory observations and CHRs subject to review and acceptance by the Ministry Representative in conjunction with the Ministry Geoscientist. Analytical results shall be compared to *Schedule 3.1* and *Schedule 3.3* of the *Contaminated Sites Regulation*.

165-A.04 Excavation of Suspected or Actual Contaminated Material – If contamination in soil is suspected or identified in situ and a remedial excavation is required, the AQP shall be on-site and direct the remedial activities in accordance with *TG1* and *TG2*. The excavation areas shall be clearly marked in the field, prior to excavation. Care shall be taken to avoid mixing of clean and contaminated soils both vertically and laterally.

165-A.05 Confirmatory Soil Sampling – If a remedial excavation is required, confirmatory soil samples from the walls and base of the excavation shall be collected by the AQP upon completion of the excavation to confirm that contaminated soils have been removed.

Discrete confirmatory soil samples shall be collected from the walls and the floor of the excavation, at a sampling frequency in general accordance to guidelines established in *TG1*. The number of in-situ samples to be collected depends on the final extents and depth of the excavation. Samples (for potential laboratory submission) shall be collected at regular intervals, for example, within the initial 0.5 m, and every 1.0 m, on vertical faces, to the maximum depth of excavation. Soil samples from each wall and floor shall then be selected for analysis based on field observations, combustible headspace screening and/or initial sampling results and be submitted by the AQP to a laboratory for the analysis of identified contaminants of concern.

The AQP shall evaluate the laboratory analytical data from all confirmatory samples submitted against the applicable regulatory standards and guidelines. If any confirmatory sample have contaminant concentrations above the applicable standards, additional excavation activities may be required, subject to approval by the Ministry Representative. If the AQP suspects that contamination may originate from the project site and be migrating offsite, they shall immediately notify the Ministry Representative, who will contact the Ministry Geoscientist.

165-A.06 Dewatering of Contaminated Area – If an excavation in a contaminated area requires dewatering, the water shall be sampled and analyzed for the PCOCs

identified by AQP prior to dewatering. The water may be sampled with a vacuum truck with sufficient capability to reach the excavation area. The laboratory results shall be compared to Schedule 3.2 of the Contaminated Sites Regulation.

If the infiltrating water is confirmed to be contaminated or is required to be dewatered prior to receiving analytical results, the water shall be removed and stored in drums or an on-site storage tank or transported directly to an off-site approved facility for disposal. If the water is confirmed to be contaminated a waste approval authorization shall be initiated with the receiving facility prior to off-Site disposal. Copies of shipping documents and receipts of delivery shall be kept as part of a contaminated groundwater tracking and record keeping system and supplied to the Ministry Representative. Any discharged water shall be managed in accordance with SS 165.03.03 and SS 165.10.02.

165-A.07 Stockpiling of Suspect or Actual Contaminated Material – If suspect contaminated soils are excavated prior to final placement or disposal, they shall be segregated and stockpiled according to direction given from the AQP and the following guidelines:

- (a) The stockpile area shall be selected to prevent potentially contaminated soil or water runoff, or particulates in air from the stockpile(s) coming in contact with the surface water, or public walkways and recreation areas. The stockpile(s) shall be managed in accordance with SS 165.03.03 and SS 165.04.
- (b) Durable polyethylene or vinyl tarp material shall be and laid out underneath and overtop of any stockpiles established, with sandbags or other heavy items to sufficiently weigh down the tarps and prevent blow-away.
- (c) If free product or hydrocarbon sheen is observed in the stockpiled soils or intercepted water, absorbent pads and/or booms shall be placed in the down-gradient containment swales and disposed of accordingly.
- (d) The Contractor shall plan to maintain and monitor any stockpiles until laboratory analytical results from stockpile samples are available and a stockpile disposal strategy is prepared by the AQP. Once analytical results from the stockpile samples are available, this soil would either be transported off-Site for disposal, or re-used on-Site, if suitable for construction activities and subject to review and approval by the Ministry Representative in conjunction with the Ministry Geoscientist.

165-A.08 Excavation Backfilling – As remedial soil excavations may be within planned construction areas, the Ministry Representative will determine if immediate backfilling is required following remedial excavation. Requirements for analytical testing, if any, will be determined by the Ministry Representative in conjunction with a Ministry Geoscientist.

165-A.09 Disposal and Tracking of Contaminated Soil – If material is confirmed to be contaminated, it shall be disposed of at an off-Site disposal facility that is licensed to receive the material in question, based on comparisons of analytical data to the applicable guidelines.

If Hazardous Waste is identified at the Site, it shall be segregated and shipped separately from other contaminated soil, under hazardous waste manifest, as outlined below.

If debris is encountered during excavation activities and appears contaminated, it shall be segregated and stockpiled for characterization (if required) and off-Site disposal.

The Contractor shall provide the Ministry Representative a letter from the AQP that certifies that the contaminated material has been disposed appropriately to legally acceptable lands. If the material is disposed at a licensed facility the Contractor shall provide the manifest forms issued by the disposal site to the Ministry Representative. If any hazardous waste soils be identified at the Site, a hazardous waste generator number shall be issued to the Ministry, in accordance with procedures available at:

<https://www2.gov.bc.ca/gov/content/environment/waste-management/hazardous-waste/registration-of-hazardous-waste-generators-and-facilities>

165-A.10 Regulatory Requirements – If the material is determined to be contaminated and will be disposed offsite, the AQP shall complete a Notification of Independent Remediation Form (NIR) to notify the Ministry of Environment Director of Waste Management promptly upon initiating remediation and within ninety (90) days of completion.

The Ministry of Environment's *Administrative Guidance 9* provides information on the process:

<https://www2.gov.bc.ca/assets/gov/environment/air-land-water/site-remediation/docs/administrative-guidance/ag09.pdf>

Protocol 17 outlines the administrative requirements for the NIR:

https://www2.gov.bc.ca/assets/gov/environment/air-land-water/site-remediation/docs/protocols/protocol_17.pdf

If more than five cubic metres of material is remediated, a Site Risk Classification Report (SRCR) shall also be completed and submitted to the Director of Waste Management, as outlined in *Protocol 12*.

https://www2.gov.bc.ca/assets/gov/environment/air-land-water/site-remediation/docs/protocols/protocol_12.pdf

A copy of the NIR and SRCR (if applicable) shall be submitted to the Ministry Representative.

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TRAFFIC MANAGEMENT FOR WORK ZONES

PART A – GENERAL

194.01 General – Traffic management requirements shall be as specified herein and as specified in the Special Provisions or by the Ministry Representative.

The Contractor's Traffic Management Plan, Construction Schedule and its provisions for traffic control and safety shall be based on providing safe passage through the Work Zone for all road users (i.e. cyclists, pedestrians and motorists). The Contractor shall at all times make provisions for traffic through the Site to a sufficiently high standard to ensure the safety of the workers on the Project, the safety and the convenience of the travelling public, driver expectancy, and the protection of the Work Area. The Contractor shall supply and maintain all traffic control devices and personnel necessary to provide all traffic control.

The Contractor's accommodation for pedestrian, bicycle and vehicular traffic, and proposed traffic control procedures for each work operation will require acceptance by the Ministry Representative prior to the start of that operation.

Any one or more of the advance warning area, transition area, buffer space and termination area of the work zone may be outside the Limits of Construction, but this will in no way diminish the Contractor's responsibility to meet the requirements of the Traffic Management Plan.

194.02 Definitions – The following definition applies within this Section:

- “Traffic Management Manual” or “TMM” means the Ministry of Transportation and Infrastructure's 2020 Traffic Management Manual for Work on Roadways, as it may be amended or replaced from time to time.

The TMM is available on-line at:

<https://www2.gov.bc.ca/gov/content/transportation/transportation-infrastructure/engineering-standards-guidelines/traffic-engineering-safety/trafficmanagementmanual>

Other general terms used within this Section are defined in TMM Appendix A.1, including without limitation:

- Construction speed zone
- Traffic Management Plan
- Traffic Control Plan
- Incident Management Plan
- Public Information Plan
- Implementation Plan

- Work Activity Area

- Work Zone

194.03 Standard Documents – Traffic strategies and plans shall be developed in accordance with standards defined in the latest version of the following documents:

- The [Traffic Management Manual](#)
- The Ministry's [Electrical and Traffic Engineering Manual](#)
- All current Ministry [Technical Circulars](#) and [Technical Bulletins](#)
- [TAC Geometric Design Guide for Canadian Roads](#)
- The Ministry's [B.C. Supplement to TAC Geometric Design Guide](#)
- Workers' Compensation Board (WCB) [Occupational Health and Safety Regulation](#)

All Ministry publications listed above may be found, free of charge, on-line through:

<https://www2.gov.bc.ca/gov/content/transportation/transportation-infrastructure/engineering-standards-guidelines>

or can be purchased from:

CROWN PUBLICATIONS
(Queen's Printer for British Columbia)
PO BOX 9452 STN PROV GOVT
VICTORIA B.C. V8W 9V7
<https://www.crownpub.bc.ca/Home/About>

Email: crownpub@gov.bc.ca

PHONE: 1-800-663-6105

FAX: (250) 387-1120

194.04 Traffic Control Supervisor – The Contractor shall designate a Traffic Control Supervisor who has the Contractor's authority to respond to traffic control requirements and who shall provide surveillance of traffic conditions through the Work Zone. This person, or qualified designate, shall be available at all times, both when the Contractor is working and during weekends or other periods when the Contractor's forces are not active on the Project, to respond to calls from the police and from representatives of the Ministry concerning damage or deficiencies in the traffic control devices and the passage for traffic through the Work Zone.

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The traffic control supervisor shall meet the requirements of the TMM and WCB Regulation and shall not be the Contractor's site superintendent.

194.05 Compliance with Acts and Regulations – The Contractor shall comply with the requirements of any legislation, Regulations and by-laws as are in force for the regulation of traffic or use of any road upon or over which it is necessary to carry out work or to haul materials or things.

PART B – PLANNING AND SCHEDULING

194.11 Provide Traffic Management Plan – The Contractor shall prepare a Traffic Management Plan (TMP) in accordance with the Ministry definitions and guidelines specified in the Traffic Management Manual.

The Special Provisions or the Ministry Representative will specify any required traffic management conditions, strategies, and required documentation for the Traffic Management Plan.

The Traffic Management Plan shall be fully integrated with the Contractor's plan and schedule for carrying out the work, comply with the requirements of the TMM, the conditions and strategies listed below, and with the provisions of the Contract.

194.12 Traffic Management Plan Review – The Ministry Representative may review the Contractor's Traffic Management Plan for general conformance with the requirements of the Special Provisions and the Contract for sufficiency of detail to provide a complete and unambiguous traffic control layout. No review or change requested by the Ministry Representative in any way relieves the Contractor of any of its responsibilities for ensuring safe and adequate traffic management. No work that affects traffic will be permitted until the Ministry Representative accepts the Traffic Management Plan.

Where the TMM shows a choice of traffic control layouts for a given specific situation, the Ministry Representative may request the Contractor to use an alternative layout which is acceptable.

194.12.01 Review Time Periods – The following time periods, subject to SS 145.34, are required for plan review and acceptance unless otherwise specified in the Special Provisions or by the Ministry Representative:

- (a) A Traffic Control Plan shall be submitted to the Ministry Representative for review at least fourteen (14) days prior to the start of any Works effecting traffic flows. The following conditions shall apply:
 - (i) Any plan modifications shall be submitted to the Ministry Representative for review at least fourteen (14) days prior to any changes being made in the field.
 - (ii) A staged plan shall be submitted to the Ministry Representative for review at least fifteen (15)

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working days prior to implementation of a new stage.

- (b) The Implementation Plan, if required, shall be submitted to the Ministry Representative for review at least twenty-one (21) days prior to the start of any Works effecting traffic flows.
- (c) The Public Information Plan, if required, shall be submitted to the Ministry Representative for review at least twenty-one (21) days prior to the start of any Works effecting traffic flows.
- (d) The Incident Management Plan, if required, shall be submitted to the Ministry Representative for review at least twenty-one (21) days prior to the start of any Works effecting traffic flows.

PART C – TRAFFIC MANAGEMENT OPERATIONAL DETAILS

194.21 Speed Zones – Reduction in speed limits may take several forms as defined in the TMM 2.4 Management of Speed. Construction speed zones within the Project will be determined at the discretion of the Road Authority or as specified in the Special Provisions.

The Contractor's work plan, Construction Schedule and its provisions for traffic control and safety shall include references to any reduced posted speed limits.

The location and length of posted construction speed zones shall be evaluated daily, or more frequently as appropriate to the Work, to provide the shortest construction speed zone practical. For example, on a resurfacing project, the reduced speed limit would usually be invoked around active operations such as milling, priming, paving, and, if necessary, at access points to pits and the plant, but will not generally be acceptable for areas of the Site used only for haul.

194.22 Use of Channelizing Devices and Taper Rates – Traffic cones, tubular markers, barrels/drums, barricades or other channelizing devices shall be used in the following circumstances for guiding and directing road users:

- (a) from one lane to another or for merging traffic because of a reduction in the number of lanes
- (b) around a hazard on the travelled way
- (c) through an area of unfinished construction where the intended traffic lanes are not self-evident
- (d) at any other location where, in the Ministry Representative's opinion, channelizing will ensure the safe passage of public traffic.

Minimum taper rates and spacing between devices shall be as shown in TMM.

194.23 Equipment on the Travelled Way – When working on the travelled way, equipment shall be positioned to

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present a minimum of interference and hazard to the travelling public. Equipment units shall be kept as close together as working conditions will permit and preferably on the same side of the travelled way. Equipment shall not be left overnight on the travelled way but shall be located so as to avoid the possibility of collision.

194.24 Roadway Configuration – The Contractor shall arrange to keep interconnected lanes of the highway open to traffic at all times with the least amount of inconvenience to the travelling public unless otherwise authorized by the Ministry Representative.

194.25 Roadway Condition – The Contractor shall keep the travelled way well-graded, free of potholes and of sufficient width that the required number of lanes of traffic may pass. The subgrade shall be completed progressively in order that gravel surfacing may follow in an orderly manner. Prior to tearing up a portion of the existing travelled way the Contractor shall obtain Ministry Representative approval and shall also have available a sufficient supply of crushed gravel, or another approved product, to ensure a smooth riding surface during the work.

194.26 Closures – Road closures shall be in accordance with the Special Provisions.

Road closures will not be permitted until the Contractor requests and receives written permission from the Ministry Representative. This request shall be submitted in writing not less than fourteen (14) days in advance of the closure.

Upon receiving written permission for the closure, the Contractor must notify the local police, fire, ambulance, municipality, school board, and public transit at least seven (7) days in advance of the closure.

The public must be notified by local radio and newspaper at least seven (7) days in advance of the closure. The Contractor shall arrange any public announcement relating to the Contract and incorporate the Ministry logo through the Ministry Representative and Corporate Writing Services. Such advertisements will be made at the Ministry's expense. Notices or other advertisements of the Contractor unrelated to the Contract, or where the Ministry's logo or name is not incorporated, may be made by the Contractor at the Contractor's expense.

194.27 Detours – Where detours or diversions are required for the execution of the work or deemed necessary by the Ministry Representative, the Contractor shall provide detours or temporary roads to facilitate the passage of traffic around the restricted construction area in accordance with Special Provisions or as otherwise directed by the Ministry Representative. The Contractor shall be responsible for all signs and their maintenance, and maintenance of the detour or temporary road. The Contractor shall consider the condition of the pavement used for detours and its impact on the safety and function of the detour. Milled surfaces upon which traffic is to run shall be clean and allow adequate drainage.

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All detours shall be adequately signed such that the detour route is clear and unambiguous to all road users.

The minimum design requirements for any detours will be specified in the Special Provisions or by the Ministry Representative.

Where the anticipated detour or diversion extends beyond the existing road surface, the Contractor shall submit an engineering design to the Ministry Representative for review, a minimum of fourteen (14) days in advance of building the detour. The engineering design shall include the design speed, the horizontal and vertical geometry, plus evidence that the design vehicle is accommodated for all movements and that all the requirements of this Contract are satisfied.

Where the anticipated deviation is minor, the Contractor may apply to the Ministry Representative, in writing, for approval to proceed without a formal design. Such approval does not relieve the Contractor of any of its responsibilities for provision of a safe site.

194.28 Access to Property – During operations the Contractor shall provide and maintain reasonable road access and egress to property fronting along, or in the vicinity of, the work unless other reasonable means of road access exists. The Ministry Representative will be the sole judge of what may be deemed reasonable road access.

194.29 Seasonal Lay-Up – In the event that the Work is stopped due to seasonal lay-up conditions (typically winter conditions or high traffic periods), the Site is to be left in a condition as specified in the Special Provisions or as deemed acceptable by the Ministry Representative.

194.30 Delays – All acceptable major and minor delays to the travelling public will be determined by the Ministry Representative.

The Contractor shall monitor the queue lengths created by any delay to the flow of traffic. Traffic must be cleared prior to starting another delay unless authorized by the Ministry Representative. If the Ministry Representative determines that the traffic delays are excessive, the Contractor shall immediately cease construction activities and make all the travel lanes available to traffic as quickly as possible. Resumption of roadway operations shall be permitted as traffic levels dictate and upon approval by the Ministry Representative.

PART D – TRAFFIC CONTROL DEVICES

194.41 Portable Traffic Signals – Portable traffic signals, in accordance with TMM 4.8, may be used to provide bi-directional traffic control. Portable traffic signals are used primarily on longer term work zones. Typically, a pair of signals is set up at the perimeter of a roadway construction site.

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Each signal unit shall have at least two signal heads for each approach and shall be positioned so that at least one signal head is overhead and one is side-mounted. Each signal head shall have a 3 lamp (red, yellow, green) signal display.

The use of portable traffic signals falls into two categories:

Category 1 is a fixed time signal used for short duration work in low speed environments of ≤ 60 km/h, and where advance warning flashers are not required. A traffic engineer is not required to prepare the timing sheet for the signal.

Category 2 is an actuated or fixed time signal used for long duration work, and/or in high speed environments ≥ 70 km/h, and/or where advance warning flashers are required. A Traffic Engineer is required to prepare the timing sheet for the signal.

The Contractor shall submit a request for the use of portable traffic signals to the Ministry Representative that includes:

- The site layout
- Anticipated volumes
- Requirements for advance warning signs
- The category of portable traffic signal proposed
- Proposed signal timing plan(s)

The acceptance of the signal installation will be determined by the Ministry Representative.

If the proposed traffic signal installation is accepted by the Ministry Representative, the Contractor shall:

- Implement any portable traffic signals in accordance with the Traffic Management Manual
- For category 2 signals, prepare a traffic signal timing plan, signed and sealed by a Traffic Engineer registered with the APEGBC
- Supply and install all traffic controller equipment
- Supply power
- Operate and maintain the signals

Traffic controller technology used shall comply with industry standard practice and shall have:

- Manual override to hold the signal in green
- A conflict monitoring mechanism that will ensure both signals in a pair cannot show green simultaneously
- Ability to revert to flashing red mode if a fault is detected

Where the work activity impacts the operation of an existing signal, the Contractor must submit a written notification to the Ministry Representative a minimum of ten (10) days in advance of the disruption to the signal.

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194.42 Vehicles as Traffic Control Devices – The use of shadow vehicles, buffer vehicles, vehicle-mounted crash attenuators, and/or pilot cars may be specified or warranted, and shall be supplied and operated in accordance with the TMM.

When required, the Contractor shall provide sufficient pilot cars, as required, to keep a steady and controlled flow of traffic moving around or through the construction area.

194.43 Use of Traffic Control Persons (TCP) – It is generally expected that TCP and/or Portable Traffic Signals will be required in the following situations:

- (a) when public traffic is required to pass working vehicles or equipment which may block all or part of the travelled roadway.
- (b) when it is necessary to institute a one-way traffic system through a construction area or other blockage where traffic volumes are heavy, approach speeds are high, and a traffic signal system is not in use.
- (c) in high volume areas where temporary protection is required while other traffic control devices (barricades, cones, signs, etc.) are being erected or taken down.
- (d) for emergency protection when other traffic control devices are not readily available.
- (e) in all situations, where adequate protection for workers, working equipment and public traffic is not provided by other traffic control devices.
- (f) Initiate traffic stoppages and reinstate flow of traffic to facilitate construction activities.

194.44 Traffic Signs – All standard signs, new and replacement, shall meet current Ministry specifications. The Ministry Specifications for Standard Highway Sign Materials, Fabrication and Supply are located on our Ministry web page at:

https://www2.gov.bc.ca/assets/gov/driving-and-transportation/transportation-infrastructure/engineering-standards-and-guidelines/traffic-engineering-and-safety/traffic-engineering/traffic-signs-and-pavement-markings/standard-traffic-signs/specifications/sign_fabrication_specs.pdf

The Contractor shall supply and maintain all necessary signs and traffic control devices required by the Contract, this specification, and the TMM or as ordered by the Ministry Representative. All signs and devices used must conform to the standards detailed in the TMM.

194.45 Barricades, Lights, Delineators, Temporary Pavement Markings – The Contractor shall provide, erect and maintain all types of temporary traffic control devices including temporary pavement markings, barricades, fences, delineators, amber flashing lights, in order to ensure safety to the workers and the general public. These devices

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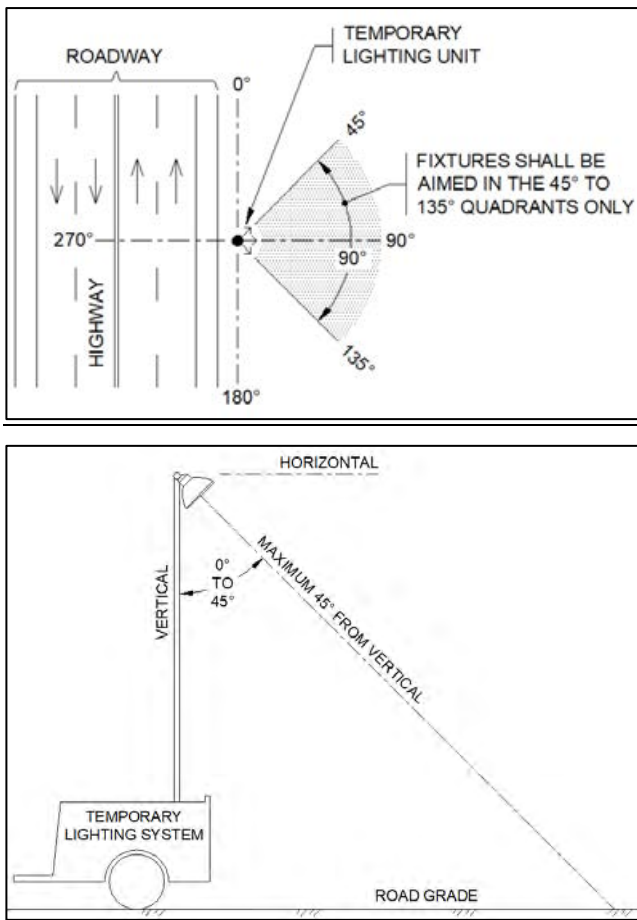
shall be as required by the Contract and the TMM, or as directed by the Ministry Representative.

Flashing amber lights, if deemed necessary by the Traffic Control Plan, are used for extra night time effectiveness and the batteries of the flashing lights shall be checked each day.

Flashing lights, if two-faced, and if used to mark the right-hand edge of a travelled lane, must have one face covered so that the light is not visible to traffic travelling in the other direction. Flashing lights having two faces showing in opposite directions may be used only if it is necessary that the obstruction or lane edge being marked be seen from both directions of travel. They should not be used in a line of delineators through an area of incomplete construction except where traffic must be diverted and a specific hazard exists.

Temporary road construction lighting installations shall be positioned as shown in Figure 194-1.

Figure 194-1: Positioning of Temporary Lighting Adjacent to a Highway



To minimize glare and resulting impaired driver vision, lights shall be aimed away from any approaching traffic and at an angle of less than 45 degrees above vertical. Where lights cannot be located as shown then temporary glare screens shall be used to shield the traffic from the light

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source. These glare screens shall be metal or an opaque plastic material and be designed to shield the lighting from the eyes of the driver.

The Contractor shall be responsible for the application and removal of all temporary pavement marking and traffic control devices. When traffic lanes have to be redefined for Long Duration Work, the Contractor shall eradicate all redundant temporary or pavement markings that are not required for the intended traffic patterns.

Class I and Class II standard barricades are illustrated in the TMM. Temporary delineator posts shall be as shown on SS Drawing SP194.02. In speed zones of 50 km/h or less, the minimum height of traffic cones shall be 450 mm. Where the maximum speed limit is over 50 km/h, the minimum height of cones shall be 700 mm.

194.46 Dynamic Message Signs –The use of dynamic message signs (DMS) may be specified in the Contract or by the Ministry Representative. Each sign must be portable. When in operation, the DMS shall be a minimum of two (2) metres from the bottom of the sign display to the road surface and shall be level and capable of pivoting for sighting purposes. The specifications of the sign are as follows:

Sign unit:	Yellow/Orange LED display
Sign display:	3 lines with 8 characters per line
Character size:	450mm (18 inches)
Character matrix:	5 x 7
Remote dial-up access:	By cellular phone or equivalent

A full matrix sign may be used given that it has the display parameters noted above.

The Contractor is responsible for the DMS including but not limited to bringing the signs to the Project, moving and setting-up, providing storage as required including moving the signs into and out of storage, maintaining the signs in full operating condition, ensuring the correct message is being displayed, damage to or loss of the signs and returning the signs to the supplier. The Contractor shall provide the Ministry Representative with a description of the messages displayed and procedures for changing DMS messages when the Contractor is on and off site.

PART E – GENERAL MAINTENANCE

194.51 Maintenance – The Contractor shall be responsible for the continuing maintenance of all traffic control devices in use as follows:

- Signs, etc., not applying to existing conditions shall be removed or covered. If covered, such as after sundown or during periods when work is suspended, the covering shall be placed over the sign so as to completely obscure the message thereon. Where operations are

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carried out in stages, only those devices or signs, which apply to the current stage, are to be left in place or uncovered.

- (b) Signs shall be checked daily for legibility, damage, suitability and location. Dirty, damaged, unsuitable or misplaced signs shall be cleaned, repaired or replaced. Signs and delineators with a reflective surface shall be cleaned as frequently as necessary to ensure full reflectivity. As considerable damage may be inflicted on signs and devices during a weekend, the Contractor shall conduct a check for replacement or repairs before the rush hour on Monday morning. Similarly, high traffic volume and recreational routes, and any site that suffers repeated damage, shall be checked during the weekend or at any other time that works is not in progress.
- (c) During hours when headlights are required, signs that are not retroreflective shall be illuminated. After sundown all signs shall be checked for visibility and those that cannot be seen clearly shall be adjusted or replaced.
- (d) Channelizing devices critical to the guidance of road users through the work zone shall be checked daily, or more frequently as specified by the Contract or Ministry Representative, for proper placement.

PART F – PAYMENT

194.91 Payment – The cost of all Traffic Management requirements, including the construction, staging, maintenance, and demolition of detours, and provision of pilot cars, all flagging and traffic control equipment, and personnel covered in this Section and the Contract Documents, or as ordered by the Ministry Representative, shall be considered to be included within the Lump Sum bid for Traffic Management, unless otherwise specified in the Special Provisions.

(a) Payment for Detour Construction – Where the Contract specifies that detour construction will be paid, such payment shall be as follows, based on the detour Design (whether prepared by the Ministry or by the Contractor), staging plan, and demolition plan, all as approved by the Ministry Representative:

(i) Contractor-Designed Detour: Any detour designed by the Contractor will be paid only as a Lump Sum and must:

- be compliant with the Contract,

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- be of reasonable scope, in the opinion of the Ministry Representative, and
- be approved by the Ministry Representative.

(ii) Lump Sum – If payment is specified as a Lump Sum, the Lump Sum price bid shall be full compensation for costs to construct, stage, and demolish all detours. Progress payments shall be determined by the Ministry Representative, in consultation with the Contractor.

(iii) Unit Prices – If payment is specified as being at the Unit Prices, quantities of Work required will be paid under the applicable Unit Price Items bid, measured and paid for each movement of material for construction, staging, and demolition of the detour. Where Work is required that is outside the scope of the existing Unit Prices, those costs shall be incidental to the Work and will not be paid separately.

(iv) Full Costs – In all cases, the payments made under SS 194.91(a)(ii) and (iii) inclusive shall be full compensation for any and all Work necessary to complete the construction of the detour, stage any changes required to the detour configurations, and to demolish the detour.

(v) Detour Maintenance – Payment for maintenance of any detour, whether detour construction is paid or incidental, will be included within the Lump Sum paid for Traffic Management.

(b) Payment for Pilot Cars and Traffic Control Persons – Where pilot cars and traffic control persons are included as bid items in the Contract Documents, a record of the days and hours that each pilot car and/or traffic control person was employed shall be submitted daily by the Contractor for verification and approval of the Ministry Representative. In such event the overtime “hours worked” by traffic control persons in accordance with the current union shall be expressed in equivalent straight time “hours earned” for payment at the rate bid. Pilot cars will be paid for at the all found rate bid in the Schedule for the actual number of hours operated as such.

SECTION 200

CLEARING AND GRUBBING

200.01 Clearing – Except for trees and shrubs to be preserved as indicated on the Drawings or designated by the Ministry Representative, "Clearing" shall mean the complete removal and disposal of all standing and fallen trees, stumps, logs, upturned roots, rotten wood and all other vegetation growth, and accumulations of rubbish of whatsoever nature, and any other objectionable material from the area shown on the Drawings, or as directed by the Ministry Representative. Protective measures for the protection and retention of vegetation are specified in SS 769.

Unless otherwise specified in the Contract, Clearing shall also include the removal of existing cribwork, guard rail, fences, siphon structures built of timber and/or metal, culvert pipes of all types and timber box culverts within the limits of the work.

Clearing shall not include the removal of concrete buildings or their foundations, concrete bridge piers or abutments and their footings, concrete siphon boxes or concrete box culverts. These items shall be classified as Type A (Solid Rock) and will be paid for as such in accordance with the measurements established by the Ministry Representative, and, where applicable, at the unit price bid in the Schedule of Approximate Quantities, and Unit Prices for Type A (Solid Rock). If the Contract does not have a Type A Item, the associated work will be paid as a Change to Work under GC 38.00.

200.01.01 Area – The area(s) to be cleared and/or grubbed shall include those areas required for the highway right-of-way, Ministry provided borrow pits, and quarry sites, Ministry or Crown gravel pits and access roads thereto, if ordered by the Ministry Representative; off-take ditches or other drainage ditches and channels for stream diversions either within or without the right-of-way; clearing of debris in streams as indicated on the Drawings, or as directed by the Ministry Representative within the Site; foundations for structures and the right-of-way as shown on the Drawings, or as directed by the Ministry Representative, for intersecting public and private road approaches or diversions of same outside the main highway right-of-way. All such areas are hereinafter referred to as the "Area."

The Contractor shall stake the limits of clearing and grubbing as defined on the Drawings. Prior to commencing the clearing and grubbing operation the Contractor shall notify the Ministry Representative of the areas to be worked. The Ministry Representative may adjust the limits of clearing and grubbing and adjust the payment accordingly.

200.01.02 Timing – Clearing and Grubbing shall only be undertaken outside the bird nesting window identified for

the Site to avoid contravention of environmental legislation (including without limitation: the [Migratory Birds Convention Act](#), the [Migratory Birds Regulations, Section 34 of the Wildlife Act](#)), unless an appropriately qualified professional conducts a survey to determine there are no occupied bird nests. The survey shall be the responsibility of the Contractor and shall be performed to the satisfaction of the Ministry Representative. If required, the survey shall be considered incidental to the Work and no additional payment will be made.

Clearing must be carried out and completed in a timely fashion, to a distance and width as necessary to facilitate utility relocation and grading operations.

200.01.03 Merchantable Timber

(a) License to Cut

(i) Ministry Obtained – Unless the Contract states otherwise, authorization from the Ministry of Forests, Lands and Natural Resource Operations (Forestry) to cut and remove trees from the Project limits will be obtained by the Ministry. Once this authorization is approved, Forestry will then issue the License to Cut to the Contractor prior to the commencement of the Clearing and Grubbing operation.

(ii) Contractor Obtained – When specified in the Contract, it shall be the Contractor's responsibility to obtain a License to Cut and Remove Timber from the Ministry of Forests, Lands and Natural Resource Operations prior to the removal of merchantable timber from within the Project limits. The Contractor shall submit a copy of the License to Cut and Remove Timber to the Ministry Representative prior to commencing any clearing operations.

(iii) Compliance – The Contractor shall fully comply with all Ministry of Forests, Lands and Natural Resource Operations regulations and the License to Cut.

(b) Merchantable Species – Merchantable timber shall be as defined by the Ministry of Forests, Lands and Natural Resource Operations. It shall be the Contractor's responsibility to determine which species the Ministry of Forests, Lands and Natural Resource Operations will class as merchantable.

(c) Ownership – Merchantable timber in areas of the highway right-of-way will become the property of the Contractor and shall be removed by the Contractor in accordance with the [Forest Act](#).

(d) **Processing** – All merchantable timber shall be processed according to industry standards. The Contractor can be charged by the Ministry of Forests, Lands and Natural Resource Operations for any merchantable timber willfully damaged or destroyed during construction operations. Any fines resulting from non-compliance of the license requirements by the Ministry of Forests, Lands and Natural Resource Operations shall be the responsibility of the Contractor and as such, will be deducted from the Contractor's progress payments until proof of payment has been presented to the Ministry Representative.

200.01.04 Vegetation Retention – All trees shall be felled within the Area but, in the event of any trees falling outside the Area, such trees shall be cut up and together with all debris and slash therefrom, disposed of in accordance with the Contract.

The Ministry Representative may designate certain trees or shrubbery to be left standing, in which case the Contractor shall take every precaution not to damage or injure such trees or shrubbery in felling adjacent timber, burning or any other clearing operations as specified in SS 769.

Such trees or shrubbery are to be limbed or thinned to such height and extent as may be directed by the Ministry Representative. No deduction will be made in the total quantity of clearing for any trees or shrubbery ordered to be left standing; no additional payment will be made for any limbing or thinning of such trees or shrubbery as may be directed by the Ministry Representative.

200.02 Disposal of Material

200.02.01 General – Unmerchantable timber, stumps, etc., shall not be disposed of by pushing outside the clearing and grubbing right-of-way.

200.02.01 Chipping – Organic debris from clearing and grubbing, including stumps, may be reduced by chipping, grinding or other means, mixed with the organic stripping, and disposed of in accordance with the following conditions:

- (a) The organic debris has no dimension exceeding 75 mm,
- (b) The organic debris is uniformly mixed with the unsuitable material at a rate not exceeding 30 parts total organic debris to 100 parts organic stripping material by volume,
- (c) Material shall either be disposed of in the designated disposal area or stockpiled for future use at the direction of the Ministry Representative, and
- (d) The resulting mixture is spread only in locations and to depths approved by the Ministry Representative.

200.02.02 Burning – Where permitted, any burning shall be carried out subject to the Provisions of the Open Burning Regulations and the Forest Act and regulations thereto.

Where burning is permitted, associated slash and debris shall be piled and burned at points located centrally in the Area.

The number of fires to be started at any one time shall be limited to the capacity of the Contractor's equipment and organization to provide adequate protection against the spreading of the fires to adjacent timber or property.

In the event of Forestry regulations prohibiting burning, the slash, debris, etc., may be piled along the sides of the Area and burned at a later time when permitted by the regulations. Where the regulations do not allow burning prior to the Completion Date, the material shall be disposed of off-Site prior to the Completion date.

200.02.03 Off-Site Disposal – Any debris from clearing and grubbing destined for off-Site disposal shall be disposed of at an off-Site disposal site provided by the Contractor, in accordance with SS 145.27.02, or at other locations as permitted under the Contract.

200.03 Grubbing – Grubbing means the entire removal and disposal by burning as described in SS 200.01 "Clearing" and SS 200.02 "Disposal of Material" of all stumps, roots and embedded logs to a depth of 0.6 m below the ground line.

Grubbing shall be carried out over the entire Area as defined in SS 200.01 "Clearing." Grubbing shall be fully completed at least 300 m in advance of grading operations.

200.04 Danger Trees – This item shall consist of removing, on written order of the Ministry Representative, any leaning or dangerous trees or snags outside the clearing Area.

Such trees shall be felled and disposed of in accordance with SS 200.01 "Clearing" and SS 200.02 "Disposal of Material."

The Contractor will not be required to grub the stumps of any danger trees.

200.06 Mailboxes – Where it becomes necessary to remove or otherwise disturb existing mail boxes within the limits of any project, the Contractor will be required to install the boxes temporarily in such a position that their usefulness will not be impaired. After construction work has been completed, the boxes shall be re-installed at positions designated by the Ministry Representative. No new posts or materials will be required, except that any damage caused by the Contractor to either boxes or supports, due to the Contractor's negligence, shall be repaired by the Contractor at the Contractor's expense.

All costs in connection with the removal and replacement of the mailboxes, as specified above, shall be considered as incidental to the construction of any improvement and shall be included in the unit prices for the various pay items of work involved in the Contract. No additional compensation will be made.

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CLEARING AND GRUBBING

200.07 Removal of Existing Signs and Symbols – The Contractor shall remove all existing signs and their associated structures as shown on the Drawings and, where applicable, in accordance with SS 635.

The Contractor must obtain approval from the Ministry Representative prior to removing any signing equipment and, if so directed by the Ministry Representative, existing signs shall not be removed until new signs or temporary signs have been installed.

The Contractor shall remove and stockpile all existing sign faces, boards, redundant posts and bases at the Ministry Maintenance Contractor's closest yard, or as otherwise directed by the Contract or the Ministry Representative.

Voids remaining after the removal of sign bases shall be backfilled and compacted with suitable native materials.

Existing Ministry signs, which are not marked for removal or relocation but are in conflict with the progress of the Project, shall be removed and temporarily relocated as approved by the Ministry Representative. No separate payment will be made for these temporary relocations which shall be considered incidental to the Works and no additional payment will be made.

MEASUREMENT AND PAYMENT

200.91 Clearing – CLEARING will be paid for at the unit price bid per hectare of clearing and will, unless otherwise specified, comprise the full length and breadth of the right-of-way as shown on the Drawings or as directed by the Ministry Representative. The price shall be accepted as full compensation for everything furnished and done in connection therewith, regardless of the nature or condition of the Area.

200.92 Merchantable Timber – All costs associated with sorting, loading hauling and cold-decking, and disposal of merchantable timber, including but not limited to, the provision of the maintenance of the cold-deck sites and the construction and maintenance of haul routes and other roads shall be considered incidental to the Work and no additional payment will be made. No separate payment will be made in respect to the time taken to remove the timber from the cold-deck sites, and all costs resulting from the presence of

timber in the cold-decking sites, until such time as it may be removed, including but not limited to, the cost of property, of scheduling the Work, of performing additional Work, and of delay in the Work.

200.93 Grubbing – GRUBBING will be paid for at the unit price bid per hectare of grubbing and will, unless otherwise specified, comprise the full length and breadth of the right-of-way as shown on the Drawings or as directed by the Ministry Representative. The price shall be accepted as full compensation for everything furnished and done in connection therewith, regardless of the nature or condition of the Area.

200.94 Danger Trees – DANGER TREES will be measured in three sizes according to the diameter of the tree trunk measured at a point 1.25 m above the ground line at the base of the tree, with compensation for removal paid in accordance with Table 200-A..

Table 200-A: Danger Tree Removal Compensation

Size (m) @ 1.25 m above ground	Compensation
< 0.75	\$140.00
0.75 to 1.5	\$330.00
>1.5	\$550.00

200.95 Remove Existing Signs – Payment for REMOVE EXISTING SIGNS including, but not limited to, excavation, removal, hauling, disposal of sign faces, removal, and stockpiling or disposal of existing posts and bases, and backfilling the excavation will be made at the Unit Price bid per sign removal and shall be considered full compensation for everything furnished and done in connection therewith. Signs without posts (pole-mounted, etc.) shall be considered the same as signs with posts for payment purposes.

No separate payment will be made for the removal and disposal of delineator posts which shall be considered incidental to the Works and no additional payment will be made.

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ROADWAY AND DRAINAGE EXCAVATION

DESCRIPTION

201.01 General – Roadway and Drainage Excavation shall include:

- (a) all excavation and the construction of all embankments required for the formation of the roadbed, pullouts, parking areas and look-outs;
- (b) excavation for any drainage ditch, off-take ditch or channel for stream diversion within or outside the right-of-way;
- (c) removal of surcharge material, topsoil and organic waste material from the roadway, and of topsoil and unsuitable overburden from any Ministry provided borrow pit or any available Ministry or Crown gravel pit;
- (d) excavation of materials below grade; excavation of borrow pits; the grading necessary to construct any frontage road, gravel pit access road, borrow pit access road or any other access road, public or private, either within or outside the right-of-way, only to Ministry provided sources as may be ordered by the Ministry Representative;
- (e) the grading of connections to intersecting roads, public or private, either within or outside the right-of-way;
- (f) the hauling and disposal of all excavated material, and the trimming and shaping of all excavations and embankments.

All Works shall be confined to the right-of-way except where agreements for access rights are in place for other lands, public or private.

201.02 Extent of Work – The dimensions of the excavations and embankments shall be as shown on the Contract Drawings but the Ministry Representative may increase or decrease roadbed dimensions and slope angles.

Construction Grade is defined as the grade upon which the Asphaltic Pavement is laid.

Subgrade is defined as the grade upon which the first layer of select granular subbase or base material is laid.

MATERIALS

201.11 Description of Material Types – Excavation shall be classified by the Ministry Representative, under the following material types:

(a) Type A

(b) Type D, with potential sub-classes

- Common
- Topsoil
- Stripping
- Suitable
- Unsuitable

The material encountered in any project excavation shall be classified as one of the two types listed above.

In no case shall a material be classified using percentages of two or more types.

201.11.01 Type A - Solid Rock – Type A shall, without limitation, include all forms of “solid rock in place” including solid formations, masses, ledges, seams or layers of dense sedimentary, igneous or metamorphic material of sufficient hardness generally requiring drilling and blasting methods, very heavy ripping, or equivalent methods, before excavation and removal.

Type A shall also include detached masses of rock or boulders individually containing a volume of 2.0 m³ or more.

The breaking and removal of frozen materials shall not be considered Type A excavation.

201.11.02 Type D – Common – Common material is all other excavation materials of a nature not included in the foregoing description of Type A, regardless of the nature or condition of the material, or the method used to excavate or remove. Type D may be broken down into further sub-classes for payment, generally depending on handling requirements and intended use:

(a) Topsoil – typically the upper organic soil layer consisting of intermixed humus, minerals and composted materials, well suited as a growth medium for plants and unsuitable for embankment construction. Used only where planting beds require higher quality growth medium.

(b) Stripping – Stripping is a sub-class of Type D that is material typically near or at the surface and contains organics, root mass, or vegetation., which is typically unsuitable for embankment construction but may be suitable for slope dressing. Mulched materials, depending on the size of the chips and proportion of soil to chips, may be considered as Stripping. Where a Contract has no specific use for Topsoil, Topsoil may be included within Stripping

(c) Suitable – material suitable for embankment construction.

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(d) Unsuitable – material which is unsuitable for embankment construction but is not Stripping or Topsoil. May include mulching, where chip sizes are large or soil to chip proportion is low.

201.12 Material Classification Changes – It is possible that the material in a cut may change at some level in the excavation and may change more than once in a single cut.

Where such a change occurs, the Contractor shall immediately, and in any event within twenty-four (24) hours of attempting to excavate the changed material, notify the Ministry Representative and clean off any material falling within the currently approved material classification of the excavation, in order to expose the horizon where the material change is thought to occur.

The Ministry Representative will review the exposed material, determine whether a classification change is warranted, and notify the Contractor if a classification change is warranted or not.

If a classification change is approved, sufficient field measurements will be taken to establish boundaries and the material horizon for volume calculations.

If the Contractor disagrees with the Ministry's assessment of the material classification, the Contractor may appeal only in accordance with SS 201.13.

201.13 Material Classification Appeal

201.13.01 Type A and Type D Material Classification – The Contractor may appeal the material classification between Type A and Type D as determined by the Ministry Representative. There shall be no appeal for any distinction between the Type D sub-classes.

201.13.02 Notice and Time Limit of Appeal – The Contractor shall serve written Notice of appeal to the Ministry Representative within:

- (a) twenty-four (24) hours of the Contractor attempting to excavate any potentially changed classification material; or
- (b) three (3) working days of the material classification designation by the Ministry Representative; or
- (c) in no event less than two (2) working days prior to the completion of the excavation.

201.13.03 Ripping Test – Where the Contractor appeals the material classification, a ripping test will be conducted under the direction and control of the Ministry Representative, including the determination of the location, time, and suitability of weather, ground, equipment and other conditions for the ripping test to occur.

The ripping test will be conducted using a Class 7 or more powerful bulldozer (as identified in the "Equipment Rental Rate Guide" published by the BC Road Builders and Heavy Equipment Association), that will develop sufficient

ROADWAY AND DRAINAGE EXCAVATION

traction and effectively deliver a minimum force of 700kN per lineal metre of ripper shank embedded in the material by a single shank static ripper as determined by the manufacturer's Drawbar Pull versus Ground Speed Charts.

If, under the above specified ripping force, the material can be broken or loosened into pieces with an individual volume of 2.0 m³ or smaller, the material will be classified as Type D. If the material cannot be broken or loosened within the time limit as directed by the Ministry Representative, the material will be classified Type A.

The test will be conducted at the Contractor's expense unless the test determines, in the sole determination of the Ministry Representative, that a material classification change is warranted. In that case, the cost of the equipment (including mobilization and demobilization if the equipment is brought in solely for the purpose of the ripping test) and the operator will be paid as Extra Work.

201.14 Unsuitable Material – Only material acceptable to the Ministry Representative shall be used in the construction of embankments.

Where Topsoil is a payment Item in the Contract, Topsoil within the excavation and embankment area, and meeting the requirements of SS 751.13 and SS 751.16, shall be excavated and stockpiled at a location acceptable to the Ministry Representative. Where the Contract has no Item for Topsoil or the available volume of Topsoil exceeds that required for the Contract, only the required volume shall be stockpiled and the balance dealt with as Stripping.

Stripping and surplus Topsoil shall be removed over the entire excavation and embankment area. Topsoil shall be stockpiled separately from Stripping.

All material above or below subgrade in an excavation or in the foundation of an embankment, which in the opinion of the Ministry Representative is unsuitable for embankment construction, shall be removed and paid as classified by the Ministry Representative. Soft clays and peats are typical examples of unsuitable materials.

Material deemed unsuitable for the construction may be wasted between the toe of the embankment and the right-of-way boundary or in other areas, all as designated by the Ministry Representative. If no waste areas are designated in the Contract then the unsuitable material shall be removed and disposed offsite at the Contractor's expense. Any such material subsequently placed in an embankment, without the approval of the Ministry Representative, shall be removed and disposed of, as directed by the Ministry Representative, and no payment will be made for the removal or haul of such unsuitable material from the embankment.

Where removal of unsuitable material requires backfilling this work shall be carried out with approved fill materials, as directed by the Ministry Representative. Payment for such backfilling and haul will be made at the Unit Price bid for "Roadway and Drainage Excavation".

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Topsoil shall be placed in designated planting areas, in accordance with the Contract.

Upon completion of the road grade, all Stripping, and Surplus Topsoil and material suitable for growth of ground cover shall be spread as slope dressing to a nominal depth of 150 mm on the cleared right-of-way and up the fill slopes to generally blend in with the cross section. Care shall be taken that proper drainage is maintained. This material shall be placed, compacted and trimmed to a neat appearance in a manner to facilitate growth of revegetation. No additional payment will be made for placing and trimming material used for slope dressing.

201.15 Surplus Material – All suitable excavation material shall, if required, be taken or hauled into the nearest embankment and to any embankment on the project to be constructed out to the extreme distance required by the Ministry Representative.

The surplus beyond what is necessary to form the nearest embankment shall be:

- disposed of by widening embankments uniformly within the limits of right of way;
- hauled to form or widen any embankment on the road to be constructed;
- stockpiled on-site or in Ministry pits; or
- lastly removed from Site for disposal by the Contractor,

all subject to the approval by the Ministry Representative, where hauling and placing shall be performed in the most efficient manner.

No material shall be wasted without the approval of the Ministry Representative. All surplus material shall be used as much as possible.

201.16 Contaminated Soils – If any contaminated soils, suspected or actual, are encountered during construction, the material is to be left in-situ or be segregated and stockpiled, and handled in accordance with SS 165 Appendix 165-A – Managing Suspect Contaminated Materials.

In general, the Appendix requires segregating, testing by a Qualified Environmental Professional retained by the Contractor, handling, and disposal of the excavated material at an approved disposal site or at a licensed disposal facility.

Prior to any disposal, the Contractor shall identify the disposal site(s) to the Ministry Representative, in accordance with SS 145.27.02.

Contaminated material hauled to off-Site disposal will be paid at applicable Unit Price for Contaminated Soil Disposal or, where there is no such bid Item, as Extra Work.

ROADWAY AND DRAINAGE EXCAVATION

CONSTRUCTION

201.31 Rock Cuts – See SS 204.

201.32 Overhanging Rock and Boulders – The Contractor shall remove overhanging rock or boulders within the excavation limits, as ordered by the Ministry Representative, and shall backfill and compact all resulting cavities to allow the safe passage of construction and support vehicles generally in accordance with the requirements of SS 135. Such work will be paid at the Unit Price bid for the class of excavation involved.."

201.33 Overbreak in Solid Rock - Overbreak in solid rock excavation encountered in the contract is that portion of any such material which is excavated, displaced or loosened outside and beyond the slopes or grades as staked or re-established (with the exception of slides as described in SS 201.42).

Any overbreak that occurs due to the inherent character of any formation encountered, as determined by the Ministry Representative in consultation with the Qualified Professional will be classified as allowable overbreak as described in SS 201.33.01.

All other overbreak, as so defined, shall be removed by the Contractor at the Contractor's expense, and shall be disposed of by the Contractor, in the same manner as provided for "Surplus Material", but at the Contractor's expense.

The Qualified Professional is defined as the Ministry's Geotechnical Engineer, Rockwork Engineer, or consultant engineer working for the Ministry whose discipline is in geotechnical engineering.

201.33.01 Allowable Overbreak – If, and only where directed by the Ministry Representative, use is made of the overbreak to replace material which would otherwise have to be excavated outside the design cut line on-Site or borrowed for the construction of the roadway, then such overbreak will be classed as allowable overbreak. Any overbreak that is not incorporated within the approved design lines and grades of the embankments shall not be considered "allowable overbreak".

201.33.02 Contractor's Negligence – Overbreak resulting from fault or negligence of the Contractor shall be removed and disposed of, as directed by the Ministry Representative, at no cost to the Ministry, subject to SS 201.33.01.

Definition of negligence will be deemed to be lack of the following: the sequence of operations, lack of a blast design, lack of preblast survey, lack of as-built blast design as described in SS 204.04.06, SS 204.04.07, and SS 204.04.08 and SS 204.04.09. Also, definition of negligence will be deemed to mean lack of cut-off drill holes on the backslope, excessive spacing of drill holes and overloading of the same.

201.34 Sealing of Grade – To prevent standing water in the cuts and fills, the Contractor shall at the end of each working

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day, grade, roll and seal the materials with a smooth wheel roller weighing not less than 5,400 kg, such that the water will not pond on the grade.

201.35 Enlargement of Excavation – In cases where the quantity of material taken from a regular excavation will not be sufficient to form the required embankment, the deficiency shall be supplied by taking material from the excavation within or outside the right-of-way, at such places as the Ministry Representative may direct, or from enlargement of the regular excavations made uniformly on one or both sides.

The sides of the excavation in all cases shall be dressed to such slopes as the Ministry Representative may require. When the sides of the excavation had previously been final-dressed payment for sloping or scaling the second time will be as Extra Work.

201.36 Rock Embankments – This section applies to embankments constructed from material containing more than 15% by volume of rock larger than 150 mm.

Embankments shall be constructed in layers equal in thickness to the largest size of the material but not exceeding 0.7 m. Greater lift thicknesses will be permitted by the Ministry Representative under special conditions provided the Contractor can spread the larger material satisfactorily and attain a degree of compaction no less than attained for layers less than 0.7 m in thickness.

Regardless of layer thickness and material particle size all material shall be well compacted to achieve the design density and stability of the embankment and to the satisfaction of the Ministry Representative.

The material shall be deposited and spread so that the larger rocks are well distributed and the intervening spaces are filled with smaller sizes as may be available to form a stable embankment. The finer portion of excavated rock shall be retained for the top transition layer. Each layer shall be compacted by routing the loading construction equipment over the entire width supplemented with additional compaction equipment, as necessary to ensure compaction is uniform, a stable embankment is achieved and to the satisfaction of the Ministry Representative.

When a rock embankment is overlain by an earth embankment or by subbase or base course materials, the top 0.5 m of the rock fill shall be sealed with smaller rock particles and suitable soil materials, if necessary, to prevent the uppermost layers penetrating into voids in this rock embankment.

Where permitted by the Ministry Representative, and where the width is too narrow to accommodate equipment, material for the toe of the side hill rock fills may be placed by end dumping until sufficient width of the embankment has been formed to carry equipment, after which the remainder shall be placed in layers and compacted as specified. Any loose material left on the slopes by end

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dumping shall be incorporated in the fill and compacted according to these standards.

When embankments are made on hillsides or where a new fill is to be applied upon an existing embankment, the slopes of the embankment or original ground (except rock) shall be terraced in a continuous series of steps a minimum of 1.5 m wide as the embankment rises.

The material from step excavation shall be spread and compacted into the adjoining embankment. No additional payment will be made for excavation or for placing step material in the adjoining fill.

201.37 Earth Embankments – This section applies to embankments constructed from material containing less than 15% by volume of rock larger than 150 mm. Embankments shall be constructed in such a manner that they shall be completely stable with reference to designated traffic loading.

The natural surface shall be excavated to remove organic soils or other unsuitable material, as directed by the Ministry Representative, and the removal of such material will be paid at the Unit Prices bid for the class of excavation involved.

When the foundation of the embankment consists of sensitive soils the Contractor shall, with the Ministry Representative's approval, place the initial lift of embankment to a minimum depth required as stated in the design to carry the Contractor's hauling equipment. Static compaction shall be used in areas of sensitive soil.

The earth embankment shall then be constructed in successive horizontal layers and thicknesses and compacted to the required percentage of Standard Proctor Maximum Dry Density obtained by ASTM D698, all as specified in Table 201-A.

Table 201-A: Lift Thickness and Required Density by Depth Below Subgrade

<u>Depth Below Subgrade (mm)</u>	<u>Maximum Uncompacted Lift Thickness (mm)</u>	<u>Minimum Density (% SPD)</u>
<u>0 to 300</u>	<u>100</u>	<u>100%</u>
<u>300 to 500</u>	<u>100</u>	<u>95%</u>
<u>Below 500</u>	<u>200</u>	<u>95%</u>

The above maximum lift thickness requirements may be waived by the Ministry Representative, where the Contractor can demonstrate that:

- the proposed compaction equipment can uniformly compact a thicker layer;

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- the equipment and compaction procedures attain the required degree of compaction at all levels within the lift;
- they have compaction testing equipment and services for Quality Control and Quality Assurance that will reliably test to the full depth of the proposed lift thickness. The supply of all testing equipment for Quality Control and Quality Assurance shall be the Contractor's responsibility.

Where the embankment material is highly variable or of such character that conventional laboratory density determination or conventional compaction testing are likely to be inappropriate, the Contractor may prepare a proposed compaction plan based on the "Target Density" provisions of SS 202.22, roll patterns or other methodology, for review and sign-off by the Quality Manager, and submission to the Ministry Representative. The Ministry Representative, in consultation with the Geotechnical Engineer of Record, may approve or reject the proposal.

No organic soils shall be placed in the embankment. Soils with high moisture content that cannot be compacted to the required density shall not be employed without prior aeration and drying.

When embankments are made on hillsides or where a new fill is to be applied upon an existing embankment, the slopes of the embankment or original ground (except rock) shall be terraced in a continuous series of steps a minimum of 1.5 m wide as the embankment rises.

If suitable, the material from step excavation shall be spread and compacted into the adjoining embankment. No additional payment will be made for excavation or for placing step material in the adjoining fill.

The Contractor shall be responsible for selecting equipment and methods of attaining the specified degree of compaction. In general, the roller mass shall be sufficient to compress the soil vertically after each pass but not sufficient to unduly rut or shear the soil.

Water shall be added and incorporated into the soil using suitable equipment such as rotary mixers, cultivators, etc., to increase the natural moisture content to the optimum moisture percentage as determined by the current ASTM D698.

In the event that the natural moisture content is greater than the optimum, the soil shall be aerated and dried employing suitable mixing equipment.

201.38 Frozen Material – The use of frozen material in embankments will not be permitted; the only exception is, with Ministry Representative approval, broken rock containing less than 15% passing a 4.75 mm sieve. Frozen excavated material which will be suitable when dry shall be stored and allowed to thaw and dry, and then placed in the embankment, as directed by the Ministry Representative.

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No compensation will be allowed for the storing and re-handling of this material.

No material is to be placed on a frozen surface unless approved by the Ministry Representative.

201.39 Snow Removal – Snow overlying the surface of a cut, or the site or surface of an embankment shall be removed and deposited beyond the slope stakes at the Contractor's own expense.

201.40 Bridge End Fill – Material for bridge end fill shall be in accordance with SS_202.04 and SS_202.05. Construction of bridge end fill shall be in accordance with SS_202.23.

201.41 Transition Sections – Subgrade, other than solid rock, shall be subcut 1 m deep at the line of transition from cut to embankment. The subcut shall taper to zero depth 8 m within the cut. The embankment construction shall be carried back over the subcut only after the embankment has reached the level of the bottom of the subcut, as measured by the Ministry Representative. Payment for the subcut will be made at the Unit Price bid for the class of excavation involved

201.42 Slopes and Slides – The slopes of all excavations and embankments shall be trimmed neatly and evenly to the line and slope indicated on the Drawings, including special slope treatment in accordance with SS Drawing SP201-01, or as directed by the Ministry Representative.

No undercutting of slopes in excavation by power shovels or other excavation equipment will be permitted.

In case slopes, finished to the lines as shown on the Drawings, slide into the roadway or out of embankments before final acceptance of the work, such slide material shall be removed by the Contractor from the roadway or replaced by the Contractor in the embankment, as the case may be, at the Unit Price bid for the class of excavation involved. The classification of material in slips and slides shall be in accordance with its condition at the time of removal regardless of its prior condition. The slopes shall be refinished by the Contractor, as directed by the Ministry Representative. Such refishing will be paid for by Order for Extra Work. Materials to replace embankment slides shall be obtained from sources designated by the Ministry Representative.

Slopes undercut at the base or destroyed in any manner by act of the Contractor shall be resloped by the Contractor at the Contractor's expense to the slope, as directed by the Ministry Representative. All materials resulting from such resloping shall be removed and deposited, as directed by the Ministry Representative. No payment will be made to the Contractor for the removal of such material.

201.43 Ditches – Ditches which may be considered necessary for the proper drainage of the work shall be constructed at such points and to such cross section, alignment and grade as the Ministry Representative may

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direct. This shall include inlets and outlets to culverts and ditching of all kinds. Ditching quantities will be considered as ordinary excavation quantities and will be measured, classified and paid for as such.

201.44 Borrow – Borrow refers only to materials sourced outside of the Site; Material sourced on-Site, within or outside of the design lines, is Type A or Type D excavation.

Borrow shall consist of rut resistant material, with less than 20% passing the 0.075 mm sieve and free of organics, high plasticity clays and other unsuitable materials, obtained from an approved source of supply (e.g. pit or quarry) outside the highway right-of-way, developed and used in accordance with SS 202.

The Special Provisions may identify potential sources, additional or different material qualities, or require representative sampling and testing of the material.

201.45 Haul – “Haul” includes all work necessary to move excavation materials from their in-situ source to any destination, on or off-Site.

201.46 Watering – Water for compacting embankments, constructing subgrades and surfacing, and for laying dust caused by grading operations and traffic, shall be applied as required. Water shall be applied by sprinkling with tank trucks equipped with spray bars and suitable control apparatus.

The Contractor shall make all necessary arrangements for obtaining and applying water, including acquiring all required permits.

Watering shall be incidental to the Work.

201.47 Finishing of Roadway – Before acceptance and final payment is made, the entire roadway, including the roadbed, shoulders and ditches shall be neatly finished and trimmed to the lines, grades and cross sections shown on the Drawings, or as directed by the Ministry Representative, to reproduce smooth surfaces, slopes and a uniform cross section. Subgrade shall be finished within a tolerance of ± 15 mm except for rock cut fills where the tolerance shall be ± 50 mm.

All drainage ditches, waterways and culverts shall be opened up and cleaned out to restore them to their full effectiveness.

All loose rock and boulders within the right-of-way resulting from grading and grubbing operations shall be gathered up and buried, or otherwise disposed of as the Ministry Representative may direct.

The Contractor shall grade all portions of the right-of-way outside the Excavation and Embankment areas to conform to the general ground line. This work will be considered as subsidiary work pertaining to the item of "Roadway and Drainage Excavation," and no extra payment will be made.

ROADWAY AND DRAINAGE EXCAVATION

201.48 Surcharging – Pre-consolidation by surcharging shall be carried out where indicated on Drawings and/or Special Provisions, or as directed by the Ministry Representative. Embankments on these areas shall be placed directly on the natural ground without removal of the organic materials, unless otherwise directed by the Ministry Representative, and shall be constructed to a surcharged height above construction grade as shown or specified.

The Contractor shall place the initial lift of embankment to a minimum depth required to carry the Contractor's hauling equipment with the approval of the Ministry Representative. Static compaction shall be used in areas of sensitive soils. The remainder of the embankments shall be constructed in accordance with procedures set forth in this Section; except that compaction will not be required on the surcharge material above construction grade.

To avoid shear failures, the rate of construction shall be rigidly controlled by instrumentation installed by the Ministry.

The surcharge shall remain on the embankments for a period of time as outlined in the Contract Special Provisions, or as may be indicated by the instrumentation, or as determined and adjusted by the Ministry Representative.

All instrumentation shall be kept in working, continuous, and operable order according to manufacturer's requirements.

Should any of the instrumentation become damaged or rendered unusable by the construction operations, the same will be replaced by the Ministry at the Contractor's expense.

201.49 Backslope Stabilization – Drain holes, rock bolts, mesh and/or shotcrete may be required where rock is being excavated.

The backslope stabilization requirements will be assessed by the Ministry Representative during construction and the amount and location of the drain holes, rock bolts, mesh and shotcrete may be changed or deleted according to the condition of the rock encountered in the field.

MEASUREMENT

201.81 Borrow – Borrow quantities will be measured in-bank at the off-Site source and computed in accordance with SS 145.21.01. The volume of boulders and unsuitable material removed from borrow pits that is not used in embankments shall be deducted.

201.82 Excavation – All "Roadway and Drainage Excavation" will be measured in-bank at the source, after Clearing and Grubbing operations have been completed, as specified in SS 200.01 through SS 200.03, and volumes computed in accordance with SS 145.21.01.

Pay quantities will be computed, in accordance with SS 145.21.01, in CUBIC METRE to the neat lines staked.

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Adjustments for curvature will be made in any cut where deemed equitable by the Ministry Representative.

201.83 Haul and Overhaul – Haul and Overhaul are incidental to the Work.

PAYMENT

201.91 Borrow – Payment for BORROW materials, imported from outside the Site, shall be at the Unit Price per cubic metre bid for the various types of material borrowed, classified in accordance with SS 201.11. All specifications in this Section relating to excavation shall apply to borrow. Payment shall be full compensation for everything furnished and done, including without limitation costs for acquisition, development, and payment of royalties for private pits or quarries, purchase price of borrow material, access road construction and maintenance, excavation, screening, crushing, stockpiling, loading, hauling, spreading, compaction and moisture adjustment (watering or drying) in place as specified.

201.92 Excavation – Payment for EXCAVATION, within the Site, shall be at the Unit Price bid per cubic metre for the various types of materials excavated, classified in accordance with SS 201.11. The Unit Price for such excavation shall include excavation, slope trimming,

ROADWAY AND DRAINAGE EXCAVATION

hauling, and placing and compacting into embankment, stockpile or disposal site and shall be accepted as full compensation for everything furnished and done in connection therewith.

Material will be paid only once as a class or sub-class of material. For clarity, if a Contract includes Items for both Type D and its subclass Topsoil, Topsoil will only be paid under the Topsoil Item and not also as Type D.

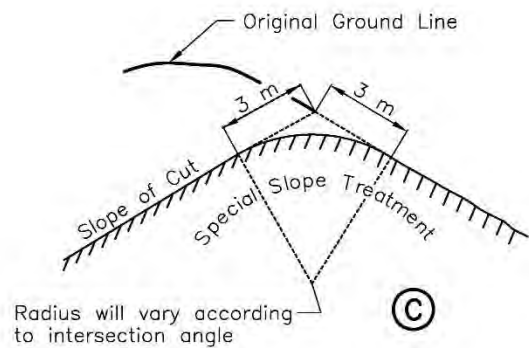
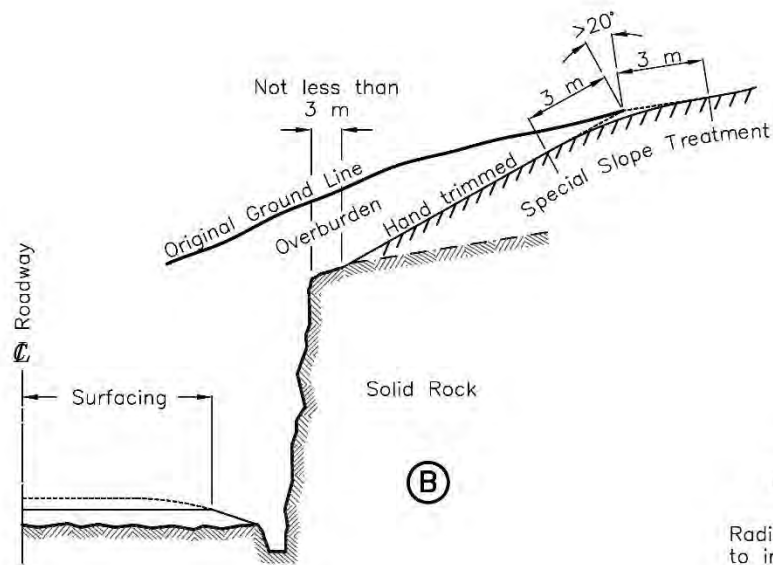
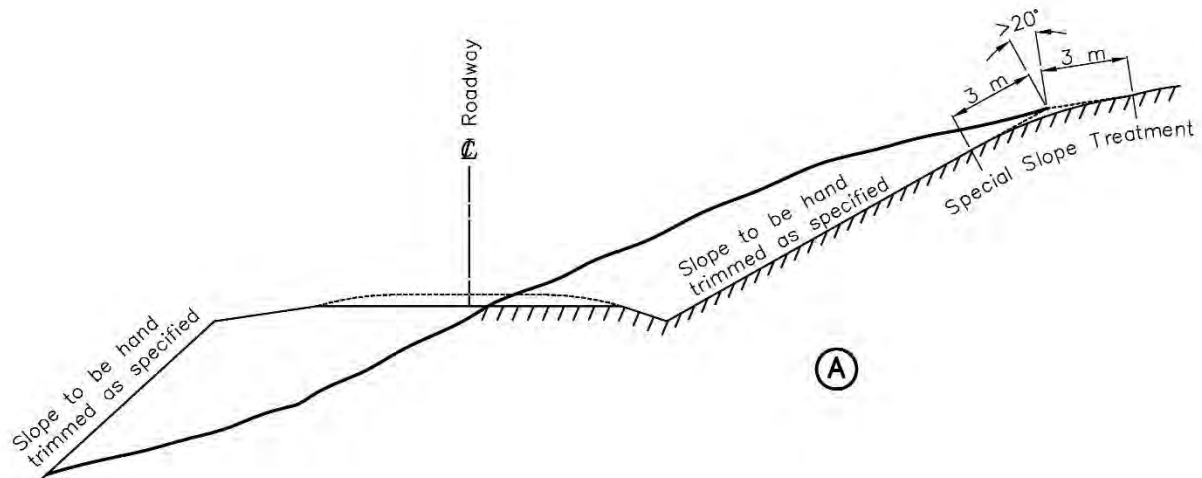
201.93 Allowable Overbreak – Allowable overbreak will be paid at 75% of the Unit Price bid for TYPE A MATERIAL. The quantity at the Unit Price for overbreak in any one cut shall not exceed 10% of the original theoretical cut, as shown on the Drawings or as directed by the Ministry Representative. Material in excess of the above 10%, if used in lieu of borrow and only on the advice of the Ministry Representative, will be paid at the Unit Price bid for TYPE D MATERIAL.

201.94 Embankment – Except where otherwise explicitly provided in the Contract, the work of embankment construction will not be paid for directly as a pay item, but shall be considered as incidental to the classes of EXCAVATION or BORROW.

201.95 Surcharge Removal – Payment for surcharge removal shall be at the Unit Prices bid for EXCAVATION.

SPECIAL SLOPE TREATMENT

SP201-01



NOTES:

1. SPECIAL SLOPE TREATMENT REFERS TO THE ROUNDING OFF AT THE INTERSECTION OF CUT SLOPES WITH THE EXISTING GROUND LINE. IT SHALL BE APPLICABLE WHERE THE MATERIAL IS OTHER THAN SOLID ROCK, AND WHEN THE DIFFERENCE IN THE INTERSECTION ANGLE BETWEEN THE BACKSLOPE AND THE ORIGINAL GROUND EXCEEDS 20 DEGREES.
2. MATERIAL REMOVED IN SPECIAL SLOPE TREATMENT SHALL BE UTILIZED OR DISPOSED OF IN ACCORDANCE WITH **SS 201.14** AND **SS 201.15**

NOT TO SCALE

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

SECTION 202

GRANULAR SURFACING, BASE, AND SUB-BASES

SCOPE OF SPECIFICATION

202.01 Preliminary and General – This specification describes the classification, the materials and the professional standards required for the production of granular aggregates and the construction of granular surfacing, granular base and select granular subbase.

MATERIALS

202.02 Aggregate Classification

202.02.01 Crushed Surfacing Aggregates:

(a) **High Fines Granular Surfacing Aggregate (HFSA):** HFSA is used for surfacing of gravel roads. Cohesion for this aggregate is achieved by plastic fines.

202.02.02 Crushed Base Course Aggregates – These are high strength, high load bearing, high quality, crushed granular base materials. As these materials are not well suited for long-term traffic, they are usually overlaid by surface course materials (HFSA or asphalt/concrete pavement).

(a) **Well-Graded Base (WGB):** WGB is the standard base course material used for pavement structures. It is well-graded with moderate permeability.

(b) **Intermediate-Graded Base (IGB):** IGB is intermediate in gradation and permeability between well-graded aggregate and open-graded aggregate.

(c) **Open-Graded Base (OGB):** OGB contains a small percentage of fine-grained material and is highly permeable, gap-graded aggregate.

202.02.03 Subbase Aggregates – A granular material placed between the sub-grade and the base course aggregates, providing strength and drainage to the pavement structure.

(a) **Select Granular Sub-base (SGSB):** SGSB is the standard aggregate, crushed or screened, placed directly above subgrade, with moderate permeability and a wide gradation.

(b) **Intermediate Graded Sub-base (IGSB):** IGSB is a higher permeability, crushed sub-base aggregate.

(c) **Open Graded Subbase (OGSB):** OGSB is a highly permeable, crushed subbase aggregate.

202.02.04 Bridge End Fill (BEF) – BEF is quality granular fill primarily used behind and below a bridge abutment to provide good drainage, a smooth transition from the bridge approaches to the bridge structure, and a suitable material through which to drive piles.

202.03 General Description of Work – The aggregates for granular surfacing, base and subbase may be supplied either by the Contractor or produced from a Ministry provided source. Production of granular surfacing and granular base requires crushing and placing in stockpile. Production of select granular subbase and bridge end fill may be by crushing, screening or direct excavation.

Granular aggregate shall be loaded from the stockpile, crushing and screening plant, or pit face, as may be applicable; hauled out to the site, placed accurately without segregation and thoroughly compacted to established line and grade.

The Contractor shall maintain the prepared surface of the base course aggregate until it has been paved or until completion of the contract, whichever is applicable.

202.04 Aggregate Quality

202.04.01 General – Granular aggregate shall be composed of inert, clean, tough, durable particles of crushed rock, gravel, sand and fines capable of withstanding the deleterious effects of exposure to water, freeze/thaw, handling, spreading and compacting and design traffic loading. The aggregate particles shall be uniform in quality and free from clay lumps, and wood.

Aggregates for surfacing, base, subbase and bridge end fill shall be tested in accordance with the tests listed in Table 202-A.

202.04.02 Primary Quality Tests – All aggregates for surfacing, base, subbase, and bridge end fill shall meet the requirements of Table 202-B.

Where the Ministry has pre-existing test results, the Ministry will make this information available to the Contractor and the Contractor will be able to rely on the factual data and not have to perform the corresponding primary quality tests on materials extracted from the investigated portions of the pit or quarry.

In circumstances where historic performance of material from a source has proven satisfactory, the Ministry may, by Special Provision, waive any or all of the testing required in SS 202.04.

202.04.03 Secondary Quality Tests – All aggregates for surfacing, base, subbase, and bridge end fill failing any primary quality test indicated in SS 202.04.02 shall further be tested as follows to determine acceptability or, at the Contractor's option and at the Contractor's expense, a new source shall be found.

(a) If the Micro Deval test results show that the material fails, then the soundness shall be reassessed by use of

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GRANULAR SURFACING, BASE AND SUB-BASES

Magnesium Sulphate (ASTM C88). The material will be considered acceptable if the loss after 5 cycles is less than:

- (i) 20% for coarse aggregate, and(the material retained on or above the 4.75 mm sieve).
 - (ii) 25% for fine aggregate (the material passing the 4.75 mm sieve)
- (b) If the aggregate fails any of the Sand Equivalent, Micro-Deval or Magnesium Sulphate tests, then the Contractor shall perform the Ministry's petrographic test (Appendix 202-B) to determine the cause of failure. The Ministry Representative may review the test results and determine whether the material is acceptable or not.

202.05 Aggregate Gradation – Gradation shall be determined in accordance with ASTM C117 (Wash Test).

202.05.01 Gradation Curve Slope – Aggregate shall have a gradation that defines a curve (% passing versus log sieve size) with a slope between adjacent sieves, equal or intermediate to the corresponding slopes of the boundary curves defined by the specification. Gradations shall fall within the limits, for the specified classification, shown in Table 202-C.

202.05.02 Rut Resistance – Any aggregate supplied must, in addition to meeting the gradation requirements specified above, not rut when proof rolled with a truck having a 9 tonne single axle dual tire or 17 tonne tandem axle group with dual tires with a tire pressure of 600 kPa. Any aggregate which does rut shall be removed and replaced, or blended with suitable aggregates, to meet both the gradation requirement and the rut resistance requirement.

USE OF MINISTRY-PROVIDED PITS OR QUARRIES

202.06 General – Ministry-provided pits and quarries are those identified in the Special Provisions which the Ministry is offering to the Contractor as potential aggregate sources.

They may include pits and quarries:

- (a) owned by the Ministry
- (b) leased by the Ministry, or
- (c) established as Map Reserve on Crown Land.

Use of such pits and quarries will be at the Contractor's option unless the Special Provisions clearly state that the source must be used for specific purposes.

When a Ministry pit or quarry is provided for the Contractor's optional use under the Contract, the

Contractor, by entering the pit or quarry, will be deemed to have accepted all obligations, risks and costs for the production of aggregate meeting the requirements of SS 202.03, SS 202.04, and SS 202.05. The Ministry gives no warranty that its pit or quarry will meet the quality and quantity requirements.

Where a pit or quarry is designated as a mandatory source, the Ministry waives the aggregate quality requirements of SS 202.04.02 except for the Fractured Faces requirement for aggregates required by the Ministry to be produced from that source.

Table 202-A: Aggregate Quality Tests

ASTM	Title of Test Standard Test Method for...
C88	Soundness of Aggregate by Use of Magnesium Sulphate.
C117	Materials Finer than 75-µm (No. 200) Sieve in Mineral Aggregates by Washing
C136	Sieve Analysis of Fine and Coarse Aggregates
D2419	Sand Equivalent Value of Soils and Fine Aggregate
D4318	Liquid Limit, Plastic Limit, and Plasticity Index of Soils
<u>D4791</u>	<u>Flat Particles, Elongated Particles, or Flat and Elongated Particles in Coarse Aggregate</u>
D6928	Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus
MoTI	Title of Test
<u>Appendix 202-A</u>	Fracture Count on Coarse Aggregate
<u>Appendix 202-B</u>	Petrographic <u>Analysis Test</u>

Table 202-B: Aggregate Properties

TEST	Test Result							
	Surfacing Aggregates	Base Course			Subbase Aggregates			Bridge End Fill
	HFSA	25mm	50mm	75mm	SGSB	IGSB	OGSB	BEF
Sand Equivalent	≥ 20	≥ 40	≥ 40	≥ 40	≥ 20	≥ 20	≥ 20	≥ 20
Micro-Deval loss factor	≤ 25%	≤ 25%	≤ 25%	≤ 17%	≤ 30%	≤ 25%	≤ 25%	≤ 30%
Fractured Faces Method "A" ¹	≥ 50 ¹	≥ 50 ¹	≥ 50 ¹	≥ 50 ¹	n/a	≥ 50 ¹	≥ 50 ¹	n/a
Flat and Elongated Particles using 4:1 ratio (%)	n/a	≤ 10	≤ 10	≤ 10	n/a	n/a	n/a	n/a
Plasticity	≤ 6	n/a	n/a	n/a	n/a	n/a	n/a	n/a

¹ Values are for total sample.

202.07 Source of Supply – The Contract documents may identify one or more pits or quarries, for use as aggregate sources at the Contractor's discretion.

The Ministry will provide those gravel pits or quarries and the right-of-way to those pits or quarries and will pay the cost of royalties. The cost of constructing and maintaining access roads shall be considered as part of the cost of producing the construction aggregates and shall be payable by the Contractor. The Ministry will exercise its best judgement in the selection of the pit or quarry sites; however, the failure of the pit or quarry to yield materials of uniform quality shall not constitute grounds for a claim by the Contractor against the Ministry.

The Contractor shall be satisfied as to the gradation and other characteristics of the raw material in the Ministry provided pit or quarry and as to the nature and amount of work required to produce materials which will meet all gradation requirements, and the Contractor shall, at the Contractor's expense, remove any objectionable materials from the aggregate.

202.08 Development of Pit or Quarry – Before any material for aggregate is excavated from the pit or quarry, the approved development area shall be cleared and grubbed, and all debris disposed of as ordered by the Ministry Representative. If topsoil or undesirable overburden exists, these materials shall be stripped and placed in separate stockpiles as shown on the Ministry pit development plan or as directed by the Ministry Representative.

The Contractor shall not operate the pit or quarry in a manner which will contaminate remaining granular material nor leave the pit or quarry in a condition which will limit its

future use. The Contractor shall follow the Ministry pit development plan, where provided. Stockpile areas in Ministry-provided sources shall be per the pit development plan. If the Ministry pit development plan conflicts with the Health, Safety, and Reclamation Code for Mines in British Columbia, then the code shall prevail.

202.09 Use of All Materials – Any Ministry provided pit or quarry, once entered and developed, shall be used to its full potential. Normally, the Contractor shall produce select granular subbase by selection or screening, provided that oversize reject aggregate is removed and properly placed in a stockpile for later crushing and use. However, if excessive (5% or greater) oversize occurs, the Ministry Representative may order that pit run aggregate be crushed to produce 75 mm crushed granular base for use in place of select granular subbase.

For the production of crushed surfacing and base aggregates, as well as IGSB and OGSB, the Contractor shall provide crushing equipment such that all aggregate which will pass through 375 mm x 450 mm slotted openings shall be used for the production of crushed aggregate; rocks which will not pass through these openings shall be stockpiled as shown on the pit development plan or disposed of to the satisfaction of the Ministry Representative. Crushing and screening equipment shall be provided with adequate facilities and capacity to bleed off reject aggregate in usable condition, or otherwise remove any excess of fine aggregate, dust or objectionable aggregate coatings that may be present in or on the aggregate, so as to make it generally acceptable for use. No portion of the products of crushers or screening plants that can be used shall be wasted, but shall be stockpiled or used as directed by the Ministry Representative.

Table 202-C: Aggregate Gradation

Percent Passing (%) Sieve Size																	
Sieve Size (mm)	Surfacing Aggregate	Base Course Aggregates										Sub-Base Aggregates		Bridge End Fill			
		WGB				IGB			OGB			SGSB	IGSB		OGSB	BEF	
		25mm	50mm	75mm	100mm	25mm	50mm	75mm	100mm	25mm	50mm						75mm
75	—	—	—	100	—	—	100	—	—	—	—	—	100	100	100	100	100
50	—	—	100	—	—	100	55 - 100	—	—	100	70 - 100	—	—	55 - 100	70 - 100	30 - 100	30 - 100
37.5	—	—	80 - 100	60 - 100	—	60 - 100	40 - 80	—	—	75 - 100	50 - 85	—	—	40 - 80	50 - 85	—	—
25	100	100	—	—	100	40 - 75	—	100	—	—	—	—	—	—	—	—	—
19	85 - 100	80 - 100	50 - 100	35 - 80	65 - 100	—	17 - 40	75 - 100	35 - 65	15 - 55	15 - 55	15 - 100	17 - 40	15 - 55	20 - 100	20 - 100	20 - 100
12.5	—	—	—	—	—	15 - 40	—	—	—	—	—	—	—	—	—	—	—
9.5	60 - 85	50 - 85	35 - 75	25 - 60	30 - 70	—	—	30 - 65	5 - 35	—	—	0 - 100	—	—	—	—	—
6.3	—	—	—	—	—	—	—	—	—	—	0 - 20	—	—	0 - 20	—	—	—
4.75	40 - 70	35 - 70	25 - 55	20 - 40	15 - 40	—	—	5 - 30	0 - 15	—	—	—	—	—	—	10 - 60	10 - 60
2.36	—	25 - 50	20 - 40	15 - 30	10 - 30	10 - 25	10 - 25	0 - 10	0 - 10	0 - 10	0 - 10	—	10 - 25	0 - 10	—	—	—
1.18	20 - 40	15 - 35	15 - 30	10 - 20	—	—	—	—	—	—	—	—	—	—	—	6 - 32	6 - 32
0.600	—	—	—	—	—	—	—	—	—	—	—	0 - 100	—	—	—	—	—
0.300	10-25	5-20	5-15	3-10	5-15	5-15	4-15	0-8	0-8	0-8	0-8	0-15	4-15	0-8	4-15	0-8	4-15
0.075	7-12	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5	0-5

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202.10 Modification of Materials – The rejection or addition of any particular size material necessary to produce aggregate conforming to specification requirements shall be the responsibility of the Contractor, and no additional compensation will be paid therefore.

202.11 Change of Ministry Provided Pit or Quarry – No payment will be made to the Contractor for moving the plant from one Ministry provided pit or quarry to another Ministry provided pit or quarry.

However, if the Ministry Representative requires a move to a new Ministry provided pit or quarry, the Contractor will be given notice in writing and upon completion of the move, the Contractor will be paid a lump sum of \$7,500 for moving the existing crushing operation and any additional equipment that may be required for material processing at the new pit. The payment will be deemed to be full compensation for all costs and expenses of the move and set-up of the crusher. If the Contractor moves the crushing plant without first securing a written order to do so from the Ministry Representative it will be considered sufficient proof that the move was unnecessary, and no allowance or compensation will be made to cover such a move.

202.12 Pit Cleanup – When the Contractor discontinues operations in any pit or quarry, the Contractor shall trim the face of pit excavations, waste piles and stockpiles to a 1.5H to 1V slope, or such other slope as the Ministry Representative may direct, open up such drains or ditches as may be required to prevent water standing therein, and leave the site in a neat condition, all to the satisfaction of the Ministry Representative. The sloping of pit excavations shall be with in-place materials and shall not be contaminated with topsoil, overburden or any reject aggregates. The trimming, sloping, ditching and draining of the pit or quarry, as well as maintenance of the access roads, shall be considered part of the cost of producing the granular aggregate and no additional payment will be allowed.

If a pit has been fully depleted and is to be abandoned, the Contractor may be requested to reshape the pit faces to a slope not steeper than 2H to 1V using the stockpiled overburden if necessary, and cover same with any topsoil previously removed and stockpiled, all as directed by the Ministry Representative. In such cases, the replacing of stockpiled overburden and topsoil will be paid for in accordance with SS 202.32.

202.13 Work in Ministry Pits or Quarries – All work in a Ministry pit or quarry shall be in accordance with SS 145.26.

CONTRACTOR SUPPLIED AGGREGATES

202.14 Preliminary and General – Aggregates shall meet the requirements of SS 202.02 to SS 202.05 inclusive.

202.15 Development and Clean Up of Private Pits or Quarries – Pit or quarry development and cleanup shall be

GRANULAR SURFACING, BASE AND SUB-BASES

in accordance with local and Provincial regulatory agency requirements.

202.16 Contractor to Pay All Costs – The Contractor shall bear and pay all costs including, applicable royalties, construction and maintenance of access roads to borrow sites, mobilization/demobilization of weigh scales, and any move of the equipment that may be ordered or required because a pit or quarry of the Contractor's choosing proves unsuitable for production of the contracted quantity of granular aggregates meeting the requirements of SS 202.

PRODUCTION OF CRUSHED AGGREGATE

202.17 Equipment and Plant – The Contractor shall provide and maintain in good operating condition, all plant and equipment necessary to comply with the requirements of this specification. Such plant and equipment shall be of a type and capacity to ensure proper construction and maintenance of access roads, development of the pit or quarry, maximum use of the pit or quarry, production of granular aggregates to the standard of uniformity required, and adherence to the construction schedule.

Adequate sampling facilities shall be installed by the Contractor at the discharge end of the production conveyor.

The Contractor shall ensure that the plant and equipment meet the requirements of SS 145.26.

202.18 Working of Pit – A pit shall be worked in such a manner as to ensure that granular material excavated and fed to the crushers, screening plants or loaders shall be as uniform as possible. This may require the use of equipment capable of deep excavating through horizontal gravel layers or carrying out selective digging or cross blending over a large area of the pit. Before commencing production, the Contractor's quality control plan shall describe the proposed method of production for the Ministry Representative and obtain the Ministry Representative's general acceptance.

202.19 Crushing Operations – The Contractor shall adjust the crushers and/or screening plants and provide such screens as may be necessary to produce and maintain acceptable gradations of granular aggregates.

202.20 Crusher Gradation Control

202.20.01 Design Aggregate Gradation - For each aggregate gradation classification specified for production, field and/or laboratory tests shall be performed by the Contractor to select a single gradation curve within the "banana" specification (Table 202-C), the "Design Aggregate Gradation" (DAG), which will be the target for crusher control. The Contractor shall declare the DAG, in writing to the Quality Manager and the Ministry Representative, within production of the first 10% of the Approximate Quantity of a given aggregate classification.

As part of the Ministry's quality assurance program and for payment purposes, the Ministry may sample the stockpile to

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assess aggregate gradation compliance to contractual requirements.

202.20.02 Variation Limits - The running average of four (4) consecutive tests, on each applicable individual sieve size specified in Table 202-C, must be maintained at all times within the limits specified in that Table. Also, each individual test must be within the specifications of Table 202-C.

The maximum permissible variation from the DAG curve, on each applicable individual sieve size specified in Table 202-C of the mean of any four (4) consecutive tests shall also be within the limits specified in Table 202-D.

202.20.03 Field Adjustment of Design Aggregate Gradation - The Contractor may make a maximum of two (2) field adjustments to the Design Aggregate Gradation, to accommodate minor changes in pit characteristics or to enhance production efficiency.

A field adjustment to the Design Aggregate Gradation is defined as a change in declared target gradation of the various aggregate sizes within the specified limits shown in Table 202-C which does not require a supplementary formal review. The maximum adjustment for individual sieve sizes for each adjustment are as permitted in the Table 202-E.

A proposed Design Aggregate Gradation field adjustment shall be submitted in writing with supporting documentation (showing original DAG, current stockpile average gradation, and proposed field adjustment and the new DAG) to the Ministry Representative. Upon receipt of the proposed field adjustments, the Ministry Representative will assess the adjustment for conformance with the contract requirements and notify the Contractor whether or not it is acceptable in a timely fashion.

Table 202-D: Aggregate Gradation Variation Limits

Sieve Size (mm)	Variation Limits (% Passing)
4.75 and larger	± 5
1.18 to 2.36	± 3.5
0.300 and 0.600	± 2
0.075	± 1

Table 202-E: Field Adjustment of Design Aggregate Gradation

Sieve Size (mm)	Maximum Field Adjustment (% Passing)
19 and larger	± 3.0
9.5	± 2.5
2.36 and 4.75	± 2.0
0.600 and 1.18	± 1.5
0.150 and 0.300	± 1.5
0.075	± 1.0

No field adjustment will be acceptable if it results in a change from the current Design Aggregate Gradation to fall outside the limits defined in Table 202-C. The variation limits specified in SS 202.20.02 will apply to the adjusted DAG.

The Contractor shall have the moisture/density relationship established per ASTM D698 for Standard Proctor Maximum Dry Density for each original DAG, and one after each field adjustment thereto.

202.20.04 Declaration of a New Design Aggregate Gradation (DAG) - Should there be a substantial change in the character of aggregate exposed in the pit face as the work proceeds, the Ministry Representative may authorize a change in the declared Design Aggregate Gradation, which would then also be eligible for field adjustment in accordance with SS 202.20.

A maximum of one change to the declared DAG will be permitted.

202.21 Stockpiling

202.21.01 Unless it is specified in the Special Provisions or ordered otherwise in writing by the Ministry Representative, all crushed aggregates shall be stockpiled prior to use on the Highway.

202.21.02 Stockpile sites shall be cleared of all vegetation, trees, brush, rocks or other debris and a uniform gravel surface prepared before the stockpile material is deposited on the stockpile site.

202.21.03 Stockpiles shall be constructed on the designated site and when completed shall be neat and regular in shape, occupying as small an area as is practicable. Spilling of material over the edges of the piles will not be permitted.

202.21.04 The Contractor shall ensure that stockpiles shall be built up in layers not to exceed 1 m in thickness.

202.21.05 The Contractor shall ensure that plank or protected runways shall be provided for operating trucks on stockpiles when the Ministry Representative deems them necessary to prevent dirt being tracked onto the crushed aggregate.

202.21.06 The Ministry Representative may, on receipt of a written request from the Contractor, permit the Contractor to build the final stockpile by bulldozing the aggregate from a feed pile at the end of a production belt provided that the following are maintained:

- (a) The bulldozer(s) to be used shall be equipped with U-shaped pushing blades.
- (b) The aggregate does not become contaminated with oversize material, mud or other objectionable material picked up from the pit floor or general working areas.
- (c) The crushed granular aggregate is distributed evenly over the final stockpile area in lifts not greater than 150 mm in depth.

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- (d) The cone of the feeder pile at the end of the production belt shall not, without express permission, be allowed to build up to a height greater than 2 m.
- (e) No appreciable segregation or degradation shall occur in the main stockpile as a result of using this method.

201.21.07 Radial stackers may be utilized only with the approval of the Ministry Representative and under Contractor-developed written work and quality management procedures governing the stockpiling operation that ensures that:

- (a) the stockpile is large enough to accommodate all required material, or that separate stockpiles are developed;
- (b) segregation does not occur; and
- (c) lift thicknesses, drop heights, and wind-driven fines migration are minimized.

CONSTRUCTION METHOD

202.22 Target Density for Compaction – For each material produced and placed under SS 202, the Contractor shall determine a Target Density in accordance with the following.

- (a) Laboratory Density – Where the material gradation is within the applicable range, the Target Density shall be the Standard Proctor Maximum Dry Density obtained under the current ASTM D698, with oversize correction in accordance with ASTM D4718; or
- (b) Field Density – Where the material falls outside the applicable range of ASTM D698 and ASTM D4718, the Target Density shall be 98% of the maximum dry density achieved through field compaction at varying moisture content and varying number of compaction passes. The methodology employed in determining the field density shall be developed by the Contractor and its Quality Manager, and be subject to the approval of the Ministry Representative

If the Contractor's methodology is not approved by the Ministry Representative, then the Contractor may propose an alternative method or the Ministry Representative may order that each lift or course of aggregate shall be continuously and thoroughly rolled until successive passes of a vibratory roller results in an increase in density of less than 10 kg/m³.

The vibrating roller shall have a minimum steel drum diameter of 1.15 m, a minimum drum width of 1.5 m, and shall be capable of being loaded so as to have a gross mass of 20 kg per lineal centimeter of drum width.

However, if the Contractor elects to use alternate compaction equipment, then upon written request by the Contractor, the Ministry Representative may give

written permission for the substitution of new or alternative compaction equipment if the Ministry Representative is satisfied that such equipment will provide equal or superior compaction performance.

(c) Proof Rolling – The Ministry Representative may require a proof rolling, in accordance with SS 202.29, at any time to assess the performance of the grade at the Target Density.

202.23 Bridge End Fill – Construction of the bridge end fill shall not commence until the Ministry Representative has assessed the pertinent foundation conditions that may affect the future stability of both the bridge and embankments and has authorized continuation of construction. Unless the procedure for construction is stated in the Special Provisions, the Contractor's proposed method must be submitted to the Ministry Representative for approval before commencing work.

Bridge end fill shall be constructed to the specified thickness and dimensions as shown on the Drawings or as described in the Special Provisions, unless otherwise directed by the Ministry Representative. The bridge end fill shall be constructed to the subgrade elevation.

The material shall consist of mineral soil with properties and gradation in accordance with SS 202.04 and SS 202.05.

The bridge end fill shall be constructed in successive horizontal layers not exceeding 150 mm in loose thickness, compacted to meet or exceed the applicable Target Density determined in accordance with SS 201.22.

202.24 Thickness of Granular Courses – Crushed surfacing course, crushed base courses and subbase courses shall be constructed to the specified thickness and dimensions as shown on the Drawings or as described in the Special Provisions, unless otherwise directed by the Ministry Representative.

Normally, on new construction, crushed bases shall be constructed 300 mm thick (in individual lifts with compacted thickness of 150 mm) unless otherwise approved by the Ministry Representative. Additional base or subbase granular materials will be of such thickness as may be considered necessary to provide supporting strength for the flexible pavement structure.

202.25 Construction of Subbase

202.25.01 Aggregates for subbase shall be delivered to the roadbed as uniform mixtures and shall be spread in layers without segregation, preferably through an approved aggregate spreader. Granular aggregate shall not be end-dumped from trucks in piles on the grade. The Ministry Representative may permit spreading from the tailgate of trucks or from centre dump units, provided the Ministry Representative is satisfied that the work will be well controlled and segregation will not occur.

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When the subgrade below granular aggregate subbases consists of material susceptible to damage from wheeled equipment, and written permission is granted by the Ministry Representative, a portion of the granular aggregate subbase or base may be dumped in piles upon the subgrade and spread ahead in sufficient quantity to stabilize the subgrade. Segregation of aggregates shall be avoided and the material as spread shall be free from pockets of large or fine material. Segregated materials shall be remixed until uniform. Static compaction shall be used in areas of sensitive soil subgrades.

Where the required thickness is 150 mm or less, the granular base or subbase may be spread and compacted in one layer. Where the required thickness is more than 150 mm, the aggregate shall be spread and compacted in two or more layers of approximately equal thickness; the maximum compacted thickness of any one layer shall not exceed 150 mm. Each layer shall be spread and compacted in a similar manner. At locations where the granular base or subbase is to be placed over areas susceptible to damage by the spreading equipment, the granular subbase or base may be spread by any means acceptable to the Quality Manager and the Ministry Representative to obtain the specified results.

202.25.02 Immediately following spreading, the material shall be compacted to meet or exceed the applicable Target Density.

202.25.03 The Contractor may water the subbase aggregates as required to aid in attaining the specified density, but shall not ensure that overwatering does not occur.

202.25.04 The completed surface of the subbase course shall conform to the required line, grade and cross section as shown on the Drawings to an accuracy of ± 15 mm, neither uniformly high nor low.

202.26 Construction of Crushed Base Course

202.26.01 Crushed base course aggregate shall be placed on a properly prepared subbase surface to such depth or at such rates as may be specified. If the Ministry Representative is of the opinion that the finished surface of the subbase does not meet the requirements of SS 202.25.04 but has been thoroughly and densely compacted and should not be disturbed, the Ministry Representative may order that the surface of the subbase be corrected to the true cross section, line and grade, and within the tolerances specified by use of a levelling course of crushed 25 mm base course aggregate. In such event, the crushed 25 mm base course aggregate so used will be paid for only at the rates bid for subbase, provided the same Contractor is responsible for the construction of both the crushed base, and subbase.

The crushed base course shall be constructed in such a manner that the aggregate is neither segregated, contaminated nor degraded. End dumping will not be permitted. The thickness of the crushed base course shall be substantially uniform and the minimum thickness shall

not be less than the nominal thickness shown on the Drawings or ordered by the Ministry Representative. If the Contractor is unable to provide adequate manually operated equipment or workers of sufficient skill to lay the crushed base course aggregate within the tolerances specified, the Ministry Representative may require that the Contractor lay the aggregate through an approved electronically controlled spreading machine.

202.26.02 Compaction – Immediately following spreading, the crushed base course aggregate shall be compacted to meet or exceed the applicable Target Density.

The method of compaction to be employed may be selected by the Contractor, but shall be subject to approval by the Quality Manager and the Ministry Representative

202.26.03 Watering – Subject to the agreement of the Ministry Representative, the Contractor may water the base course aggregates as required to aid in attaining the specified density.

202.26.04 Tolerance of Completed Surface – The completed surface of the granular base course shall conform to the required line, grade and cross section as shown on the Drawings to an accuracy of ± 10 mm, neither uniformly high nor low.

202.27 Equipment for Watering – Water shall be applied from a pressure type distributor, equipped with spray bar mounting nozzles similar to those used on asphalt distributors and capable of applying the water accurately and uniformly. Splash plate type distributors or those equipped with spray bars that eject fine streams of water will not be permitted. The distributor must be provided with a satisfactory means for accurately measuring the quantity of water sprayed. If the Ministry Representative so requires, measuring equipment shall be calibrated under the Ministry Representative's inspection. All necessary arrangements and permits for obtaining water shall be performed by the Contractor and at the Contractor's expense.

202.28 Weather and Job Conditions

202.28.01 No construction shall be undertaken during snow, heavy rain, freezing or other unsuitable conditions. Aggregate shall not be placed upon a frozen, wet, muddy, rutted or undulating or rutted subgrade, subbase, base or surface unless otherwise directed by the Ministry Representative.

202.28.02 When the subgrade is soft due to excessive moisture, the placing of granular sub-base shall be stopped until rutting or displacement of the initial subbase layers can be prevented by reduced loading or by other means approved by the Ministry Representative.

202.29 Proof Rolling and Stabilizing Crushed Base Course – Before acceptance, each compacted course of base course aggregate shall receive one complete coverage by the tires of a truck having a 9 tonne single axle dual tire or 17 tonne tandem axle group with dual tires with a tire

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pressure of 600 kPa. The truck shall be loaded to its gross vehicle weight and driven at a speed of 1 to 2 m/s.

There shall be no deflection, reaction (noticeable movement in a localized area directly under the vehicle tire) or pumping of the ground surface (vertical displacement of top surface not directly under vehicle tires) observed under the moving vehicle's tires. This testing shall be performed under the observation of the Ministry Representative.

Any areas where rutting or displacement occurs shall be either excavated or replaced and proof rolled or stabilized by the addition of suitable blending material incorporated uniformly into the base to the satisfaction of the Ministry Representative.

The supply, load, haul, placing, proof rolling, and mixing of such stabilizing aggregates as necessary to correct deficiencies in aggregate stability shall be incidental to the Work for Base aggregates. Blending may be performed at the pit or quarry or on the highway in a manner acceptable to the Ministry Representative.

202.30 Benkelman Beam Testing – At any time during the course of the work, when considered necessary by the Ministry Representative, Benkelman Beam testing may be undertaken at the Ministry's expense.

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202.31 Water Applied to the Highway – Water for compaction or dust control shall be incidental to the bid price for the granular material.

202.32 Development and Cleanup of Ministry-Provided Pits and Quarries – Authorized clearing and grubbing of Ministry provided pits and quarries and construction of the access road is the responsibility of the Contractor. Removal and stockpiling of overburden/topsoil during development of the proposed work areas, and spreading overburden on a depleted pit or quarry shall be the responsibility of the Contractor and the work shall be completed to the satisfaction of the Ministry Representative.

Unless explicitly stated otherwise in the Special Provisions, no separate payment will be made for clearing, grubbing, disposal or relocation of stockpiles, debris or contaminated materials, or for any other costs of site preparation, pit development, remediation, or access, or for any delay or other cost arising from the use of Ministry-provided pits or quarries by the Contractor, and all costs thereof shall be covered in the prices for the Items under which payment is provided for the applicable materials.

202.33 Stockpile Reject Aggregate

202.33.01 Aggregate from screening or crushing operations in a Ministry-provided pit or quarry, whether:

(a) Oversize under SS 202.09, or

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(b) Rejected at the crusher in order that the product meets the requirements of SS 202.05,

shall be stockpiled separately according to size in accordance with SS 202. Stockpiles of reject aggregate shall not be contaminated by organic or other deleterious materials.

202.33.02 No payment will be made for any reject aggregate.

202.34 Blending Materials for Stabilizing Aggregates on the Highway – Blending materials, supplied, loaded, hauled, placed and mixed into the granular aggregate on the highway to correct deficiencies in aggregate stability, will be paid for at the same rate as the granular aggregate which has to be stabilized. The final blended gradation must still meet the design aggregate gradation.

Normally, no additional payment will be made to cover the costs of work required to correct stability failure in the aggregate unless the Ministry Representative is satisfied that such instability is inherent in the type of aggregate available to the Contractor from a mandatory use Ministry-provided pit or quarry, and that the lack of stability has not arisen because of unsatisfactory production methods or improper construction practices. In these circumstances, the Ministry Representative will issue an Order for Extra Work to the Contractor to cover the additional cost of mixing blending material into the granular aggregate to correct the stability deficiency.

202.35 Aggregate Supplied in Stockpile Only – Aggregate, supplied in stockpile only, will be paid for at the Unit Price bid per CUBIC METRE, measured in stockpile and computed by surface to surface or, at the Ministry Representative's option, average end areas.

Payment at the unit price bid price shall be accepted as full compensation for everything furnished and done to supply (where applicable), produce and stockpile the crushed aggregate.

202.36 Aggregates Supplied in Place

202.36.01 Unit Price Payment – Crushed or screened aggregates (surfacing, base, subbase and bridge end fill) supplied and constructed in place will be paid on a neat line basis for at the applicable Unit Price bid per CUBIC METRE. Payment shall be full compensation for everything furnished and done, including without limitation costs for acquisition, development, and payment of royalties for private pits or quarries, purchase price of aggregate, access road maintenance, screening, crushing, stockpiling, loading, hauling, spreading, compaction and moisture adjustment (watering or drying) in place as specified.

202.36.02 Advance Payment: Crushed or Screened Aggregates Supplied in Place – An advanced payment of the greater of 70% of the demonstrated production costs or 30% of the extended amount for the Item will be made for crushed or screened aggregates (surfacing, base or subbase)

in stockpile. No advance payment will be made for pit run aggregates. Recovery of the advance payment will be made on each progress estimate as the respective aggregate is subsequently withdrawn from the stockpile, with an additional adjustment on the final progress estimate for any material remaining in stockpile such that the entire advance payment is recovered.

202.36.03 Surplus Aggregate – Where actual aggregate needs on the Contract fall below the Approximate Quantity indicated on Schedule 7, the Ministry will compensate the Contractor at the rates indicated in **Error! Reference source not found.** for a portion of surplus aggregates produced by the Contractor which meet all the applicable quality and gradation specifications of the contract.

For each classification of aggregate, the maximum quantity of surplus aggregate eligible for compensation shall be the Approximate Quantity of that aggregate on Schedule 7 minus the quantity of that aggregate paid on the final progress estimate.

There will be no compensation for surplus aggregate for pit run aggregates or for any portion of the production on other aggregates which are beyond the Approximate Quantity. Where aggregates are purchased from a pre-existing commercial pit or quarry operation, there will be no compensation for any aggregate beyond that actually used in the Contract.

Surplus aggregates, originating from private land, in stockpile on Ministry lands are the property of the Ministry and no additional compensation will be paid. Surplus aggregate stockpiles on private lands will be the property of the Contractor and there will be no compensation for any aggregate beyond that actually used in the contract.

Table 202-F: Compensation for Surplus Aggregate

Stockpile Location	Compensation for Surplus Aggregates (\$/m ³)	
	Crushed	Screened
Ministry land	\$7.50	\$3.75
Private land	\$5.00	\$2.50

202.36.04 Ministry Materials Processed on Private Lands – Where the Contractor elects to source raw materials from a Ministry pit or quarry, but process those materials on private lands:

(a) The Contractor shall extract only as much raw material from the Ministry source as is necessary to produce the quantity of product required to be incorporated in the final Project, inclusive of staging and maintenance

requirements, with a small additional contingency amount determined by the Contractor and the Quality Manager and approved by the Ministry Representative.

(b) Ministry-sourced materials shall not be used in any way to improve the private lands, including without limitation for such purposes as access development, maintenance, or stockpile site preparation, unless approved in advance by the Ministry Representative under any conditions the Ministry Representative may require.

(c) Any surplus raw or processed material remaining on the private lands at the end of construction shall:

(i) be returned by the Contractor to the Ministry source and stockpiled at a location determined by the Ministry Representative, all at the Contractor's expense; or

(ii) upon request by the Contractor and at the Ministry Representative's sole discretion, the Contractor may be offered the options of:

(A) of leaving the materials on the private land and providing a written agreement with the owner of the property which shall state that the Ministry will have free access to and exclusive use of the remaining materials for a period of twenty-four (24) months after the completion of the Contract, after which time the Ministry relinquishes all claims to the material; or

(B) purchasing the remaining material at a negotiated amount to be back-charged to the Contractor.

(d) Where the volume of material remaining is less than the additional contingency amount determined pursuant to SS 202.30.04(a), the Ministry Representative will make the offers in SS 202.30.04(c)(ii). If agreement is not reached, SS 202.30.04(c)(i) will apply

202.37 Stockpile Volume Determination

The Ministry will determine the volume of aggregate by surveying the stockpile(s) using string-line techniques, determine volume using prismatic volumes between surfaces, and deducting 10% for stockpile loss. Alternatively, the Ministry Representative may elect to survey using cross section techniques and/or determine volumes using end-area volumes. The base of the pile will be determined from a pre-stockpile survey or, where such a survey is not available, from a surface determined by the Ministry Representative as being a reasonable interpolation from the intersection of the pile sides with the adjacent ground level. Where there are two or more stockpiles of aggregate meeting the same gradation classification, the compensation will be based on the cumulative quantity of those stockpiles.

APPENDIX 202-A
FRACTURE COUNT FOR COARSE AGGREGATE
(BCH 1-13)

Purpose:

This test determines the amount of fractured material in the coarse part of an aggregate sample.

- i) **Method "A"** is used for crushed granular surfacing and base aggregates.
- ii) **Method "B"** is used for paving aggregates.

Apparatus:

- Drying oven, preferably forced draft, capable of maintaining a uniform temperature of $110 \pm 5^\circ \text{C}$.
- A nest of sieves of the following sizes:
 - Method "A"**: 37.5 mm, 25.0 mm, 19.0 mm, 12.5 mm, 9.5 mm, 4.75 mm
 - Method "B"**: 19.0 mm, 13.2 mm, 9.5 mm
- Balance with sufficient capacity and accurate within 0.1 percent of the mass of sample.

Test Sample:

The sample of aggregate to be tested shall be representative, oven-dry, and reduced to an amount suitable for testing. The mass of the sample shall conform to the following:

Maximum Nominal Size (mm)	Minimum Dry Mass (kg)
37.5	4.0
25.0	2.5
12.5	1.5
9.5	1.0

Procedure:**a) Method "A"**

- Sieve the coarse aggregate to be tested on the 37.5 mm, 25.0 mm, 19.0 mm, 12.5 mm, 9.5 mm 4.75 mm sieves.

- Separate each sieve size into two groups, fractured and unfractured. Each rock in the fractured group shall have at **least one** fractured face or shall be naturally angular with sharp edges. (See Discussion 1).
- For each sieve size, count the number of fractured rocks and the total number of rocks (fractured and unfractured).
- Calculate and record the % fracture for each sieve size and report the total % fracture for the sample using Equation 202-A-1 and Equation 202-A-2.

b) Method "B"

- Separate the coarse aggregate to be tested from the fine aggregate by sieving it over the 4.75 mm screen.
- Separate the coarse aggregate for each sieve size into two groups, fractured and unfractured. Each rock in the fractured group shall have **two or more** fractured faces produced by a crusher or shall be naturally angular with sharp edges. Each rock in the unfractured group shall be unfractured or fractured only in one dimension.
- Obtain the mass of the fractured group, also the mass of the total coarse aggregate (fractured + unfractured groups).
- Calculate and report the total % fracture for the sample using Equation 202-A-3.

Discussion:

- Fractured face shall be defined as a fracture in any plane whose area is 15% or more of the largest cross section area in a parallel plane.
- The required sample can be conveniently obtained from the separated fractions of the Dry or Wash Sieve Test.

Equation 202-A-1: Percent Fracture for Each Sieve Size (e.g. 19 mm)

$$\% \text{ Fracture (19 mm)} = \frac{\text{Number of Fractured Rocks (19 mm)}}{\text{Total No. of Rocks (19 mm)}} \times 100\%$$

Equation 202-A-2: Percent Fracture for the Total Sample (by Number of Rocks)

$$\text{Total \% Fracture} = \frac{\text{Number of Fractured Rocks (All Sieve Sizes)}}{\text{Total No. of Rocks (All Sizes)}} \times 100\%$$

Equation 202-A-3: Percent Fracture for the Total Sample (by Mass)

$$\% \text{ Fracture} = \frac{\text{Mass of Fractured Rock}}{\text{Mass of Total Rocks}} \times 100\%$$

APPENDIX 202-B
PETROGRAPHIC ANALYSIS TEST

Purpose:

The purpose of the Petrographic Analysis Test is to identify the various rock types and rock characteristics in the aggregate fraction of material retained on the 9.5 mm sieve of a gravel deposit. The test is performed after a Micro Deval soundness test or a magnesium sulphate soundness test indicates that the granular materials are weak, non durable or suspect. The petrographic analysis test is to prove or determine the cause of the poor performance of the aggregate and to determine the extent or contributing factors of specific rock types or the extent of deleterious materials or clay particles. This information will be used to determine the possible use of the materials.

Scope:

The test shall be performed by a qualified professional with experience in rock type identification, rock classification and gravel/aggregate sources and use; and shall be presented in a signed report that details findings. Recognized procedures for rock identification shall be used. The procedures involve simple physical and chemical tests, such as the hardness scratch test, hydrochloric acid reaction and visual surface examination assisted by magnifying lens. The predominance of easily distinguishable types in petrographic samples will permit reasonably accurate testing. Important accessories are a sample kit, the rock and mineral descriptions and a good text.

Rock types are to be listed by percentage weight under the broad categories of origin (sedimentary-metamorphic-igneous) with condition descriptions (i.e. hardness, weathering and cementation). Note that the descriptions of quality regarding durability or soundness (good, fair, poor, deleterious) are vital to the test results.

Additional information on general condition including weathering, shape, porosity, coatings etc. shall also be recorded.

Apparatus:

1. Balance – 20,000 gram capacity, accuracy ± 0.1 gram.
2. Oven capable of maintaining 110°C.
3. Sieves 19 mm (3/4 inch) and 9.5 mm (3/8 inch) and shaker.
4. Sample splitter with pans.
5. Hand-lens (10X magnification).
6. A ten percent solution of hydrochloric acid.
7. Aluminum pans.
8. A stereoscopic microscope.

Sample Preparation

A 1500 gram sample of representative aggregate passing 19 mm and retained on 9.5 mm screen is required.

1. For pit run materials, a minimum of 18 kg, or one full gravel bag of representative aggregate is required.
2. This material is oven-dried at $110^{\circ}\text{C} \pm 5^{\circ}\text{C}$ overnight, or for 16 hours.
3. After drying, the sample is quartered in the sample splitter.
4. A one-quarter sample is sieved to obtain passing 19 mm aggregate. Additional quarters are sieved if the initial quarter provides less than 1500 grams of the required sizes.
5. The passing 19 mm retained on 9.5 mm aggregate is mixed and 1500 grams separated by splitter.
6. Fractured materials are separated from non-fractured materials and a fracture count test shall be performed using Method A (Appendix 202-A).
7. Following removal (by hand) of clay balls and very soft weak material, the non-fractured rocks are washed to assist visual identification of mineral characteristics. The soft fractured rocks shall be washed with great care as their degradation may prevent correct identification. The clay balls and lumps shall be weighed and the percent of sample noted.

Rock Type Classification:

Assignment of each particle to rock type and character is required.

Identification of specific rock type may be preceded by general identification under the major rock categories of: igneous, sedimentary and metamorphic. Following simple physical, chemical and visual tests, each particle is then identified with a specific rock type and description.

Rock types are weighed to the nearest 0.1 gram. Weights are recorded on the test sheet and the percentage of each is calculated from the total sample weight.

Comments on each rock type include descriptions of physical and chemical qualities such as:

- Hardness: hard, medium, soft.
- Weathering: non weathered, slightly or intensely weathered.
- Porosity
- Cementation: firmly cemented to friable.
- Coatings

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Quality Characterization (good, fair, poor, deleterious)

Basis of Quality System:

The following quality distinctions are relative estimates of a rock's physical and chemical condition and of probable engineering quality.

- **Good:** Particles are hard, durable, free from fracture potential, little or no capillary absorption.
- **Fair:** Particles are soft but sound and tough, medium hard, slightly to moderately weathered, have small to moderate capillary absorption, are relatively smooth and impermeable.
- **Poor:** Particles are soft to very soft, friable, potential slaking when wetted and dried, highly fractured potential, capillary absorption moderate to high.
- **Deleterious:** Particles react chemically with alkali in Portland Cement causing expansion in concrete, although they may be suitable in base course.

Report Submission Requirements:

The report shall describe the tests performed, present the test results, including descriptions of weathering, shape, porosity, coatings, angularity and provide a petrographic number.

A description of suitability shall be stated by the individual performing the test.

The report shall contain an executive summary of rock types and conditions as shown in the example below.

“e.g. Igneous granodiorite 90% hard, non-weathered. volcanics 10% hard and fresh, with some volcanics (30% of volcanics) slightly weathered and some volcanics (5% of volcanics) are vesicular, soft and intensely weathered. No clay lumps, some particles (less than 5% of sample) contained coatings of clay.”

Sample Petrographic Number:

The Petrographic Number (PN) for the sample is calculated by multiplying the percentage of each of the four quality classification by the following multipliers:

- Good 1
- Fair 3
- Poor 6
- Deleterious 10

and then summing the results.

Sample Petrographic Number	Overall Sample Rating
100 to 125	Good
>125 to 140	Fair
>140 to 155	Poor
> 155	Deleterious

SECTION 204

ROCK CUTS

DESCRIPTION

204.01 Scope – This Section describes the controlled blasting techniques required to produce rock cut slopes at the locations shown on the Drawings or as authorized by the Ministry Representative. Rock shall be excavated to subgrade elevation as shown on the Drawings. The work shall be conducted, whether by blasting or other methods, in a manner that complies with SS 165, minimizes blast damage to the excavation backslope, and minimizes the requirement for stabilization. The responsibility for the results of every blast lies with the Contractor. Nothing within this specification shall be interpreted to mean the Ministry accepts responsibility for the results of any blast.

204.02 Definitions

204.02.01 Backline Holes – A line of holes detonated or intermittently detonated along the specified excavation backslope.

204.02.02 Bench – A near-horizontal or shallow slope surface at the top of a near-vertical rock face where blast holes are collared.

204.02.03 Bench Width – The horizontal distance on the bench between the excavation backslope and the crest of the existing rock face. Generally, this dimension defines the width of the rock to be removed and is perpendicular to the highway alignment.

204.02.04 Blast Crater – A local depression in rock formed by blasting.

204.02.05 Buffer Holes – The line of production holes located closest to and parallel to the backline holes.

204.02.06 Burden – The distance between the blast hole and the nearest free face.

204.02.07 Charge – A specific quantity of explosive such as a cartridge.

204.02.08 Controlled Blasting – Controlled blasting is the controlled use of explosives and accessories in carefully spaced and aligned production and backline drill holes to produce the specified excavation backslope within the specified excavation limits. The purpose of controlled blasting is to minimize damage to the rock backslope, to help ensure long-term stability and to minimize flyrock.

204.02.09 Controlled Delay Sequence – The delay sequence required for controlled blasting.

204.02.10 Cushion Blasting – A blasting method whereby the production holes are detonated before the backline holes.

204.02.11 Delay – A blast initiator with a specific delay time period before detonation.

204.02.12 Excavation Backslope or Smooth Wall – This is the location of the free surface or shear plane in the rock produced by blasting along the backline holes.

204.02.13 Excavation Limits – This is the boundary within which rock is removed.

204.02.14 Explosive Decoupling – The separation of an explosive charge from the wall of the blast hole.

204.02.15 Flyrock – The throw of fragmented material during blasting.

204.02.16 Free Face – The rock face that provides relief for a blast.

204.02.17 Guide Holes – Those holes along the backline that are not loaded with explosive and are not stemmed.

204.02.18 Hole Deviation – Borehole misalignment distance measured in two directions, within and perpendicular to the excavation backslope. Within the plane of the excavation backslope, hole deviation is borehole misalignment as measured by the horizontal offset distance between any point in the design hole location and the point at a corresponding depth in the actual hole. Perpendicular to the plane of the excavation backslope, hole deviation is the horizontal offset distance between the plane of the design excavation backslope and the centreline of the borehole being measured.

204.02.19 Lift – The vertical distance between the top and bottom of an area to be blasted.

204.02.20 Overbreak in Rock – Overbreak in rock is any rock outside of the specified excavation limits (except slide material described in SS 201.42) that is excavated, displaced or loose due to the inherent character of any formation encountered or due to any other cause.

204.02.21 Overburden – This is Type D material (as described in SS 201.11) overlying rock.

204.02.22 Presplit (or Pre-Shear) Blasting – A blasting method whereby backline holes are detonated before production holes are detonated.

204.02.23 Production Blasting – Blasting of production holes detonated in a controlled delay sequence.

204.02.24 Production Holes – All holes within the specified excavation limits that are not backline holes. Production holes are often more widely spaced than backline holes.

204.02.25 Rock Excavation – Rock excavation includes drilling holes, loading holes with explosives, detonation by

controlled blasting, mucking to subgrade and ditch level, and removal of material to a designated area. Rock excavation also includes secondary breakage of oversize rock.

204.02.26 Rock – Rock is Type A material as defined in SS 201.11.01.

204.02.27 Scaling – The removal of all loose material using scaling bars, portable hydraulic jacks, other hand tools, wire rope cables, compressed air blow pipes, blasting without the need for drilled holes and other methods authorized by the Ministry Representative.

204.02.28 Setback Distance – The setback distance is the horizontal distance between the top of the excavation backslope and the toe of the overburden material. Setback is created by removing overburden.

204.02.29 Spacing – The distance between blast holes perpendicular to the burden.

204.02.30 Stemming – Material placed on top of the explosive to provide confinement of the explosive gases.

204.02.31 Subdrill – The distance drilled below subgrade level.

204.02.32 Trimming – The removal by drilling and controlled blasting of potentially unstable rock remaining at the exposed excavation backslope. Trimming may not require backline holes.

204.03 Quality Control

204.03.01 General – Quality control shall be conducted for all blasting operations.

204.03.02 Blasting Consultant – The Contractor shall retain a blasting consultant specialist, acceptable to the Ministry Representative, to provide quality control. The consultant shall not be an employee of the Contractor, explosives manufacturer or explosives distributor.

204.03.03 Vibration Specialist – The Contractor shall provide a blast vibration monitor if required. The blast vibration monitoring shall be under the direction of the Vibration Specialist.

204.03.04 Quality Control Activities For Blasting Operations – Quality control for blasting operations shall be performed for test sections, full scale blasting operations and trimming. A field report generated by the Consultant shall be provided to the Ministry Representative within one day of each site visit by the Consultant. Quality control for blasting includes, but is not limited to the following activities by the blasting consultant:

- (a) Viewing the proposed blast area at least one (1) week before drilling operations commence.
- (b) Preparing and submitting blast designs for production and backline holes.

- (c) Intermittently viewing the site during drilling for each blast area.
- (d) Intermittently observing the loading of holes with explosives and tying in to the delay system.
- (e) Observing the blasts and reviewing the excavated areas.
- (f) Attending on site to view the excavation backslope after each lift and reviewing excavation backslope for quality control purposes.
- (g) Viewing the site while reviewing proposed changes to the blast design and preparing new designs.
- (h) Attending on site at other times as are appropriate to assist the Contractor in the setting up, assessment and adjustment of the various procedures to be employed for blasting.
- (i) Preparing a field report for each site visit, including details of the progress of blasting operations, a statement whether the blast design is being complied with, any changes to the blast design, any recommendations made to the Contractor and any problems encountered by the Contractor.

204.04 Submittals

204.04.01 General – The Ministry may perform Quality Audits. The receipt of submittals by the Ministry shall not relieve responsibility from the Contractor for the accuracy and adequacy of the submittals. Submittals are for quality assurance and record keeping purposes. Inadequate or incorrect submittals will be returned for revisions prior to acceptance. The Contractor shall submit to the Ministry Representative the following documentation for review.

204.04.02 Qualifications and Experience of Contractor – The Contractor shall provide a statement of the qualifications, experience and work function of all personnel assigned to drilling and blasting duties. A statement of previous work experience on similar projects shall also be provided. This statement shall include the project name, location, volume of rock, year constructed and the owner/client contact name. The company, the driller and the blaster shall each have a minimum of 5 consecutive years demonstrated experience in drilling and controlled blasting work on at least 3 projects involving rock cuts over 8 m height along transportation corridors.

204.04.03 Qualifications and Experience of Proposed Blasting Consultant – The Contractor shall provide the name of the consulting firm, and the name and qualifications of the blasting consultant's on-site representative who will be providing the quality control for rock excavation. The blasting consultant shall have a minimum of 5 consecutive years demonstrated experience in preparing successful blast designs along transportation corridors for at least 3 projects. The following information shall be included in the qualifications submitted:

- (a) Project name, location and experience.

- (b) Name and phone number of owner/client contact who can verify the experience of the blasting consultant's site representative.

204.04.04 Qualifications and Experience of Proposed Vibration Specialist – The Contractor shall provide the name and qualifications of the vibration specialist who will be providing the quality control for rock excavation. The vibration specialist shall have a minimum of 5 consecutive years demonstrated experience in the field of vibration monitoring for at least 3 projects. The following information shall be included in the qualifications submitted:

- (a) Project name, location and experience.
 (b) Name and phone number of owner/client contact who can verify the experience of the vibration specialist.

204.04.05 Proposed General Construction Plan – The Contractor shall submit a general construction plan to the Ministry Representative showing proposed blasting sequence numbers defining the order of blasts for the contract. The plan shall also show the following information for each blast:

- (a) Blast sequence number.
 (b) The location of the blast in relation to the remaining rock on the site.
 (c) Approximate dimension of the rock to be removed.
 (d) Estimated volume of rock to be removed.
 (e) Location of the disposal site.

204.04.06 Sequence of Operations – The Contractor shall adopt a logical, systematic sequence of operations to ensure blasting is conducted safely and effectively. The following general sequence of blasting operations outlines minimum requirements of the Contractor to maximize the stability of the excavation backslope and does not limit or supersede any other requirements in this specification:

- (a) Retain a blasting consultant to perform quality control.
 (b) Submit blast designs for review and authorization by the Ministry.
 (c) Accurately survey locations of proposed blast holes.
 (d) Drill holes, load explosives, detonate blast and remove muck pile.
 (e) Check the excavation backslope to determine hole offsets, hole angles, hole alignment and compliance with allowable tolerances.
 (f) Review blast results and make changes to blasting operations as necessary.
 (g) Perform backslope stabilization to the satisfaction of the Ministry Representative before subsequent lifts are detonated.

204.04.07 Blast Design – The Contractor shall provide and follow a blast design, approved and signed by the blasting consultant, not less than one week prior to commencing drilling and blasting operations and a minimum of one day before the Contractor proposes to implement any changes to the previously utilized drilling or blasting methods. The design may be prepared by the blaster, but shall be reviewed by the Contractor and forwarded to the blasting consultant for signing. The design shall contain full details of the drilling and blasting patterns and controls that the Contractor proposes to use for controlled blasting. The blast design shall contain the following minimum information:

- (a) Date the design was prepared and proposed date of blast.
 (b) Station limits of proposed blast.
 (c) Plan and section views of proposed drill pattern including free face, burden, blasthole spacing, blast hole diameters, blast hole angles, lift height, drill offset, hole depth, and subdrill depth.
 (d) The location of production (including buffer) and backline holes.
 (e) Loading diagram showing type and amount of explosives, primers, initiators, and location and depth of stemming.
 (f) Initiation sequence of blast holes including delay times and delay system.
 (g) Manufacturer's data sheets for all explosives, primers, delays, and initiators to be used.
 (h) Blasting consultant's signature, printed name, and company name.
 (i) Blaster's signature, printed name, company name, and blaster's certificate number.

204.04.08 Pre-blast Survey – The Contractor shall conduct a pre-blast survey a minimum of one day before blasting operations commence. The pre-blast survey shall include a complete description of the existing condition of any nearby buildings, structures, wells and utilities that potentially may be damaged by blasting operations. The survey method used shall be acceptable to the Contractor's insurance company.

204.04.09 As-Built Blast Design – Within one day after each blast, the Contractor shall submit, unless otherwise authorized by the Ministry Representative, an as-built blast design showing all actual blast details in a format that permits direct comparison with the proposed blast design.

204.04.10 Blasting Consultant's Field Report – The Contractor shall provide the blasting consultant's field report to the Ministry Representative within one day after each visit by the blasting consultant.

204.04.11 Vibration Control Records – As required, the Contractor shall provide all seismograph records of

vibration monitoring and interpretation of results within one day after each blast to the Ministry Representative.

204.04.12 Peak Overpressure Records – As required, the Contractor shall provide a permanent signed and dated record of the peak overpressure measurements within one day after each blast to the Ministry Representative.

MATERIALS

204.11 Explosives and Related Products

204.11.01 Manufacturer – All products and materials used for any blasting operations shall be products of a company regularly engaged in the manufacture of explosives and related products.

204.11.02 Expired Explosives – Explosives with an expired shelf life shall not be used.

204.11.03 Water Resistance – Water resistant explosives may be required for the work.

CONSTRUCTION

204.31 Permits and Regulations – The Contractor shall obtain all necessary permits and shall comply fully with the laws, rules and regulations of Municipal, Provincial and Federal agencies in connection with the use, transportation, storage and safe handling of all explosives, including those regulations contained in the WCB Occupational Health and Safety Regulation.

204.32 Supervision – The Contractor shall provide at least one person thoroughly trained and experienced in the use of explosives who shall be present at all times during the execution of all blasting operations and who shall direct such work.

204.33 Personnel – The Contractor shall ensure that all persons conducting blasting operations have a valid blaster's certificate issued by the WCB or is under the direct supervision of a certificate holder.

204.34 Safety – The Contractor shall meet all WCB regulations. All work shall be performed in a manner that prevents injury or harm to any personnel employed in the rock excavation area. Warning signs shall be posted and readily recognizable audible warning signals shall be used. The perimeter of the area affected by blasting operations shall be patrolled and controlled by direct voice communication. The Ministry Representative shall stop the work if the safety of the public is being jeopardized by the Contractor's blasting operations.

204.35 Flyrock Control – Before the detonation of any blast in areas where flying rock or other debris may result in personal injury or damage to property, the area within the excavation limits shall be covered with suitable blasting

mats, soil or other equally serviceable material to prevent flyrock.

204.36 Other Damage – The Contractor shall be responsible for any damage resulting from blasting. Occupants of local buildings shall be notified by the Contractor, prior to the commencement of the blasting, as to the timing, size of blasts, types of warning and other signals.

204.37 Subgrade and Ditch Areas

204.37.01 General – The subgrade shall be constructed to ± 50 mm of the specified line and grade before the placement of Select Granular Subbase Material. Any pinnacles of intact rock protruding above the design subgrade elevation shall be removed.

204.37.02 Supporting Rock – The Contractor shall exercise care and use appropriate methods to prevent breaking, loosening or otherwise damaging supporting rock below subgrade level and ditch bottom. The Contractor shall be responsible for the methods used and for any damage to the rock structure resulting from the operations.

204.37.03 Drainage – Excavated rock shall be free draining unless otherwise authorized by the Ministry Representative. If craters formed in rock by blasting below subgrade elevation are not free draining, then the Contractor shall provide drainage by trenching to a free outlet. These blast craters and drainage trenches shall be backfilled to subgrade elevation.

204.37.04 Shatter Zone for Drainage – Unless otherwise authorized by the Ministry Representative, drainage shall also be provided by shattering the upper portion of rock below subgrade and ditches within the entire excavation limits. The shatter zone shall be 300 mm thick. A shatter zone thickness of 900 mm may be required near cut to fill transition zones for 10 m beyond the transition, or as requested by the Ministry Representative.

204.37.05 Backfill Material – Backfill shall be free draining material such as clean broken rock or coarse clean granular material from a source authorized by the Ministry Representative. All backfill shall be placed and compacted as specified.

204.37.06 Ditches – The ditches shall be formed and cleaned before any base material is placed on the subgrade.

204.38 Blasting Test Section(s)

204.38.01 General – All requirements for full scale blasting shall also apply to test sections unless otherwise authorized by the Ministry Representative. Prior to commencing full-scale blasting operations, the Ministry Representative may require the Contractor to demonstrate the adequacy of the proposed blast design by drilling, blasting, and excavating short test sections, up to 30 m in length, to determine which combination of method, hole spacing, timing, and charge

yields acceptable results. The length of the blast test section shall be as specified in the blast design.

204.38.02 Reduced Section Length – Where requested by the Ministry Representative, the Contractor shall use test section lengths of less than 30 m.

204.38.03 Backline Hole Spacing – The Contractor shall begin the tests by drilling backline holes 750 mm apart along the specified excavation backslope, then adjust if needed, until the Ministry Representative authorizes the spacing to be used for full-scale blasting operations.

204.38.04 Review of Test Section Results – The Contractor shall not drill beyond the test section until it has been excavated and the results reviewed by the blasting consultant and continuation authorized by the Ministry Representative.

204.38.05 Revision of Methods – If either the blasting consultant or the Ministry Representative determines that the results of the test section are unsatisfactory, then notwithstanding the Ministry Representative's prior review of such methods, the Contractor shall adopt such revised methods as are necessary to achieve the required results.

204.38.06 Additional Test Sections – If at any time during the progress of the work the methods of drilling and blasting do not produce the required excavation backslope geometry within the tolerances specified for backline holes, then the Contractor will be required to drill, blast and excavate short sections, not exceeding 30 m in length, until a technique is achieved that will produce the desired results.

204.39 Overburden Removal and Setback – The setback distance shall be 3.0 m. Unless otherwise authorized by the Ministry Representative, the Contractor shall, before drilling the backline holes, remove all overburden within the excavation limits or 10 m beyond the limits of the production holes in a direction parallel to the backline. The overburden surface shall be sloped at 1.5:1 unless otherwise authorized by the Ministry Representative.

204.40 Backline and Production Holes

204.40.01 Stemming – The upper portion of all holes between the topmost charge and the hole collar shall be stemmed. Stemming materials shall be sand or other inert angular granular material with similar specific gravity, passing a 9.5 mm sieve.

204.40.02 Hole Obstructions – Before placing charges, the Contractor shall determine that the hole is free of obstructions for its entire depth. All necessary precautions shall be exercised so that the placing of the charges will not cause spalling of material from the walls of the holes.

204.40.03 General Orientation – All holes shall be drilled downward unless otherwise specified in the blast design and authorized by the Ministry Representative. In general, slash holes (horizontal, near horizontal or fanned out holes) shall not be drilled along the excavation backslope or on

pioneering routes excavated to provide access for backline hole drilling, unless authorized by the Ministry Representative.

204.40.04 Insertion of Explosive – Explosive materials shall not be inserted into the holes until the blast design has been reviewed by the Ministry Representative.

204.41 Backline Holes

204.41.01 General – The Contractor shall control the hole layout and drilling operations using proper equipment and techniques to ensure that backline holes are located and oriented correctly. The proposed location of each backline hole shall be accurately surveyed and staked.

204.41.02 Hole Location – For each blast, the line of backline holes shall extend 10 m to 20 m beyond the limits of the production holes to be detonated or to the end of the specified excavation backslope, as applicable.

204.41.03 Hole Location Tolerance – Backline holes shall be drilled within 75 mm of the staked collar location.

204.41.04 Hole Deviation – Backline holes shall not deviate from the plane of the specified excavation backslope by more than 150 mm as measured perpendicular to the slope. Backline holes shall not deviate more than 150 mm as measured within the plane of the excavation backslope.

204.41.05 Hole Diameter – The backline holes shall be 50 mm to 75 mm in diameter, or as recommended by the blasting consultant and authorized by the Ministry Representative. Trim blast holes may be as small as 25 mm diameter.

204.41.06 Hole Length – The length of backline holes for any individual lift shall not exceed 8 m unless the Contractor can demonstrate to the Ministry Representative that the Contractor can stay within the tolerances and produce the required excavation backslope geometry.

204.41.07 Guide Holes – Guide Holes shall be of the same diameter and drilled in the same plane and to the same tolerance as the remaining backline holes.

204.41.08 Hole Length Increase – Upon satisfactory demonstration, the length of holes may be increased to a maximum of 12 m with written authorization of the Ministry Representative. If more than 5% of the backline holes are misaligned in any one lift, then the Contractor shall reduce the height of the lifts until the 150 mm alignment tolerance is met.

204.41.09 Control of Drilling Orientation – All drilling equipment used to drill the backline holes shall have mechanical devices attached to that equipment to accurately determine the orientation of the drill steel entering the rock. Backline hole drilling will not be permitted if these devices are either missing or inoperative.

204.41.10 Offset Between Lifts – When the cut height requires more than one lift, a maximum 0.5 m offset

between lifts is permitted to allow for drill equipment clearances. The Contractor shall begin the backline hole drilling on the design backline for the top lift, and shall adjust the start of lower lifts to compensate for drill offset and any drift which may have occurred in the upper lifts. The excavation angle required to compensate for drill offset can be determined as shown in Appendix 204-A, Figure 204-A-1.

204.41.11 Length for Toe Berm Removal – Drilling 0.5 m below ditch bottom is permitted to facilitate removal of the toe berm.

204.42 Presplit Blasting

204.42.01 General – Unless otherwise specified in the blast design and authorized by the Ministry Representative, presplit blasting shall be conducted for all blasting to the excavation backslope. In general, cushion blasting may proceed when the bench width is less than three times the lift height.

204.42.02 Explosive Type and Accessories – Drill hole conditions may vary from dry to filled with water. The Contractor shall use explosives and blasting accessories appropriate for the drill hole conditions encountered to accomplish the specified results. Only standard explosives manufactured for presplit blasting shall be used in backline holes, unless otherwise specified in the blast design and authorized by the Ministry Representative. Bulk ammonium nitrate and fuel oil (ANFO) shall not be loaded into the backline holes.

204.42.03 Explosive Decoupling – Explosives shall be evenly distributed and decoupled to the maximum extent possible.

204.42.04 Explosive Charges – The bottom charge of backline holes may be larger than the remaining charges but shall not be large enough to cause overbreak. The top charge of backline holes shall be placed far enough below the collar and be sufficiently small to avoid overbreak and heaving of rock beyond the excavation backslope.

204.42.05 Modified Blasting Sequence – The Contractor may detonate the backline holes before drilling production holes, provided satisfactory excavation backslopes are obtained.

204.42.06 Backline Hole Delays – If required to reduce ground vibrations or noise, backline holes may be delayed, provided the effective hole-to-hole delay time is not more than 25 ms.

204.42.07 Excavation Backslope Geometry – The excavation backslope shall not deviate more than 150 mm from a plane passing through adjacent drill holes except where the character of the rock is such that, as determined by the Ministry Representative, irregularities are unavoidable. The 150 mm tolerance shall be measured perpendicular to the plane of the slope.

Should any blast result in a non-compliant backwall, the blast design shall be modified, prior to any subsequent blasting, to prevent future non-conformance.

In no case shall any portion of the slope encroach on the ditch.

204.43 Cushion Blasting

204.43.01 General – Where the horizontal distance from the specified excavation backslope to the existing free face is less than 5 m, the Contractor may use cushion blasting instead of presplit blasting.

204.43.02 Delay Time – The difference in delay time between the backline holes detonated after the buffer holes shall be between 25 ms and 75 ms. With the exception of these criteria, requirements for presplit blasting shall also apply to cushion blasting.

204.44 Production Blasting

204.44.01 Hole Diameter – Production holes shall not exceed 150 mm in diameter, unless otherwise specified in the blast design and authorized by the Ministry Representative.

204.44.02 Hole Location – Production blast holes (not including buffer holes) shall not be drilled closer than 2.0 m to the backline holes, unless otherwise specified in the blast design and authorized by the Ministry Representative. The bottom of the production holes (including buffer holes) shall not be lower than the bottom of the backline holes.

204.44.03 Delay Sequence – All sequences shall be delayed in such a manner that successive delays promote the movement of rock in the direction of a free face at all times.

204.44.04 Damage to Backslope – It is the Contractor's responsibility to take all necessary precautions during production blasting to minimize blast damage to the excavation backslope.

204.44.05 Buffer Holes – A line of buffer holes shall be drilled along a plane parallel to the backline holes. Buffer hole diameters shall be between 50 mm and 75 mm unless otherwise specified in the blast design. Unless otherwise specified in the blast design, the line of buffer holes shall be drilled approximately 1 m away from the backline holes and spaced 1.0 to 1.5 m centre to centre.

204.44.06 Buffer Hole Charge and Detonation – The explosive charge in buffer holes shall not exceed 50% of the full explosive load that could be placed in a 75 mm diameter production hole. Detonation of the buffer holes shall be on a delay sequence toward a free face. Ammonium nitrate and fuel oil shall not be used for buffer holes unless otherwise specified in the blast design and authorized by the Ministry Representative.

204.45 Vibration Control and Monitoring

204.45.01 Vibration Control – The Contractor shall use blasting methods designed to limit the intensity of ground

vibrations originating within the excavation limits. When blasting near buildings, structures, wells, utilities or other works that may be subject to damage from blast induced ground vibrations, the ground vibrations shall be controlled using properly designed delay sequences and allowable charge weights per delay.

204.45.02 Charge Weight Per Delay – Allowable charge weights per delay shall be based on vibration levels that will not cause damage. The Ministry Representative may monitor vibration levels at the blast site by requesting trial blasts to determine actual vibration levels reached during blasting.

204.45.03 Monitoring – Monitoring shall meet the International Society of Rock Mechanics (ISRM) standards. Whenever vibration damage to adjacent structures is possible, the Contractor shall monitor each blast with approved seismograph(s) located between the blast area and the closest structure(s) subject to potential blast damage. The geophone shall be placed as close as possible to the structure(s) but not directly above the structure(s). The seismograph(s) shall be set to record particle velocity, accelerations, and frequency in the range generally found with controlled blasting. The peak particle velocity shall be calculated as the maximum vector sum of three mutually perpendicular components of vibration. All components and peak particle velocity shall be recorded.

204.45.03 Vibration Limits – Peak particle velocity, accelerations and frequency shall not be allowed to exceed the safe limits of the nearest structure subject to potential vibration damage. The Contractor shall employ a qualified vibration specialist to establish the safe vibration limits.

204.45.04 Interpretation – The vibration specialist shall interpret the seismograph records to ensure that the seismograph data is effectively used in the control of the blasting operations with respect to the existing structures.

204.46 Air Concussion and Noise Control

204.46.01 General – When requested by the Ministry Representative, an air concussion monitoring system shall be installed in a representative location between the blasting area and the nearest structure subject to potential blast damage or annoyance. The equipment used to make the air concussion measurements shall be the type specifically manufactured for that purpose.

204.46.02 Monitoring – The air concussion monitoring system shall be set to record air overpressure on the linear setting. When requested by the Ministry Representative, human annoyance (A weight setting) and human ear response (C weight setting) shall also be recorded.

204.46.03 Overpressure – Peak overpressure shall be controlled using appropriate blast hole patterns, detonation systems and stemming to prevent venting of blasts, and to minimize air concussion and noise levels produced by the blasting operations. The contractor shall use a qualified vibration specialist to establish safe overpressure limits.

204.47 Excavation Backslope Stabilization

204.47.01 Stabilization – The excavation backslope shall be stabilized as recommended by the Ministry's geotechnical engineer and to the Ministry Representative's satisfaction, during or upon completion of the excavation of each lift. Unless otherwise authorized by the Ministry Representative, drilling of the next lift shall not proceed until stabilization has been completed. Drilling of the next lift shall not proceed until all concerns about stability raised by the Ministry's geotechnical engineer and the Ministry Representative are addressed by the stabilization work. Stabilization shall be completed before any base material is placed upon the subgrade. Stabilization shall be at the Contractor's expense.

204.47.02 Stabilization Methods – Stabilization methods include scaling, trimming, the application of rock bolts, shotcrete, slope mesh, drains or other stabilization techniques recommended by the Ministry's geotechnical engineer and authorized by the Ministry Representative.

204.48 Special Use of Excavated Rock – Excavated rock of suitable quality required for walls, riprap, paving, or other special use shall be sorted and deposited in stockpiles if so requested by the Ministry Representative.

MEASUREMENT

204.81 Type A Excavation – Type A excavation will be measured by the CUBIC METRE.

204.82 Excavation Backslope (Smooth Wall) – The excavation backslope will be measured by the SQUARE METRE only where the height of the cut exceeds 2 m, and shall be measured over the entire height of such cuts. Measurement will apply to the condition of the slope prior to scaling. Measurement for payment, including measurement of smooth wall surface area, hole deviation and slope deviation, will be conducted by the Ministry.

PAYMENT

204.91 Type A Excavation – Payment for TYPE A EXCAVATION will be at the Contract Unit Price per cubic metre and will be authorized by the Ministry Representative after submittal of all documentation required in this Section. The Unit Price will be considered full compensation for the requirements of this Section.

Design slope lines including approved offsets, subgrade and ditch lines shall represent the pay lines of Type A excavation.

If so determined by the Ministry Representative, stabilization necessitated by improper blasting operations shall be at the Contractor's expense.

Trimming will be paid at the Contract Unit Price per cubic metre for TYPE A EXCAVATION.

SECTION 204

ROCK CUTS

All costs associated with blasting test sections and any revised methods necessary to produce acceptable results shall be incidental to Type A excavation.

Additional drilling, blasting and excavation required to construct subgrade to the required tolerances and excavating, loading, hauling, placing and compacting backfill material for blast craters and drainage trenches to subgrade level shall be incidental to Type A excavation.

All costs associated with vibration control, air concussion and noise control, and all monitoring shall be considered incidental to Type A excavation.

204.92 Excavation Backslope (Smooth Wall) – Payment for EXCAVATION BACKSLOPE will be at the Contract

Unit Price per square metre for only that part of the exposed cut face meeting the geometric tolerances for excavation backslope in this Section. Geometric tolerances to be met include hole location tolerance, hole deviation, offset between lifts and deviation of excavation backslope geometry.

Scaling of loose material disturbed by blasting including removal and disposal of overbreak shall be considered incidental to payment for excavation backslope.

Where there is no Excavation Backslope (Smooth wall) Item in Schedule 7, the Excavation Backslope (Smooth wall) Work shall be performed incidental to Type A Excavation.

PROPOSED BLAST DESIGN

File #: _____
PROJECT NO. _____

(TO BE SUBMITTED NOT LESS THAN 1 DAY BEFORE DRILLING)

GENERAL:

DATE PREPARED _____
CONTRACTOR'S NAME _____ PROPOSED BLAST DATE _____
BLASTER'S NAME _____ BLAST # _____
BLASTER'S CERTIFICATE NUMBER _____
HIGHWAY# _____
BLAST LOCATION AT: _____

UTILITY PROTECTION OR STANDBY REQUIRED: (CIRCLE) YES NO

SITE DETAILS:

HEIGHT ABOVE HWY _____ (m) SLOPE ANGLE _____ (°) SLOPE LENGTH _____ (m)
ROCK TYPE _____
DISTANCE TO NEAREST STRUCTURE (UTILITY) _____ (m)

ANTICIPATED DRILLING DETAILS:

PROPOSED NO. OF BACKLINE HOLES _____ PROPOSED APPROX. NO. OF HOLES _____
PROPOSED AVG. DEPTH _____ (m) HOLE DIA. _____ (mm)
PROPOSED NO. OF ROWS _____ APPROX. HOLE SPACING _____ (m) BURDEN _____ (m)
PROPOSED MAXIMUM DEPTH _____ (m) PROPOSED HOLE INCL.: (CIRCLE) VERTICAL HORIZ. VARIABLE
PROPOSED TOTAL DEPTH _____ (m)

PROPOSED BLAST DETAILS:

EXPLOSIVE TYPE _____ EXPLOSIVE SIZE _____ mm by _____ mm
APPROX. NUMBER OF CARTRIDGES _____ APPROX. TOTAL WEIGHT _____ (kg)
PROPOSED NUMBER OF DELAYS _____ DELAY TYPE AND LENGTH _____ (ms)

INITIATION DEVICE: (CIRCLE) E.B. CAPS SAFETY FUSE OTHER (SPECIFY) _____
BLASTING MACHINE: TYPE _____ CAPACITY _____

APPROX. VOLUME OF ROCK TO BE BLASTED _____ (m³)

PROPOSED BLAST SKETCH:

SHOW THE FOLLOWING:

- SCALE
- PLAN VIEW
- CROSS SECTION
- ROCK GEOMETRY
- APPROX. HOLE LOCATIONS
- APPROX. HOLE DEPTH
- PROPOSED TIE-IN PATTERN
- ROW BY ROW DELAYS
- DETONATION DIRECTION
- HIGHWAY LOCATION
- UTILITY LOCATION
- NORTH ARROW

PLAN VIEW:	CROSS SECTION VIEW:
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BLASTING CONSULTANT NAME _____ SIGNATURE _____ COMPANY _____

BLASTER'S SIGNATURE _____ COMPANY _____

RECEIVED BY MINISTRY REPRESENTATIVE (SIGNATURE) _____

(Use Additional Sheets if Necessary)

AS-BUILT BLASTING RECORD

File #: _____

PROJECT NO. _____

(TO BE SUBMITTED NOT MORE THAN 1 DAY AFTER EACH BLAST)

GENERAL:

DATE PREPARED _____

CONTRACTOR'S NAME _____ ACTUAL BLAST DATE _____

BLASTER'S NAME _____ BLAST # _____

BLASTER'S CERTIFICATE NUMBER _____

HIGHWAY# _____

BLAST LOCATION AT: _____

UTILITY PROTECTION OR STANDBY USED: (CIRCLE) YES NO TYPE OF UTILITY PROTECTION _____

SITE DETAILS:

HEIGHT ABOVE HWY _____ (m) SLOPE ANGLE _____ (°) SLOPE LENGTH _____ (m)

ROCK TYPE _____

DISTANCE TO NEAREST STRUCTURE (UTILITY) _____ (m)

DRILLING DETAILS:

NUMBER OF BACKLINE HOLES _____ TOTAL NUMBER OF HOLES _____

AVERAGE DEPTH _____ (m) HOLE DIAMETER _____ (mm)

NUMBER OF ROWS _____ HOLE SPACING _____ (m) BURDEN _____ (m)

MAXIMUM DEPTH _____ (m) HOLE INCLINATION: (CIRCLE) VERTICAL HORIZ. VARIABLE

TOTAL DEPTH _____ (m)

BLAST DETAILS:

EXPLOSIVE TYPE _____ EXPLOSIVE SIZE _____ mm by _____ mm

TOTAL NUMBER OF CARTRIDGES _____ TOTAL WEIGHT _____ (kg)

TOTAL NUMBER OF DELAYS _____ DELAY TYPE AND LENGTH _____ (ms)

INITIATION DEVICE: (CIRCLE) E.B. CAPS SAFETY FUSE OTHER (SPECIFY) _____

BLASTING MACHINE: TYPE _____ CAPACITY _____

VOLUME OF ROCK BLASTED _____ (m³)

BLAST SKETCH:

SHOW THE FOLLOWING:

- SCALE
- PLAN VIEW
- CROSS SECTION
- ROCK GEOMETRY
- HOLE LOCATIONS
- HOLE DEPTH
- TIE-IN PATTERN
- ROW BY ROW DELAYS
- DETONATION DIRECTION
- HIGHWAY LOCATION
- UTILITY LOCATION
- NORTH ARROW

PLAN VIEW:	CROSS SECTION VIEW:
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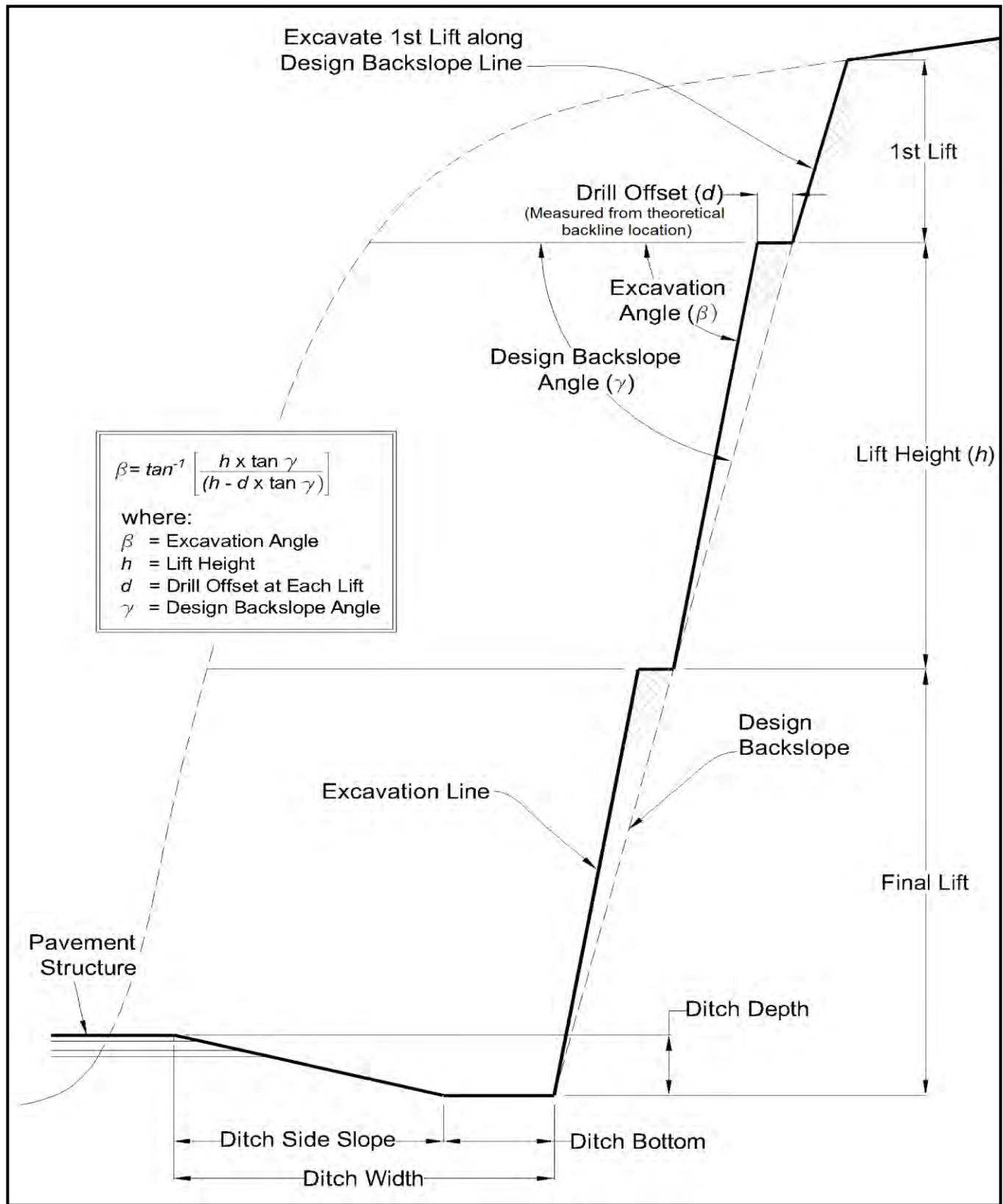
BLASTER'S SIGNATURE _____ COMPANY _____

RECEIVED BY MINISTRY REPRESENTATIVE (SIGNATURE) _____

(Use Additional Sheets if Necessary)

Appendix 204-A

Figure 204-A-1: Calculation of Excavation Angle (β) for Drill Offset



SECTION 205

RIPRAP

205.01 General – This Section covers the protection of embankments and channels, by using the Class of riprap, at the designated locations, specified in the Drawings and Special Provisions.

Work within any watercourse shall be carried out in conformity with the environmental protection provisions in the Contract and SS 165, all to the satisfaction of the Ministry Representative.

205.02 Quality Control

205.02.01 General – The Contractor shall be fully responsible for all quality control testing, inspection and documentation to achieve compliance with the Contract.

205.02.02 Material Testing

(a) **Initial Testing and Gradation** – The Contractor shall provide documented test results for the riprap material properties, per Table 205-A, and the gradation of the riprap to the Ministry Representative for review at least one week prior to starting riprap delivery to the Site.

(b) **Gradation Sampling Areas** – For riprap gradation, quality control shall continue to occur throughout the project and include, but not be limited to the following:

- (i) gradation of representative samples of riprap at the source while production is underway; and
- (ii) gradation of the placed riprap at the Site while placement is underway and at completion of the placement.

(c) **Gradation Sampling Frequency** – Gradation quality control shall occur at a frequency of not less than once per week, or once per 1000 tonnes or 500 m³ of riprap produced or placed, whichever is more frequent, at each source and placement site that is active at any time during the week.

205.02.03 Gradation Control – The Contractor may elect to perform gradation quality control using either mass or dimensional assessment in accordance with the following.

(a) **Light Riprap** – For riprap of Class 50 or less, quality control may be done visually, supplemented with measuring dimensions or masses as necessary to confirm the accuracy of the visual assessment.

(b) **Heavy Riprap by Mass** – The quality control for the gradation of the Class 100 and heavier riprap by mass shall:

- (i) Conform to the gradation specified in using Table 205-B ; and
- (ii) be done in accordance with the ASTM D5519 Method A or Method C.

Table 205-A: Tests for Riprap Material Properties

Property	ASTM Test Designation	Allowable Value
Specific Gravity	D6473	≥2.50
Absorption	D6473	≤2%
Soundness by use of Magnesium Sulphate	D5240	≤10% (following 5 cycles)
Micro-Deval Abrasion Loss Factor	D6928	≤20%

(c) **Heavy Riprap by Dimension** – The quality control for the gradation of Class 100 and larger riprap by size shall:

- (i) Conform to the gradation specified in Table 205-C with a tolerance of -5% to +15 % of the values listed; and
- (ii) be done using ASTM D5519 Method B, the [FHWA FLH T 521](#) Wolman count method, or other method approved in advance by the Ministry Representative.

(d) **Documentation** – Documentation of the gradation testing and inspection shall be provided to the Ministry Representative within 24 hours from its completion.

205.02.04 Hold Points – The following hold points, as defined in SS 145.12, will apply for Class 100 kg riprap and larger unless otherwise directed by the Ministry Representative:

- Gradation QC and material property test results for riprap are provided to the Ministry Representative prior to starting delivery to Site
- Completion of Toe/Terminal end key excavations
- Preparation of back slope/surface
- Application of filter(s)
- Completion of toe construction

205.03 Quality Assurance – The Ministry may carry out quality assurance by auditing the Contractor's quality control program and by testing and inspection at its discretion. Ministry quality testing shall not relieve the Contractor of responsibility for providing quality control.

205.04 Riprap Properties

205.04.01 General - Rock shall be hard, durable, and angular quarry rock of a quality that will not disintegrate on exposure to water or the atmosphere.

205.04.02 Size and Gradation – The size and gradation of rocks shall be in accordance with the following:

- well-graded;
- Table 205-B for mass or Table 205-C for dimensions; and
- the minimum dimension of each individual rock shall be greater than one-third of its maximum dimension;

205.04.03 Material Properties – Rocks used for riprap shall only break with difficulty, have no earthy odour, no closely spaced discontinuities, and should not absorb water easily. Rocks composed of appreciable amounts of clay or silt shall not be accepted for use as riprap.

Any riprap source shall be tested by the Contractor for conformance to the requirements of Table 205-A, prior to use on the Site.

Where the Ministry has provided a riprap source and has previously tested its material properties, the Contract will indicate that, and the Contractor will not be required to undertake further material property testing.

Where the Contractor elects to use another source, or the Ministry does not have test results for an available Ministry source, the Contractor will be responsible to test to, and ensure compliance with, the allowable values for the tests given in Table 205-A.

Representative samples may be broken off representative riprap rocks and crushed to allow performance of the tests.

Rocks shall be tested for Acid Rock Drainage and Metal Leaching as required by the Ministry Representative. Testing shall meet the Ministry requirements outlined in the Ministry's [Technical Circular T-04/13](#), available at the link below.

<https://www2.gov.bc.ca/assets/gov/driving-and-transportation/transportation-infrastructure/engineering-standards-and-guidelines/technical-circulars/2013/t04-13.pdf>

205.05 Preparation – Areas to receive riprap shall be trimmed to a uniform surface and to the slope(s) indicated on the Drawings or as directed by the Ministry Representative.

Before rock placement commences, loose material shall be removed and minor hollows filled with surrounding native materials well tamped-in to the approval of the Ministry Representative.

Table 205-B: Gradation of Rock by Class of Riprap

Class of Riprap (kg)	Rock Mass (kg)			
	Percentage Smaller Than Given Rock Mass			Max. Size
	15%	50%	85%	
10	1	10	30	50
25	2.5	25	75	125
50	5	50	150	250
100	10	100	300	500
250	25	250	750	1 250
500	50	500	1 500	2 500
1000	100	1 000	3 000	5 000
2000	200	2 000	6 000	10 000
4000	400	4 000	12 000	20 000

Table 205-C: Gradation and Intermediate Dimension of Rock by Class of Riprap

Class of Riprap (kg)	Intermediate Dimension (mm)			
	Percentage Smaller Than Intermediate Dimension			Max. Size
	15%	50%	85%	
10	90	200	285	350
25	125	270	385	450
50	155	340	485	600
100	200	425	610	750
250	270	575	830	1 000
500	340	725	1 050	1 250
1000	425	915	1 325	1 600
2000	535	1 150	1 650	2 000
4000	675	1 450	2 100	2 500

Note: Table 205-C shows the intermediate dimension as defined in the Wolman method as per [FHWA FLHT 521](#) corresponding to the rock mass shown in Table 205-B, based on spherical volume, using Specific Gravity = 2.50. Regardless of actual source Specific Gravity, the dimensions indicated remain applicable (subject to the limits specified in Table 205-A).

205.06 Filter Layers – Filter layers and placement, where required, shall be as per the Drawings and Special Provisions or as directed by the Ministry Representative.

205.07 Foundations and Placement – The nominal thickness and the surface width for each Class of riprap shall conform with the requirements specified in Table 205-D.

Each truckload of rock brought to the site shall provide a complete range of the rock sizes in the gradation.

Work shall be carried out to prevent cracking or breaking of rock riprap by crushing under machine tracks. Work shall be carried out to avoid disturbing the filter layer(s). Damage shall be repaired at the Contractor's expense.

Placement shall be as indicated on the Drawings or as directed by the Ministry Representative.

Layout and installation details for riprap at bridge embankments and for embankment protection works parallel to waterway flow shall be in accordance with SS Drawing SP205-1, unless otherwise specified. SS Drawing SP205-1 shall not apply for riprap for culvert end treatment or around in-stream piers.

At the toe of sloped riprap, a sufficient number of the larger rocks shall be placed to form a firm foundation. The remaining larger rocks shall be distributed evenly throughout the mass.

Rocks shall be placed to the required thickness, providing a reasonably well-graded mass with the minimum of voids. Clusters of small or large stones shall be avoided.

205.08 Machine Placed Riprap – The controlled placement of rock of the Class specified shall produce a well-graded rock mass of the nominal or required thickness over the area indicated. Placement of riprap shall not be by end-dumping. The rock shall be machine manipulated as necessary to provide mass stability and a regular surface with a minimum of voids.

205.09 Hand-Laid Riprap – Hand-laid riprap, normally Class 10 or 25, shall conform to the size, gradation and requirements set out in SS 205.04. Individual rocks too large to handle shall be machine manipulated for satisfactory setting and spacing.

205.10 Grouted Riprap – Where grouted riprap is shown or required, the surfaces of the rocks shall be cleaned and wetted and the interstices filled with cement mortar, well rodded and pounded in for a minimum mortar depth of 300 mm or as otherwise detailed or required by the Ministry Representative. The mortar shall consist of one-part Portland cement to three parts well-graded clean fine aggregate (1:3) mixed to a proper consistency.

Table 205-D: Placement Dimensions by Class of Riprap

Class of Riprap (kg)	Nominal Thickness of Riprap* (mm)	Surface Width, W* (mm)	
		2H:1V Slope	1.5H:1V Slope
10	350	783	631
25	450	1006	811
50	550	1230	992
100	700	1566	1262
250	1000	2236	1803
500	1200	2684	2163
1000	1500	3355	2704
2000	2000	4473	3606
4000	2500	5591	4507

* See SS Drawing SP205-1 for the description of the Nominal Thickness and Surface Width dimension "W".

MEASUREMENT

205.11 Measurement – Measurements will be made by multiplying the facial area by the average thickness dimensions as shown on the Drawings or as directed by the Ministry Representative. No allowance will be made for the quantity of rock placed in excess of these dimensions.

PAYMENT

205.12 Payment – Payment shall be made at the Unit Price bid per cubic metre for the Class of riprap specified or required. The Unit Price bid shall be accepted as full compensation for everything completely furnished and done in connection therewith, including supply, haul, placing, and quality control.

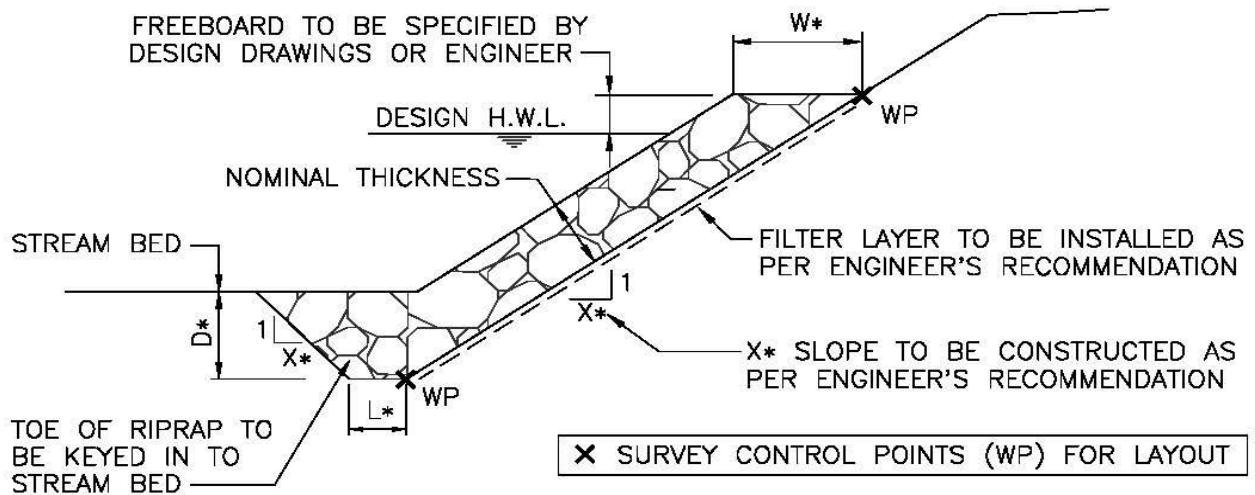
Payment for the excavation work at the placement site shall be paid for under "Roadway Drainage and Excavation" or "Foundation Excavation", whichever is specified in the Contract.

Where the source of supply for the riprap is off-Site or outside the design excavation limits as shown on the Drawings, the Unit Price for Riprap shall also include all costs to develop the source and produce the riprap.

Where the source of supply is on-Site and within the design excavation limits as shown on the Drawings, or within any slip, payment to excavate and haul the rock for riprap will be included within the Contract Unit Price for Type A, as defined in SS 201.

TYPICAL RIPRAP INSTALLATION AT BRIDGE EMBANKMENTS AND EMBANKMENT PROTECTION PARALLEL TO WATER FLOW

SP205-1



L* & D* TO BE SPECIFIED BY ENGINEER
 W* TO BE AS PER TABLE 205-D
 OR AS PER DESIGN DRAWINGS

NOTE: DETAIL NOT APPLICABLE TO CULVERT END PROTECTION

NOT TO SCALE
 ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE NOTED



BRITISH COLUMBIA

Ministry of Transportation & Infrastructure

SECTION 206

ROCK BOLTS

DESCRIPTION

206.01 General – Unless otherwise specified, the [Post Tensioning Institute \(PTI\) D35.1-14 Recommendations for Prestressed Rock and Soil Anchors](#) shall apply to rock bolts and other rock anchoring systems. Rock bolting operations shall include, but not be limited to, pre-construction and construction work including quality control, site preparation, selection of appropriate installation method, drilling, sealing, bar insertion, grouting and testing of rock bolts.

Rock bolts shall be installed in the areas designated by the Ministry Representative. Unless otherwise stated below, all rock bolts shall be installed and tensioned to the rock bolt manufacturer's specifications.

206.02 Codes and Standards

CSA G30.18	Carbon Steel Bars for Concrete Reinforcement
CSA G40.21	Structural Quality Steels
CSA A23.2-9C	Compressive Strength of Cylindrical Concrete Specimens.
ASTM A123	Standard Specification for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products
ASTM A153	Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware

206.03 Quality Control – The Contractor shall provide quality control for rock bolt operations.

206.04 Submittals – The Contractor shall supply the following to the Ministry Representative:

206.04.01 Proposed Rock Bolt System – Provide documentation as follows for the proposed rock bolt system for review at least 5 working days before commencing rock bolt installation.

- (a) **Rock Bolts** – Type, dimensions and manufacturer of the bar, centralizers, face plate, washer (where applicable), and nut.
- (b) **Resin** – Published specifications showing name of material, manufacturer, graph of unconfined compressive strength development versus time, gel time, viscosity, shelf life, storage and handling requirements.
- (c) **Grout** – Published specifications showing name of material, manufacturer, 7 day and 28 day unconfined compressive strength, composition, shelf life, storage and handling requirements.

(d) **Data Sheets** – Upon request by the Ministry Representative, supply the manufacturer's product data sheets.

(e) **Properties of Steel** – Upon request by the Ministry Representative, supply the physical and chemical properties for each lot or heat number of the steel.

(f) **Test Results** – Upon request by the Ministry Representative, submit a certified statement from an independent testing laboratory of the physical dimensions and mechanical properties of the rock bolt bar proposed for use. This statement shall include:

- (i) Minimum cross sectional area of the threaded portion of the bar.
- (ii) Minimum cross sectional area of the unthreaded portion of the bar.
- (iii) Yield strength of the bar.
- (iv) Ultimate tensile strength of the bar.
- (v) Percent elongation of the bar at yield and ultimate strength.

(g) **Quick Setting Mortar** – Name of material, manufacturer, 7 day and 28 day unconfined compressive strength, and composition.

(h) **Sealing** – Proposed method of sealing fractures and voids in drill holes to prevent loss of grout.

206.04.02 Calibration Certificates – Provide tensioning jack calibration certificates 5 working days before commencing rock bolt installation. Calibration of jacks shall have been performed by an authorized testing agency not more than 30 days prior to rock bolt testing. The certificate shall show the relationship between gauge pressure and applied load. Pumps and jacks shall be paired for calibration.

206.04.03 Contractor's Rock Bolt Records – Provide daily rock bolt tensioning records within one day of each day of rock bolt operations. Records shall include contractor name, date of tensioning, weather, temperature, test jack identification number, pump identification number, name of person who tested the rock bolt, rock bolt location, rock bolt number, rock bolt length, resin or grout details, test start time, test end time, gauge reading for each minute of the creep test, and lock off load. For resin systems, records shall also include the start and end times of spinning the bar through the resin in the hole.

206.05 Definitions – The following definitions refer to words and terms used in this Section. For definitions not covered here, refer to definitions provided by the Post Tensioning Institute.

206.05.01 Bonding Agent – Material such as grout or resin used to bond the rock bolt bar to the rock in the bond zone or free stressing length.

206.05.02 Bond Zone – The distal or lower portion of the hole where the bonding agent transmits the applied tensile load to the surrounding rock.

206.05.03 Free Stressing Length – The proximal or upper portion of the hole not within the bond zone.

206.05.04 Grout Sock – A geotextile encasement within all or part of the drill hole, used to control grout loss in highly permeable rock conditions.

206.05.05 Sealing Grout – Portland cement based grout that is pumped or gravity fed into the hole or used as drilling fluid to reduce the permeability of the rock immediately surrounding the hole.

MATERIALS

206.11 Materials – Steel materials shall be hot dip galvanized to ASTM A123 or ASTM A153, as appropriate. All resin, grout and galvanized steel materials shall be the products of established manufacturers regularly engaged in the manufacture of rock bolt materials for at least five years.

Materials shall meet the following additional requirements:

(a) Rock Bolt Bars

- (i) Steel hot-rolled Grade 517 MPa meeting CSA G30.18.
- (ii) Nominal solid bar diameter 22 mm unless otherwise specified.
- (iii) Threadlike surface deformations for full length of bar and suitable for mechanical coupling.
- (iv) Cut-thread reinforcing bar not permitted.

(b) Miscellaneous Hardware

- (i) Steel hardware to be compatible in size and strength with rock bolt bars.
- (ii) Face plates to be square and suitable for dome shaped nuts.
- (iii) Face plate size to be matched to bar size.
- (iv) Face plates to meet CSA G40.21 Grade 300W.
- (v) Face plate dimensions 10 mm by 150 mm by 150 mm unless otherwise specified.
- (vi) Face plates date stamped after galvanizing on the side visible when installed with the current year (in the format YYYY) in numbers 10 mm high.
- (vii) Face plates slotted for grout tube if grout is used.

(c) Resin

- (i) Fast-set and slow-set resin in cartridge form.

- (ii) A minimum unconfined compressive strength when fully mixed and cured of 90 MPa, tested in accordance with CSA A23.2.

- (iii) Encased in a plastic film that provides optimum resistance to moisture, and is easily ruptured to enable complete mixing during installation.

- (iv) Suitable thixotropic and viscous properties to permit adequate mixing of the resin components by rotation of the rock bolt bar and to contain the resin within the drill hole.

- (v) Easily identifiable gel time and as recommended by the resin manufacturer.

- (vi) Reach 80% of its ultimate strength within a time interval equal to five times the gel time.

- (vii) Non-shrink after the gel time.

- (viii) Unaffected by mild acids or mild alkalis.

- (ix) Cartridge boxes labelled with the resin expiry date.

(d) Grout

- (i) Pre-mixed, unsanded, non-metallic, and non-shrink cementitious grout containing silica fume.

- (ii) Can be mixed to a flowable consistency, typically with a water to cement ratio of 0.35 or as specified by the grout manufacturer to achieve or exceed the required strength.

- (iii) Minimum 7 day compressive strength of 30 MPa and a minimum 28 day compressive strength of 40 MPa, tested in accordance with CSA A23.2.

- (iv) Admixtures to be used according to the manufacturer's specifications and as authorized by the Ministry Representative.

- (v) Calcium chloride accelerator is not permitted.

(e) Mortar Pads

- (i) Portland cement based.
- (ii) Quick setting.

CONSTRUCTION

206.31 Execution

206.31.01 General – The entire rock bolt system shall be stored under cover away from deleterious materials. All grease and other deleterious material shall be removed from the steel prior to rock bolt installation.

206.31.02 Site Preparation – Where rock bolts may be adversely impacted, rock removal above and around proposed rock bolt locations shall be completed before installation of rock bolts commence. Any minor rock scaling performed in conjunction with rock bolting shall be considered incidental to rock bolting.

206.31.03 Drill Holes

- (a) **Location, Orientation and Depth** – The location, direction, angle and depth of the holes will be dependent on field conditions encountered and will be detailed by the Ministry Representative prior to and if necessary during construction.
- (b) **Hole Diameter** – The diameter of the holes shall be suitable for the rock bolt system chosen. Where grout is used, the hole size shall be according to the rock bolt manufacturer's recommendations. Where resin is used, the hole size shall be according to the resin manufacturer's recommendations.

206.31.04 Cleaning – All water, grease, oil, cuttings and other deleterious materials shall be removed from finished holes by a water and/or air jet as required.

206.31.05 Sealing – The Contractor shall take measures incidental to rock bolting to maintain open drill holes, seal holes and prevent anchor grout loss as follows:

- (a) Fill fractures and small voids;
 (b) Bridge large voids;
 (c) Consolidate around the hole.

including but not limited to using the following measures:

- (d) Sealing grouting;
 (e) Lining the hole with steel or pvc casing;
 (f) Grouting and re-drilling;
 (g) Using grout sock to contain anchor grout

Alternative sealing methods shall be as authorized by the Ministry Representative.

206.31.06 Installation

- (a) **Installation Methods** – Rock bolt bars shall be surrounded and secured in the hole using one of two possible methods (See SS Drawing SP206-01):
- (i) Grout in bond zone, grout in free stressing length.
 (ii) Resin in bond zone, grout in free stressing length.
- A third method may be used as follows, if authorized by the Ministry Representative, due to field conditions:
- (iii) Resin in bond zone, resin in free stressing length.
- (b) **General** – Rock bolts shall be inserted (or rotated) into the drill holes and fully encapsulated in resin or grout to the drill hole collar. When resin is used, the bolt shall be advanced and rotated at a rate recommended by the resin manufacturer.
- (c) **Anchorage Length** – The anchorage length or bond zone shall be the last 1000 mm of the inserted end of the bar, unless otherwise specified.

- (d) **Centralizers** – If grout is used, centralizers on 3.0 m centres shall centralize the rock bolt in the drill hole before grout is placed. Centralizers shall be suitable for holes in rock and shall be specifically manufactured for rock bolt application.
- (e) **Resin** – Resin cartridges shall be installed as follows or as specified by the resin manufacturer:
- (i) **Fast Set Resin** – A sufficient number of fast-setting cartridges shall be placed in the bond zone at the bottom of the hole for the anchorage.
- (ii) **Slow Set Resin** – A sufficient number of slow-setting cartridges shall be placed in the free stressing length between the bond zone and the collar of the hole.
- (f) **Grout** – Grout shall be prepared and placed as follows unless otherwise specified by the grout manufacturer:
- (i) **Mixing** – Grout shall be mixed in a colloidal or high shear grout mixer according to the grout manufacturer's published instructions. Mixing paddles shall be slotted and perforated. Mixing time shall be not less than two minutes.
- (ii) **Batching** – All ingredients for the grout mix shall be batched by mass. Water shall be added to the drum first and dry ingredients afterwards. Grout shall not be re-tempered after initial mixing. Grout shall be placed immediately after mixing.
- (iii) **Grout Placement** – Grout shall be pumped or gravity fed through a grout tube extending to the bottom of the hole. The inserted end of the tube shall remain below the level of the grout in the hole to affect a continuous air free column as the grout level rises. Grout shall be placed quickly and continuously to avoid overworking, segregation, bleeding and disturbance of initial set. Grout that has stiffened due to delay in placing shall not be used in the work and shall be disposed of at an authorized location.
- (g) **Rock Face Preparation** – The bearing surface shall be prepared to allow the face plate to be oriented within the limits recommended by the anchor manufacturer. If necessary, rock shall be chipped from around the face plate contact area.
- (h) **Mortar Pad Construction** – Mortar pads shall be constructed as required to ensure the bar alignment is within twenty degrees (20°) of a line perpendicular to the face plate. Mortar pads shall be installed where face plates deform excessively under load. The pad shall not crack or deform when loaded. Sufficient time shall be provided to allow pads to achieve sufficient bearing capacity prior to test-tensioning.

(i) End Hardware Installation

- (i) Nuts shall bear uniformly against the face plate.
- (ii) The bolt extension beyond the nut shall be 100 mm \pm 10 mm.

206.31.07 Tensioning – All rock bolts shall be test-tensioned and locked off following set-up (or curing) of the bond zone and before set-up of the free stressing length. Prior to testing, the bond zone grout and/or resin shall meet the strength specified by the anchor manufacturer. The following procedure applies to 22 mm diameter Grade 517 MPa steel bars. An alternative procedure may be specified for different bar sizes.

- (a) **Equipment** – Equipment required for tensioning shall be supplied by the Contractor and shall be of a size adequate to provide the required tension. A torque wrench shall not be used for tensioning.
- (b) **Test-Tensioning and Creep Test** – Rock bolts shall be test-tensioned to 186 kN (42,000 lb). The test load shall be held for 10 minutes for the creep test.
- (c) **Lock-Off Tension** – Rock bolts shall be locked-off to a design tension of 140 kN (31,500 lb) after testing.
- (d) **Acceptance Criteria** – During the creep test a load loss of greater than 10% of the load applied shall be indicative of anchorage failure. Creep movement at the anchor head shall not exceed 2 mm during the creep test. A replacement rock bolt shall be installed at the Contractor's expense where these criteria are not met.

206.31.08 Rock Bolt Evaluation – The Ministry Representative will implement a program of evaluation of rock bolts installed. The Contractor shall perform the following test procedure on randomly selected bolts. After locking off the anchor, the load shall be re-applied to determine the lift-off load. The lift-off load shall be the tension level at which the anchor nut can be loosened by hand. Lift-off tests shall be performed on rock bolts chosen by the Ministry Representative to a minimum of 5% of the total number of rock bolts. One additional lift-off test on a different bolt shall be performed for each bolt whose lift-off load is not within 10% of the specified lock-off load. Following lift-off testing, all bolts shall be locked off as specified.

MEASUREMENT

206.81 Rock Bolts – Rock bolts will be measured by the METRE installed. The measurement length shall be the length of bar in the rock.

PAYMENT

206.91 Rock Bolts – Payment for ROCK BOLTS will be at the Contract Unit Price per metre. Payment for rock bolts will be authorized after installation to the contract specifications and after submittal of the completed Contractor's Daily Rock Bolt Testing and Tensioning Record. The Unit Price will be considered full compensation for all work and materials supplied according to the requirements of this Section.

ROCK BOLTS	SP206-01
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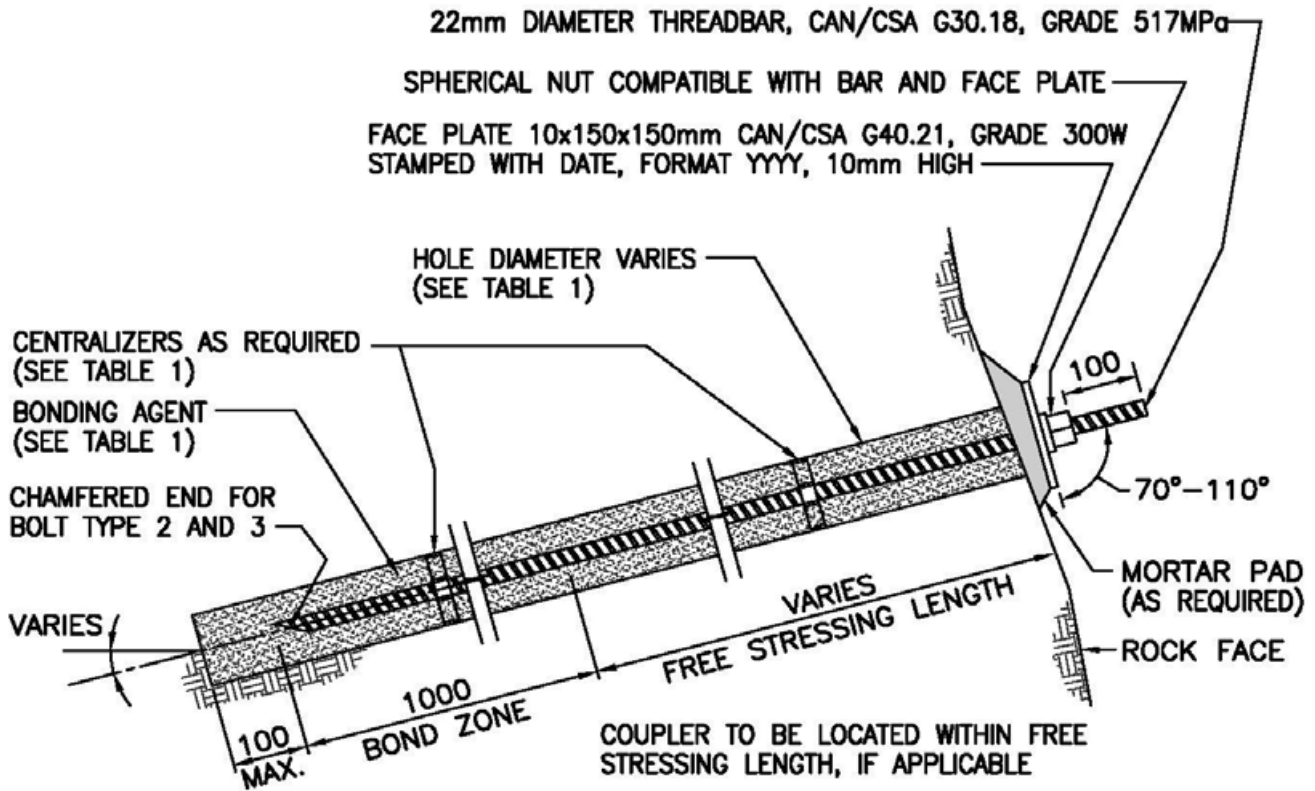


TABLE 1 BOLT TYPE, BONDING AGENT, HOLE DIAMETER AND CENTRALIZERS

BOLT TYPE *	BONDING AGENT		HOLE DIAMETER mm **		CENTRALIZERS
	BOND ZONE	FREE STRESSING LENGTH	BOND ZONE	FREE STRESSING LENGTH	
1	GROUT	GROUT	42 MIN.	42 MIN.	YES
2	RESIN	GROUT	38	42	NO
3	RESIN	RESIN	38	41	NO

* BOLT TYPE 3 MAY BE USED DUE TO FIELD CONDITIONS IF TYPE 1 AND 2 ARE NOT POSSIBLE AND ONLY IF AUTHORIZED BY THE MINISTRY REPRESENTATIVE. COUPLER IS NOT PERMITTED FOR BOLT TYPE 3.

** HOLE DIAMETER MAY VARY DEPENDING ON MATERIAL SUPPLIER. INCREASE HOLE DIAMETER IN FREE STRESSING LENGTH TO 56mm IF BAR COUPLER IS USED.

NOTES:

1. GROUT TUBE OMITTED FOR CLARITY.
2. TEST TENSION TO 186kN, HOLD FOR 10 MINUTES.
3. LOCK-OFF LOAD IS 140kN.
4. ALL STEEL TO BE HOT DIP GALVANIZED TO ASTM A123 OR A153, AS APPROPRIATE.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



Ministry of
Transportation
& Infrastructure

SECTION 207

SLOPE MESH FOR ROCK CUTS

DESCRIPTION

207.01 Scope – This Section applies to slope mesh structures which are installed to provide rockfall protection.

207.01.01 Scaling – Rock slope scaling, tree removal and overburden excavation performed in conjunction with slope mesh installation shall be considered incidental to slope meshing. Rock slope scaling, tree removal and overburden excavation shall be completed before the mesh support system is installed unless otherwise authorized by the Ministry Representative. Trees to be removed will be detailed by the Ministry Representative.

207.01.02 Materials Disposal – Discarded mesh components, scaled rock, trees and debris generated by the slope meshing work shall be removed from the Site and disposed of by the Contractor in accordance with SS 145.27.02. Temporary storage of felled trees in ditches will not be permitted.

207.01.03 General Layout – The Contractor shall layout in the field all anchor and suspension cable locations for each section for review by the Ministry Representative prior to installation.

207.01.04 Layout Variations – Field conditions may require final anchor, cable and mesh configurations to vary from the Ministry's Drawings. All variations shall be authorized by the Ministry Representative.

207.01.05 Mesh Sections – The area to be provided with slope mesh protection shall be divided into sections with a maximum width of 24 m. Each section shall have a separate suspension cable, end main anchors and end anchors. End main anchors of adjacent sections shall be positioned 200 mm apart. The gap between sections shall be closed with mesh.

207.01.06 Mesh Height – The maximum mesh height shall be 80 m.

207.01.07 Impact Energy – The maximum rock impact energies shall be as follows:

- 3 kJ above cut crest
- 30 kJ below cut crest

MATERIALS

207.11 Slope Meshing Materials – Unless otherwise specified, preparation and installation of materials shall be according to manufacturer's recommendations.

All components shall be hot dip galvanized by the manufacturer conforming to ASTM A123 or ASTM A153,

as appropriate. Damaged galvanizing shall be re-galvanized at the Contractor's expense.

207.11.01 Mesh – Mesh shall be 11 gauge (2.95 mm dia.) hexagonal double-twist (i.e. three one-half turns) gabion type mesh. Mesh wire shall meet ASTM A975 style 1 consisting of carbon steel with a class 3 zinc coating of not less than 244 g/m² meeting ASTM A641. The weight of zinc coating shall be determined by ASTM A90. The coating shall withstand four one-minute dips by the Preece test, ASTM A239. Mesh opening shall be hexagonal in shape and uniform in size measuring 80 mm by 100 mm.

207.11.02 Threadbar – Threadbar shall be steel hot-rolled Grade 517 MPa, conforming to CSA G30.18.

207.11.03 Eye nuts – Eye nuts shall be cast or manufactured eye nuts supplied by the threadbar manufacturer.

207.11.04 Cables – All cables shall be fibre core conforming to CSA G4. Cables shall be unspliced.

207.11.05 Thimbles – Thimbles shall meet US Federal Specification FF-T-276 Type III.

207.11.06 Clips – Clips shall meet US Federal Specification FF-C-450 Type 1, Class 1.

207.12 Grout

207.12.01 Anchors – Main anchors shall be secured in the drill hole using grout. Auxiliary anchors shall be secured in the drill hole using grout or resin.

207.12.02 Grout – Grout shall have the following properties:

- be pre-mixed, unsanded, non-metallic, and non-shrink cementitious grout containing silica fume.
- may be mixed to a flowable consistency, typically with a water to cement ratio of 0.35 or as specified by the grout manufacturer to achieve or exceed the required strength.
- minimum 7 day compressive strength of 30 MPa and a minimum 28 day compressive strength of 40 MPa, tested in accordance with CSA A23.2-09.
- admixtures to be used according to the manufacturer's specifications and as authorized by the Ministry Representative.

calcium chloride-based accelerators are not permitted.

207.12.03 Resin – Resin shall have the following properties:

- be supplied in cartridge form.

- (b) a minimum unconfined compressive strength of 90 MPa when fully mixed and cured and tested in accordance with CSA A23.2-09.
- (c) be encased in a plastic film that provides optimum resistance to moisture and is easily ruptured to enable complete mixing during installation.
- (d) suitable thixotropic and viscous properties to permit adequate mixing of the resin components by rotation of the rock bolt bar and to contain the resin within the drill hole.
- (e) an easily identifiable gel time recommended by the resin manufacturer.
- (f) reach 80% of its ultimate strength within a time interval equal to five times the gel time.
- (g) be non-shrink after the gel time.
- (h) be unaffected by mild acids or mild alkalis.
- (i) be in cartridge boxes labelled with the resin expiry date.

207.13 Conformance Documents – Prior to installation the contractor shall supply documents of conformance to project specifications of all materials upon request.

CONSTRUCTION

207.31 Anchor Installation

207.31.01 Overburden – Overburden shall be excavated to rock at anchor locations unless specified otherwise. The Contractor shall minimize disturbance of surrounding soil and rock when excavating. Overburden shall be removed as necessary so that cables do not contact ground surface.

207.31.02 Ground Clearance – The minimum ground clearance for the suspension cable shall be 0.6 m between anchors, or up to 1.5 m if so specified. A maximum of one connector may be used above the ground to extend any main anchor as necessary to achieve the required clearance. Suspension cable clearance shall be maximized where possible by appropriate selection of anchor locations.

207.31.03 All Anchors – Anchor holes shall be a minimum of 1.5 times anchor diameter and in strong, competent rock. Anchors shall be centered in the hole and grouted. Anchors shall not be loaded within 3 days of grouting.

207.31.04 Main Anchors – Main anchors shall be located at local high points where practicable to maximize clearance between suspension cable and ground surface. Main anchors shall be vertical and centered in the hole unless otherwise authorized by the Ministry Representative. Main anchor height above ground surface may be reduced where authorized by the Ministry Representative for field conditions.

207.31.05 Auxiliary Anchors – Auxiliary anchors shall be located to minimize potential for main anchor bending.

207.31.06 Guy Anchors and Cables – Main anchors may be substituted, where authorized by the Ministry Representative, with a limited number of guy cables directly connected to the suspension cable where no suitable main anchor locations can be found. Guy cable anchors shall be located to maximize suspension cable elevation. See Section Y-Y, on SS_Drawing SP207-02 and Detail B on SS_Drawing SP207-03.

207.31.07 Embedded Length – Auxiliary or guy anchor embedded length may require extension if weak rock conditions are encountered.

207.32 Soil Anchors – Soil anchors are incidental to slope meshing and may be required in place of end main, main, guy and/or auxiliary anchors where:

- Bedrock is not at ground surface.
- Overburden thickness is greater than 0.6 m.
- Required at any other locations or as ordered by the Ministry Representative.

For soil anchor details see SS_Drawings SP207-04 and SP207-05.

Concrete requirements for soil anchors:

- Minimum compressive strength at 28 days = 30 MPa
- Maximum nominal size of aggregate = 28 mm
- Air content = $5 \pm 1\%$
- Slump = 55 ± 20 mm
- Maximum w/c ratio by mass = 0.45

Upon request by the Ministry Representative, the Contractor shall load test 10% of the overburden soil anchors to 10 kN to verify capacity or as authorized by the Ministry Representative.

207.33 Cable and Mesh Installation – Suspension, auxiliary, end, and guy cables shall be installed to nominal tension to remove slack before and after installing mesh. Terminate lacing cable ends with a loop through the end main anchor eye nut, secured to the suspension cable or the lacing cable itself using two clips, 19 mm or 6 mm respectively.

A maximum of two horizontal mesh seams (200 mm overlap) shall be permitted along the entire mesh height. The upper mesh portion shall be between the slope and lower mesh at the overlap. The horizontal seam connections shall be similar to the vertical seams.

Unless otherwise specified on the Contract Drawings or by the Ministry Representative, the mesh shall terminate 1500 mm above the highway edge of pavement elevation.

SECTION 207

SLOPE MESH FOR ROCK CUTS

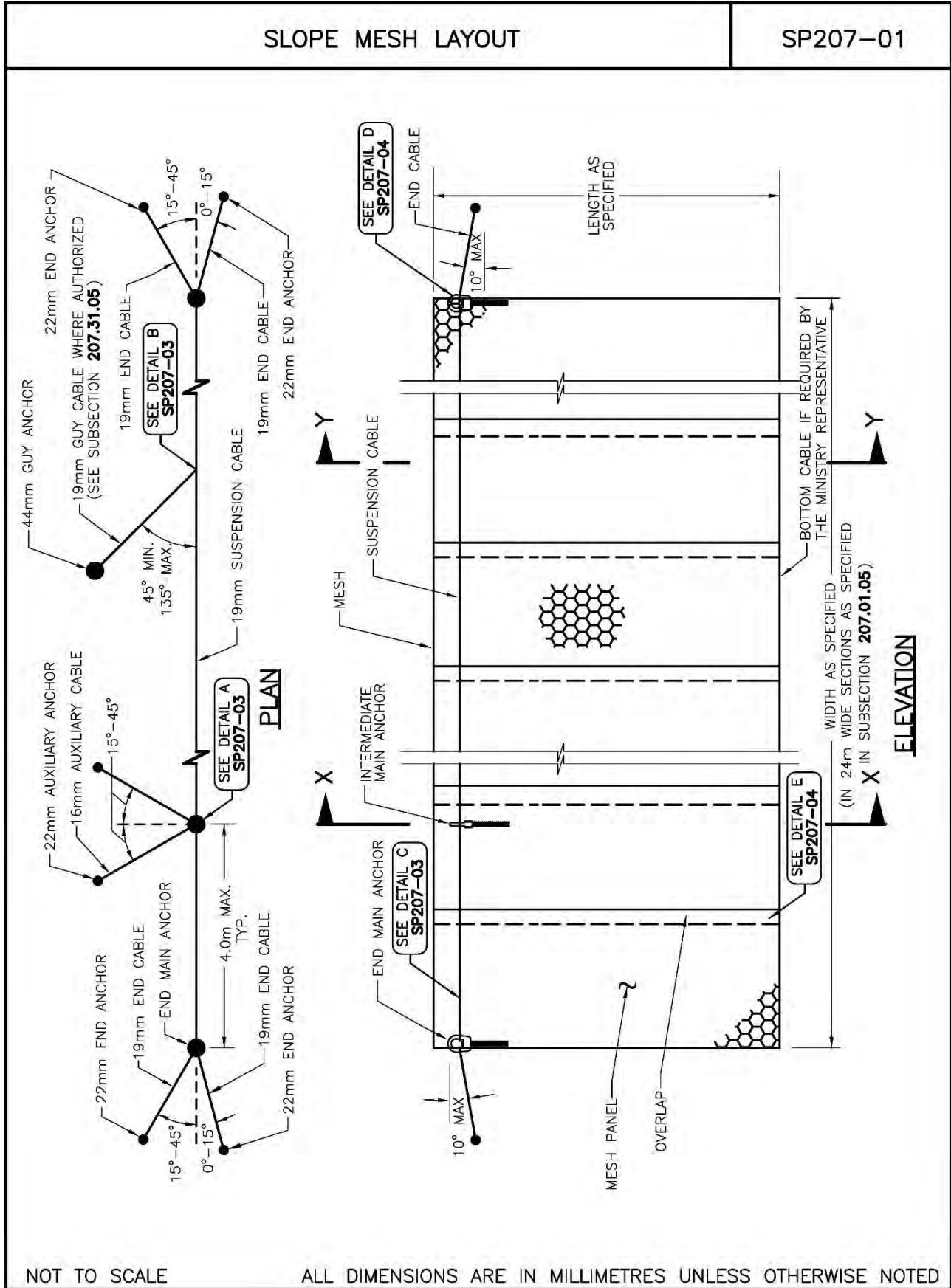
The bottom of the mesh shall be evenly trimmed parallel with the highway elevation. The bottom of the mesh shall be bent to remove the curl.

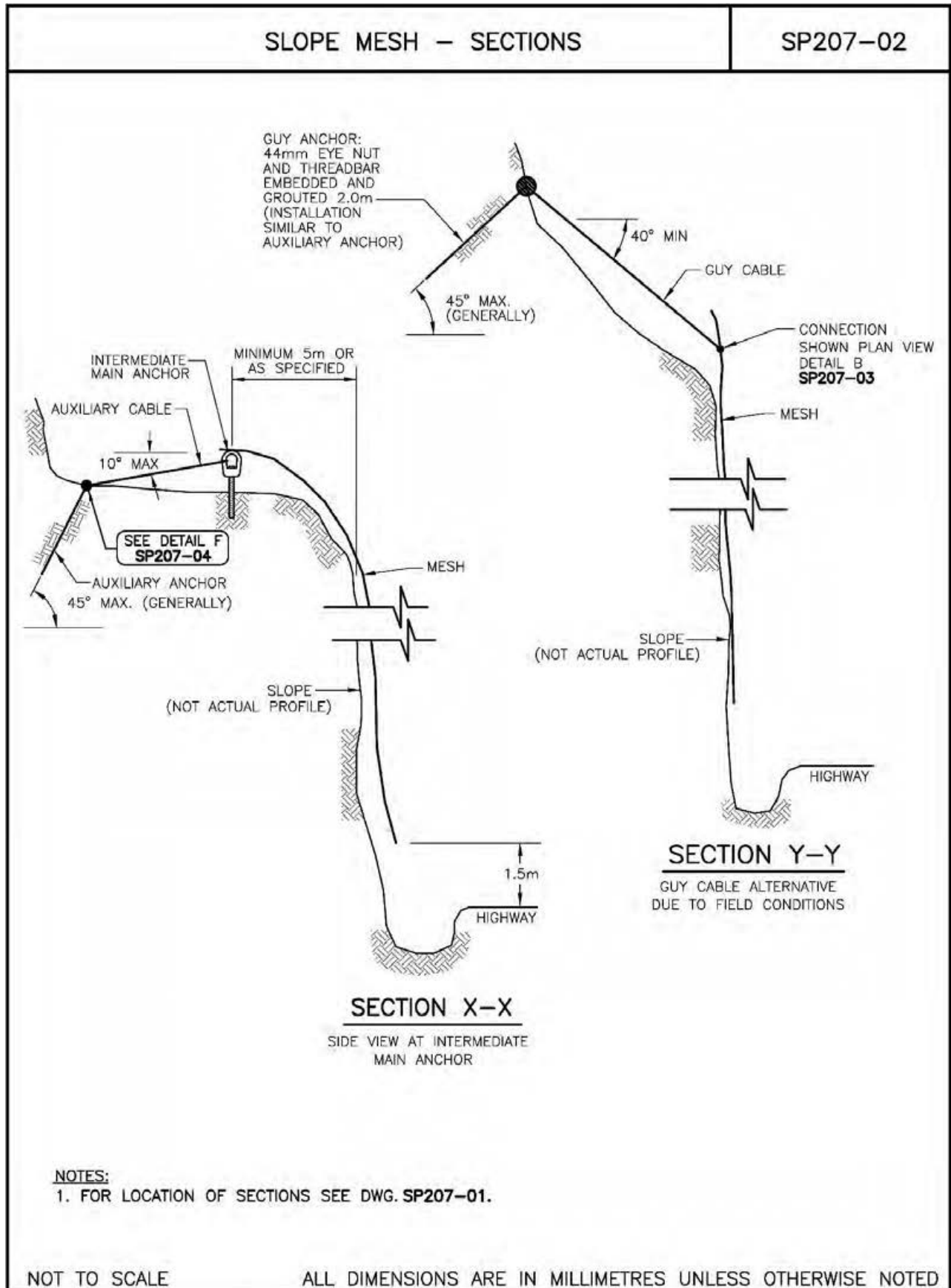
MEASUREMENT

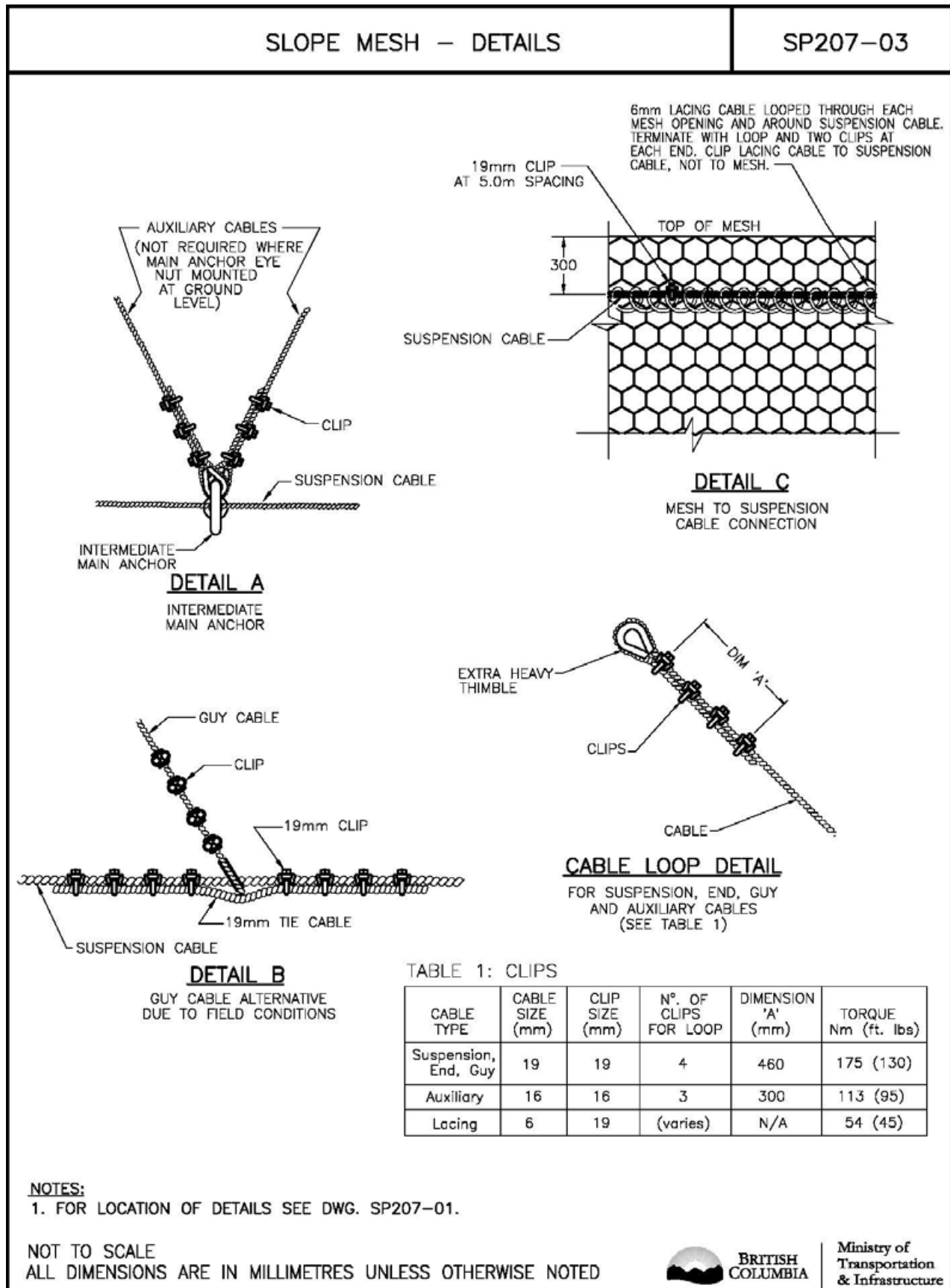
207.81 Slope Mesh – Slope mesh shall be measured by the SQUARE METRE of slope meshed area.

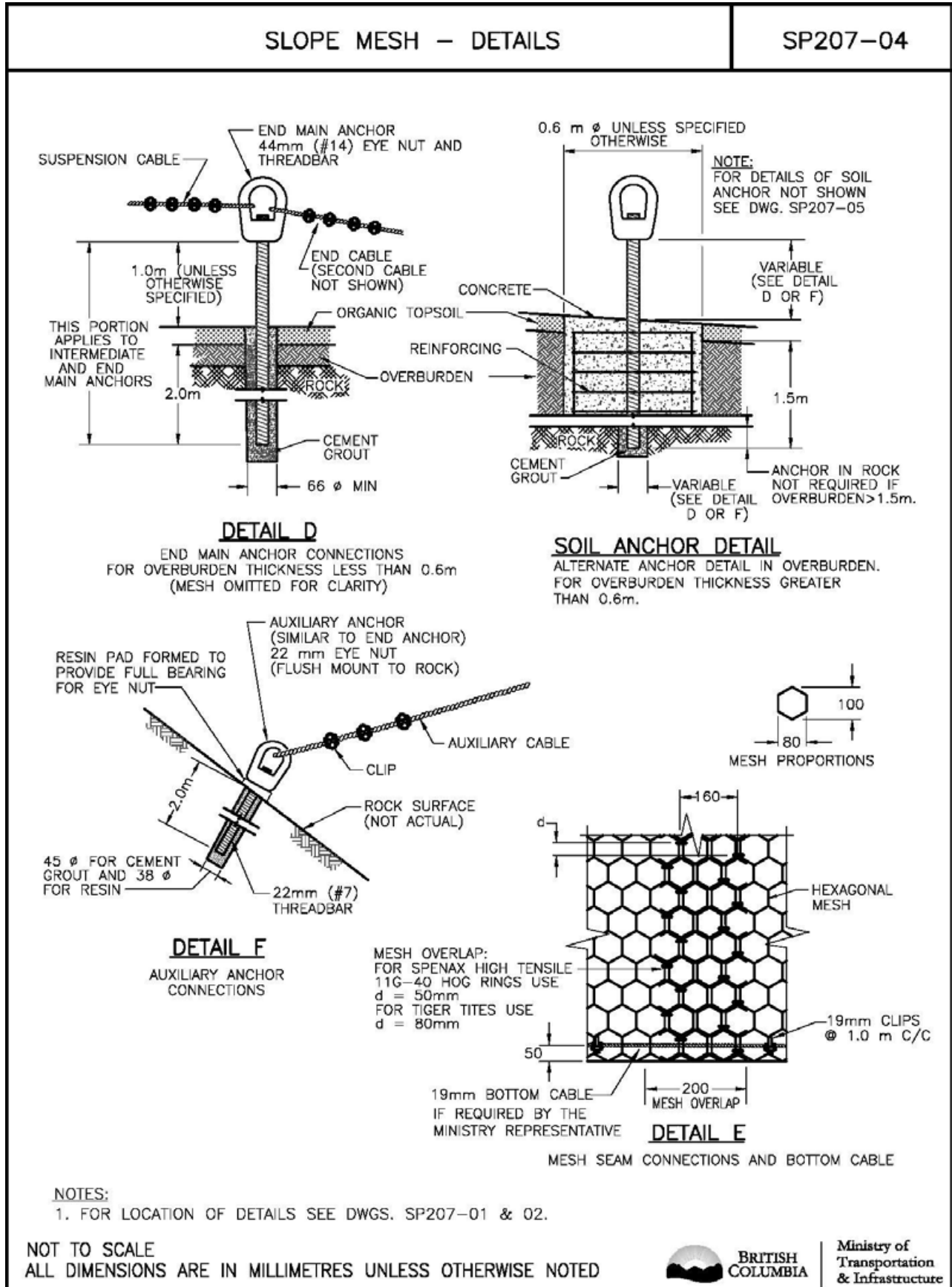
PAYMENT

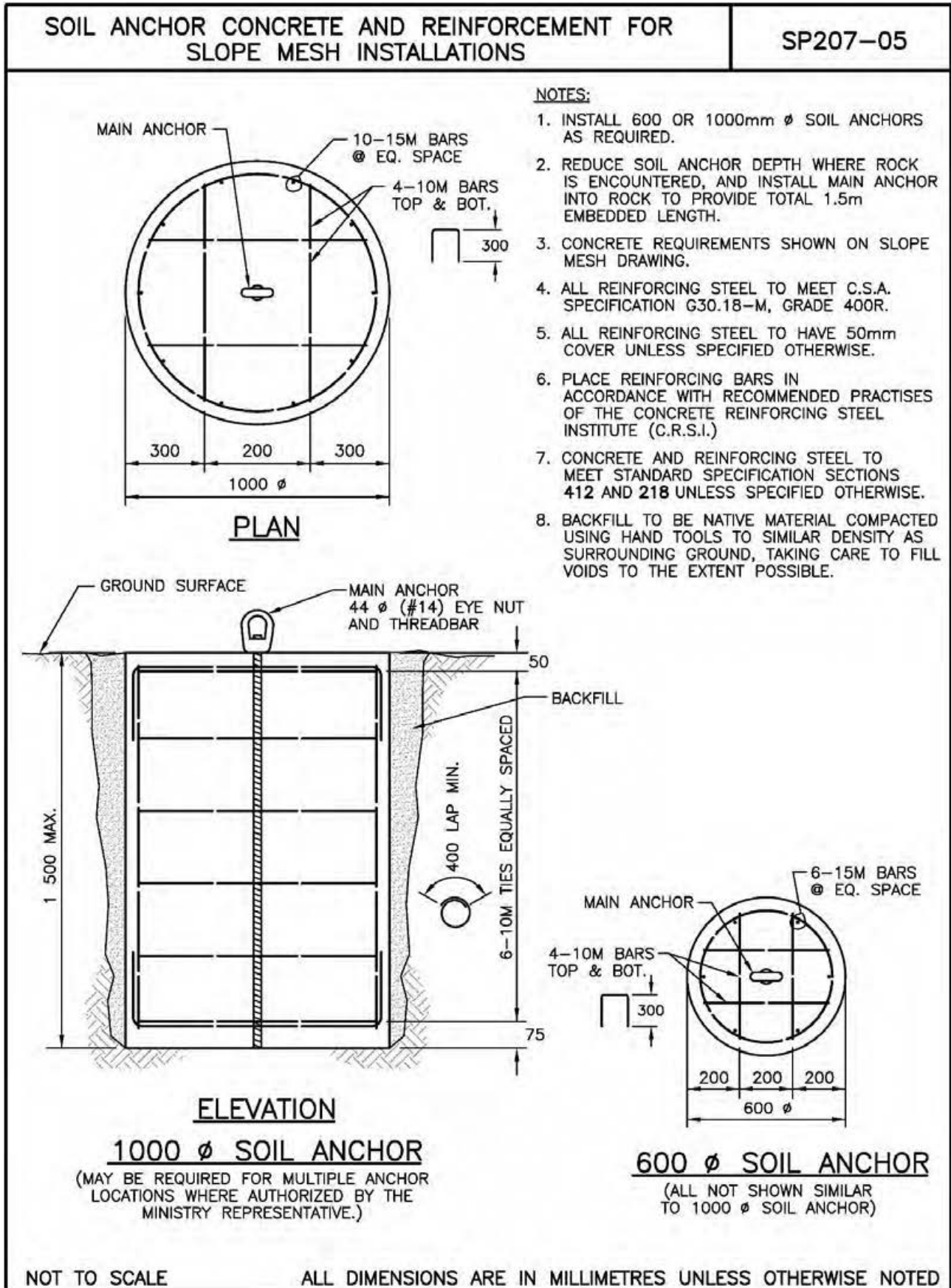
207.91 Slope Mesh – Payment for SLOPE MESH will be at the Contract Unit Price per square metre installed. The Unit Price will be full compensation for all requirements in this specification. Partial payment may be authorized where all components have been installed in a portion of the designated slope meshing area. No separate payment will be made for mesh overlap.











SECTION 208

DRAINS FOR ROCK CUTS

DESCRIPTION

208.01 Scope – This Section covers the installation of lined drains in the designated areas to the specified depth. The exact locations of drain holes will be dependent on field conditions and will be detailed by the Ministry Representative.

MATERIALS

208.11 PVC Pipe Materials – Drain hole lining shall consist of Schedule 40 slotted PVC pipe. The outside diameter of the pipe shall not be less than 67% of the borehole diameter, and the internal diameter of the pipe shall be a minimum of 19 mm. The slots shall be along the entire length of the pipe in one row. Unless otherwise specified, the slots shall be 0.5 mm wide, spaced 6 mm apart and have a minimum length of 30% of the outside circumference of the pipe.

CONSTRUCTION

208.31 Drilling – Advancement of drilling for drain holes shall be in an upslope direction. The dip angle for drain holes shall be five degrees (5°), or as specified by the

Ministry Representative. Finished holes shall be flushed with air to remove deleterious materials.

208.32 PVC Pipe Installation – PVC pipes shall be installed into the finished holes immediately after flushing. Pipes shall be coupled according to the manufacturer's specifications. The pipe shall be installed along the entire length of the holes and shall protrude 0.30 m out of the rock face. The slots shall be on top when the pipe is installed. The pipe shall be secured in the hole, using plastic or wooden wedges driven into the hole collar if necessary, so that the pipe cannot be pulled out by hand.

MEASUREMENT

208.81 Lined Drains – Lined drains will be measured by the METRE of drain hole drilled.

PAYMENT

208.91 Lined Drains – Payment for LINED DRAINS will be at the Contract Unit Price per metre. Payment for lined drains will be authorized after installation to the Contract specifications. The Unit Price will be considered full compensation for all work and materials necessary to complete the installation prescribed in this Section.

SECTION 209

SHOTCRETE FOR ROCK CUTS

DESCRIPTION

209.01 General – Unless otherwise specified, the shotcrete standards of American Concrete Institute ([ACI](#)) 506.2 Specification for Shotcrete and the standards of good practice in ACI 506R Guide to Shotcrete shall apply. SS 211, Portland Cement Concrete, shall also apply for shotcrete, unless otherwise specified.

Shotcrete operations shall include, but not be limited to pre-construction and construction work including preparation of the mix design(s), quality control, surface preparation; installation of wick drains, weep holes and pipes; supply and installation of the shotcrete; preconstruction and construction testing, and curing; the supply, transportation and testing of shotcrete panels; any core extraction of the in-place shotcrete, and any other testing to determine curing or protection requirements.

209.02 Quality Control

209.02.01 General – The Contractor shall engage at its own expense, a specialist firm (the Contractor's Consultant) to provide on-site quality control for shotcrete operations. The Contractor's Consultant shall not be an employee of the Contractor.

209.02.02 Qualifications – The Contractor's Consultant shall have designed shotcrete for at least three (3) projects over the last five (5) years. The Contractor's Consultant's representative conducting on-site shotcrete tests shall be an [ACI](#) or [Canadian Council of Independent Laboratories \(CCIL\)](#) certified concrete technician.

209.02.03 Specific Requirements – For wet and dry process shotcrete, quality control shall include, but not be limited to the following:

- Observation of surface preparation before shotcrete application.
- Observation and testing during shotcrete application.
- Observation of curing operations.

209.02.04 Other Requirements – The Contractor's Consultant shall be present at other times as are appropriate to assist the Contractor in the preparation, assessment and adjustment of the various procedures to be used for shotcrete operations.

209.02.05 Field Report – The Contractor shall ensure that the Contractor's Consultant prepares a field report for each site visit, signed by the Contractor's Consultant. The report shall include details of the progress of shotcrete operations, any recommendations made to the Contractor and any problems encountered by the Contractor.

209.03 Submittals

209.03.01 The Contractor shall submit for review the following documentation by the pre-construction meeting:

- (a) Qualifications and Experience of the Contractor** – Provide a statement of the qualifications, experience and work function of all personnel assigned to shotcrete duties. A statement of previous work experience on similar projects shall also be provided. This statement shall include the project name, location, type of shotcrete, volume of shotcrete, year constructed and the owner contact name. The company, the nozzle operator and the predampener/pot operator shall each have a minimum of five (5) years demonstrated experience on shotcrete work.
- (b) Qualifications and Experience of the Contractor's Consultant** – The qualifications shall include the name of the consulting firm, and the name and qualifications of the Consultant's on-site representative who will be providing the quality control. The following information shall be included in the qualifications submitted:
 - Project name, location and experience.
 - Contact name and phone number of the owner who can verify the experience of the Contractor's Consultant's site representative.
- (c) Proposed Laboratory Testing Agency** – Provide a statement of the testing agency's experience in performing laboratory tests on shotcrete. The agency shall be an independent, qualified laboratory with a proven record of performing tests on shotcrete on at least five (5) previous projects. The testing agency shall be certified by CCIL to CSA A283 *Qualification Code for Concrete Testing Laboratories*.
- (d) Certification of the Concrete Technician** – Provide evidence of [ACI](#) or [CCIL](#) Certification for the concrete technician who will be providing on-site shotcrete tests.

209.03.02 Documentation – The Contractor shall submit the following documentation at least ten (10) days prior to commencing shotcrete operations to the Ministry Representative:

- (a) Proposed Shotcrete Operations** – Provide a description of the proposed shotcrete operations including slope preparation method, shotcrete application method, and shotcrete curing and protection methods.

(b) **Materials Records** – Provide records showing source and proof of conformance to project specifications of the following materials:

- (i) Portland Cement (provide mill certificates)
- (ii) Silica Fume (provide mill certificates)
- (iii) Mix Water
- (iv) Aggregates
- (v) Admixtures (provide supplier data sheets)
- (vi) Fibre Reinforcement (if specified), include load/deflection curves to ASTM C1609 for the proposed mix design
- (vii) Mesh Reinforcement (if specified, provide mill certificates)

(c) **Previous Performance Data** – Provide previous performance data for the proposed shotcrete mix design, if available.

(d) **Proposed Shotcrete Mix Design (Proportions)** – Provide documentation for the proposed shotcrete mix design. Include such back-up data as requested. The proposed mix design submittal shall include:

- (i) Mix design number.
- (ii) Batch quantities in kg/m³ based on aggregates in an SSD (Saturated Surface Dry) moisture state for wet process shotcrete and ready mix supplied dry process shotcrete. Batch quantities for dry bagged supply shall be based on mass of aggregates in a dry state.
- (iii) A total cementitious content of not less than 20% by mass of the total ingredients.
- (iv) Aggregate source, bulk density, absorption, combined gradation data and a grain size distribution graph scaled between 0.075 mm and 20 mm showing the percent passing curve for the combined aggregate.

(e) **Proposed Shotcrete Imprint** – Provide a drawing of the proposed shotcrete imprint including content, imprinting method and materials that will be used to produce the imprint.

209.03.03 The Contractor shall submit for review the following documentation within 1 day following each day of shotcrete application:

- (a) **Field Report** – Provide original field reports signed by the Contractor's Consultant.
- (b) **Daily General Records** – Provide daily shotcrete records with the following information (certified by the Contractor's Consultant in the case of wet process shotcrete):

- (i) Contractor Name

- (ii) Nozzleman Name

- (iii) Helper Name

- (iv) Contractor's Consultant's Name (for wet process shotcrete)

- (v) Contractor's Consultant's Representatives Name (for wet process shotcrete)

- (vi) Date

- (vii) Weather

- (viii) Contractor's start and end time

- (ix) Shotcrete equipment type

- (x) Temperature

- (xi) Type of shotcrete (wet or dry process)

- (xii) When slump is measured (before or after admixture and fibres)

(c) **Shotcrete Batch Records** – Submit the following information for each batch of shotcrete:

- (i) Batch number

- (ii) Location of shotcrete

- (iii) Mix design number

- (iv) Batch time at plant (wet process)

- (v) Batch arrival time on-site

- (vi) Truck departure time (wet process)

- (vii) Placement start time

- (viii) Placement end time

- (ix) Quantity used

- (x) Water/cementitious materials ratio (wet process)

- (xi) As-shot air content (wet process)

- (xii) Slump (wet process) – indicate when measurement is made (e.g.: after air entrainment is added, before superplasticizer is added and before fibre reinforcement is added)

- (xiii) Percent rebound (visual estimate)

- (xiv) Average thickness of shotcrete

- (xv) Test panel number

- (xvi) Shotcrete delivery tickets from batch plant (wet process)

- (xvii) Comments on the quality of the shotcrete and any problems that arise

- (xviii) Proposed revised mix design

209.03.04 The Contractor shall submit the laboratory test results within two (2) working days after testing to the Ministry Representative. These test results shall compare

the shotcrete performance with the requirements given in Table 209-A.

Records shall include all reporting requirements outlined for construction testing. Records shall also include an original summary report of all test results, showing the sample number, sample origin, panel number, panel origin, date the panel was sprayed, panel shipping date, date panel received at the laboratory, photos of prepared test samples, test date, sample age at testing, test results for each sample, average test results of each set of three samples taken pursuant to SS 209.39 for each test at a given age, and acceptability of the results.

209.04 Definitions – The following definitions refer to words and terms used in this specification. For definitions not covered in this document, refer to ACI 506R and ACI 506.2.

209.04.01 Blow pipe – Air / water jet operated by nozzle operator's helper during shotcrete placement to assist in keeping rebound and overspray out of the work.

209.04.02 Dry Process Shotcrete – A process where most of the water added to the shotcrete mixture is added at the nozzle.

209.04.03 Overspray – Shotcrete material deposited away from the intended receiving surface.

209.04.04 Pot (Sometimes called Gun) – Machine used to meter the shotcrete into the hose.

209.04.05 Predampener – Device used to mix a portion of the mixing water with the shotcrete. Can also be used to mix accelerator with shotcrete.

209.04.06 Saturated Surface Dry – The state of a material in which it will neither add nor subtract moisture from other material placed in contact with it.

209.04.07 Slake – A process of deterioration of freshly exposed rock due to exposure to the atmosphere and/or water.

209.04.08 Sloughing (also called sagging) – Subsidence of shotcrete, due generally to excessive water in the mix or placing too great a thickness of shotcrete in a single pass.

209.04.09 Weep Pipes – Plastic pipes inserted in water bearing fissures to relieve water pressure from behind the shotcrete.

209.04.10 Wet Process Shotcrete – Shotcrete in which all of the ingredients are mixed before introduction into the delivery hose. Compressed air is introduced to the material flow at the nozzle. If an accelerator is used, it is normally added at the nozzle.

MATERIALS

209.11 Materials

209.11.01 Cement – Cement shall be Type GU or HE, or where approved by the Ministry Representative, Type GUL. GUL cement may be used only if:

(a) Shotcrete is not subject to very severe sulphate exposure conditions on Site (CSA A23.1 exposure condition S-1); and

(b) GUL cement, silica fume, and any other supplementary cementitious materials and additives proposed to be use are assessed for compatibility, and collectively will not produce any deleterious effects on the long-term properties of the shotcrete or its bond to the substrate.

209.11.02 Silica Fume – Silica fume shall meet the requirements of CSA-A23.5 [See CSA A3000], Supplementary Cementing Materials, Type U Silica Fume, with the following additional requirements:

- Minimum SiO₂ content – 90 percent (90%) by mass
- Maximum carbon content: 5 percent (5%) by mass

The Contractor may propose alternatives to silica fume, which shall be subject to approval by the Ministry Representative.

209.11.03 Pozzolan – Pozzolan shall be Type F flyash.

Table 209-A: Shotcrete Performance Requirements

Test Description	Test Method	Age (Days)	Specified Requirement
*Maximum Water / Cementitious Materials Ratio		-	0.45
Air Content – As Shot, %	CSA A23.2-4C	-	4 ± 1½
*Slump at discharge into shotcrete pump, mm	CSA A23.2-5C	-	80 ± 30
Minimum Compressive Strength, MPa	CSA A23.2-14C	7 28	30 40
Minimum Flexural Strength, MPa	ASTM C1609	7	4
Minimum Toughness Performance Level (TPL)	ASTM C1609 and this specification	7	III
Maximum Boiled Absorption, %	CSA A23.2 – 11C	7	8
Maximum Volume of Permeable Voids, %	ASTM C642	7	17

Notes: *Properties apply to wet process shotcrete only.

209.11.04 Water – Surface or ground water may be used for cleaning the slope and curing provided it is free of deleterious substances. Verification that the water intended for use is acceptable is at the discretion of the Ministry Representative and at the expense of the Contractor.

209.11.05 Aggregates – Aggregate shall be normal density. The combined aggregate gradation limits shall meet the limits shown in Table 209-B.

Table 209-B: Combined Gradation Limits for Shotcrete Aggregate

U.S. STD Square Mesh	Metric Sieve Size (mm)	Total Passing Each Sieve Size (% By Mass)
1/2	14	100
3/8	10	90 – 100
#4	5	70 – 85
#8	2.5	50 – 70
#16	1.25	35 – 55
#30	0.630	20 – 35
#50	0.315	8 – 20
#100	0.160	2 – 10

209.11.06 Admixtures

- (a) **General** – Chemical admixtures shall include air-entraining, water reducing and set retarding admixtures where appropriate.
- (b) **Superplasticizers** – Superplasticizers shall meet the requirements of ASTM C1141.
- (c) **Accelerating Admixtures in General** – Accelerating admixtures shall only be used with the written consent of the Ministry Representative.
- (d) **Accelerating Admixture Requirements** – Accelerating admixtures:
 - (i) shall be non-chloride based accelerating admixtures meeting the requirements of CSA A23.1M Clause 15.1.7.1;
 - (ii) shall be compatible with the cement being used when tested in accordance with ASTM C1141;
 - (iii) shall have a demonstrated ability, when used at the required proportions, to produce accelerated shotcrete meeting the performance criteria specified in Table 209-A;
 - (iv) wet process shotcrete shall be liquid, dispensed at the nozzle in closely controlled quantities;

- (v) dry process shotcrete shall be liquid, dispensed at the nozzle in closely controlled quantities; or be powdered and either:
 - Integrally mixed in the case of dry bagged material or;
 - Mechanically proportioned through the predampener in the case of ready mix supply; metering of accelerators directly into the pot will not be permitted.

209.11.07 Fibre Reinforcement – Unless otherwise specified, shotcrete reinforcement shall consist of steel or polyolefin fibres. The fibres shall be capable of meeting the performance criteria specified in Table 209-A and the following requirements:

- (a) **Steel Fibres** – Fibres shall meet the requirements of ASTM C1116 Type I.
- (b) **Polyolefin Fibres** – Fibres shall only be used in wet process shotcrete and shall meet the requirements of ASTM C1116 Type III.

209.11.08 Mesh Reinforcement – Where specified instead of or in addition to fibre reinforcement, shotcrete shall be reinforced with mesh anchored to the surface before shotcrete application. For fibre reinforced shotcrete, additional reinforcement may be required by placing mesh panels up to 0.60 m by 0.60 m without anchors between successive shotcrete lifts where final shotcrete thicknesses are expected to exceed 300 mm in local areas.

- (a) **Mesh Materials** – Mesh shall consist of welded wire fabric 102 mm by 102 mm opening (4 by 4 inch) by MW13.3 / MW13.3 (8 gauge) wire meeting ASTM A1064 and hot dip galvanized meeting ASTM A123.
- (b) **Mesh Anchors** – Anchors shall consist of Grade 517 MPa steel, conforming to CSA G30.18, minimum 19 mm nominal diameter.

Face plates shall be 150 mm by 150 mm by 6 mm. Face plates, nuts and washers shall be capable of developing full anchor pullout capacity.
- (c) **Mesh Anchorage** – Mesh anchors shall be secured by resin or grout.

(i) Grout – Grout shall have the following properties:

- (A) be pre-mixed, unsanded, non-metallic, and non-shrink cementitious grout containing silica fume.
- (B) may be mixed to a flowable consistency, typically with a water to cement ratio of 0.35 or as specified by the grout manufacturer to achieve or exceed the required strength.
- (C) minimum 7-day compressive strength of 30 MPa and a minimum 28 day compressive

strength of 40 MPa, tested in accordance with CSA A23.2-09.

(D) admixtures to be used according to the manufacturer's specifications and as authorized by the Ministry Representative.

(E) calcium chloride-based accelerators are not permitted.

(ii) Resin – Resin shall have the following properties:

(A) be supplied in cartridge form.

(B) a minimum unconfined compressive strength of 90 MPa when fully mixed and cured and tested in accordance with CSA A23.2-09.

(C) be encased in a plastic film that provides optimum resistance to moisture and is easily ruptured to enable complete mixing during installation.

(D) suitable thixotropic and viscous properties to permit adequate mixing of the resin components by rotation of the rock bolt bar and to contain the resin within the drill hole.

(E) an easily identifiable gel time recommended by the resin manufacturer.

(F) reach 80% of its ultimate strength within a time interval equal to five times the gel time.

(G) be non-shrink after the gel time.

(H) be unaffected by mild acids or mild alkalis.

(I) be supplied in cartridge boxes labelled with the resin expiry date.

(d) Mesh Coverage and Overlap – Mesh shall be installed over the entire area to be shotcreted, unless otherwise specified. Mesh joints shall be lapped 1.5 mesh openings in each direction (horizontally and vertically) and tied at minimum 100 mm intervals with 16 gauge tie wire to achieve full contact between mesh layers.

(e) Mesh Anchor Spacing – Mesh shall be anchored on minimum 1.2 m centres in each direction or closer where required to form the mesh to within 50 mm of the slope surface. The mesh shall be in full contact with the surface where practicable. The mesh may also be anchored to any previously installed rock bolts provided the spacing is consistent with the spacing required for the above anchors.

(f) Anchor Installation – Anchors shall be installed as recommended by the anchor manufacturer. The minimum anchor embedment length shall be 1.0 m or as specified. Resin or grout shall be applied to full anchor embedment.

(g) Mesh Anchor Testing – A total of 5% of the anchors, selected by the Ministry Representative, shall be load

tested to 186 kN. This load shall be held for 10 minutes. A load loss of greater than 10% of the applied load shall be indicative of anchorage failure. Any failed anchors shall be replaced at the Contractor's expense.

209.11.09 Anchor Reinforcement – Where specified, anchor reinforcement shall be installed. Anchor materials, installation and testing shall meet the requirements for mesh anchors unless otherwise specified. Face plates for these anchors shall be “Spider Plates”. These anchors and plates shall be fully embedded in the shotcrete.

209.11.10 Materials Handling and Storage

(a) General – All materials shall be stored and handled in a manner that will prevent damage, deterioration or contamination.

(b) Hot Weather Storage – During hot weather prebagged material shall be stored in a manner that will allow maximum cooling of the material.

(c) Cold Weather Storage – During cold weather prebagged material shall be stored in a manner that will provide maximum heat retention of the material.

209.12 Performance Requirements

209.12.01 General – Shotcrete shall meet the performance requirements in Table 209-A.

209.12.02 Air Content – The air content shall be determined on wet process shotcrete sprayed into a CSA-A23.2-4C air pressure meter base or freshly applied material removed from in-place and consolidated into the air meter base by rodding.

CONSTRUCTION

209.31 Batching, Mixing and Conveyance

209.31.01 General – Shotcrete shall be batched, mixed and supplied using (one of) the following system(s):

(a) dry bagged pre mix supply to ASTM C928 or:

(b) ready mix supply

209.31.02 Mixing – Shotcrete supplied shall be uniformly mixed with no evidence of segregation or improper mixing.

209.31.03 Batching and Conveyance – Ready mix supplied shotcrete shall be batched and conveyed at a rate that allows a consistent application of shotcrete. Irregularities of conveyance that result in delays or a surplus of aged material on-site shall be avoided. Shotcrete that is more than 90 minutes old from the time of batching will not be accepted, unless hydration controlling admixtures are used to maintain suitable slump. Shotcrete that has stiffened excessively shall not be used.

209.32 Pre-construction Testing

209.32.01 General – The Contractor shall perform pre-construction shotcrete tests (prior to full-scale shotcrete operations), at a shotcrete test site chosen by the Ministry Representative, to demonstrate the competence of the crew, the adequacy of the equipment and the adequacy of the shotcrete mix design. The requirement for pre-construction testing may be waived by the Ministry Representative if sufficient documentation of previous satisfactory performance on similar work is provided.

209.32.02 Previous Performance – Documentation for previous satisfactory performance shall meet the following requirements:

- (a) **Company** – The company shall have applied more than 100 m³ of shotcrete on a minimum of 3 projects in the last 5 years.
- (b) **Nozzle Operator** – The nozzle operator shall have applied more than 100 m³ of shotcrete on a minimum of 3 projects in the last 5 years.
- (c) **Predampener/Pot Operator** – The predampener/pot operator shall have mixed more than 100 m³ of shotcrete on a minimum of 3 projects in the last 5 years.
- (d) **Contacts** – The contact name and number for each project above shall be supplied upon request.

209.32.03 Application – Shotcrete applied at the shotcrete test site shall be applied in accordance with the general shotcrete provisions in these specifications.

209.32.04 Test Panel – One test panel shall be shot and tested for the pre-construction shotcrete testing as described for construction testing.

209.32.05 Test Results – The Contractor shall not apply shotcrete outside of the shotcrete test site area until the pre-construction shotcrete test results have been evaluated by the Ministry Representative and authorization is given to proceed with the work.

209.32.06 Revised Methods – If the results of the shotcrete tests are unsatisfactory in the opinion of the Ministry Representative, then, notwithstanding the Ministry Representative's prior review of the Contractor's pre-construction submittals, the Contractor shall adopt such revised methods as are necessary to achieve the required results.

209.32.07 Additional Tests – The Contractor shall perform, at any time during the progress of the work, additional shotcrete tests at designated test sites if the shotcrete applied in the areas specified in the Contract are unsatisfactory in the opinion of the Ministry Representative.

209.33 Weather Conditions

209.33.01 Heavy Rain or High Wind – During periods of heavy rain or high wind, the Contractor shall ensure that the quality of the finished shotcrete product is not compromised. Shotcrete shall not be applied during such

weather unless protective measures can be practicably taken.

209.33.02 Hot Weather – During periods of hot weather, when ambient temperatures are expected to rise over 30°C, the following requirements shall be met

- (a) **Curing** – The Contractor shall implement curing immediately after the shotcrete has been placed.
- (b) **Temperature at Placement** – The Contractor shall take any steps necessary to ensure that the temperature of the shotcrete supplied to the site is less than 25°C at the time of placement.

209.33.03 Cold Weather – During periods of cold weather, where the minimum ambient air temperature falls below 5°C, the following requirements shall be met:

- (a) **Temperature at Placement** – The Contractor shall take whatever steps are necessary to ensure that the in-place shotcrete temperature is between 10°C and 25°C at the time of placement.
- (b) **Initial Curing Temperature** – The Contractor shall ensure that, during the initial curing period (4 days), the in-place shotcrete temperature will not fall below 5°C. Shotcrete shall not be placed unless measures can be practicably taken to ensure these requirements are met.
- (c) **Temperature at the Pot** – The Contractor shall not allow the temperature of the dampened dry process shotcrete at the pot to exceed 30°C.
- (d) **Ice, Snow or Frozen Surfaces** – Shotcrete shall not be applied to frozen surfaces, or surfaces covered with frost, ice or snow.

209.34 Surface Preparation – Surfaces to which shotcrete is applied shall be cleaned of loose rock, dirt and debris which if left in place could impair the bond of shotcrete to rock, concrete or existing shotcrete. Water or air and water jets shall be used to clean rock surfaces. Compressed air only shall be used to clean rock surfaces that have a tendency to slake from contact with water.

209.35 Drainage – Unless otherwise controlled, water seepage from slopes shall be controlled by installing weep holes, weep pipes, and wick drains. Surface runoff shall be controlled using suitable water diversion systems.

209.35.01 Weep Holes – Weep holes (and weep pipes as required) shall be installed before shotcrete is applied. These weep holes shall be on 1 m to 1.5 m centres in the areas to be shotcreted in joints, fractures and faults, or at a closer spacing where required by the Ministry Representative. Holes and pipes shall be inclined to permit free drainage.

209.35.02 Weep Pipes – To prevent blockage of weep holes due to entry of shotcrete or rebound materials, weep pipes shall be secured in weep holes and temporarily plugged at the outer end of the pipe. The plugs shall be removed when

shotcreting is completed. Weep pipes shall consist of 25 mm minimum outside diameter Schedule 40 plastic pipe.

209.35.03 Active Seepage Areas – Where active seepage occurs, weep holes shall be drilled approximately 150 mm or more to intercept fractures or openings wherever practicable and weep pipes shall be installed prior to the application of shotcrete. Where drilling is not practicable, the weep pipes shall be secured in the fractures by other appropriate means.

209.35.04 No Active Seepage Areas – Where no active seepage occurs, weep holes shall be installed in joints, fractures and faults by one of the following methods:

- (a) by securing the weep pipes by appropriate means before shotcreting;
- (b) by marking locations using sticks or other appropriate means before or during shotcreting and drilling at the marked locations after shotcreting. Weep holes shall be 25 mm diameter and shall be drilled to intersect the joints, fractures and faults under the shotcrete at the marked locations. Weep pipes are not required for such holes.

209.35.05 Weep Pipe Removal or Cutting – Weep pipes shall be removed or cut level with the shotcrete surface after the shotcrete has reached initial set, within one day after application.

209.35.06 Additional Weep Holes – Additional weep holes shall be drilled after shotcreting where there are visible signs of water pressure, such as seepage and damp spots in the hardened shotcrete.

209.35.07 Wick Drains – In areas of active seepage, wick drains shall be installed before shotcreting in addition to weep holes if requested by the Ministry Representative. Wick drains shall consist of a flexible polypropylene core drain wrapped with a proven durable geotextile filter fabric. Wick drains shall be 3 mm by 100 mm size (variable length), with #120 sieve filter, 0.17 mm/s filter permeability, and 0.8 kN core grab tensile strength. They shall be able to withstand all handling, abrasion and distortion that occur during installation. Wick drains shall be secured to the surface on maximum 0.5 m centres with pneumatically driven nails, or authorized substitute fasteners. Fastener spacing shall be reduced where required to ensure wick drains are in full contact with the surface. Wick drains shall be installed and covered with shotcrete in a manner that allows gravity drainage to the toe of the slope.

209.35.08 Surface Runoff – Where the bond of shotcrete to rock may be impaired by surface water runoff, this water shall be controlled and directed away from the area to be shotcreted.

209.36 Existing Rock Bolts – Within or near areas designated for shotcrete where rock bolts were installed (not for the sole purpose of securing mesh) prior to shotcrete application, the rock bolts shall be protected or extended

through the shotcrete as described in the following subsections.

209.36.01 Extension of Rock Bolts – Unless otherwise specified, where rock is fractured within 0.5 m of the existing rock bolts, the face plates and nuts shall be removed before shotcrete is applied. The bar shall be extended, where necessary, using a short piece of bar and a coupler. The shotcrete shall be applied up to and against the existing bar. The face plates and nuts shall then be reinstalled over the shotcrete after the shotcrete has cured. Each face plate shall be replaced to the same bar from which it was removed. The exposed bar length shall be 100 mm following reinstallation of the face plate. The bar shall be nominally tensioned (using a wrench to the nut).

209.36.02 Protection of Rock Bolts – All other rock bolts shall be protected from overspray. Where overspray was not anticipated, the affected rock bolts shall be cleaned off by appropriate means.

209.37 Application

209.37.01 General – Prior to application of any shotcrete, the Contractor shall obtain authorization to proceed after the areas prepared for shotcrete have been reviewed by the Ministry Representative. Review by the Ministry Representative shall not relieve the Contractor from the responsibility for ensuring that the shotcrete operations are conducted in a satisfactory manner in accordance with these specifications.

209.37.02 Wetted Surface – Not more than one hour prior to application of shotcrete, all surfaces (except slaking ground) to be shotcreted shall be sprayed with water. Wetted surfaces shall be allowed to dry back to a saturated-surface-dry condition prior to application of shotcrete. If necessary, a blow-pipe shall be used to facilitate removal of surface water. Compressed air used in the blow pipe shall be oil-free.

209.37.03 Shotcrete Layers – Wherever possible, shotcrete shall be applied to the full thickness in a single layer, provided it does not slough or become delaminated. If multiple lifts are required, the previous shotcrete lifts shall be scraped or broomed prior to the time of set to remove any loose material, rebound, overspray, laitance or any other material that may impair bond of subsequent layers. Previous lifts shall be prevented from drying out by moist curing. Construction joints shall be to a 45° edge.

209.37.04 Rebound and Overspray – A blow pipe shall be used during the course of the work to remove accumulations of rebound and overspray from areas to be shotcreted. Rebound and overspray shall not be incorporated in the completed work.

209.37.05 Thickness and Area Covered – In general, an average thickness of 100 mm of shotcrete shall be applied over fractures, joints and faults in the areas designated for shotcrete, unless otherwise specified by the Ministry

Representative. In general, shotcrete shall cover laterally to a maximum of 0.50 m beyond fractures, joints and faults over solid rock or concrete areas.

209.37.06 Access – Suitable devices shall be employed to allow access to the work for shotcreting. Access shall be such that fresh shotcrete is not damaged by hoses, equipment or personnel.

209.37.07 Surface Finish – The shotcrete surface shall be left in a natural gun state unless otherwise specified.

209.37.08 Embedments – A cover of 50 mm of shotcrete shall be applied over any reinforcing steel or other embedments. If greater total thicknesses of shotcrete are applied to encapsulate the embedments this work shall be performed at no cost to the Ministry.

209.37.09 Imprint – The plastic shotcrete surface shall be neatly imprinted with capitalized lettering 75 mm high with the following information arranged in the order listed below:

- MoTI
- The shotcrete completion date, in the format MMM, YYYY
- The name of the Contractor

The imprint shall be located in a small area in the bottom right corner of each continuous shotcrete area, unless otherwise authorized by the Ministry Representative. Where practicable, the information shall be readable unaided from highway level following shotcrete curing. Imprints produced by scraping letters into the plastic shotcrete will not be acceptable.

209.38 Curing and Protection

209.38.01 Curing Compounds – Membrane curing compounds shall meet ASTM C309 and shall be applied if necessary and generally in accordance with the manufacturer's recommendations. The application rate of a curing compound on the rough shotcrete surface shall generally be twice the rate used for smooth concrete surfaces. Curing compounds shall not be used on the substrate or in areas where additional shotcrete will be applied. If a membrane curing compound is not used, then moist curing shall be implemented.

209.38.02 Moist Curing – The surface of the shotcrete shall be maintained in a moist condition for a minimum period of 4 days following shotcrete application or until an in-place compressive strength of 20 MPa has been reached. Wet burlap and polyethylene sheet or water sprays shall be used to provide moist curing.

209.38.03 Protection from Freezing – The shotcrete shall be protected from freezing for a minimum period of 7 days after placing or until an in-place compressive strength of 20 MPa has been reached, whichever occurs first.

209.38.04 Sampling and Testing – If compressive strength samples are used to determine curing or protection requirements, the cost for obtaining the samples, shipping the samples and performing the tests shall be the responsibility of the Contractor. Extraction of cores shall be in accordance with CSA A23.2-14C. Compressive strength testing shall be performed according to requirements for construction testing.

209.39 Construction Testing

209.39.01 On-Site Tests – For wet process shotcrete, the following on-site tests shall be conducted for each truck:

- As-placed air content; and
- Slump at discharge into pump.

209.39.02 Thickness Monitoring – The Contractor shall monitor the thickness of shotcrete applied using depth gauges or probes. Thickness shall be measured on 1.0 m centres or closer as required to provide thickness control.

209.39.03 Test Panels in General – The Contractor shall produce test panels in accordance with the requirements of ASTM C1140, but panels shall have minimum base dimensions of 500 mm x 500 mm and a minimum depth of 125 mm. The form base shall be made from 19 mm thick sealed plywood, and side pieces shall be made from two by six timbers. The test panel molds shall have 45° sloped sides to permit escape of rebound.

209.39.04 Test Panel Frequency – One construction test panel shall be shot on each day of shotcrete placement or for every 50 m³ of shotcrete being placed, whichever results in more panels. The time of shooting the panels may be chosen by the Ministry Representative.

209.39.05 Test Panel Shooting and Marking – Test panels shall be shot using the same nozzle and length of hose as the production shotcrete. The Ministry may request test panels to be shot at the location of the in-situ shotcrete. Construction test panels shall contain no reinforcement or embedments (other than fibre reinforcement). For each test panel the Contractor shall provide a record of the test panel number, date cast, date shipped and date received by the laboratory.

209.39.06 Test Panel Curing and Transportation – Test panels shall be field cured, in the forms, in the same manner as the specified shotcrete work, for a minimum period of 48 hours, prior to transport to the test laboratory. Panels shall be covered with wet burlap and plastic sheet to prevent drying. In cold weather, panels shall be protected from temperatures below 5°C by covering with thermal insulation blankets or by placing panels in a suitably heated enclosure. Test panels shall be transported in their forms and kept moist, with care being taken not to crack or damage the shotcrete.

209.39.07 Test Panels in Laboratory – The test panels inside the forms shall be placed in a moist room in the

laboratory, maintained at temperature of $23 \pm 2^\circ\text{C}$ and relative humidity of $98 \pm 2\%$. After age 3 days the test panels shall be removed from the forms and returned to the moist room until the time of testing.

209.39.08 Test Specimen Extraction and Storing – At specified test ages, test specimens shall be extracted from the panels using either diamond sawing or coring. Test specimens shall be maintained in a moist state in the moist room in the laboratory or in a lime-saturated water bath maintained at $23 \pm 2^\circ\text{C}$ until the time of testing.

209.39.09 Compressive Strength Test Specimens – Compressive strength test specimens shall preferably be 75 mm diameter cores. Length/diameter ratios shall not exceed 2:1 and shall not be less than 1:1.

209.39.10 Compressive Strength Testing – For each panel, three specimens shall be tested for compressive strength at each specified age. Compressive strength tests shall be conducted in accordance with CSA_A23.2-14C. Measured compressive strengths shall be corrected to equivalent 2:1 length:diameter cores, using the core correction factors given in CSA A23.2-14C (Table 1). Reporting of results shall be according to Section 5 (Reporting) in CSA A23.2-14C.

209.39.11 Flexural Strength and Toughness Test Specimens – Flexural strength test specimens shall have dimensions of 100 mm x 100 mm x 350 mm. Reporting of results shall be according to "11. Report" in ASTM C1609 and shall include all load – deflection graphs.

209.39.12 Flexural Strength and Toughness Testing – For each panel, three specimens shall be tested for flexural strength and toughness at the specified age. Flexural strength and toughness tests shall be conducted in accordance with ASTM C1609 with the following additional requirements:

- (a) **Measuring System** – A "Japanese Yoke" or other measuring system that precludes any extraneous deflection measurements shall be used. An automatic data acquisition system or equivalent shall also be used during the tests.
- (b) **Accuracy of Measurement** – The accuracy of the first crack deflection shall be verified by comparing the measured value with the theoretically calculated value by using the formula:

$$\delta = \frac{23PL^3}{1296EI} \times \left[1 + \frac{216H^2(1 + \mu)}{115L^2} \right]$$

Where:

δ = mid-span deflection in mm

P = load at first crack in kN

L = load span in mm

E = modulus of elasticity (for convenience usually based on an estimated compression modulus of elasticity) in GPa

I = moment of inertia in mm^4

H = height of specimen in mm

μ = Poisson's Ratio (for convenience usually based on an assumed compression Poisson's Ratio of 0.2)

For a 100 mm x 100 mm x 350 mm beam tested on a 300 mm span the first crack deflection for typical fibre reinforced shotcrete will have first crack deflection in the range of about 0.03 mm to 0.05 mm. Extraneous deflections have been eliminated if first crack deflections are within this range.

- (c) **Recording of Deflections** – An LVDT connected to an x-y plotter or digital recording shall be used to capture load vs. deflection response. Dial gauges shall not be used for recording deflections. Bending Load shall be plotted against Net Midspan Deflection.
- (d) **Testing Machine** – A deflection-control testing machine shall be used, but shall not be of the closed-loop or soft testing type.
- (e) **Total Deflection** – Deflection measurements shall continue until a total deflection of 2 mm is reached.

209.39.13 Toughness Performance Level Data Analysis – Toughness Performance Level of flexural strength specimens shall be obtained by the following analysis:

- (a) **Template** – A template shall be created using the percent of design flexural strength shown in Table 209-C, but expressed in load equivalent units.

The following formula shall be used to determine load equivalent units;

$$P_d = \frac{\sigma \times B \times H^2}{L} \times \frac{1}{1000}$$

where:

P_d = design flexural load in kN

σ = design flexural strength in MPa

B = beam width in mm

H = beam depth in mm

L = load span in mm

- (b) **Master Template** – A master template shall be produced to the same scale as the load vs. deflection data recorded from the toughness test. The master template shall be superimposed over the actual load deflection curves and compared with the recorded data.

- (c) **Acceptability Criterion** – To conform to a given Toughness Performance Level the recorded data shall meet the specified flexural strength and fall above the line representing the given Toughness Performance Level at both the 0.5 mm and 2 mm net midspan deflection.

Table 209-C: Toughness Performance Level

Toughness Performance Level	Residual Flexural Strength By % of Design Flexural Strength	
	1/600 Span (0.5 mm)	1/150 Span (2.0 mm)
III	50%	30%

209.39.14 Boiled Absorption and Permeable Voids Test Specimens – Specimens for boiled absorption and permeable voids testing shall be 75 mm cubes cut from the broken ends of flexural test prisms, or extracted 75 mm diameter cores at least 100 mm long.

209.39.15 Boiled Absorption and Permeable Voids Testing – For each panel, three specimens shall be tested for boiled absorption and permeable voids at the specified age. Boiled absorption and permeable voids tests shall be conducted in accordance with ASTM C642. The report of results shall include all information recorded for “6.1 Calculation” in ASTM C642.

209.39.16 Laboratory Test Results – For each type of test, test results shall be considered acceptable if the average of three (3) specimens tested at a given age meets or exceeds the criteria specified in Table 209-A. The test results for an individual specimen shall not be less than 80% of the criteria specified for compressive strength, and not more than 120% of the criteria specified for boiled absorption and permeable voids.

209.40 Shotcrete Deficiencies and Repair

209.40.01 General – The Contractor shall, whenever possible, correct deficiencies while the shotcrete is still plastic. For any proposed changes to the initial mix design, a solid volume calculation shall be performed and submitted upon request. The calculation shall include the mass, density and volume of each constituent material, the total yield, and a comparison between measured (ASTM C138) and the calculated plastic density.

209.40.02 Deficiency Types – Shotcrete deficiencies other than failure to meet the performance requirements include, but are not limited to, the following:

- (a) Improper consolidation of the shotcrete because of incorrect spraying angles or improper distance of the nozzle from the receiving surface;
- (b) Improper control and removal of overspray and rebound material;

- (c) Excessive shotcrete or fibre rebound;
- (d) Any evidence of excessive plastic or drying shrinkage cracking;
- (e) Segregation, incorporation of sand lenses, excessive voids, tears, sags, sloughs or delaminations in the work; and
- (f) Inadequate thickness.

209.40.03 Shotcrete Repair – At the discretion of the Ministry Representative, shotcrete with deficiencies as defined above shall be removed and replaced to these specifications.

209.41 Shotcrete Evaluation – The Ministry Representative will implement a program of evaluation of the in-place shotcrete.

209.41.01 Core Extraction – When shotcrete is considered defective or deficient based on test results of test panels, the Ministry Representative may require cores to be extracted from the production shotcrete. Cores shall be extracted from the in-place shotcrete in accordance with CSA A23.2-14C, at locations detailed by the Ministry Representative, and tested for compliance to the project specifications. Generally, up to 6 cores shall be extracted from every 200 m² of shotcreted surface. Core extraction shall be at the Contractor’s expense.

209.41.02 Non-Destructive Testing – The shotcrete will be checked for delaminations, voids or other deficiencies using appropriate non-destructive testing procedures.

MEASUREMENT

209.81 Shotcrete – Shotcrete will be measured by the CUBIC METRE supplied and installed in-place on the slope in the designated areas.

PAYMENT

209.91 Shotcrete – Payment for SHOTCRETE will be at the Contract Unit Price per cubic metre. Payment for up to 75% of the shotcrete installed will be authorized after the Ministry Representative has received 7 day test results for this shotcrete. Payment for the remaining 25% will be authorized after 28 day test results and complete documentation have been received. The Ministry may consider reduced payment for defective or deficient shotcrete based on test results on test panels and/or in-situ shotcrete. The Contract Unit Price for shotcrete shall be considered full compensation for all requirements in this specification. The Ministry will not pay for volume losses to rebound, sloughing, rejected or otherwise discarded shotcrete.

DAILY SHOTCRETE RECORD

File #: _____
 PROJECT NAME _____

PROJECT NO. _____

(TO BE SUBMITTED WITHIN 1 DAY AFTER EACH DAY'S OPERATION)

Contractor Name _____ Date _____
 Nozzleman Name _____ Weather _____
 Helper Name _____ Temperature _____ °C
 Contractor's Consultant's Name _____ Contractor's Start _____ End _____ Time
 Contractor's Consultant's Representative _____ Shotcrete Equipment Type _____

Shotcrete Type: (circle) WET DRY Process

For wet process, when is slump measured (circle)
 BEFORE AFTER Air entrainment added
 BEFORE AFTER Superplasticizer added
 BEFORE AFTER Fibre Reinforcement added

On-Site Details and Tests for Shotcrete:

Batch #	Location		Mix Design #	Batch Times			Spray Time		Quantity Used (m ³)	Water/Cement Ratio	Sample (As-Shot) Air Content (%)	Slump (wet mix) (mm)	Re-bound est. (%)	Average Thickness of In-Place Shotcrete (mm)	Test Panel No.
				From Plant	On-Site		Start	End							
	From	To													

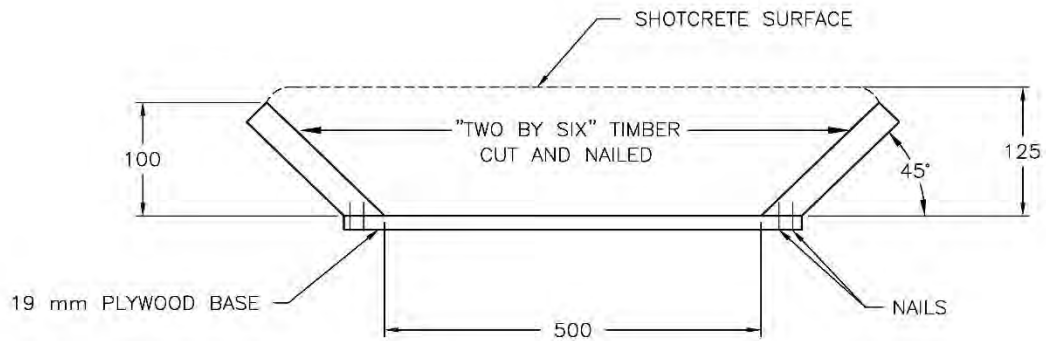
Comments: _____

Certified Correct: _____
 (Contractor's Consultant's Representative) (Title)

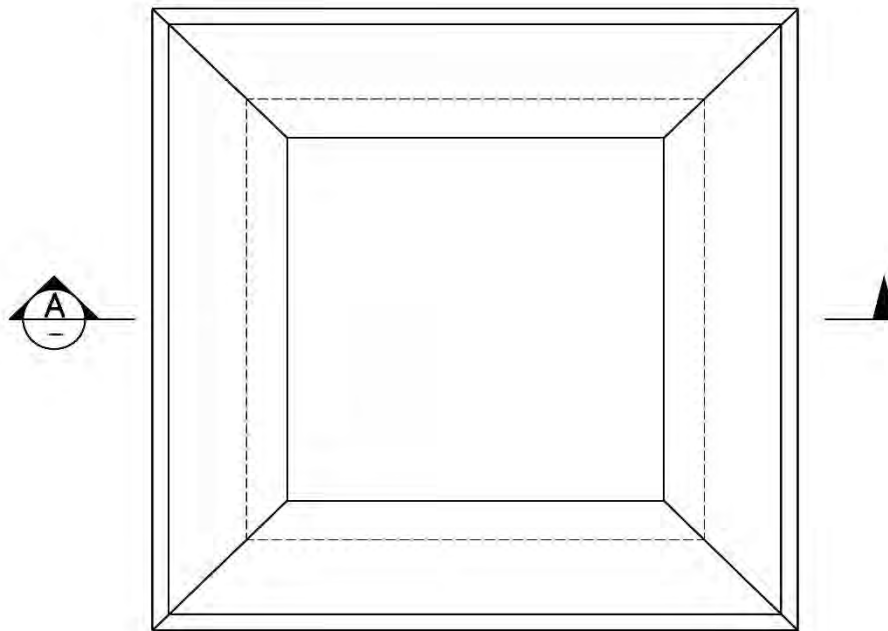
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SHOTCRETE TEST PANEL FORM

SP209-01



SECTION (A)



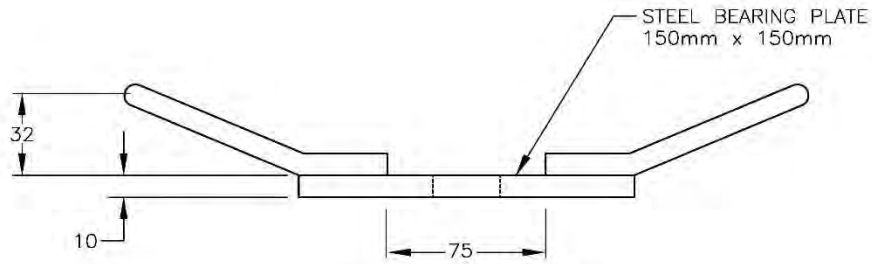
PLAN VIEW

NOT TO SCALE

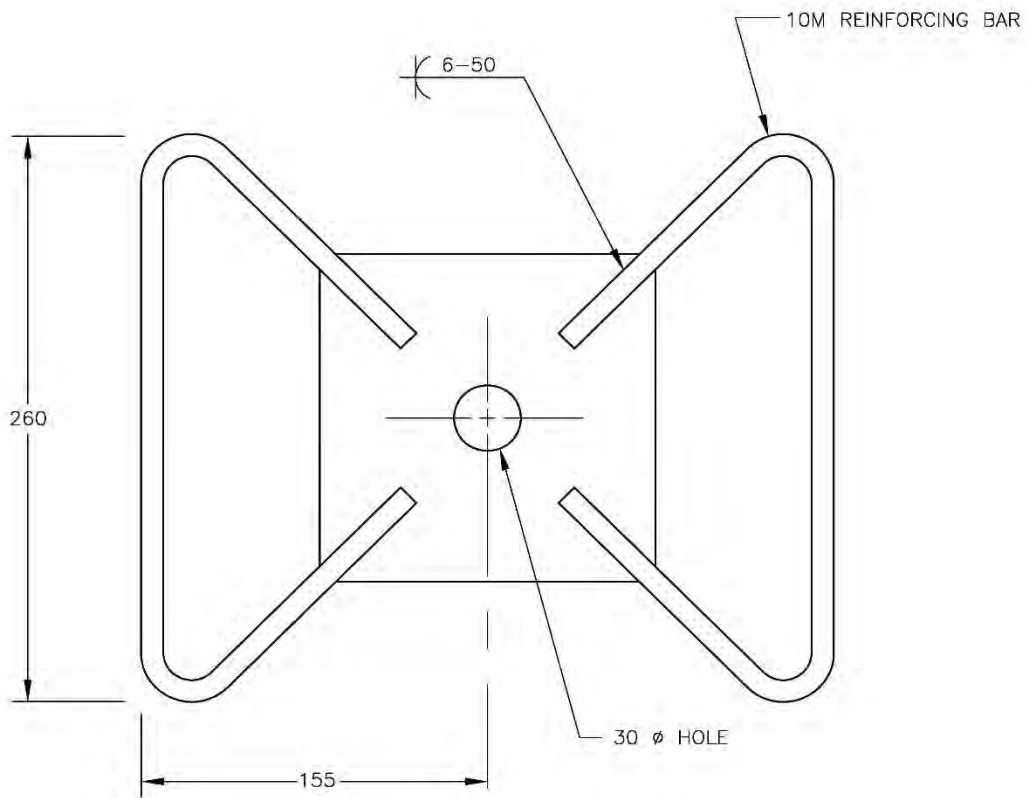
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

SHOTCRETE SPIDER PLATE DETAIL

SP209-02



ELEVATION VIEW



PLAN VIEW

NOT TO SCALE

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

SECTION 211

PORTLAND CEMENT CONCRETE

211.01 General – This Section describes the requirements for Portland Cement Concrete. Requirements include quality control and quality assurance, constituent materials, mix design proportioning, sampling and testing of the materials and concrete, batching, mixing, transporting, placement, finishing and curing. The Contractor shall supply all necessary materials.

211.02 Applicable Standards – The standards listed in Table 211-A shall apply unless specified otherwise herein or in the Special Provisions or Drawings. All referenced standards shall be to the current editions at time of Award.

Table 211-A: Applicable Standards

CSA_A23.1	Concrete materials and methods of concrete construction
CSA_A23.2	Test methods and standard practices for concrete
CSA_A3000	Cementitious Materials Compendium
CAN/CSA-S269.1	<u>Falsework and formwork</u>
CSA_A283	Qualification Code for Concrete Testing Laboratories

211.03 Concrete Mix Designs and Submission Requirements

211.03.01 General – The Contractor shall be responsible for quality control of all components of the concrete operation, including but not limited to, aggregate and component quality, proportioning, test batching, batching, mixing, transporting, placing, consolidating, finishing, curing, and all necessary quality control and verification testing of the components and the fresh and hardened concrete.

211.03.02 Proportioning – The Contractor shall be responsible for proportioning and designing all concrete in full compliance with the required concrete mix parameters as listed in the project Special Provisions or Drawings. All mix designs shall use the “Absolute-Volume Method” for mix proportioning.

211.03.03 Alkali-Aggregate Reactivity (AAR) Preventative Requirements – Contrary to CSA A23.2-27A, the total alkali content (Na₂O equivalent) contributed by the Portland cement to the concrete mix shall not exceed;

- 2.4 kg/m³ for aggregates classed as moderately reactive.
- 1.8 kg/m³ for aggregates classed as highly reactive.
- Aggregates classed as extremely reactive shall not be used.

The Ministry Representative reserves the right to alter this limit or to redefine the concrete mix design parameters based on the review of the mix design submittal.

Unless otherwise specified the required service life of the concrete structure or element shall be 75 years.

211.03.04 Mix Design Submittals – The Contractor shall submit a report outlining the proposed mix design for each classification of concrete to the Ministry Representative for review and acceptance at least 2 weeks in advance of when concrete production is scheduled to commence. Acceptance of the mix design by the Ministry does not constitute acceptance of the concrete. Acceptance of the concrete will be based on the test results and the performance and quality of the concrete and concrete components placed in the Work. No concrete shall be placed prior to receiving Ministry acceptance of the mix design.

Each mix design submittal shall include all of the following (Incomplete submittals by the Contractor may result in rejection of the mix design submittal):

(a) Project Specifics

- (i) Name and location of the proposed supplier.
- (ii) Distance and expected travel time from batch plant location to project Site.
- (iii) Expected method of batching, transporting and placing concrete.
- (iv) Specified mix parameter requirements as listed or referenced in the project’s Special Provisions or Drawings.
- (v) Name and contact information of certified quality control testing laboratory and certified testing personnel. Certification shall be in accordance with the requirements of CSA A23.1.

(b) Materials – All materials shall comply with the requirements as listed in SS 211.04 of this Standard Specification.

(i) **Cementitious Materials** – Types and source of each material including mill test reports and manufacturer’s certificates of compliance.

(ii) **Aggregates** – Type and source of all individual aggregate products including individual gradations and all other aggregate quality testing as described in Table 211-B for each product and each aggregate source.

**Table 211-B: Required Aggregate Testing for Normal Density Coarse and Fine Aggregate
(Per Individual Product & Aggregate Source)**

Test Method	Test Description	Test Data Validity ⁽⁵⁾
CSA A23.2-2A ⁽¹⁾ CSA A23.2-5A ⁽¹⁾	Sieve Analysis of Fine and Coarse Aggregate Amount of Material finer than 80 µm in Aggregate	Within 90 days ⁽⁶⁾ Within 90 days ⁽⁶⁾
CSA A23.2-3A	Clay Lumps in Natural Aggregate	Within 3 years
CSA A23.2-3B	Determination of total or water-soluble sulphate ion content of soil	Within 3 years
CSA A23.2-4A	Low Density Granular Material in Aggregate	Within 3 years
CSA A23.2-4B ⁽¹⁰⁾	Sampling and determination of water-soluble chloride ion content in hardened grout or concrete	Within 3 years
CSA A23.2-6A, 12A	Relative Density and Absorption of Fine and Coarse Aggregate	Within 1 year
CSA A23.2-7A	Test for Organic Impurities in Fine Aggregates for Concrete	Within 2 years
CSA A23.2-8A ⁽³⁾	Measuring Mortar-Strength Properties of Fine Aggregate	Within 2 years
CSA A23.2-9A ⁽²⁾	Soundness of Fine and Coarse Aggregate by Use of Magnesium Sulphate	Within 3 years
CSA A23.2-13A	Flat and Elongated Particles in Coarse Aggregate - Procedure A (Length to Width Ratio 4:1)	Within 3 years
CSA A23.2-14A ⁽⁹⁾	Potential Expansivity of Aggregates (Procedure for Length Change Due to Alkali-Aggregate Reaction in Concrete Prisms at 38°C)	Within 3 years
CSA A23.2-15A ⁽⁴⁾	Petrographic examination of aggregates	Within 3 years
CSA A23.2-16A ⁽⁷⁾	Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine	Within 3 years
CSA A23.2-23A ⁽²⁾	Test Method for the Resistance of Fine Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus	Within 3 years
CSA A23.2-24A ⁽⁸⁾	Test Method for the Resistance of Unconfined Coarse Aggregate to Freezing and Thawing	Within 3 years
CSA A23.2-25A ⁽¹¹⁾	Test Method for Detection of Alkali-Silica Reactive Aggregates by Accelerated Expansion of Mortar Bars	Within 3 years
CSA A23.2-29A ⁽²⁾	Test Method for the Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus	Within 3 years

- Each aggregate product that may be used as a component of the proposed mix must have a washed sieved analysis performed showing the cumulative percent passing for each of the following sieve sizes; 56, 40, 28, 20, 14, 10, 5, 2.5, 1.25, 0.630, 0.315, 0.160 and 0.080 mm. Sieve analysis reports are to include all relevant sampling information (date/sampled by/location/field sample size) and include the actual test specimen sample mass.
- Subject to approval by the Ministry Representative, Test Method A23.2-9A can be performed as an alternative to performing Test Method A23.2-23A and A23.2-29A. Refer to CSA A23.1, Section 4.2, Table 12 for further information.
- Test Method A23.2-8A is required only if the aggregate fails to meet the requirements of Test Method A23.2-7A as described in CSA A23.1 Section 4.2.3.3.2
- Petrographic Examination of Aggregates: The Contractor shall submit the results of the Petrographic Analysis, certified by a qualified professional registered with the Association of Professional Engineers and Geoscientists of the Province of B.C. and indicating the aggregate's suitability for its intended use. Test shall be

performed on coarse and fine aggregates. The petrographic analysis shall not be used to forfeit the requirement of performing other necessary aggregate quality testing.

- Test data validity requirements are based on time periods prior to the expected initial date of concrete production. The Ministry Representative, based on submitted test data and/or visual observations, may waive or alter the above listed time periods. For multiyear projects, test data validity shall be maintained relative to the date of concrete placement.
- Quality control during aggregate production shall include the following test methods at the specified test frequencies: CSA A23.2-2A & A23.2-5A (1 per 300 tonnes). Testing shall be completed on a blend of aggregate reflecting the mix design.
- Subject to approval by the Ministry Representative, Test Method CSA A23.2-16A can be waived if CSA A23.2-29A test results meet the specified criteria.
- This test shall be required should any of Test Methods 4A, 9A, 16A or 29A fail to satisfy the allowable limits. Test shall be performed on

both coarse and fine aggregates or a blend of aggregates representative of the proposed mix.

9. Testing is waived if CSA A23.2-25A test results classify the aggregates as non-reactive.

- (iii) **Admixtures** – Type and source of all admixtures, including expected dosage rates, point of addition to the mix, compatibility documentation, and individual technical data sheets.
- (iv) **Water** – The source of mixing water. (depending on the source, the Ministry Representative may request additional test documentation).
- (v) **Miscellaneous** – Documentation for all other materials proposed for the mix, showing conformance with applicable Ministry requirements and manufacturer/industry guidelines and standards.

(c) Proportioning

- (i) The concrete proportions expressed in terms of quantity of each component.
- (ii) Fresh and hardened concrete properties.
- (iii) The mass of materials in kilograms (kg) and the absolute volume in cubic metre (m³) contributed by each material in the mixture.
- (iv) The quantity of each individual cementitious material added in a powder state shall be expressed as kilograms per cubic metre (kg/m³).
- (v) Materials added in slurry shall have their respective solid and water contents expressed as kilograms per cubic metre (kg/m³).
- (vi) The quantity of each separately batched size of coarse aggregate and fine aggregate shall be expressed as kilograms per cubic metre (kg/m³) in a saturated-surface-dry (SSD) condition.
- (vii) The quantity of water shall be the maximum amount allowed based on the maximum specified water-to-cementitious material ratio (W/C_m), corrected to account for any additional water that may be contributed by other materials used in the proposed mix.
- (viii) The quantity of water shall be expressed as kilograms (or Litres) per cubic metre (kg/m³ or L/m³). If ice is used for temperature control, it shall be expressed in incremental units (sacks, bags, or pails, etc.) and kilograms per cubic metre (kg/m³) and shall be included in the maximum allowable water quantity.
- (ix) Admixtures dispensed as liquids shall be expressed as Litres per cubic metre (L/m³) and where applicable, expected dosage range stated.
- (x) The quantity of any pre-measured, pre-packaged additives, such as fibres, shall be expressed in

10. ASTM C1218 may be used in place of A23.2-4B

11. Test shall be performed on both coarse and fine aggregates or a blend of aggregates representative of the proposed mix

incremental units (sacks, bags, or pails, etc.) and kilograms per cubic metre (kg/m³).

- (xi) The absolute volume of each material, air content and the total sum of the absolute volumes of all materials shall be provided in cubic metres (m³).
- (xii) Slump, unit weight, air content, strength characteristics, and any other specified characteristic shall be reported for each set of mixture proportions intended for use.

211.03.05 Trial Mixes – The Contractor shall produce test batches of 4.0 m³ volumes on all proposed bridge deck mix designs. The Contractor shall produce test batches of 4.0 m³ volumes on other mix designs where deemed necessary by the Ministry Representative.

Point of discharge sampling and testing shall be carried out by the Contractor to verify the pertinent parameters of the proposed mix design. If placement is by pump, concrete shall be sampled and fully tested at both the truck chute discharge and at the pump discharge locations. Sampling and testing shall be in accordance with SS 211.09 and [Table 211-C](#).

For concretes containing superplasticizers or high range [water reducing admixtures](#) or supplementary cementitious materials then sampling and testing in accordance with [Table 211-D](#) shall also apply.

Test parameters include all required properties of the fresh and hardened concrete, workability and finishability of the mix, strength gain characteristics, including 7 and 28-day compressive strengths, and any other additional testing of the hardened concrete that may be required by the Ministry Representative. Batching, haul times and conveyance procedures used for the test batch must emulate the procedures expected during the actual concrete placement.

The Contractor shall be fully responsible for all costs associated with the test batches. Test batch trials that do not meet requirements shall be repeated.

211.03.06 Adjustments to Mix Designs – If, during the progress of the work, the mix design is found to be unsatisfactory, the Contractor shall make the necessary adjustments. Adjustments shall be accepted by the Ministry Representative prior to subsequent concrete placement. Notwithstanding the Ministry Representative's review and acceptance of the proposed mix design, it remains the Contractor's responsibility to ensure that the concrete meets all Contract requirements.

Table 211-C: Standard Test Methods for Sampling and Testing

Test Methodology	Test Description	Minimum Test Frequency
CSA A23.2-1C	Sampling Plastic Concrete	As stated below
CSA A23.2-3C	Making and Curing Concrete Compression and Flexural Test Specimens	One set for every 35 m ³ or portion thereafter for each classification of concrete placed within an individual structural element or component, and placed on any one calendar day from a single supplier. The Ministry Representative may request additional sets.
CSA A23.2-9C	Compressive Strength of Cylindrical Concrete Specimens	
CSA A23.2-4C	Air Content of Plastic Concrete by the Pressure Method	Every individual load unless approved otherwise by the Ministry Representative Retests shall be performed following any load adjustments.
CSA A23.2-5C and 19C	Slump and Slump flow of Concrete	
ASTM C1064	Temperature of Fresh Concrete	
CSA A23.2-6C	Density and Yield of Plastic Concrete	Density and Yield tests shall be performed with every air test and strength test.

Table 211-D: Additional Test Methods

Test Methodology	Test Description	Minimum Test Frequency
ASTM C457 ⁽¹⁾	Microscopic Determination of Parameters of the Air-Void System in Hardened Concrete	One test per individual test batch for deck concrete. Acceptance criteria shall be in accordance with CSA A23.1 Section 4.3.3.2 and 4.3.3.3. The procedure used shall be reported. Further testing, during scheduled concrete pours may be required, as directed by the Ministry Representative.
ASTM C1202 ⁽¹⁾	Electrical Indication of Concrete's Ability to Resist Chloride Ion Penetration (Test age at 91 days)	One test per individual test batch for deck concrete. Acceptance criteria shall be a coulomb rating not exceeding 1500 coulombs. Further testing, during scheduled concrete pours may be required, as directed by the Ministry Representative.
ASTM C157 ⁽¹⁾	Length Change of Hardened Hydraulic- Cement Mortar and Concrete (Drying shall commence after 7 days of curing and shrinkage determined after 28 days of drying)	One test per individual test batch for deck concrete. Sampling to be at final point of discharge.

(1) Testing Requirement may be waived at the discretion of the Ministry Representative.

SECTION 211

PORTLAND CEMENT CONCRETE

MATERIALS

211.04 Materials

211.04.01 General – All material properties used in concrete production shall conform to CSA A23.1, unless otherwise specified in these specifications. Laboratory testing shall be carried out by a testing laboratory certified in accordance with the requirements of CSA A283. Field test procedures shall be undertaken by experienced personnel certified in accordance with CSA A23.1.

211.04.02 Cementitious Material – The Contractor shall obtain, from the supplier, a mill test report and a manufacturer's certificate of compliance representing each type of cementitious material for each month period representing the delivered shipment.

(a) **Portland Cement** – Unless otherwise specified, the Contractor shall use Type GU (General Use) or Type GUL (General Use Limestone) Cement in all concrete. Cement shall conform to the requirements of CSA A3000. Cement shall have a C3A content greater than 4.0% (by standard Bogue calculation) or 5% by mill certificate, and GUL cement shall have a Blaine Fineness of less than 480 m²/kg.

The total alkali content [Na₂O equivalent, (Na₂O + (0.658 x K₂O))] of the Type GU Portland Cement shall not exceed 0.60% by mass.

(b) **Silica Fume** – Condensed silica fume shall conform to the requirements of CSA A3000 for Type SF silica fume. Silica Fume application rates shall be 5% to 8% by mass of cementitious materials.

(c) **Fly Ash** – All fly ash shall conform to the requirements of CSA A3000 for Type F fly ash. Fly ash application rates shall not exceed 25% by mass of cementing materials in substructure concrete. Unless specified, fly ash shall not be used in bridge deck concrete without the acceptance of the Ministry Representative. Upon such acceptance the application rate shall not exceed 15% by mass of cementitious material.

For mixes containing both Type F fly ash and silica fume the combined mass of cementitious materials shall be 15% to 20%, with each type within;

- Type F fly ash by mass of cementitious material between 10% to 15%,
- Silica fume by mass of cementitious material between 5% to 8%,

(d) **Blended Hydraulic Cement** – Blended Hydraulic Cement shall not be used without the acceptance of the Ministry Representative.

211.04.03 Water – Water to be used for mixing concrete or mortar and for curing shall conform to the requirements of CSA A23.1 and shall be free from contamination by oil, acid, alkali, organic matter, sediment or other deleterious substances. The

Contractor shall not use water from shallow, stagnant or marshy sources.

211.04.04 Admixtures – All admixtures shall conform to CSA A23.1.

Admixtures shall not be incorporated into the mix design and/or added to the concrete without the acceptance of the Ministry Representative.

Unless otherwise specified, all concrete shall contain Air-Entraining Admixtures.

Air-Entraining Admixtures shall conform to ASTM_C260.

Chemical admixtures shall conform to ASTM C494.

Type F High Range Water Reducer (superplasticizer) shall be used when concrete contains silica fume.

All accepted admixtures shall be compatible with all other admixtures and constituents used in the mix.

Type B, Retarding or Type D, Water-Reducing and Retarding (Hydration Stabilizing) Admixtures shall not be incorporated into the mix design and/or added to the concrete without the acceptance of the Ministry Representative. When accepted by the Ministry Representative, the guidelines given in SS 211.08.02 shall apply.

Calcium chloride or any admixture containing chloride ions shall not be used. If requested, a written statement from the manufacturer stating that the admixture contains no intentionally added calcium chloride shall be provided to the Ministry Representative.

211.04.05 Aggregates – All aggregates are to be natural, processed or manufactured granular material composed of hard, sound and durable particles, free of adherent coatings, shale, clay, organic materials and other soft or disintegrated pieces.

(a) **Coarse Aggregate** – All coarse aggregate materials shall conform to the following requirements:

(i) Sampling shall be according to CSA Test Method A23.2-1A.

(ii) Gradation shall be determined in accordance with CSA Test Method A23.2-2A and 5A. (All gradations shall be based on a washed sieve analysis, see Table 211-B Footnote 1).

(iii) Gradation of the coarse aggregate shall be within the limits shown in Table 211-E for the particular size range or nominal maximum size of coarse aggregate specified.

Table 211-E: Coarse Aggregate Gradations

NMSA ⁽¹⁾ mm	Product Size mm	Total Passing Each Sieve (Percentage by Mass)								
		56 mm	40 mm	28 mm	20 mm	14 mm	10 mm	5 mm	2.5 mm	1.25 mm
40	40 – 5 ⁽²⁾	100	95-100	–	35 - 70	–	10 - 30	0 - 5	–	–
28	28 – 5 ⁽²⁾	–	100	95 - 100	63 - 83	30 - 65	–	0 - 10	0 - 5	–
20	20 – 5	–	–	100	90 - 100	50 - 90	25 - 60	0 - 10	0 - 5	–
14	14 – 5	–	–	–	100	90 - 100	45 - 75	0 - 15	0 - 5	–
10	10 – 2.5	–	–	–	–	100	85 - 100	10 - 30	0 - 10	0 - 5

(1) NMSA – nominal maximum size of coarse aggregate. Definition: The standard sieve size opening immediately smaller than the smallest through which all of the aggregate must pass.

(2) To prevent segregation, aggregates that make up the above gradings shall be stockpiled and batched in two or more separate sizes as per CSA A23.1

Table 211-F: Limits for Deleterious Substances In and Physical Properties of Aggregates

Test Method	Substance or Property	Maximum Percentage by Mass of Total Sample	
		Coarse Aggregate	Fine Aggregate
CSA A23.2-3A	Clay Lumps	0.30	1.0
CSA A23.2-4A	Low-Density Granular Materials	0.5	0.5
CSA A23.2-5A	Material Finer than 0.08 mm for Concrete Subject to Wear	1.0	3.0
	Material Finer than 0.08 mm for Other Concrete	1.0	5.0
CSA A23.2-9A	Magnesium Sulphate Soundness Loss	12	16
CSA A23.2-16A	Los Angeles Abrasion Loss for Concrete Subject to Wear	50 (35 for all deck concrete)	N/A
CSA A23.2-23A and -29A	Micro-Deval Abrasion Loss	17	20
CSA A23.2-12A	Absorption Percentage	1.75	N/A
CSA A23.2-7A	Organic Impurities in Fine Aggregate	N/A	Standard colour or lighter
CSA A23.2-13A	Flat and Elongated Particles in Coarse Aggregate - Procedure A (Length to Width Ratio 4:1)	20	N/A
CSA A23.2-24A	Test Method for the Resistance of Unconfined Coarse Aggregate to Freezing and Thawing	6	N/A

- (iv) The gradation of coarse aggregates used shall be such that the percentage passing any one sieve does not vary more than 5% from the initially approved coarse aggregates used in the mix design.
 - (v) The maximum Petrographic Number (PN) of coarse aggregate shall be determined in accordance with CSA A23.2-15A and shall not exceed 125.
 - (vi) The amount of deleterious substances in coarse aggregate shall not exceed the limits prescribed in Table 211-F.
 - (vii) Other specified physical properties measuring aggregate quality referenced in Table 211-B shall not exceed the limits prescribed in Table 211-F.
- (b) Fine Aggregate** – Fine aggregate shall conform to the following requirements:
- (i) Sampling shall be according to CSA Test Method A23.2-1A.
 - (ii) Gradation shall be within the limits given in Table 211-G when tested in accordance with CSA Test Method A23.2-2A and 5A. (All gradations shall be based on a washed sieve analysis).
 - (iii) Fineness modulus shall be between 2.3 and 3.1.
 - (iv) Fineness modulus tolerance shall be ± 0.2 (except that the above sentence shall apply) from the fineness modulus of the initially approved fine aggregate used in the design mix.
 - (v) A maximum 45% shall be retained between any two consecutive sieves.
 - (vi) The amount of deleterious substances in fine aggregate shall not exceed the limits prescribed in Table 211-F.
 - (vii) Other specified physical properties measuring aggregate quality referenced in Table 211-B shall not exceed the limits prescribed in Table 211-F.

211.04.06 Steel Fibres – Steel fibres shall meet the requirements of ASTM Standard A820 Type 1 Cold Drawn High Tensile Deformed Steel Fibres. A mill certificate showing compliance with ASTM Standard A820 shall accompany each delivered lot.

211.05 Storage and Handling of Materials - General – The storage and handling of all materials used in the production of concrete shall conform to CSA A23.1, unless otherwise specified herein. All materials shall be stored and handled in a manner that will prevent contamination or deterioration, otherwise they shall be rejected. Access shall be provided to the storage facilities to allow for inspection and sampling. Where applicable, materials shall be identified by MSD sheets.

Table 211-G: Fine Aggregate Gradation

Sieve Size (mm)	Total Cumulative Passing Each Sieve (Percentage by Mass)
10	100
5	95 - 100
2.5	80 - 100
1.25	50 - 90
0.630	25 - 65
0.315	10 - 35
0.160	2 - 10

211.05.01 Cementitious Material – All cementitious materials shall be stored in a suitable weather-tight structure capable of protecting the materials from dampness, hydration and contamination, otherwise they shall be rejected. Materials shall be free of lumps at all times during their use in the work.

211.05.02 Tanks – Tanks used for storing and/or hauling, concrete mixing and curing water shall be free from contamination by oil, acid, alkali, organic matter, sediment, rust or other deleterious substances.

211.05.03 Admixtures – All admixtures shall be stored in manufacturers' original labelled containers with clearly legible labels and be kept above freezing at all times and in accordance with the manufacturers' technical data sheets. Admixtures damaged by any cause shall be rejected.

211.05.04 Other Materials – Other materials, such as steel fibres, etc. shall be stored and handled in accordance with the manufacturer's recommendations and instructions.

211.05.05 Aggregates – Each nominal size of aggregate, including coarse and fine shall be stockpiled separately, stored and handled in a manner that will prevent contamination, intermixing and segregation. A minimum of 10 m³ of each aggregate size, in excess of the requirements of the daily concrete, shall be maintained in identifiable stockpiles at the batch site in order to ensure proper continuity of the work with approved aggregates.

Stockpiled aggregate, which is segregated, contaminated or intermixed with foreign matter of any kind, shall be rejected. Separate batch plant storage bins shall be provided for each size of aggregate to be batched.

Precautions shall be taken when moving, handling and transferring aggregates to prevent contamination, segregation or degradation in accordance with the following recommendations:

- (a) Aggregate blanket 150 mm thick should be left on the ground as stockpiles are depleted.

- (b) Aggregate transfer over stockpiles or storage bins of other sized aggregates should be avoided.
- (c) Storage bins should be filled only to 150 mm from the top unless a suitable extension rim is provided to prevent contamination.
- (d) Front-end loader buckets should be filled by raising them vertically up the face of the stockpile, not by thrusting into the stockpile in one place.
- (e) Depositing aggregates into the batch plant storage bins should be made directly over the bins' outlet.
- (f) Storage bins should be kept as full as possible during batching to minimize segregation as the aggregates are withdrawn from the bins.
- (g) Acceptable and uniform aggregate moisture content shall be obtained and maintained by necessary means to include the following:
 - (i) Washed or wetted aggregates should remain in stockpiles for a minimum of 12 hours to permit a uniform moisture content throughout the stockpile to be reached before aggregate transfer to batch plant storage bins.
 - (ii) After each day's concrete production during wet weather, fine aggregate bins should be either emptied or covered with a suitable waterproof covering.

CONSTRUCTION

211.06 Batching

211.06.01 Batching Equipment – Batching equipment shall comply with the requirements of CSA A23.1.

- (a) **General** – Batching equipment, such as weighing mechanisms, gates, water lines and dispensing systems, to be maintained in good working order. Batching bins to be completely emptied of all material before the first and succeeding batches are measured.

Batching equipment to have the following capabilities and facilities:

- (i) bins or silos for storage of aggregates, silos for storage of cement and pozzolan.
- (ii) free movement of each type or size of material to discharge openings without contamination.
- (iii) design of storage and hauling facilities to prevent:
 - loss or intermingling of different sizes and types of materials.
 - contamination by deleterious substances.
 - harmful segregation and breakage.
- (iv) covers to stockpiles of fine aggregates to ensure constant moisture content.

- (v) all fulcrums, clevises and similar working parts maintained in clean condition.
- (vi) provision made, in remote control plants and/or where batch cycles are timed, for the complete filling and discharge of the measuring unit for each batch.

- (b) **Batch Charging Mechanism** – Batch Charging Mechanism requirements to be as follows:

- (i) material flow control within the specified tolerances.
- (ii) hopper construction to eliminate material accumulation and to permit the complete discharge of every batch.
- (iii) cement weighing hopper:
 - self-cleaning and properly ventilated to allow air to escape.
 - accessible for inspection.
 - dust sealed between the charging mechanism and the hopper to maintain weighing accuracy.

- (c) **Scales and Dispensing Mechanism** – Scales and Dispensing Mechanisms to conform to the following requirements:

- (i) beam type or springless dial type.
- (ii) other methods of weighing (electric, hydraulic, load cells, etc.) are acceptable provided the specified weighing tolerances and accuracy requirements are met.
- (iii) all accurate to $\pm 0.4\%$ of the total capacity of the scale when static load tested.
- (iv) zero balance adjustment capability.
- (v) unaffected by binding or vibration due to vibrators or other appurtenances.
- (vi) working range between 10% and 90% of the scale reading.
- (vii) equipped with prominent markers with individual batch weight setting capabilities for dial scales.
- (viii) calibration facilities including an adequate number of standard test masses.
- (ix) beam type scales with provision for indicating to the operator that the required load in the hopper is being approached; the device shall indicate at least the last 100 kg of the load.
- (x) all weighing and indicating devices to be in full view of the operator while charging the hopper, and the operator shall have convenient access to all controls.

(d) Volumetric Devices for Water Measurement – Volumetric devices for water measurement to be:

- (i) fitted with such valves and connections as are necessary to divert the water measured for a batch to easily verify the accuracy of measurement.
- (ii) arranged so that measurements will be unaffected by variable pressures in the water supply line.
- (iii) calibrated to an accuracy of 2% of the batch volume.

(e) Dispensing Systems for Liquid Admixtures – Dispensing systems for liquid admixtures, other than weigh systems, to have the following capabilities:

- (i) visual, volumetric measuring or readout units located as close as possible to eye level for easy reading by the operator.
- (ii) positioned in such a manner that discharge into the batch is observed by the operator from a normal working position.
- (iii) volumetric measuring unit for periodic check of dispenser accuracy where a positive displacement system is used.
- (iv) visual measuring unit for an accurate visual check of increments of 30 mL of air entraining agent or of chemical required to treat a maximum of 25 kg of cement.

(f) Certificate or Report of Inspection – A certificate or report of inspection, from a Weights and Measures approved, independent testing authority, and which is not more than one year old for the plant in its present position, shall be on display in the plant at all times. The certificate or report will be accepted as proof of accuracy of the scales or weighing devices. Where there is reasonable doubt concerning the accuracy of the scales or weighing devices the Ministry Representative may require their recalibration and recertification at the Contractor's expense, during progress of the work. Plant relocation or major alterations shall require such recalibration and recertification.**211.06.02 Batching of Materials** – Material may be weigh-batched separately or cumulatively.

Aggregates shall be batched by mass, cement and mineral admixtures in the powder form by mass or bag.

The mass of any ingredient to be batched shall not be less than 10% nor greater than 90% of the scale capacity.

When batched by 40 kg bags, only full and sealed bags of cement shall be used with no fractional bags permitted. Cement batched by mass shall be to an accuracy of $\pm 1\%$ for batch quantities between 30% and 90% of the scale capacity. When the mass of cement being batched is between 10% and 30% of the scale capacity, the mass of cement batched shall not be less than the required mass nor more than 4% in

excess. Intermediate bulk cement shall be weighed to an accuracy of $\pm 1\%$.

When aggregates are measured by mass, batch masses shall be based on the required mass of saturated surface dry aggregate corrected for the moisture conditions of the aggregate at the time of batching.

Field determination of free moisture in the aggregates shall be made by any proven method such as CSA A23.2-11A or by the use of a device such as the "Speedy Moisture" gauge. When doubt exists as to the accuracy of such methods, total moisture in the aggregate shall be determined in accordance with ASTM C566.

When individual aggregate weigh batchers are used, the scale reading for each material shall be within 2% of the specified mass.

In a cumulative aggregate weigh batcher, the cumulative mass after each measurement shall be within 1% of the required cumulative amount when the scale is used in excess of 30% of its capacity. For cumulative measurement less than 30% of scale capacity, the allowable variation shall be $\pm 0.3\%$ of scale capacity or $\pm 3.0\%$ of the required cumulative mass, whichever is less.

Mixing water shall be measured by mass or volume to an accuracy of $\pm 1\%$. Adjustments for free water contained in batched materials, such as free water in the aggregates, water contained in admixture solutions, shall be made to the quantity of water to be batched. The total amount of mixing water in the batch obtained from all sources shall be within $\pm 3\%$ of the specified quantity and shall not vary the designed water-to-cementitious material ratio of the mix by more than ± 0.02 . Mixers shall be completely emptied of all water prior to the loading of a concrete batch.

Admixtures shall be batched in liquid form by either mass or volume; in powdered form by mass or bag. Volumetric measurements of admixtures or air entraining agents shall be to an accuracy of $\pm 3\%$ of the required amount or 30 mL, whichever is greater. Measurement by mass shall be to an accuracy of $\pm 3\%$ of the required amount.

The addition of cement to a fully-batched load of aggregates will not be accepted.

211.07 Mixing

211.07.01 General – Sufficient plant capacity and transporting equipment shall be provided by the Contractor to ensure continuous delivery of concrete at the rate required with the necessary intervals between batches, for the proper placing and finishing of the concrete without the formation of cold joints in the finished concrete.

211.07.02 Concrete Mixers

(a) General – Concrete mixers shall conform to the following requirements:

- (i) manufacturer's rating plate carried in a prominent position, indicating rated mixing capacity, and

recommended speed of mixing which must be in the range of not less than 4 nor more than 18 rpm.

- (ii) demonstrated capability of operating satisfactorily at the recommended mixing speed.
 - (iii) number of revolutions of the drum or blades registered with a counter in working condition.
 - (iv) charge and discharge openings and chute free from appreciable accumulations of cement or concrete, and hopper and chute surfaces clean and smooth.
 - (v) batch water measuring equipment in good operating condition.
- (b) **Volumetric Mobile Mixers** – Volumetric mobile mixers shall conform to the following requirements:
- (i) shall be auger type mixers.
 - (ii) the mobile mixers shall be calibrated with the actual cement and aggregate products used in the concrete mix. Calibration tables and curves shall be submitted to the Ministry Representative. The volume of the concrete produced shall be confirmed and shall be within 2%.
 - (iii) mobile mixers shall have separate flow controlled supply hoppers for each individual aggregate gradation and cementitious constituent material to be added volumetrically.
 - (iv) may be used provided that the accuracy of batching and uniformity of concrete is as required by CSA A23.1.
 - (v) cement flow rates shall be checked before (during if required) each placing operation.
 - (vi) the scale being used to calibrate the mobiles shall have a minimum capacity of 50 kg.
 - (vii) the water tank shall have a stand pipe showing the amount of water in the tank.
 - (viii) the mobiles shall have individual flow meters and admixture tanks for each admixture to be used as part of an admixture injection system.
 - (ix) the mobiles shall be able to advance backwards and forwards from controls located at the back of the mobile.
 - (x) the mixer auger shall be maintained at minimum of a 25° angle from horizontal when mixing and discharging, unless otherwise indicated by the manufacturer.

211.07.03 Mixing Concrete – Mixing drums shall be clean and empty before being charged. The drum shall be rotated at the manufacturers' recommended mixing speed during charging and mixing. Concrete shall be mixed to the uniformity requirements of CSA A23.1, Section 5.2.4.5. When a satisfactory mixing time is established it shall be maintained for all batches of the same design and mixed with the same equipment.

211.08 Delivery

211.08.01 General – Off-site mixed concrete shall, after being mixed, be transported to the Site in either truck agitators or truck mixers operating at the speed designated by the manufacturer of the equipment as agitating speed.

When concrete is being mixed in mixer trucks while in transit, the speed of the mixing drum shall be reduced to agitating speed once the mixing time has elapsed. Delivery of mixed concrete in non-agitating equipment will not be permitted.

211.08.02 Time of Placement – Time of placement guidelines for bridge decks including diaphragms, bridge parapets, bridge medians, bridge sidewalks and approach slabs are given in SS 413, Bridge Decks and Concrete Overlays.

All other concrete shall be fully discharged and placed within 90 minutes after water and cement have been combined. Under conditions contributing to rapid stiffening of concrete the Ministry Representative may require a time of less than those stated above.

Modifications to the time of placement will be considered on a case by case basis and may require the use of Hydration Stabilizing Admixtures (HSA's).

(a) **Hydration Stabilizing Admixtures (HSAs)** – HSAs shall only be used if approved in advance by the Ministry Representative.

If accepted by the Ministry Representative, the use of HSA's shall be as follows:

- (i) HSAs shall conform to the requirements of ASTM C494:
 - Type B, Retarding; or
 - Type D, Water-Reducing and Retarding Admixtures.
- (ii) Concrete shall be fully discharged and placed within 3 hours after water and cement have been combined.
- (iii) When HSA's are used, these time extensions are subject to preconstruction trials being conducted by the Contractor to establish the appropriate HSA dosage to provide suitable extended slump life of concrete without increasing the water/cementitious materials ratio of the concrete above that which would be required if HSA's were not used.
- (iv) The use of HSA's shall in no instance modify the maximum concrete temperature required at time of placement.
- (v) A one-time only addition of HSA will be allowed; during initial batching of the concrete or immediately at completion of batching as recommended by the admixture manufacturer. Addition of HSA at any other time will be cause for rejection of the concrete.

211.08.03 Temperature Control – Temperature control guidelines for bridge decks including diaphragms, bridge parapets, bridge medians, bridge sidewalks and approach slabs are given in SS 413, Bridge Decks and Concrete Overlays.

The temperature of all other concrete, from the time of batching to complete discharge shall remain between 10°C and 25°C. Any concrete containing silica fume shall not have a maximum temperature in excess of 20°C at time of placement. Concrete used in elements having a minimum section thickness greater than 1000 mm shall have temperature controls as per CSA A23.1. Methods used for maintaining temperature controls shall not diminish the quality of the concrete, and shall not alter nor exceed the specified maximum W/C_m ratio.

When the atmospheric temperature is 25°C or higher, SS 211.18 – Hot Weather Concreting shall apply.

211.08.04 Addition of Water – No water shall be added after the initial introduction of the mixing water for the mix, except at the start of discharge within the initial 10% of the total load volume, and when:

- the specified water-to-cementitious materials ratio is not exceeded.
- the measured slump is less than that specified.
- no more than 60 minutes have elapsed after water and cement have been combined.
- the concrete is in a mixer or truck mixer.

An amount not exceeding 16 L of water per cubic metre of concrete or 10% of the mix design water whichever is less may be added. After addition of water, the drum or blades shall be turned an additional 30 revolutions or more if necessary at mixing speed. The amount of water added shall be recorded on the delivery ticket. Water shall not be added to the batch at any later time. The Contractor assumes full responsibility for the on-site addition of water and its subsequent effect on the quality of the concrete.

211.08.05 Addition of Superplasticizer (High Range Water Reducer - HRWR) – When superplasticized concrete falls below the designated slump, it shall be re-tempered with superplasticizing admixtures only, not water, and shall only receive a maximum of one re-tempering. The amount of superplasticizer added shall be recorded on the delivery ticket. The Contractor assumes full responsibility for the on-site addition of superplasticizer and its subsequent effect on the quality of the concrete. The use of superplasticizers shall not be used for extending the time of placement requirements described in SS 211.08.02.

211.08.06 Control of Air Content on the Site – The air content of the concrete shall, if necessary, be brought up to the specified range by the addition of an air-entraining agent in the field. Mixing shall follow to ensure proper dispersion and upon completion of mixing, the air content shall be retested. The amount of admixture added shall be recorded

on the delivery ticket. The Contractor assumes full responsibility for the on-site addition of the air entraining agent and its subsequent effect on the quality of the concrete.

211.08.07 Delivery Ticket – Concrete delivered to the Site shall be accompanied by a ticket providing the Contractor and Ministry Representative with legible information, as follows:

- serial number of ticket
- date, name and location of concrete supplier.
- name of the Contractor.
- specific job designation (name and location).
- specific designation of concrete mix design.
- truck number and volume of concrete in cubic metres.
- addition of any materials added at the Site, at the request of the Contractor including but not limited to water and all chemical admixtures.
- time concrete was batched, arrived at Site, and completely discharged.

211.09 Quality Control Inspection, Testing and Acceptance

211.09.01 General – The Contractor shall be fully responsible for hiring, scheduling, overseeing, performing and documenting all quality control testing and inspection in full compliance with the Contract.

The Ministry may perform on-site sampling and testing as a function of the Ministry's quality assurance and/or audit. Any Ministry quality testing shall not relieve the Contractor of responsibility for providing quality control.

211.09.02 Sampling and Testing – Laboratory testing shall be carried out by a testing laboratory certified in accordance with the requirements of CSA Standard A283. Field test procedures shall be undertaken by experienced personnel. All testing personnel shall be certified under CSA Standard A283 or certified as an ACI Concrete Field Testing Technician – Grade 1 (Minimum). Field inspectors shall have extensive experience in on-site quality control testing of concrete, with the applicable admixtures being used, and with on-site batch adjustments. Testing personnel shall be on-site and available to test concrete for the complete duration of any concrete placement operation. Sampling and testing shall be in accordance with the test methods and minimum frequency levels as listed in Table 211-C and Table 211-D (if applicable). Any test outside the specified range shall be immediately reported to the Contractor and Ministry Representative. Copies of all test results shall be received by the Ministry Representative and the concrete supplier within one (1) day of the test date.

211.09.03 Sampling – Point of Acceptance – Determination of concrete properties for acceptance will be made based on samples taken at the final discharge end of the placement system. The Ministry Representative may accept sampling at the truck discharge end provided that testing during the test

batches confirm that the concrete mix parameters are within the stated specification requirements at the actual placement point. The Ministry Representative may require additional testing to be performed at the point of placement at a predetermined frequency throughout the placement period. Such additional testing, including all costs shall be deemed to be part of the Contractor's quality control program.

211.09.04 Strength Tests – Unless otherwise specified in the Contract, a strength test shall be the average 28-day strength of three standard 100 mm x 200 mm test cylinders, sampled, cast, cured, transported, and tested in accordance with CSA A23.2-3C and -9C. The cylinder size shall however, meet the aggregate size limitations specified in Clause 8.2 of CSA A23.2-3C.

For each strength test, a fourth cylinder shall be cast for a 7-day strength determination, and if requested by the Ministry Representative, a fifth cylinder shall be cast for a 56-day strength determination.

If one specimen in a strength test, in the opinion of the testing agency shows evidence of improper sampling, casting, or testing, including damage from improper handling and transporting (and if agreed to by the Ministry Representative), it shall be discarded and the remaining two specimen strengths averaged.

Additional cylinders may be cast, at the discretion of the Ministry Representative or Contractor.

The strength of each classification of concrete shall be considered satisfactory if:

- the averages of all sets of three consecutive strength tests equal or exceed the specified strength.
- no individual test shows strengths less than 90% of the specified strength.

Each "classification of concrete" will be taken as all of the concrete in one pay item in the Schedule 7. Where fewer than three strength tests are taken, the results of one test or the average of two will be used.

211.09.05 Test Cylinders – Making and curing concrete test cylinders shall be carried out in accordance with CSA A23.2-3C. The test cylinders shall be cast by the Contractor using standard CSA approved moulds. The Contractor shall provide properly designed temperature-controlled storage boxes for test cylinder storage, for a period of at least 20 hours, and further protection from adverse weather and mishandling before and during delivery to the testing laboratory for curing and testing.

The Contractor shall provide a maximum-minimum thermometer for each storage box and record site curing temperatures for all test cylinders.

Storage in a portable building which will be used by the Contractor's personnel or the concrete testing personnel during the first 24 hour storage period will not be permitted.

Storage facilities shall be provided, installed, and accepted before any concrete is placed.

The Contractor shall deliver the test cylinders to a CSA certified testing laboratory. Handling and transporting of the cylinders shall be in accordance with CSA A23.2-3C. No extra laboratory curing time will be allowed for cylinders that are delivered late to the laboratory. A copy of the test results shall be forwarded to the Ministry Representative within 2 days of each individual strength test date. Test results shall include all information as described in CSA A23.2, Annex B, Form for Reporting Compressive Strength of Concrete Test Cylinders.

211.09.06 Field Cured Test Cylinders – The Ministry Representative may require field cured test cylinders to be cast to check the adequacy of the Contractor's curing or cold weather protection. The Contractor may also cast field cured test cylinders for determining interim strengths that may be required for further work progress approval such as opening to traffic, formwork removal and/or erection, etc.

Casting, handling, transporting and testing shall be in accordance with CSA A23.2-3C and -9C.

Test cylinders shall be stored as near as possible to the point in the structure that the test cylinders represent, and shall be afforded the same temperature protection and moisture environment as the structure.

At the end of the curing period the test cylinders shall be left in place, exposed to the weather in the same manner as the structure.

Test results on field cured test cylinders shall not be used as a basis for acceptance or rejection of the concrete.

211.09.07 Failure to Meet Minimum Strength Specifications

(a) **Understrength Concrete** – When the strength of concrete is not satisfactory as specified in SS 211.09.04, the Contractor shall prepare a plan for remedial measures and the plan shall be submitted to the Ministry Representative for review and acceptance. If the proposed plan is not acceptable to the Ministry Representative then the Ministry Representative may order replacement, supplemental reinforcing, or other remedial measures, at the Contractor's expense, for portions of the structure. Alternatively, at the sole discretion of the Ministry Representative, the understrength concrete may be accepted at a reduced price.

(b) **Coring** – the coring of concrete, to verify its strength, will be allowed only for concrete, which is otherwise to be replaced or reinforced. Coring will not be allowed for concrete for which reduced payment is to be made for understrength. When coring is allowed, cores shall be taken at locations as directed by the Ministry Representative, who shall be present during the coring.

Where coring is allowed, it shall be carried out at the Contractor's expense by an independent qualified testing firm. Three cores shall be obtained and tested in accordance with CSA A23.2-14C "Obtaining and Testing Drilled Cores for Compressive Strength Testing" at each test location.

Concrete in the area represented by the core tests may be considered structurally adequate if:

- (i) the average strength test result of each set of three cores from the portion of the structure in question is equal to at least 100% of the specified strength
- (ii) the strength test result of any single core is not less than 80% of the specified strength

Notwithstanding the final result, the Contractor will be responsible for all costs associated with the coring operation and testing, and shall not be reimbursed for inconvenience or other associated costs.

211.09.08 Failure to Meet Slump or Air Content Specifications – In the event that the slump and/or air content are outside the specified limits, the Ministry Representative may accept adjustments to correct the deficient condition as an alternative to rejection. In such cases, adjustments must be completed within the maximum time allowed as specified in SS 211.08.02 and additional testing shall be required to verify specification compliance. If compliance is not achieved, the concrete shall be rejected.

211.10 Placing of Concrete

211.10.01 General – Equipment for conveying concrete at the Site, such as buggies, buckets, hoppers, chutes, belts and pumps, shall be of such design, size and condition to deposit a continuous and adequate supply of concrete of the specified mix and consistency without segregation at the required locations.

Placing of concrete shall comply with CSA A23.1. The equipment specifications in SS 211.10.02 to SS 211.10.07 inclusive shall apply unless the use of alternative equipment is accepted by the Ministry Representative.

211.10.02 Buggies – Buggies shall have inflatable rubber tires. Inner surfaces of the bucket shall be smooth with filleted corners, mortar-tight and free of any excessive accumulation of hardened concrete, obstructions or deterioration interfering with the proper discharge of concrete. Grade and ramp-way surfaces shall be sufficiently smooth to prevent segregation of concrete being carried in buggies.

211.10.03 Bottom-dump Buckets and Hoppers – Bottom-dump buckets and hoppers shall have side slopes not less than 60° from the horizontal and shall be equipped with wide free-working and tight-closing discharge gates. The discharge gates shall be constructed to regulate the concrete flow, spring loaded to ensure complete closure and be capable of immediate closure at any time during discharge. Buckets and hoppers shall be cleaned of any accumulation of

partially hardened or hardened concrete before and during concrete placement. Gate control mechanisms shall be cleaned and lubricated before being used.

In the use of crane and bucket, segregation of concrete from jarring or shaking shall be prevented. Side-dump buckets shall be used whenever necessary to avoid conflict with extended reinforcing steel or formwork.

211.10.04 Chutes – Chutes shall be of rounded cross section to avoid the accumulation of concrete in corners; be capable of slope adjustment sufficiently steep to permit flow without requiring a slump greater than that specified or required for placement (slope normally required is 1 vertical to 2 or 2½ horizontal).

Baffles and changes in direction may be used to control flow, but not vibration, paddling or water spray.

Chutes or belts shall be rigidly supported but sufficiently mobile to permit discharge as close as possible to the placement location as it progresses. Any long line of chutes or belts shall be covered during hot, dry or windy weather to prevent drying of concrete and excessive slump loss.

211.10.05 Conveyor Operation Rate – Conveyor operation rate shall be dependent on the proper placement and consolidation of the concrete with the belt inclination such that no segregation occurs from any sliding or roll-back of the concrete.

211.10.06 Hoppers – Hoppers, set approximately level, shall be supported on specially framed bearers transmitting only vertical loads to the top of the formwork.

211.10.07 Pumps – Pumps shall be of sufficient capacity to supply un-segregated concrete, using the design mix proportions at the design slump, to meet the required placement volume at the forms. Satisfactory operation and performance of the proposed pumping equipment with the concrete materials and mixes otherwise suitable and appropriate for the job, along with the line layout and grate size opening, shall be demonstrated to the Ministry Representative's satisfaction and acceptance.

Any necessary priming of the concrete pump shall be done with a water/cement or water/cement/sand slurry. The prime slurry followed by approximately 0.25 m³ of concrete shall not be included in the concrete placement. No other pump aid type admixtures will be permitted.

211.11 Falsework and Formwork

211.11.01 General – Formwork and falsework shall be designed, supplied, installed and removed in accordance with CSA-S269.1 and shall also meet all the requirements for concrete formwork and falsework given in the Occupational Health and Safety Regulation Clauses 20.17 to 20.26 inclusive.

Formwork and falsework support accessories shall not be welded or otherwise attached to the permanent structural steelwork or precast concrete structural members.

Where required, formwork and associated temporary falsework design and drawings shall be prepared and sealed by a professional engineer registered with the Association of Professional Engineers and Geoscientists of the Province of British Columbia and experienced in the design and construction of falsework and formwork structures similar to those required to construct the Work. No load shall be placed on the formwork or falsework until the professional engineer responsible for the design has inspected and certified, in writing, that the work has been carried out in accordance with the formwork drawings and specifications and a copy of the formwork design engineers' certification has been received by the Ministry Representative.

Bridge deck formwork design shall include a check for the lateral stability of the girders during the placement of deck concrete. Stringers shall be temporarily braced, if necessary, to facilitate deck concreting operations.

Detailed falsework and formwork drawings shall be submitted to the Ministry Representative for review. The drawings shall be submitted a minimum of two weeks before the start of installation.

211.11.02 Design – All forms shall be designed and built mortar-tight and of sufficient rigidity to prevent distortion due to the pressure of vibrated concrete and other loads incidental to the construction operation. The forms shall be substantial and unyielding, and shall be designed so that the finished concrete will conform to the design dimensions and contours. The shape, strength, rigidity, water tightness and surface smoothness of re-used forms shall be maintained at all times. Forms which are unsatisfactory in any respect shall not be used.

All formwork and falsework shall be designed to be completely removable, no stay in place formwork will be permitted.

No dry ties shall be permitted; form tie rods shall remain embedded and terminate not less than 50 mm from the formed face of the concrete. Removable embedded fasteners on the ends of the rods shall be such as to leave holes of a regular shape for reaming and filling.

For narrow walls and columns, where the bottom of the form is inaccessible, removable panels shall be provided in the bottom form panel to enable cleaning out of extraneous material immediately before placing the concrete.

211.11.03 Forms for Exposed Surfaces – All forms for exposed surfaces shall be mortar-tight. All exposed edges shall be chamfered by use of a chamfer strip and return corners filleted. Metal bolts or anchorages within the forms shall be so constructed as to permit their removal to a depth of at least 50 mm from the concrete surface. Break-back type form ties shall have all spacing washers removed and the tie shall be broken back a distance of at least 50 mm from the concrete surface. All fittings for metal ties shall be of such design that, upon their removal, the cavities which are left will be of the smallest possible size. Torch cutting of steel hangers and ties will not be permitted. Formwork hangers for

exterior surfaces of decks and curbs shall be an acceptable break-back type with surface cone, or removable threaded type. Cavities shall be filled with cement mortar and the surface left sound, smooth, even and uniform in color. See SS 211.17 for details on finishes, mortar mixes and other relevant information.

211.11.04 Deck Formwork – Formwork for decks, curbs, sidewalks and parapets shall be fabricated so that the lines and grades shown on the drawings are achieved. For suspended slab bridge decks the formwork shall be designed to allow for an adjustable haunch to facilitate the casting of a constant depth deck slab as shown on the Drawings. Girders will be erected to normally accepted standards of tolerance; it shall be necessary to adjust the formwork to compensate for camber, variances in girder dimensions, positioning, and sweep.

Prior to commencing deck formwork and subsequent to the completion of the erection of girders, with the girders in a thermally neutral condition and with no dead load other than the dead load of the girders and necessary fall protection, the Contractor shall profile all the girders. Camber elevations are to be taken at 1000 mm centres maximum (or as indicated on the Drawings or Special Provisions), centrelines of all bearings and ends of girders. The camber elevation locations are to be laid out to an accuracy of 25 mm horizontally and camber elevations taken to the nearest 1 mm vertically. Within 14 days of the Contractor supplying the camber elevation to the Ministry Representative, the Contractor will receive haunch heights for setting the deck slab soffit, and the design finished grade elevations for the deck, complete with anticipated dead load deflections at 1000 mm stations along the centreline of roadway.

In the event that actual girder camber values vary significantly from the design values, the Contractor will be required to modify the original design finished grade as directed by the Ministry Representative.

211.11.05 Removal of Falsework and Formwork – Where formwork and associated falsework have been engineered by a professional engineer, forms and their supports shall not be removed without the approval of the professional engineer responsible for the formwork and falsework design and without the acceptance of the Ministry Representative. In determining the time for the removal of falsework and formwork, consideration shall be given to the location and character of the structure, the weather and other conditions influencing the curing of the concrete, and the materials used in the mix.

Supports shall be removed in such a manner as to permit the concrete to uniformly and gradually take the stresses due to its own weight together with any construction loads likely to be imposed. All formwork must be removed from the completed structure.

211.12 Construction Joints

211.12.01 General – Definition – Joints between adjacent or successive lifts of concrete where the previously cast concrete cannot be plasticized with a vibrator, or any joint required by the Contractor’s method of construction. Construction joints generally shall be in accordance with the following requirements:

- kept to a minimum.
- concealed by a natural break or line in the structure.
- provided for by the formwork design and placing techniques.

Emergency construction joints shall conform as nearly as possible to planned joints. In any case, concrete must be levelled as well as possible and, if lifts are partial, a vertical bulkhead shall be installed. Permission from the Ministry Representative shall be required to complete the cast following any emergency joint installation.

211.12.02 Joint Edges – Joint edges shall be preformed whenever possible for a true line upon form removal. Pour-strip shall be positioned relative to form ties so as to minimize form bulge at the bottom of the subsequent cast, but concrete may be cast to the top of a form instead of the pour-strip. Concrete surfaces outside of the reinforcing steel shall be steel trowelled to a true line.

211.12.03 Surfaces Preparation – Joint surfaces of the hardened concrete shall be thoroughly cleaned of foreign matter and laitance to present clean, sound surfaces with the coarse aggregate partially exposed and achieved by:

- brushing immediately following initial set.
- treatment with a surface set retardant, followed by green cutting.
- sand blasting.
- water blasting.

Where roughening of the hardened concrete is specified, the surface shall be roughened to a full amplitude of at least 5mm.

211.12.04 Bonding of Joints – Bonding of joints shall be achieved through the use of a water-cement slurry or, if specified, an epoxy-type bonding agent conforming with ASTM C 881M applied in strict accordance with the manufacturer’s specifications. Slurry, when used, shall have a water-cement ratio equal to that of the concrete being placed and be brush applied to a 3 to 5 mm thickness for 100% coverage on to the cleaned existing concrete surface within five minutes of placing concrete.

211.13 Consolidation

Consolidation shall meet the requirements of CSA A23.1. The equipment specifications in SS 211.13.01 to SS 211.13.02 inclusive shall apply unless the use of alternative equipment is accepted by the Ministry Representative.

211.13.01 Vibrators – General – The number, type, design and operation of vibrators will be subject to the acceptance of the Ministry Representative.

Vibrators shall have the following capabilities and requirements:

- diameter and frequency of vibrators shall conform to Table 211-H.
- vibration transmission to the concrete at frequencies not less than 130 Hz.
- vibration intensity sufficient to be visually observed on concrete with not more than 25 mm slump over a radius of at least 450 mm.
- sufficient number of vibrators available to properly compact each 8 m³ batch within 15 minutes after it is placed in forms, with at least two vibrators provided for each concrete placing unit.
- stand-by vibrator and generator available on the job at all times in case of breakdown.

211.13.02 Mechanical Vibrators – Mechanical vibrators shall thoroughly consolidate concrete immediately after placing.

Vibrator usage shall be as follows:

- not inserted between reinforcing steel and formwork nor directly on to the reinforcing steel.
- inserted vertically at a rate of 75 mm per second and penetrating into the preceding layer at least 50 mm.
- withdrawn at a rate of 150 mm per second.
- grid pattern movement starting 100 mm from a formed surface and at a maximum spacing of 300 mm thereafter.

211.13.03 Form Vibrators – Form vibrators may be permitted at the discretion of the Ministry Representative where rebar configuration would prohibit the use of internal vibrators.

Table 211-H: Requirements for Diameter and Frequency of Vibrators

<u>Diameter (mm)</u>	<u>Frequency (Hz)</u>	<u>Rate of Placement of Concrete per Vibrator (m³/h)</u>	<u>Application</u>
<u>20 – 40</u>	<u>170 – 250</u>	<u>1 – 4</u>	<u>very thin members</u>
<u>30 – 60</u>	<u>150 – 225</u>	<u>2 – 8</u>	<u>walls and slabs 200 – 300 mm</u>
<u>50 – 90</u>	<u>130 – 200</u>	<u>5 – 15</u>	<u>larger members</u>

211.14 Finishing

211.14.01 Finishing Top Surfaces – Plastic concrete, struck off immediately following casting and before the accumulation of bleed water, shall be finished by hand or mechanical floating with the least possible disturbance. Water, cement or proprietary topping material shall not be added to the surface during finishing.

No steel floats or trowels shall be used on exposed surfaces.

211.14.02 Strike-off and Finishing Machines – Strike-off and finishing machines for surfacing concrete such as slabs on grade, pavements, etc. shall conform to the following:

- capable of accurately finishing concrete surfaces to the grade and elevation as shown on the Drawings or as directed by the Ministry Representative.
- surface finish production free of open texturing, plucked aggregates or projections.
- maintained in good mechanical repair and free of hydraulic fluid leaks, oil leaks, excessive grease and other contaminants.

211.14.03 Trowel Finishing – Trowel finishing where required:

- by the Drawings or specifications,
- for unformed surfaces exposed to view from a close distance,
- by close tolerance requirements (such as bearing surfaces),

shall be commenced after bleed waters have disappeared to reduce the texture of the concrete surface and produce a smooth blemish-free surface.

211.14.04 Other Textured Finishes – Other textured finishes shall be applied as and where specified.

211.14.05 Damage or Scaling – Any damage to or scaling of surfaces shall be repaired to the satisfaction of the Ministry Representative.

211.15 Curing

211.15.01 General – Freshly deposited concrete shall be protected from freezing, abnormally high temperatures or temperature differentials, premature drying, excessive moisture, moisture loss, heavy shocks, excessive vibrations and high stresses, for the period of time necessary to develop the desired properties of the concrete.

The curing period will begin following the placement of the concrete in the section cast. It shall be the responsibility of the Contractor to ensure that the system of curing and protection is properly planned, constructed and maintained throughout the entire curing period.

211.15.02 Acceptable Methods and Procedures – Acceptable curing methods and procedures shall include the following:

- (a) concrete surfaces shall be cured for a period of seven consecutive uninterrupted twenty-four hour periods, however, if the Ministry Representative has accepted the use of high-early strength cement (HE), this period may be reduced as permitted by the Ministry Representative.
- (b) surface temperature maintained between 10°C and 35°C, unless high-temperature curing (i.e., by hot blankets or steam) is acceptable to the Ministry Representative.
- (c) hot weather and cold weather special requirements are covered by SS 211.18 and SS 211.19. The determination and recording of air and concrete temperatures to verify compliance with the requirements for hot and cold weather concreting shall be the responsibility of the Contractor. The location and methodology for the determination and recording of temperatures shall be acceptable to the Ministry Representative.
- (d) formed concrete protection, with formwork left in place during the curing period (usually seven days), is generally not necessary except during extremes of hot or cold weather temperatures. Where formwork is removed before the end of the curing period, the concrete shall be protected in the manner specified below for unformed surfaces during the remainder of the curing period.
- (e) Unformed surface protection using burlap and water or where permitted, other moisture-retaining method considered adequate to promote curing, shall include the following common methods and requirements:
 - (i) **Burlap** – Soaking wet burlap of acceptable quality shall be carefully laid on the surface of the concrete as soon as the concrete has set sufficiently to support the burlap without marring the surface. Strips shall be overlapped 150 mm, adequately held down and maintained in place in a thoroughly wet condition throughout the curing period (usually seven days). Wetness of burlap shall be maintained at all times.
 - (ii) **Vapour Barrier** – Vapour barrier of acceptable quality of opaque white-pigmental 0.10 mm thick ("4 mil") polyethylene sheet, white upper surfaced paper or other permitted material shall be of such size and placement to minimize the number of laps. The material shall be laid, lapped and held down around the edges and at the laps:
 - to prevent displacement.
 - to provide and maintain an effective vapour barrier.
 - prevent any flow of air between the vapour barrier material and the concrete.

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- kept in place throughout the curing period (usually seven days).

However, the barrier material shall not be pulled tight against the fresh concrete surface so as to smooth the required surface texture.

211.15.03 Removal of Protective Materials – Protective materials shall be removed entirely from all concrete surfaces at the end of the curing period and the surfaces left clean.

211.16 Dimensional Tolerances – Dimensional tolerances shall be as shown in Table 211-I.

211.17 Finishing Formed Surfaces

211.17.01 Class_1 Finish – Class 1 Finish is the basic finish to be produced on all formed surfaces not exposed to view unless a better finish is specified or required by the Drawings or Special Provisions.

In order to produce a Class 1 finish, the formwork shall be mortar tight. Panel marks and texture are of no importance.

All ties, bolts, nails and other metal specifically required for construction purposes shall be removed or cut back to a depth of 50 mm from the surfaces of the concrete and the resulting holes filled.

Form tie rods shall remain embedded and terminate not less than 50 mm from the formed face of the concrete. Removable embedded fasteners on the ends of the rods shall be such as to leave holes of a regular shape for reaming and filling. Removable form tie rods inside of sleeves shall not be permitted.

Honeycombs and voids over 500 mm² in area shall be filled. Honeycombs and voids shall not be repaired until inspected by the Ministry Representative as special methods of repair may be required where occurring in structural elements. Otherwise, concrete surfaces shall be repaired as follows:

- remove all porous concrete by cutting 20 mm into sound concrete.
- thoroughly clean reinforcing steel without damaging same.
- cut edges of cavities as nearly perpendicular to the concrete surface as possible and deep enough to hold the patching mortar.
- keep surrounding concrete saturated for 30 minutes before patching.
- after the sheet of free water has disappeared from the concrete surfaces, apply a latex bonding agent, in strict accordance with the manufacturer's instructions.
- patch with mortar, proportioned as specified in SS 211.17.05 and mixed with the minimum amount of water necessary to make a workable paste: for voids deeper than 10 mm, multiple layers of mortar each with a maximum thickness of 10 mm shall be used with 30 minute intervals allowed between layers.

- trowel the surface of the patch flush with the surface of the surrounding concrete.

Table 211-I: Dimensional Tolerances

ELEMENT	TOLERANCE
Trueness of bearing surfaces	such that there is no gap over 1 mm under a 1 m straight edge
Trueness of bridge deck surfaces	such that there is no gap over 3 mm under a 3 m straight edge
Trueness of slope pavement surfaces	such that there is no gap over 12 mm under a 3 m straight edge
Trueness of other surfaces exposed to view	such that there is no gap over 12 mm under a 3 m straight edge
Curbs and parapets	such that there is no gap over 3 mm under a 3 m straight edge
Horizontal distance to concrete surfaces	±50 mm
Elevations of concrete surfaces except bearings and decks	±25 mm
Elevations of bridge deck surfaces	±15 mm
Elevations of bearing surfaces	±5 mm
Variation from plumb	1:400, but not more than 30 mm
Dimensions of members over 10 m	±25 mm
Dimensions of members under 10 m	-6 mm, +15 mm
Location of anchor bolts	± 6 mm
Projections of anchor bolts, other embedments	±13 mm

Note: *The tolerances listed in Table 211-I are acceptable only in so far as they do not prevent the proper fit of structural members.*

211.17.02 Class_2 Finish – Class_2 Finish is to be produced on all formed surfaces exposed to view from a moderate distance, such as surfaces of abutments and piers, and to any surfaces for which a Class_2 finish is specified or required by the Drawings or the Special Provisions. A Class_2 finish shall provide surfaces of uniform colour and texture as viewed from 25 m.

In order to produce a Class_2 finish, formwork shall be mortar tight and shall render a true surface. Fins 3 mm wide (maximum) shall be allowed at the panel joints; however, sheathing joints must be mortar tight. Irregularities of 3 mm in height with areas of 50 mm x 75 mm shall be allowed to a maximum of four such areas per 3 m² of formwork. Patches of dissimilar material will not be permitted. Horizontal and vertical joints shall be aligned.

All ties, bolts, nails and other metal specifically required for construction purposes shall be removed or cut back to a depth of 50 mm from the surfaces of the concrete and the resulting holes filled.

Form tie rods shall remain embedded in the concrete and terminate not less than 50 mm from the formed face. Removable embedded fasteners on the ends of the rods shall be such as to leave holes of a regular shape for reaming and filling. Removable form tie rods inside of sleeves shall not be permitted.

Honeycombs and voids over 25 mm diameter shall be filled, and all bugholes over 5 mm diameter shall be pointed. All fins and projections shall be removed with a hand stone or power grinder. The use of a power grinder shall be kept to a minimum and confined to the areas required.

When a rubbed finish is not called for, patches shall be textured with a mortar float or lightly brushed after trowelling smooth.

Where more than 50 voids or bugholes over 5 mm diameter occur per square metre, or if the surfaces are not acceptably uniform in colour or texture, the entire area affected shall be given a rubbed finish, as follows:

- allow patches to attain an initial set.
- keep surfaces saturated with water for 60 minutes before applying mortar.
- remove free water from surfaces.
- apply mortar, proportioned as specified in SS 211.17.05 and mixed at least 60 minutes before application, to the concrete surfaces with a sponge, float or a hand carborundum stone, and work mortar well into the surfaces.
- allow mortar to dry 30 - 60 minutes.
- remove residues from the surfaces by rubbing with clean dry burlap but with no addition of neat cement to the surface during the "sacking" operation and finally cure the surfaces as specified in SS 211.15.

211.17.03 Class_3 Finish - Class_3 Finish is to be produced on all formed surfaces exposed to view from close distances, such as surfaces of curbs and parapets, and to any surface for which a Class_3 finish is specified. A Class_3 finish shall provide surfaces of uniform colour and texture when viewed from less than 15 m.

In order to produce a Class_3 finish, the formwork shall render a true smooth surface, free from fins and projections. New plywood or steel is necessary to produce the required finish. Re-use of plywood forms will be permitted only if in an "as new" condition. Repairs to the forms shall be with full panels of sheathing only.

All ties, bolts, nails and other metal specifically required for construction purposes shall be removed or cut back to a depth of 50 mm from the surfaces of the concrete and the resulting holes filled.

Form tie rods shall remain embedded and terminate not less than 50 mm from the formed face of the concrete. Removable embedded fasteners on the ends of the rods shall be such as to leave holes of a regular shape for reaming and filling. Removable form tie rods inside of sleeves shall not be permitted.

Patching shall be:

- patch installation projecting slightly from the surrounding concrete surface.
- after 24 hours, saturated and carefully finished to render the surfaces of the patch true to the surrounding concrete, but any grinding using a hand carborundum stone kept to a minimum.

When a rubbed finish is required, the applied mortar shall be carefully worked into the surface area being finished and, using a hand carborundum stone manipulated in a circular motion, the entire surface rendered true and smooth with all excess mortar removed.

211.17.04 Class_3 Finish Alternative Procedures – Class_3 Finish alternative procedures when, in the opinion of the Ministry Representative, the formwork may safely be removed at an early age and the concrete finished while still partly plastic. Procedures shall include the following:

- careful removal of the formwork when the concrete has sufficiently hardened so as to hold its shape.
- finishing the surfaces immediately (within 4 to 8 hours of casting, depending upon weather).
- careful rubbing of the concrete surfaces with a carborundum stone, occasionally wetting same to produce a paste from the surface of the concrete.
- rubbing continued, adding necessary sand-cement mortar in small quantities to fill voids, until all voids and form marks are removed.
- surface curing as specified.

Modifications to the surface texture shall be achieved by grinding or other means.

SECTION 211

PORTLAND CEMENT CONCRETE

211.17.05 Patching Mortar – Patching mortar where specified above shall be selected from the Ministry Recognized Products List, subject to the acceptance of the Ministry Representative, or constituted as follows:

Cement:	60% Normal Portland 40% White Normal Portland
Liquid:	70% Water 30% Latex Bonding Agent
Sand:	Passing a 1.25 mm sieve

NOTE: Cement blend above shall be adjusted to provide close match to the concrete colour.

211.18 Hot Weather Concreting

When ambient air temperature is 25°C or higher or there is a probability of it rising to 25°C during the placing period (as forecast by the nearest official meteorological office), hot weather concreting procedures as outlined below, shall be used, for all concrete other than deck concrete. Additional requirements for hot weather concreting for bridge decks including diaphragms, bridge parapets, bridge medians, bridge sidewalks and approach slabs are given in SS 413, Bridge Decks and Concrete Overlays.

Prior to the placement of concrete, the Contractor shall submit to the Ministry Representative for acceptance, the proposed placing operations complying with the requirements for hot weather concreting.

Curing shall be accomplished by water spray or by using saturated absorptive material, such as burlap.

Formwork, reinforcement and concreting equipment shall be protected from the direct rays of the sun or cooled by fogging and evaporation.

Concrete for piers, abutments and footings of bridges shall not have a temperature greater than 25°C at any time prior to placement and curing.

The initial temperature of concrete prior to placement may be estimated from the temperatures of its ingredients by using the following equation:

$$T = \frac{0.22(T_a M_a + T_c M_c) + T_w M_w + T_{wa} M_{wa}}{0.22(M_a + M_c) + M_w + M_{wa}}$$

where

T_a , T_c , T_w and T_{wa}

= temperature in °C of aggregates, cementitious materials, added mixing water, and free water on aggregates, respectively

M_a , M_c , M_w and M_{wa}

= mass in kilograms of aggregates, cementitious materials, added mixing water, and free water on aggregates, respectively

If deemed necessary, the Contractor shall employ the following hot weather concreting procedures to reduce the concrete temperature:

(a) Mixing Water

- (i) Shading of storage tanks and water supply lines;
- (ii) Refrigeration of mixing water; and/or
- (iii) Adding cubed, shaved or chipped ice directly to the concrete mixer drum, and ensuring ice is of a particle size to be completely melted before the concrete is discharged from the mixer drum for placement.

(b) Cement

- (i) Maintain all cementitious material to below a maximum temperature of 50°C

(c) Aggregate

- (i) Shading of coarse and fine aggregate stockpiles; and/or
- (ii) Wetting of coarse aggregate and allowing time for drainage of free water.
- (iii) Wetting down of stockpiles of fine aggregate is not permitted.

(d) **Mixing** – Mixing time should be kept to a minimum, allowing adequate time for complete mixing of the concrete. The external surface of the mixing drum should be frequently wetted down.

(e) **Placing** – Production of concrete during hot weather shall be scheduled for a continuous supply of concrete at the Site.

Adequate manpower, equipment and standby provisions shall be provided in order to place and consolidate the delivered concrete within 45 minutes after initial mixing. Further, the time interval between placing batches shall not exceed 30 minutes.

Time of placement extensions during hot weather concreting may be considered by the Ministry Representative and may require the use of Hydration Stabilizing Admixtures (HSA's) conforming to the requirements of ASTM C494 Type B, Retarding or Type D, Water-Reducing and Retarding Admixtures. Use of HSA's shall be in accordance with SS 211.08.02.

For large unformed surfaces, the quantity and rate of placing of concrete shall be dependent upon the rate of finishing and climatic condition.

When the rate of evaporation from unformed surfaces as estimated from SS Drawing SP211-01 "Surface Evaporation Rate" exceeds 1.0 kg/m²/hr, either concrete operations shall cease or, upon acceptance of the Ministry Representative, necessary precautions taken to prevent plastic shrinkage as detailed in CSA A23.1 Section 7.6.

- (f) **Curing** – Curing during hot weather shall be commenced as soon as possible.

Forms shall be kept moist and loosened as soon as this can be done without structural damage, with a flow of water introduced and allowed to run down inside.

Newly exposed concrete surfaces shall be protected from premature drying by the application of pre-wetted curing materials or a continuous water spray as the forms are removed.

Large exposures of concrete require special precautions as follows:

- (i) burlap application commenced as soon as the surface will support the mass without excessive deformation, with this surface kept continuously wet by fog spraying until a waterproof barrier is securely in place.
- (ii) concrete under the curing blanket is not allowed to dry out for a minimum of seven days from the time of initial covering, necessitating frequent checking and soaking by the addition of water under the top evaporation barrier.
- (iii) wet curing followed by the removal of the evaporation barrier only under such conditions that the wetted burlap is permitted to dry slowly before removal and the surface is exposed to prevent excessive thermal shock.

211.19 Cold Weather Concreting – Cold weather concreting procedures shall be used when the ambient temperature is, or is forecast to be, below 5°C during placement, and/or is forecast to fall below 5°C during the first seven days after placing. The Contractor shall be fully responsible for the protection of concrete during cold and adverse weather conditions and shall maintain a minimum concrete temperature of 10°C for a minimum of seven continuous days. Prior to the placement of concrete, the Contractor shall submit to the Ministry Representative for acceptance, the proposed placing operations complying with the requirements for cold weather concreting.

211.19.01 Provisions – The following provisions for cold weather concreting shall apply:

- (a) **Material Temperatures** – The temperature of the combined water and aggregates shall not exceed 40°C when combined with the cement. The temperature of the concrete shall be between 10°C and 25°C at the time of placement.
- (b) **Aggregates** – Aggregates shall be heated to eliminate frozen lumps, ice and snow without overheating or excessive drying. Aggregates shall not be heated above 65°C, and all lumps of frozen aggregate shall be excluded from the mix.
- (c) **Formwork, Reinforcing Steel** – Before any concrete is placed, all ice, snow or frost shall be completely removed from the forms and the temperature of contact

surfaces raised to a minimum of 5°C, with such minimum established and maintained for at least one hour prior to placement.

Heat shall be applied uniformly and at a rate which will not induce excessive thermal stresses in the section being heated. Ambient air temperatures shall not exceed concrete temperatures by more than 13°C and shall be raised at a maximum rate of 2°C/hr.

Projecting reinforcing steel shall be insulated when temperatures are between 0°C and -10°C for a minimum of 300 mm away from the fresh concrete and for a minimum of 600 mm when temperatures are below -10°C.

- (d) **Other Contact Surfaces** – Concrete shall not be placed on frozen surfaces except in pile founded footings where a minimum layer of 25 mm rigid insulation shall be installed. Rock surfaces shall be heated by forced air. Heating shall continue until all standing frost crystals are dissolved. Frozen embankment shall be thawed and re-compacted prior to placing concrete thereon. The temperature of existing concrete work shall be raised to a minimum of 5°C and maintained at this temperature for a time sufficient to raise the entire mass of concrete to this temperature.
- (e) **Curing and Protection** – Curing and protection necessary to maintain a minimum concrete temperature (10°C) shall be provided throughout the curing period.

Heated enclosures shall be kept at 95% minimum relative humidity. Concrete surfaces shall be protected by formwork or impermeable membranes from direct exposure to the combustion gases of heating. The use of salamanders, coke stoves, oil or gas burners and similar spot heaters which have an open flame and intense local heat is prohibited without the Ministry Representative's acceptance. The enclosed air temperature shall not be less than 10°C nor more than 26°C.

Where practical, insulated forms, capable of maintaining the surface of the concrete at not less than 10°C for a period of seven days, may be used instead of enclosures and heating. If forms are insulated, exposed horizontal surfaces shall be protected with a similar layer of the insulating material securely fastened in place. If the insulated forms do not maintain the proper temperature at the concrete surface, auxiliary protection shall be used to provide additional heat as described above.

The determination and recording of air and concrete temperatures to verify compliance with the requirements for cold weather concreting shall be the responsibility of the Contractor.

The Contractor is responsible for furnishing and using a 24-hour minimum/maximum or continuous temperature recording thermometer to record the air temperature within the enclosure and/or insulated forms. Daily

records shall be submitted to the Ministry Representative upon request.

At the end of the curing period, heating and protection shall be withdrawn in such a manner so as not to induce thermal shock stresses in the concrete. The temperature of the concrete shall be gradually reduced at a rate not exceeding 10°C per day to that of the surrounding air. To achieve this, in a heated housing, the heat shall be slowly reduced and then shut off, and the whole housing allowed to cool to air temperature before the housing itself is removed. However, the protection shall not be removed until the temperature of the concrete has fallen to within 10°C of the temperature of the outside air.

211.20 Underwater Concreting – Underwater concreting procedures shall be used when structures, or parts of structures, are to be constructed under water. This section is also applicable to tremie placement in the dry, unless specified otherwise.

211.20.01 Requirements – The concrete, methods of placement either by pumping or tremie pipes, and equipment shall be in accordance with the following, or as otherwise directed by the Ministry Representative:

- (a) Proposals outlining procedures shall be submitted for review to the Ministry Representative 3 weeks prior to placement of concrete. Placement shall comply with the provisions of the proposal.
- (b) Materials shall conform to the requirements of SS 211.04, except that coarse aggregates shall contain not more than 25% angular particles by mass.
- (c) Mix proportions shall be as specified in SS 211.03, except:
 - (i) cement content not less than 400 kg/m³.
 - (ii) water/cement ratio no greater than 0.40.
 - (iii) plasticized slump of 180 mm ±20 mm.
 - (iv) admixtures used to achieve plastic, cohesive and flowable concrete if acceptable to the Ministry Representative.
 - (v) suspension type admixtures should be considered
 - (vi) retarding type admixture if accepted by the Ministry Representative shall be used to prevent the formation of cold joints with the lift method of placement or where the concrete surface area is large.
- (d) Heating of water or aggregates or both, to obtain a suitable placing temperature, shall be done in accordance with SS 211.19. Concrete, when mixed, shall have a temperature of between 15°C and 25°C and shall not be placed in water having a temperature less than 2°C.
- (e) Cofferdams or forms into which concrete is to be placed shall be sufficiently tight to prevent the loss of mortar and, if in running water, reduce the velocity of flow within the cofferdam or forms to not more than 3 m per minute. Dewatering will not be permitted until at least 24 hours after concreting has been completed.
- (f) Pumping and tremie pipe requirements shall be as follows:
 - (i) length sufficient to reach the lowest point of deposit but held 150 mm to 250 mm above the bottom of the placement.
 - (ii) submerged into the placed concrete and held in a near vertical position at all times.
 - (iii) supported in such a manner to allow vertical movement.
 - (iv) discharge at locations not farther apart than 7 m.
 - (v) discharge end fitted with a 3 m section of steel pipe.
 - (vi) fed by either chutes or pumps with a continuous flow of concrete.
- (g) Priming of tremie pipes shall be by inserting a plug of burlap, 25 mm less in diameter than the tremie pipe, in the top of the pipe and carefully controlling concrete flow as the plug passes through the pipe. Where priming is lost, the priming procedure shall be repeated.
- (h) Concrete flow, once begun, shall be continuous through the pipe, at a minimum of 15 m³ of concrete supplied to each tremie or pump per hour, but controlled so as not to overflow the feed hopper. The flow of concrete shall be continuous to the end of the batch and the pipe lowered in the placed concrete to maintain its prime.
- (i) Placement of concrete underwater shall be either to the full depth of placement or in lifts and shall conform to the following requirements:
 - (i) seals shall be placed by beginning at one end and continuing until final grade is achieved.
 - (ii) point of placement shall be relocated by approximately 3 m, either laterally or longitudinally and placement continued to grade.
 - (iii) above procedures repeated to the completion of the seal or when the lift method of placement is used.
 - (iv) flow of concrete shall be regulated to produce approximately horizontal surfaces.
 - (v) each lift placed before the preceding lift has taken initial set.
 - (vi) depth and area of each lift dependent upon the rate of placement.
- (j) Concrete in its plastic state shall not be disturbed either directly or indirectly by puddling or vibration.
- (k) Should interrupted placement be sufficient for the concrete to take initial set, the laitance shall be removed from the concrete surface before concreting is resumed, but with the resumption of concreting delayed indefinitely, laitance shall be removed not later than 36 hours after the interruption.

211.21 Measurement and Payment

211.21.01 Formwork – Payment for formwork will be made at the Unit Prices bid per square metre. Payment shall be for the supply, construction and removal of formwork, and for the finishing, as required, of formed surfaces. Areas shall be measured of the faces of forms in contact with the concrete.

Payment will be made for forming one face of vertical construction joints which are shown on the Drawings or ordered by the Ministry Representative. No payment will be made for forming construction joints not so required. Payment will be made for forming both faces of joints formed by casting in sheets of styrofoam or other material, where such joints are required by the Contract Documents.

Payment will not be made for forming surfaces which are cast against permanent embedded items which are paid for separately.

Upper surfaces which form an angle of 45° or less with the horizontal, will not usually be considered to be formed surfaces.

The undersides of members which are shown on the Drawings to rest on ground or fill will not be considered formed surfaces. The undersides of members which are not shown on the Drawings to rest on ground or fill will be considered formed surfaces.

Where sides of footings or faces of walls are cast against ground or fill (if permitted by the Ministry Representative) the surfaces so formed will be considered formed surfaces. Only the areas of the surfaces shown on the Drawings will be paid for.

Progress payments will be made according to Table 211-J.

Table 211-J: Progress Payments

Progress Payments	Application
60%	For formwork in place
20%	For Formwork removed
20%	For surface finishing completed

211.21.02 Concrete – Payment for concrete will be made at the Unit Prices bid per cubic metre (or at a reduced price for understrength concrete - see fourth paragraph of this subsection) prorated at 90% for the supply, transportation,

placing, and finishing (except the finishing of formed surfaces) of concrete, and for everything supplied and done in connection therewith, except formwork and reinforcing steel.

Payment for curing, prorated at 10% will be made pending satisfactory completion of the specified curing process. No payment will be made for partial curing. Failure to comply strictly with the specified curing process will result in forfeiture of payment for curing. The Ministry Representative shall take measures as required to provide the specified curing. Should the cost of these measures exceed the monies recovered from the forfeiture of curing payment, the balance will be deducted from the remaining payment for concrete.

Payment for concrete shall include cement.

Payment shall also include all costs associated with quality control.

Measurement of volume will be made to the surfaces shown on the drawings or ordered by the Ministry Representative. Deductions will be made of the volumes of embedded ducts, timbers, etc., but not of the volume of reinforcing steel. No payment will be made for concrete placed outside the surfaces shown or ordered, as to fill spaces left by over excavation or irregularities in excavation.

The reduced price to be paid for understrength concrete shall be calculated by the following formula:

$$P_R = \frac{S_{AV}}{S_{SP}} \times P_U$$

Where

P_R = reduced price

P_U = bid price

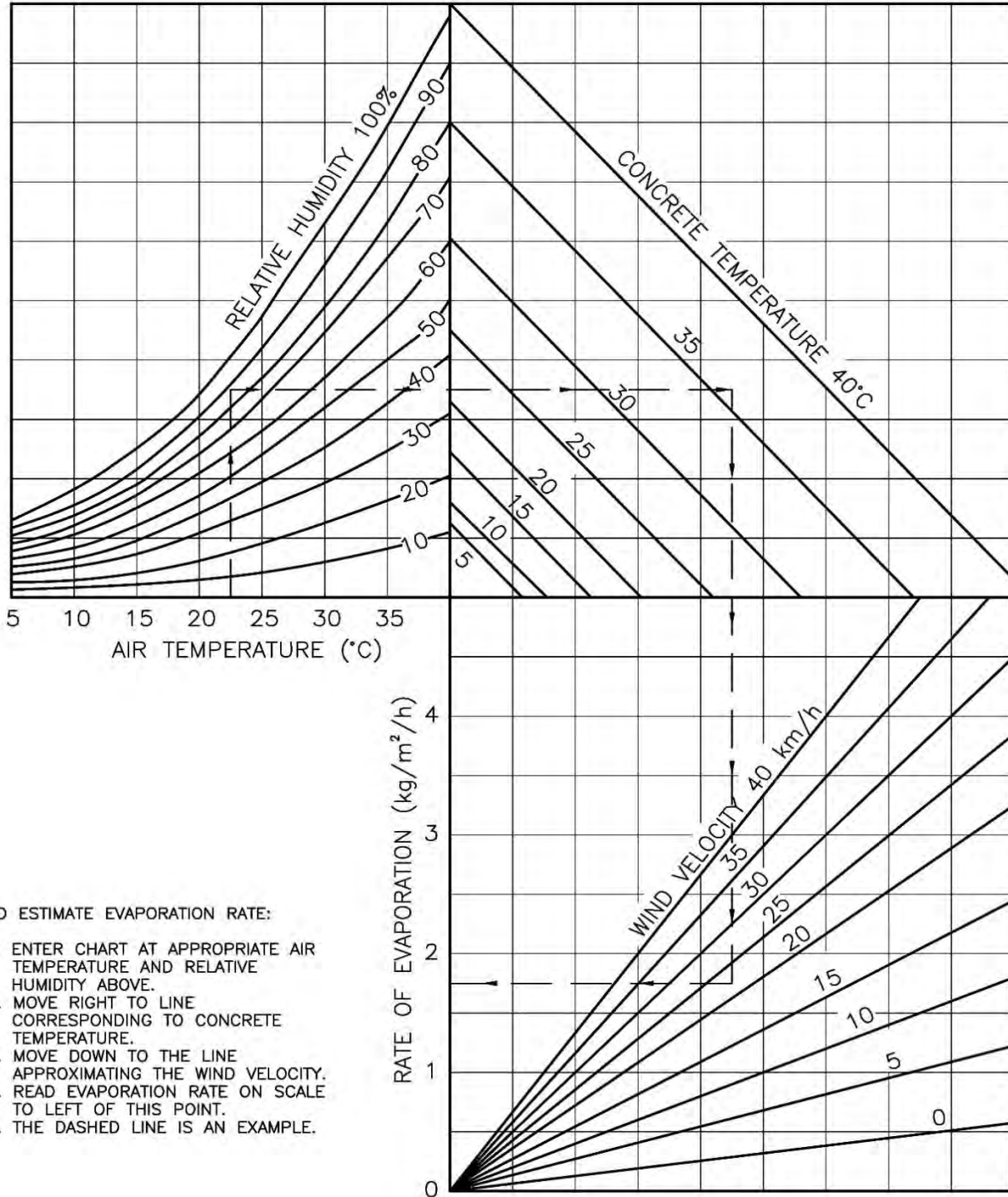
S_{SP} = specified strength

S_{AV} = the lowest average of three consecutive strength tests for one classification of concrete within the same structural element. Where less than three strength tests are taken, the results shall be the average of all the actual strength tests taken.

See SS 211.09.07 for circumstances under which a reduced price will be paid. Where a reduced price is paid, it will be applied to all of the concrete in the applicable pay item.

SURFACE EVAPORATION RATE

SP211-01



TO ESTIMATE EVAPORATION RATE:

1. ENTER CHART AT APPROPRIATE AIR TEMPERATURE AND RELATIVE HUMIDITY ABOVE.
2. MOVE RIGHT TO LINE CORRESPONDING TO CONCRETE TEMPERATURE.
3. MOVE DOWN TO THE LINE APPROXIMATING THE WIND VELOCITY.
4. READ EVAPORATION RATE ON SCALE TO LEFT OF THIS POINT.
5. THE DASHED LINE IS AN EXAMPLE.

SECTION 213

TIMBER BRIDGES – CONSTRUCTION

213.01 General – Timber materials may be used in the completed Work only when explicitly identified in the Contract as an acceptable product.

213.02 Materials, Handling, Storage and Construction – All materials, handling, storage and construction shall be as specified and shall conform to the relevant requirements of SS 214, SS 904, SS 905, SS 906, and SS 908.

All materials shall be handled and stored with reasonable care to prevent damage such as puncture, cutting or crushing of fibre.

213.03 Logs for Cribs and Log Stringers – Logs for cribs shall be cut from live trees and shall be peeled before use.

Log stringers shall be cut from live trees not less than 30 days before use and shall be seasoned with the bark on. Immediately before use, all bark shall be peeled and the log trimmed smooth of all knots and projections.

213.04 Work Practice – The standard of work shall be first-class throughout and in accordance with AASHTO LRFD Bridge Construction Specifications.

Details for construction of log stringer bridges and log cribs shall be in accordance with “Log Bridge Construction Handbook 1980” published by Forest Engineering Research Institute of Canada (FERIC), except as otherwise specified in the Contract.

213.05 Framing – All the cutting, boring, framing, match marking, etc. required on all timber shall be done by competent bridge framers in a thorough manner, in accordance with good work practice. It shall be done such that surfaces in contact shall bear evenly and fully; no shims or open joints are permitted. All measurements shall be accurate. Gains and daps shall have plane, smooth surfaces.

Holes shall be bored with an auger to the sizes shown in Table 213-A.

Table 213-A: Hole Sizes

<u>COMPONENT</u>	<u>FIR AND HARD-WOODS</u>	<u>CEDAR AND SOFT-WOODS</u>
BOLTS	1/16” Large	Same size as bolt
DRIFT BOLTS	Same size as bolt	1/16" Small
DOWELS	1/16" Large for ends of posts, but in sills make same size as dowel	1/16" Large for ends of posts, but in sills make same size as dowel

Spikes that are likely to crack or unnecessarily damage the timber shall have holes bored 1/16" small.

Hardware and metalwork in contact with treated wood shall be galvanized or stainless steel and shall be in accordance with the requirements of SS 908.08.

213.06 Field Treatment – Cutting, framing and boring of timbers to receive preservative treatment shall be done before treatment insofar as possible.

In the event that cutting or drilling becomes necessary after treatment, a field treatment preservative specified in CSA O80.3 or the American Wood Protection Association (AWPA) Standard M4 shall be used and applied in accordance to its label. At least two coats shall be applied and where possible, the colour of the preservative treatment used for protecting field cuts shall match the original preservative treatment colour.

For bridge components, creosote and copper naphthenate shall be the only permitted field preservatives.

For untreated timber, all surfaces in contact with any other surface and all bolt holes shall be swabbed with a water-borne preservative.

Preservative chemicals shall not be permitted to spill onto the ground or into any waterway during field treatment.

213.06 Preparation of Laminated Decks for Wearing Surface

213.06.01 For Asphalt Plank – Depressions in laminated decks shall be levelled by placing and compacting a fine sand levelling course.

The levelling course shall consist of a mixture of fine durable sand conforming to the gradation shown in Table 213-B.

The sand shall be thoroughly mixed with asphalt emulsion, Type SS-1 which shall conform to the requirements of SS 952.08. The asphalt emulsion content shall be such that an air void content of 5% to 8% in the compacted mixture is obtained. This will normally require an asphalt emulsion of 15% to 25% by weight of the total mixture.

Before laying and nailing asphalt plank, laminated decks shall be swabbed with oxidized mopping asphalt Type 2 conforming to the requirements of Table 213-C.

213.06.02 For Asphalt – Prior to placing asphalt wearing surface, creosote-treated laminated decks shall be lime-scrubbed where creosote oil has "bled" to the surface.

To prevent "bleeding" of the creosote oil, the mix temperature of hot-mix asphalt placed on these decks shall be as low as is practicable.

Table 213-B: Consistency of the Levelling Course

STANDARD SIEVE SIZES		PERCENTAGE PASSING BY WEIGHT (%)
ASTM	METRIC	
#8	2.36 mm	100
#16	1.18 mm	60 – 100
#30	0.600 mm	46 – 80
#50	0.300 mm	35 – 62
#100	0.150 mm	25 – 46
#200	0.075 mm	18 – 35

213.07 Basis of Payment – Payment will be made at the unit prices bid, for the various items in the Schedule of Approximate Quantities and Unit Prices, for material remaining in and forming a part of the finished structure. Such payment shall be accepted as full compensation for all labour, equipment, tools, materials, including hardware and furnishing and placing all other items required to complete the work.

Table 213-C: Requirements for Oxidized Mopping Asphalt Type 2

TEST DESCRIPTION	ASTM TEST METHOD	TYPE 2 ASPHALT	
		MINIMUM	MAXIMUM
<u>Softening Point (R & B), °C</u>	<u>D36</u>	<u>74</u>	<u>79</u>
<u>Flash Point, C.O.C., °C</u>	<u>D96</u>	<u>232</u>	<u>---</u>
<u>Penetration @ 0°C</u>	<u>D5</u>	<u>12</u>	<u>20</u>
<u>Penetration @ 25°C</u>	<u>D5</u>	<u>20</u>	<u>35</u>
<u>Penetration @ 46°C</u>	<u>D5</u>	<u>---</u>	<u>70</u>
<u>A.P.I. Gravity @ 15°C</u>	<u>D71</u>	<u>4.6</u>	<u>5.8</u>
<u>Ductility @ 25°C, 5 cm minimum</u>	<u>D113</u>	<u>3</u>	<u>---</u>
<u>Loss on Heating to 163°C (325°F), 5 hr., %</u>	<u>D6</u>	<u>---</u>	<u>0.3</u>
<u>Penetration of Residue, % of original</u>	<u>D5</u>	<u>80</u>	<u>0</u>
<u>Total Bitumen (soluble in CS₂) %: Mineral Stabilized Asphalt</u>	<u>D4</u>	<u>80</u>	<u>---</u>
<u>Total Bitumen (soluble in CS₂) %: Asphalt Without Mineral Stabilizer</u>	<u>D4</u>	<u>99</u>	<u>---</u>

SECTION 214

TIMBER PILING CONSTRUCTION

Note: Timber piles may be used only when explicitly identified in the Contract as an acceptable material.

214.01 Materials – All piling shall conform to requirements of SS 906 and preservative treated piles shall meet the requirements of SS 908.

Hardware, fasteners and metalwork for timber piling construction shall be galvanized or stainless steel and shall also meet the requirements of SS 908.08.

214.02 Pile Lengths – Piling shall be supplied to the minimum lengths specified. At the Contractor's expense, allowances will be made for such additional lengths of pile as may be required to provide fresh heading and to suit the Contractor's methods of operation.

Pile lengths determined by the Ministry, are those expected to remain in the completed structure plus 600 mm for cut-off, except for end bearing piles where the allowance for cut-off and bearing depth irregularity is 3.5 m or as otherwise specified on the Drawings or the Special Provisions.

214.03 Handling of Piles – Handling of piles shall be in accordance with SS 906.08 and SS 908.

214.04 Hammer – Piles may be driven with a drop hammer or a mechanical hammer. Drop hammers shall weigh not less than 1,400 kg for piles less than 15 m long and not less than 1,600 kg for piles 15 m and over in length, and in no case shall the weight of the hammer be less than the combined weight of driving head and pile. Mechanical hammers shall develop a total energy not less than 3,900 kg m per blow.

214.05 Leads – Pile drivers with fixed leads or with hanging or swinging leads that can be held in a fixed position during driving shall be used to drive all piles.

Battered piles shall be driven using leads adaptable to their driving.

When driving treated timber piles, the use of spuds and chocks in the leads shall be kept to a minimum in order that the treated surface is not damaged.

214.06 Preparation for Driving

214.06.01 Driving Caps and Bands – The heads of timber piles, shall be protected by suitable driving caps of approved design, preferably having a rope or other suitable cushion next to the pile head and fitted into a casting which in turn supports a timber shock block. Alternatively, the pile head may be banded with a pile ring and covered with a steel wire mat. When driving treated piles, a driving cap shall be used.

214.06.02 Pointing – Steel points shall be supplied and installed as shown on the Drawings or Special Provisions.

Where soil conditions, in the opinion of the Ministry Representative, require the pointing of piles, the Contractor shall supply and install the piles with steel points fabricated in accordance with SS Drawing SP214-01 Pile Shoe Type A.

214.06.03 Excavation – Footing excavation and seal excavation, where applicable, shall be completed with any necessary allowance made for upheaval before the driving of footing piles. Should too great an allowance be made, backfilling with gravel will, in general, be permitted to raise the bottom of the excavation to the correct elevation. Any material forced up between the piles to above the footing level shall be removed to the correct elevation. No extra payment will be made for the supply and placing of gravel or any additional excavation to obtain the correct elevation.

214.07 Driving – Unless otherwise specified, all piles shall be driven with the small end downward. The fall of a drop hammer shall be regulated to avoid damage to the piles and under no circumstances shall the drop exceed 3 m.

Each pile shall be square-headed at the top, properly pointed at the tip, and set properly in the leads; if any pile becomes split or broomed on the top it shall be fresh headed.

Piles shall be driven to the minimum depth (maximum tip elevation) indicated on the Drawings or as otherwise specified and unless ordered otherwise by the Ministry Representative, to practical refusal, which shall be to the depth at which the piles have an allowable load resistance of 20 tonnes and an average penetration per blow determined by the following Engineering News Record (ENR) formulae:

Equation 214-1: For Drop Hammers

$$s = \frac{wh}{6p} - 0.03$$

Equation 214-2: For Mechanical Hammers

$$s = \frac{wh}{6p} - 0.003$$

Where:

s = average penetration per blow in metres under last 10 blows

w = weight of moving part of hammer in newtons

h = effective fall of hammer in metres

p = allowable load of pile in newtons

Where it is required by the Ministry Representative to penetrate a thin stratum (which may show practical refusal

in driving) to obtain greater depth in a less resistant formation, driving shall be continued through the stratum regardless of the definition laid down for practical refusal.

Any pile which, in the opinion of the Ministry Representative, is damaged by driving so as to be deemed unfit for the use for which it is intended, or is so far out of position, or off vertical or designated batter, as to require excessive force or manipulation to restore it to its proper position, shall be withdrawn and replaced by a new pile. If this is not possible, a new pile shall be driven adjacent to the defective pile. The damaged, out-of-position or misaligned piles shall be for the Contractor's account.

Where boulders or other large obstructions make it impossible to drive piles in the locations shown on the Drawings, the Ministry Representative may direct the Contractor to drive an additional pile, or piles, for which the Contractor will be paid in accordance with the various Items of SS 214.11 – Payment.

214.08 Additional Equipment – Where, in the opinion of the Ministry Representative, the bounce of the hammer is excessive or where the required depth or resistance is not obtained by the use of a hammer complying with the above minimum requirements the Contractor shall provide a heavier hammer at the Contractor's expense.

214.09 Accuracy of Driving – All piles shall be driven in such a manner that any variation from the vertical, or batter as shown on the Drawings, shall not exceed 20 mm/m. Piles in trestle bents shall be driven so that the cap may be placed in its proper location without incurring excessive stresses in the piles. Cutting, springing, dapping or facing-off for placing bracings and planking on piles will not be permitted.

After driving, footing piles shall not be more than 75 mm out of the horizontal position shown on the Drawings.

214.10 Pile Heads – The tops of all piling shall be cut off to a true plane as shown on the Drawings or as established by the Ministry Representative. Piles shall show a solid head at the plane of cutoff.

Pile heads not encased in concrete shall be protected as follows.

214.10.01 Treated Piles – After cut-off, the pile heads shall be covered with alternate layers of hot roofing asphalt and loosely woven glass fabric, using four applications of asphalt and three layers of fabric.

The fabric shall measure at least 150 mm more in dimension than the diameter of the pile and shall be neatly folded down and secured by large headed galvanized roofing nails or by binding with not less than seven complete turns of galvanized wire securely held in place by large-headed galvanized roofing nails or staples. The edges of the fabric projecting below the fastening shall be neatly trimmed.

214.10.02 Untreated Piles – After cut-off, the pile head shall be thoroughly swabbed with three coats of hot creosote oil and if they are to be capped, they shall also be coated with hot roofing asphalt.

214.11 Payment – Payment for timber piling construction will be made under the following Items.

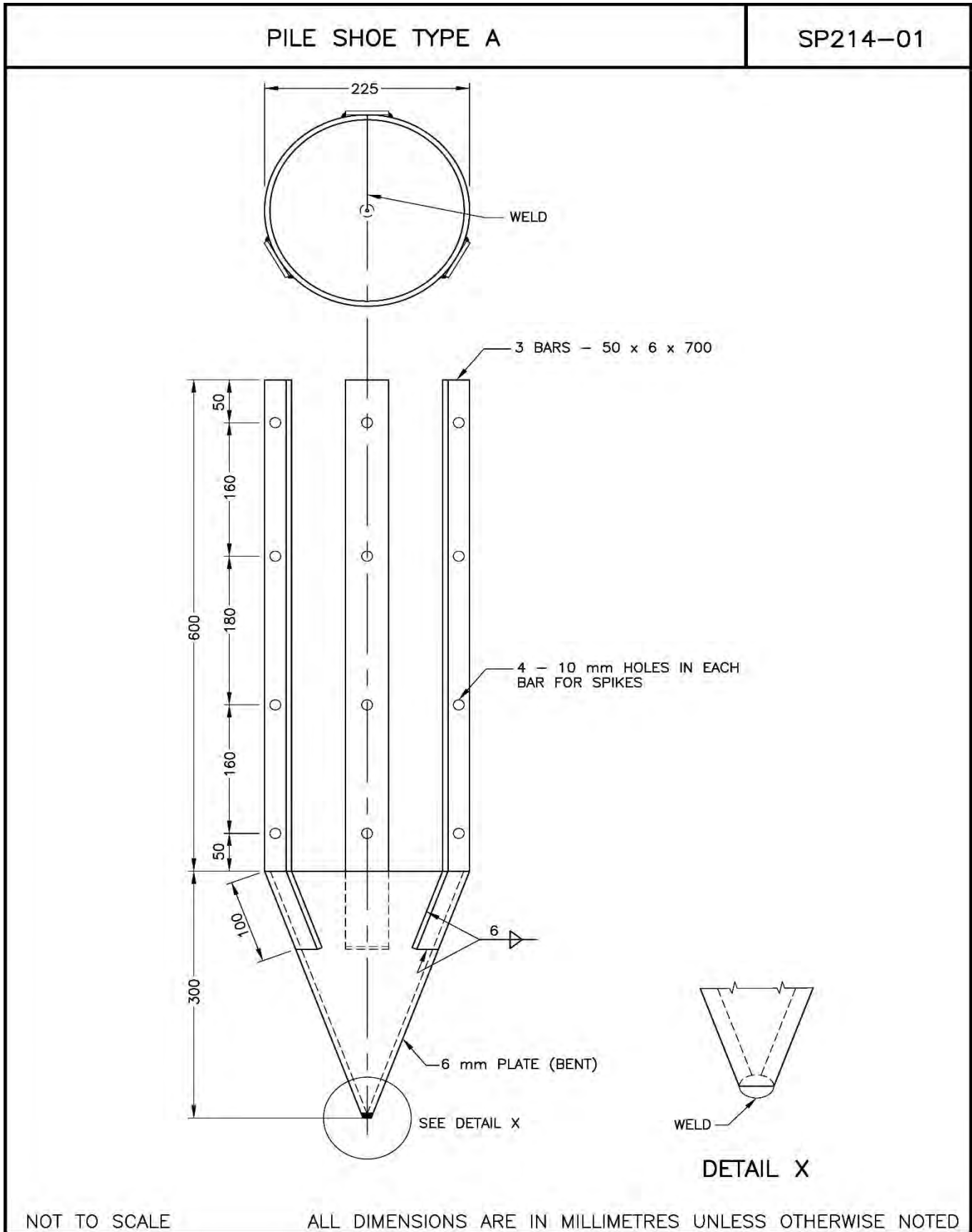
214.11.01 Mobilization and Fixed Costs for Pile Installation – Payment will be made at the Lump Sum price bid for providing, setting up and removing the plant and equipment for driving the piles, and shall be accepted as full compensation for everything furnished and done in connection therewith.

214.11.02 Supply – Payment will be made at the Unit Price bid per metre for the total number of metres of piling as shown on the Drawings or as specified by the Ministry Representative. Such payment shall be accepted as full compensation for everything furnished and done in connection with the supply and delivery of the piles.

214.11.03 Installation – Payment shall be accepted as full compensation for all costs in connection with supply and installation of pile points and installation of piles.

If the length of piling is more than anticipated, the Ministry will pay documented costs of delivery of less than full loads of additional piling, without markup.

If pile points are not specified on the Drawings or Special Provisions, but are ordered by the Ministry Representative, payment will be as Extra Work.



SECTION 215

BRIDGES

215.01 General - Bridges shall be designed in accordance with the Canadian Highway Bridge Design Code (CHBDC), CSA S6 and the Ministry's [Bridge Standards and Procedures Manual](#), including the Ministry's [Supplement to CHBDC S6](#).

Construction shall be in accordance with the current Ministry standards, practices and the [Contract documents](#) prepared for the particular structure.

SECTION 216

COATING OF STEELWORK

216.01 Scope – This Section covers the operations required for shop, field and/or maintenance coating of structural and miscellaneous steel surfaces. The coating process includes pre-cleaning, surface preparation, coating, and curing of coatings.

216.02 Definitions

216.02.01 Good Painting Practice – The application of generally accepted methods and equipment to clean, contain and coat industrial structures in a safe, practical, environmentally sensitive manner resulting in a long-lasting quality protective coating system. These principals are laid out in this Section. Many of the points are drawn from the SSPC (Society for Protective Coatings) publication Steel Structures Painting Manual Volume 1, Good Painting Practice. References are to the current edition unless otherwise stated. These practices are based on experience in the United States and where they conflict with Canadian or British Columbia laws, regulations or guidelines, the local regulations shall prevail.

216.02.02 Shop, Field and Maintenance Coating – This consists of the series of operations that include the surface preparation, pre-treatment (if required) and application of coatings to structural steel surfaces either in the shop or in field. It also includes the supplying of all labour, equipment and materials. In addition, it encompasses the protection of newly coated surfaces during curing, transport and erection as well as the protection of traffic, property and the environment. Additionally, it also includes the disposal of waste material in accordance with all applicable environmental, fisheries and local regulations and bylaws.

216.02.03 Painting / Coating – The terms painting and coating are considered synonymous for the purposes of this document. Techniques such as metallizing or galvanizing, while not typically thought of as painting, are considered here to be painting or coating since they form part or all of a protective system.

216.02.04 Shop Coating – Shop coating is the coating of steel or other components in a shop or plant before shipment to the site of erection. This may include part of a coating system or the whole system including the topcoat.

216.02.05 Field Coating – Field coating is the coating of steel or components at the place of erection.

216.02.06 Maintenance Coating – Maintenance coating is the coating of structures in service that have previously been partially or completely coated. The maintenance coating may be one of four general types:

(a) **Touchup** – Touchup is the cleaning and coating of small selected areas on a structure. Typically, this is used to repair damaged or failed areas of coating. For

repairs, sufficient coats are added to bring the new coating up to the level of the existing coating surrounding the repairs. It may also be used as an interim maintenance strategy to extend the lifetime of a coating system overall.

(b) **Zone Coating** – Zone coating is the coating process where specific whole parts of a structure are coated and others are deliberately left. This deals with larger portions of a structure than touchup coating. For example, an above deck arch portion of a bridge would be zone coated, but the edges of the flanges would be touched up. Zone coating may use touchup, overcoat, or recoat strategies to meet its objectives. Zone coating also includes coating new steel added to old structures for example steel added for seismic upgrades.

(c) **Overcoating** – Overcoating is the coating of portions of a structure where the structure is touched up, then coated with a topcoat overall. It may include top coating part or all of the structure.

(d) **Recoating** – Recoating is the coating process where larger portions of old coating on a structure are removed to bare metal and a completely new coating system is applied over the entire area.

216.02.07 Anchor Profile – The anchor profile is a measure of the roughness of the prepared surface. The profile is normally measured from surface peak to surface valley and expressed as a range (for example 50 to 75 µm or 2 to 3 mils). Besides the depth, the surface profile is dependent on the grit type used to create it. Mineral abrasives such as sand, slag, garnet, etc. produce a ‘sharp’ profile. Steel shot produces a rounded profile unless mixed with 15% steel grit. Normally the anchor profile is specified for both depth and sharpness.

216.02.08 Containment / Hoarding – The process of encapsulating the work site so that hazardous or nuisance materials cannot escape to the surrounding environment. SSPC Guide 6 Guide for Containing Debris Generated During Paint Removal Operations specifies types and levels of containment necessary for various cleaning operations.

216.02.09 Ambient Conditions – Ambient conditions refer to such things as wind, relative humidity, dew point, air temperature, substrate temperature etc. that affect the cleaning, application, and curing of coatings. They are very important in obtaining a properly adhered, long life coating. All coating works shall be within the specified parameters as given in SS 216.08.04, SS 216.08.05 and SS 216.08.09. Where ambient environmental conditions do not fall within these conditions, coating work may continue if the conditions are being met within the containment. This may mean extra hoarding, dehumidification, ventilation and/or

SECTION 216

COATING OF STEELWORK

temperature control for the duration of cleaning, application and curing of the coating system.

It is worthy of note that some coatings are designed to be applied in conditions outside those mentioned above. These coatings and their conditions of use will be subject to the approval of the Ministry Representative prior to use.

216.03 Reference Standards – It is recognized that standards are changed or amended from time to time. All specifications mentioned shall be the current editions at the date and time of tendering unless a specific edition of a reference or standard is stated here or in the Contract Drawings and Special Provisions.

216.03.01 Recognized Products List (RPL) – The Ministry publishes a Recognized Products List indicating products that are considered generally acceptable for particular purposes. This is available on the Ministry website at:

https://www2.gov.bc.ca/assets/gov/driving-and-transportation/transportation-infrastructure/engineering-standards-and-guidelines/recognized-products-list/recognized_products_list.pdf?forcedownload=true

Only those coating systems and products listed under the heading of “Coating Systems” in the RPL or as otherwise specified in the Special Provisions or on the Drawings shall be used on Ministry Projects. The Ministry may, at its sole discretion, designate alternative materials to fit a unique situation.

216.03.02 SSPC – Society for Protective Coatings – The SSPC publishes two volumes: SSPC Painting Manual Volume 1, Good Painting Practice and SSPC Painting Manual Volume 2, Systems and Specifications, which lay out the basics for painting steel structures.

The following SSPC standards and guidelines shall apply unless noted otherwise.

(a) Surface Preparation Standards

(i) **General** – SSPC-SP COM Surface Preparation Commentary for Steel and Concrete Substrates.

(ii) Visual Standards:

- SSPC-VIS 1, Guide and Reference Photographs for Steel Surfaces Prepared by Dry Abrasive Blast Cleaning
- SSPC-VIS 2 Standard Method of Evaluating Degree of Rusting on Painted Steel Surfaces
- SSPC-VIS 3, Guide and Reference Photographs for Steel Surfaces Prepared by Hand and Power Tool Cleaning
- SSPC-VIS 4, NACE No. 7 Guide and Reference Photographs for Steel Prepared by Water Jetting

- SSPC-VIS 5, NACE VIS 9 Guide and Reference Photographs for Steel Surfaces Prepared by Wet Abrasive Blast Cleaning

(iii) Surface Preparation Standards:

- SSPC-SP 1 Solvent Cleaning
- SSPC-SP 2 Hand Tool Cleaning
- SSPC-SP 3 Power Tool Cleaning
- SSPC-SP 5, NACE No. 1 White Metal Blast Cleaning
- SSPC-SP 5 (WAB)/NACE WAB-1, White Metal Wet Blast Cleaning
- SSPC-SP 6, NACE No. 3 Commercial Blast Cleaning
- SSPC-SP 6 (WAB)/NACE WAB-3 Commercial Wet Blast Cleaning
- SSPC-SP 7, NACE No. 4 Brush-off Blast Cleaning
- SSPC-SP 7 (WAB)/NACE WAB-4 Brush-Off Wet Blast Cleaning
- SSPC-SP 10, NACE No. 2 Near-White Blast Cleaning
- SSPC-SP 10 (WAB)/NACE WAB-2 Near White Wet Blast Cleaning
- SSPC-SP 11 Power Tool Cleaning to Bare Metal
- SSPC-SP 13, NACE No. 6 Surface Preparation of Concrete
- SSPC-SP 14, NACE No. 8 Industrial Blast Cleaning
- SSPC-SP 14 (WAB)/NACE WAB-8 Industrial Wet Blast Cleaning
- SSPC-SP 15 Commercial Grade Power Tool Cleaning
- SSPC-SP 16 Brush-off Blast Cleaning of Non-Ferrous Metals
- SSPC-SP WJ-1, NACE WJ-1 Waterjet Cleaning of metals Clean to Bare Substrate
- SSPC-SP WJ-2, NACE WJ-2 Waterjet Cleaning of metals Very Thorough Cleaning
- SSPC-SP WJ-3, NACE WJ-3 Waterjet Cleaning of metals Thorough Cleaning

- SSPC-SP WJ-4/NACE WJ-4 Waterjet Cleaning of metals Light Cleaning

(b) Abrasive Specifications:

- SSPC-AB 1 Mineral and Slag Abrasives
- SSPC-AB 2 Cleanliness of Recycled Ferrous Metallic Abrasives
- SSPC-AB 3 Ferrous Metallic Abrasive
- SSPC-AB 4 Recyclable Encapsulated Abrasive Media (in a compressible cellular matrix)

(c) Application Standards, Guides and Specifications:

- SSPC-PA COM Commentary on Paint Application
- SSPC-PA 1 Shop, Field and Maintenance Painting of Steel
- SSPC-PA 2 Procedure for Determining Compliance to Required DFT, Restriction Level 2
- SSPC-PA 17, Procedure for Determining Conformance to Steel Profile/Surface

(d) Guides:

- SSPC – Guide 6 Guide for Containing Surface Preparation Debris
- SSPC – PA Guide 11 Protecting Edges, Crevices, and Irregular Steel Surfaced by Stripe Coating
- SSPC – Guide 12 Guide for Illumination of Industrial Painting
- SSPC – Guide 14 Guide for the Repair of Imperfections in Galvanized, Organic or Inorganic Zinc-Coated Steel Using Organic Zinc-Rich Coatings
- SSPC – Guide 15 Field Methods for Extraction and Analysis of Soluble Salts on Steel and Other Nonporous Substrates

(e) Technical Updates:

- SSPC-TU 3 Overcoating
- SSPC-TU 6 Chemical Stripping of Organic Coatings from Steel Surfaces
- SSPC-TU 7 Conducting Ambient Air, Soil, and Water Sampling During Surface Preparation and Paint Disturbance Activities

- (f) Qualification Procedures** – These requirements will be laid out specifically in the Contract for each project and will depend primarily on the work to be done. Contractors may require current certification in one or

more of the following levels to perform Ministry work as called for in the particular tender.

- SSPC-QP COM Commentary on Qualification Procedures
- SSPC-QP 1 Certification (Field Application to Complex Industrial and Marine Structures)
- SSPC-QP 2 Certification (Field Removal of Hazardous Coatings)
- SSPC-QP 3 Certification (Shop Painting Certification Program)
- SSPC-QP 6 Thermal Spray Contractor Qualification
- SSPC QP 7 Certification (Painting Contractor Introductory Program)
- NACE Coating Inspection Program Level 2
- SSPC Bridge Coatings Inspection Program (BCI) Level 2

216.03.03 Hot Dip Galvanizing – ASTM A123 Standard Specification for Zinc (Hot Dip Galvanized) Coatings on Iron and Steel Products.

216.03.04 Metallizing – SSPC-CS23.00 / AWS C2.23M / NACE No. 12, Specification for the Application of Thermal Spray Coatings (Metallizing) of Aluminum, Zinc, and Their Alloys and Composites for the Corrosion Protection of Steel.

216.03.05 Colour Standards – Refer to AMS-STD-595A: Colors Used In Government Procurement. Topcoat colours shall be in accordance with the Special Provisions and/or the Drawings or as determined by the Ministry Representative. For weathering steel, unless noted otherwise, the topcoat colour shall match the expected colour of the oxidized surfaces (Federal Color 30045 unless noted otherwise). The proposed colour shall be subject to the acceptance of the Ministry Representative.

216.04 Environmental Considerations – All coating works including preparation shall comply with SS 165, Protection of the Environment.

216.05 Containment of Coating Operations

216.05.01 General – Full containment shall be provided to prevent contamination of the environment. This enables work to be accomplished by physically limiting the spread of debris and facilitates cleanup. Current containment technology is able to contain virtually all debris generated by washing, water jetting, hand, power tool, or abrasive blasting, and coating operations.

216.05.02 SSPC Guide 6 – Containment shall be provided in accordance with SSPC Guide 6, Guide for Containing Debris Generated During Paint Removal Operations. In general, the Ministry wishes to minimize the environmental

impact of all Ministry coating work. To this end, all coating works shall be fully enclosed to prevent contamination of the environment. The coating specification will contain specific language to exactly determine the desired containment end performance requirements. Containment shall be in accordance with the pertinent requirements of the Workers Compensation Act, the Occupational Health and Safety Regulation, and Environment and Fisheries Authorities.

216.05.03 Containment of Debris and Disposal of Debris

– All debris from cleaning and coating operations must be contained in such a way as not to contaminate the environment. All debris shall be contained, tested and disposed of in accordance with current environmental, fisheries and local regulations. The Contractor shall provide written proof of disposal of all materials including way-bills and transport logs.

216.06 Construction Site Safety

216.06.01 General – General site safety shall comply with SS 135 Construction Site Safety, the Workers Compensation Act and the Occupational Health and Safety Regulation

216.06.02 Specific Safety Issues Relating to Bridges and Coating Work

- (a) **Working at Height** – All work occurring at height shall conform to all Workers Compensation Act and Occupational Health and Safety Regulation requirements.
- (b) **Lead Coatings** – Many of the Ministry structures have coatings containing lead. While the Ministry will attempt to indicate the presence of lead coatings, it is the responsibility of the Contractor to ascertain the presence and extent of lead coatings if any; and to conform to all regulations for work undertaken. This includes but is not necessarily limited to the Workers Compensation Act, the Occupational Health and Safety Regulation and the Ministry of Environment regulation requirements for working with, storing, transporting, disposing of and documenting any work process involving lead coatings.
- (c) **Enclosed Spaces** – Many of the Ministry structures contain enclosed spaces or spaces that the Workers Compensation Act and the Occupational Health and Safety Regulation define as enclosed spaces. In some instances, the hoarding or containment is considered an enclosed space. It is the responsibility of the Contractor to ascertain whether a space is defined as an enclosed space and to adjust their work procedures to meet all regulation requirements.

216.07 Surface Preparation

216.07.01 General – Surface preparation of areas to be coated shall be in accordance with the specifications herein in SS216, the Drawings and the Special Provisions. Where

the specified surface preparation differs from the coating manufacturer's recommended surface preparation, the more stringent surface preparation, shall apply, unless approved otherwise by the Ministry Representative.

Where surface preparation is not defined herein in SS216, the Drawings and the Special Provisions, the manufacturer's specifications shall be used.

216.07.02 Washing

- (a) **General** – Washing is a procedure for removing surface contaminants including oil, grease, drawing and cutting compounds, soil, salts, etc. from either steel or coated surfaces. Removal of the contaminants shall be accomplished using any one or a combination of the following: potable water, alkaline cleaners, solvents, emulsion cleaners, steam cleaning (with detergents), high pressure water, scrubbing (with detergent) or other approved method.

Alkaline cleaners, detergents, solvents, etc. shall not be used over streams, lakes, rivers, etc. or agricultural land without first ensuring compliance with all applicable environmental legislation (federal, provincial and local).

The washing procedure shall be conducted so that any part of the structure which has already been cleaned, or newly coated, shall not be contaminated with the cleaning chemicals or solvents.

Immediately after treatment with cleaning chemicals, the surfaces shall be thoroughly washed with clean water to remove detrimental residues.

(b) Residual Non-Visible Contaminants (NVC) –

Immediately prior to abrasive blast cleaning and immediately prior to the application of any coating, surfaces shall be checked for residual non-visible contaminants in accordance with SSPC Guide 15 swabbing extraction method 5.2.5 Latex Sleeve Methodology. Surfaces that exceed any of the following "designated NVC levels" shall be cleaned in accordance with SSPC-SP WJ-4/NACE WJ-4 to achieve these levels:

- 7 µg/cm² chloride ion,
- 10 µg/cm² soluble ferrous ion, and
- 17 µg/cm² sulfate

216.07.03 Water Cleaning/Jetting – Waterjet cleaning is the use of clean, pressurized water for removing coatings and other materials, including hazardous materials, from a substrate to achieve a defined degree of surface cleanliness. Waterjet cleaning includes various methods such as low-pressure water cleaning (LP WC), high-pressure water cleaning (HP WC), high-pressure water jetting (HP WJ), and ultra high-pressure water jetting (UHP WJ). Four levels of surface cleanliness are each addressed in separate standards (SSPC-SP WJ-1, to SSPC-SP WJ-4)

Water jetting is inherently dangerous and safety precautions as listed in SSPC Steel Structures Painting Manual, Volume 1, Good Painting Practice, shall be followed. Where the current Workers Compensation Act and the Occupational Health and Safety Regulation conflict with any of the SSPC-SP WJ specifications, the Workers Compensation Act and the Occupational Health and Safety Regulation shall prevail. Water used in water jetting units shall be clean and free of contaminants that may leave deposits on the surface being cleaned or affect the adhesion of subsequent coatings applied. Water supply to the water jetting units must meet current Federal and Provincial regulations for water taken from streams if on-site water from streams is used.

All waste water and coating debris shall be contained and disposed of in accordance with Environmental and Fisheries Agencies or other applicable regulations.

Wastewater must meet regulations for disposal on-site if it is to be disposed of on-site. These regulations are typically those of Fisheries and Oceans Canada and the Ministry of Environment, but may not be limited to those governmental agencies. As well, SS 165 Protection of the Environment lays out Contractor responsibilities.

216.07.04 Manual and Power Tool Cleaning – Manual cleaning is a procedure for removing loose mill scale, loose rust, loose coating and other detrimental foreign matter by chipping, scraping, grinding, and wire brushing, etc. The tools used for this method of cleaning may be either hand or power tools; they shall be of high quality and maintained in good repair.

The standard of cleaning obtained by this method shall be SSPC-SP 2, SP 3, SP 11, or SP 15 as specified in the Contract. Where surface preparation is not defined in the Contract, the manufacturer's product data sheet specifications shall be used.

In specific circumstances the methods available may be limited by the Drawings and Special Provisions.

In all four preparation methods listed above, the surface shall be cleaned of dirt, grease, and soluble salts prior to treatment. Heavier layers of rust shall be removed by chipping, before any of the other cleaning tools are used.

Finally, all traces of dust shall be removed prior to coating.

216.07.05 Abrasive Blast Cleaning – Abrasive blast cleaning is a procedure for removing mill scale, rust, rust scale, coating or other foreign matter by use of abrasives propelled through nozzles or by centrifugal wheels. It is also used to roughen the surface of the substrate to enhance adhesion.

If compressed air blasting is employed, proper line filters and dryers, which remove all oil and water from the air, shall be used. Compressed air shall be periodically tested for presence of these contaminants.

The standard of cleaning obtained by this method shall be SSPC SP 5, SP 6, SP 7, SP 10, or SP 14 as specified in the Drawings and Special Provisions. Where the standard of cleaning is not defined in the Drawings and Special Provisions, the manufacturer's specifications shall be used.

Prior to abrasive blast cleaning, the surface shall be cleaned of dirt, oil, grease and residual ions in accordance with SS 216.07.02. If necessary, heavier layers of rust shall be removed by chipping before the blast cleaning operation commences.

A cleaned and blasted surface shall be coated within a maximum of 8 hours of cleaning or less depending on humidity, temperature or other factors. The surface immediately prior to coating shall meet the specification in its entirety. The Ministry shall be the sole arbitrator on determining the minimum time between surface preparation and coating. If the surface preparation criteria are not met immediately prior to coating, the surface shall be rejected and re-prepared in accordance with the specification.

Finally, all traces of dust and sand, etc. shall be removed prior to coating.

216.07.06 Abrasive Materials – Abrasive materials, unless specified, must be accepted by the Ministry Representative. Silica Sand shall only be used in accordance with the Workers Compensation Act and the Occupational Health and Safety Regulation.

216.07.07 Recycled Steel Abrasive – Recycled steel abrasives are permitted, however the working material must meet SSPC-AB 2 requirements and must contain a minimum 15% steel grit to produce an angular/sharp profile.

216.07.08 Containment of Debris and Disposal of Debris – All debris from cleaning, blasting and coating operations must be contained in such a way as to not contaminate the environment. All debris shall be contained, tested and disposed of in accordance with current environmental, fisheries and local regulations. The Contractor shall provide written proof of compliant disposal of all materials including way-bills and transport logs.

216.08 Application of Coating Materials

216.08.01 Pretreatment – Newly cleaned surfaces may require pretreatment other than washing or abrasive blasting prior to the application of the prime coat of paint. The application of the pretreatment shall be strictly in accordance with the manufacturer's directions. When pretreatments are used, particular care shall be taken to prevent contamination of adjacent surfaces.

216.08.02 General – In general, the Contractor shall follow SSPC-PA 1 Shop, Field, and Maintenance Painting of Steel when applying coatings. Where SS 216 and SSPC-PA 1 disagree, SS 216 shall be taken as applicable.

216.08.03 Materials Storage and Handling – Materials handling and use shall be as per SSPC-PA 1. Coatings shall

be stored in a secure building on-site, but not on the structure itself. Coatings taken onto the structure in cans, pails or totes shall be contained so that should they spill, no coating will escape to contaminate the adjacent area or environment.

216.08.04 Temperature – Coatings shall only be applied when the ambient, surface and coating temperature are within the range indicated by the manufacturer's written instructions. Coating shall not be applied when the substrate temperature is less than 3°C above the Dew point. Winter or quick dry catalysts or coating formulations shall not be used. Some coatings are designed for use in marginal conditions. Their use is at the sole discretion of the Ministry Representative. In unique situations where coatings must be used outside these limits, they shall only be used at the sole discretion of the Ministry Representative. Lack of permission may necessitate removal, re-cleaning and reapplication at the Contractor's expense.

216.08.05 Humidity – Coatings shall not be applied in wet weather, fog, mist, rain, snow and/or relative humidity in excess of 85%. Coatings shall not be applied to wet, damp or icy surfaces and care shall be taken to ensure that any overnight dew has completely evaporated before coating commences in the morning.

216.08.06 Mixing and Thinning – All ingredients in any container of paint shall be thoroughly mixed before use and shall be agitated often enough during application to keep the paint in a uniform condition. In all cases of heavily pigmented paints (e.g. "Zinc Rich" paints) the material shall be continuously agitated during application.

Mixing shall be done in accordance with the manufacturer's instructions and in accordance with good painting practice as outlined in SSPC Steel Structures Painting Manual, Volume 1, Good Painting Practice and SSPC-PA 1. No coating system shall be applied beyond one year from the manufacturing date.

Thinner shall only be added with the written consent of the manufacturer and only after it is shown that satisfactory application cannot be obtained with proper adjustment of the spray equipment and air pressures. In no case shall more than 12.5% of thinner by volume be added to a paint.

Only thinner specified and supplied by the manufacturer for that product shall be added to a product. Thinners used for cleanup shall not be recycled as thinning agents. Paint shall be mixed by mechanical methods. Paint shall be mixed in a manner that will ensure breaking up of all lumps, complete dispersion of settled pigment and a uniform composition. After mixing, all paint shall be strained to remove skins and other undesirable matter; the strainer shall not be so fine as to remove any pigment.

Thinner, when approved by the Ministry Representative, shall be added after initial mixing is completed. The quantity of thinner added shall be measured in and

thoroughly mixed until the paint is again of uniform consistency.

216.08.07 Induction Time – Manufacturers' induction times shall be strictly observed before the coating is applied. Since induction times are a function of temperature, temperatures outside the normal range may necessitate changing the induction period. Revised induction times may be accepted by the Ministry Representative, subject to the coating manufacturer providing the revised induction times in writing.

216.08.08 Pot Life – Manufacturer's estimates of pot life shall be adhered to and no coating shall be used after the recommended pot life has expired. Since pot life is a function of temperature, in hot or cold conditions the pot life shall be revised to reflect the ambient temperature. Written revised pot life recommendations from the coating manufacturer must be reviewed and accepted by the Ministry Representative prior to their use.

216.08.09 Wind – Paint shall not be sprayed when the wind speed or wind gusts are over 15 km per hour. Wind speed shall be measured where the work is taking place, for example within the containment.

216.08.10 Curing Conditions – Coating lifetime may be affected by curing conditions. Coatings shall be protected from rain during their curing. Coatings shall be cured under the coating manufacturer's stated conditions. This may require heating, cooling, humidity or ventilation adjustments to bring the ambient conditions into line with the coating manufacturer's recommended conditions for curing. In enclosed spaces, ventilation must be provided to remove evaporation from the coating.

Failure to achieve proper curing or curing conditions may necessitate removal, re-cleaning, reapplication or repair of the coating at the Contractor's expense.

216.09 Inspection

216.09.01 General – The Contractor shall provide the quality control functions on the Contract work. Quality assurance will be done by the Ministry. The Contractor shall produce a quality plan for all work to be done and submit this plan for the Ministry's review and acceptance. The Ministry will audit the quality process via the Ministry Representative.

216.09.02 Quality Control Inspection – Quality Control inspectors shall be a third-party qualified NACE CIP Level 2, or SSPC BCI Level 2 (i.e. Certified Coating Inspectors independent from the applicator). Inspection records produced shall be forwarded to the Ministry Representative daily. The records shall be electronic files, preferably Microsoft Excel files, and shall be forwarded to the Ministry Representative in that format to speed dissemination and aid any data manipulation required for the quality audit. Where this is not possible due to lack of communication technology, the records may be hard copy or hand delivered electronic files. The Quality Control

inspector(s) shall follow the quality plan as accepted by the Ministry.

All washing and cleaning work shall be inspected by the Contractor before any coating is applied. Each coat of paint shall be inspected by the Contractor and passed by the Quality Control Inspector before the succeeding coat is applied.

Coated surfaces rejected shall be made to comply with all requirements of this specification at the sole expense of the Contractor.

(a) **Hold Points** – Hold points (as defined in SS 145.12) for inspection shall include, but may not be limited to:

- Post installation of scaffolding and other accesses
- Post installation of containment
- On site coating systems review (manufacturing dates/batch numbers, etc.)
- Surface after washing
- Surface after cleaning prior to coating application
- Coating product mixing/thinning
- Surface after Prime coat
- Surface after Penetrating Sealer if any
- Surface after Stripe Coat
- Surface after each midcoat
- Surface after caulking if any
- Surface after topcoat
- Post removal of containment
- Post removal of scaffolding and other accesses and repair of any coating damage

(b) **Accessibility for Inspection** – The Contractor shall provide the Ministry Representative safe access to the specific areas of the jobsite under inspection. In order to limit the work disruption, the Contractor shall notify the Ministry Representative of upcoming hold points at least 24 hours prior to the hold point. The Ministry may perform quality assurance/audit inspections in conjunction with or subsequent to the Contractor's quality control inspections. The Ministry Representative will attempt to limit delays to the Contractor as much as possible, but retains the right to obtain as much information from the quality assurance/audit inspections as needed to justify acceptance or rejection of the Work.

The Contractor shall maintain a minimum 200-foot-candles illumination at all areas of the work to be inspected. This may include adding lighting to augment natural light. Lighting if added shall be according to SSPC Guide 12, Guide for Illumination of Industrial Painting Projects.

216.09.03 Thickness Measurement – Large areas amenable to the application of SSPC-PA 2 shall be measured by this method. The use of PA 2 is not considered to limit the number of measurements should additional readings be needed for any reason.

The Contractor shall set aside a suitable area of steel on the structure to be coated and dry abrasive blast clean the area with the Contractor's equipment and blast abrasive to the standard required by the coating specification. This area shall be protected from moisture and shall be used as a standard for acceptable surface cleanliness and anchor profile. A portion of the area shall also be used as an area to verify the calibration of dry film thickness (DFT) gauges. It is recognized that the magnetic plane on the blast cleaned surface is above the bottom of the profile. The Contractor shall fill that difference with coating. If requested by the Ministry Representative, the Contractor shall produce and calibrate a secondary, portable calibration standard for dry film thickness gauges. The dry film thickness of small and/or more complex areas shall be measured as agreed upon by the Contractor's Inspector and the Ministry Representative. Gauges are to be calibrated according to SSPC-PA 2 at the beginning and end of each work shift.

216.09.04 Tinting Multilayer Systems – Each separate coat, penetrant, stripe, midcoat(s) and topcoat shall be coloured or tinted to differentiate it visually from the coats above and below. This is an aid to application and inspection and also serves as an indicator of wear for the life of the coating.

216.09.05 Coating Systems – SS 308 lists coating systems used or identified by the Ministry and suggests situations where the systems are best applied. Coating systems meeting SS 308 requirements are listed in the Recognized Products List. Only specified coating systems from the Recognized Products List shall be used.

Occasionally, special requirements mandate the use of coating materials not already listed in SS 308. These requirements will normally be specified in the Contract Special Provisions section or will be shown on the Contract Drawings.

Unless otherwise specified, surface preparation, application and curing for individual coatings shall be in accordance with the manufacturer's Product Data Sheet. Coating thickness shall be within the range recommended by the manufacturer's Product Data Sheet as judged by SSPC PA-2 Level 3 inspection unless otherwise specified.

216.09.06 Coatings from Different Manufacturers – The Contractor shall not mix or interchange coatings or solvents from different manufacturers.

216.09.07 Coating Flaws – A coat of paint will only be accepted if none of the following faults are apparent:

- Coatings showing sags, runs, holidays, pinholes, orange peel, dry spray, shadowing or other application defects.
- Poor coverage at rivet heads, bolts, threads, plate edges, crevices, pockets, corners or re-entrant angles.
- Surfaces damaged mechanically by moving scaffolding, equipment, etc. or damage by rain, flying dust, etc. or damage from any other cause.

Sags and runs may be repaired or treated as per SSPC-PA 1.

Coatings shall be applied as per SSPC-PA_1, except where directed otherwise by this specification.

216.09.08 Rejection – Areas not meeting this specification shall be repaired at the sole expense of the Contractor. At the sole discretion of the Ministry Representative, small areas may be spot repaired. Larger areas shall be re-cleaned to specification and the coating built up again to specification.

216.10 Application Methods (Excluding metallizing and Hot Dip Galvanizing)

216.10.01 General – Coatings are designed to be applied by specific application techniques. These techniques are specified by the manufacturer to ensure that the risk of failure is minimized. Manufacturer's product application sheets shall be followed when applying coatings.

Where an application method is not possible due to circumstances at a particular structure, the Contractor shall obtain written instructions from the manufacturer for a new application method. Contractor use of the new application method is subject to approval by the Ministry Representative.

216.10.02 Brush or Roller Application – Brushes or rollers shall be of high quality and of a style and material that will enable proper coating application. The paint shall be brushed or rolled so that there are no runs or sags, minimum brush marks and a uniform thickness. Achieving the specified thickness for a coating may require more than one coat with brush or roller. The Contractor is responsible for achieving the specified thickness.

216.10.03 Air Spray Application – The equipment shall be capable of properly atomizing the paint to be applied. The equipment shall be kept clean and maintained in excellent condition. Equipment that has dirty or malfunctioning parts shall not be used until the parts are cleaned, repaired or replaced as necessary.

Paint shall be applied in a uniform layer, with overlapping at the edge of the spray pattern. The spray pattern shall be adjusted so that the paint is deposited uniformly. During application, the gun shall be held perpendicular to the surface and at a distance that will ensure that a wet layer of paint is deposited on the surface. The trigger of the gun shall be released at the end of each stroke.

All runs and sags shall be brushed out immediately, or the paint shall be removed and the surface repainted.

Areas inaccessible by spray such as cracks and crevices and the blind sides of rivets and bolts shall be painted by brush. Stripe coat material may be applied by spray to speed up the transfer of material to the surface, but all stripe coating must be brushed in to ensure complete wetting and coverage.

The air caps, nozzles and needles shall be those recommended by the manufacturers of both the material being sprayed and the equipment being used.

Proper air line filters and dryers shall be provided to remove oil and condensed water from the air. The air from the spray gun impinging against the surface and into a clean rag or blotter paper shall show no condensed water or oils.

The pressure on the material in the pot and of the atomizing air shall be adjusted for optimum spraying effectiveness. Both material and atomization pressures shall be controlled by suitable pressure regulators with gauges. The atomizing air pressure shall be high enough to properly atomize the paint, but not so high as to cause excessive fogging of paint, excessive evaporation of solvent or loss by overspray.

The air compressor used shall be of sufficient size to maintain the regulated air pressures constant at all times, irrespective of the amount of equipment that is being operated from the compressor.

216.10.04 Airless Spray Application – The equipment shall be capable of properly atomizing the paint to be applied. The equipment shall be kept clean and maintained in good working condition. Equipment that has dirty or malfunctioning parts shall not be used until the parts are cleaned, repaired or replaced as necessary.

Paint shall be applied in a uniform layer, with overlapping at the edge of the spray pattern. The spray pattern shall be adjusted so that the paint is deposited uniformly. During application, the gun shall be held perpendicular to the surface and at a distance that will ensure that a wet layer of paint is deposited on the surface. The trigger of the gun shall be released at the end of each stroke.

All runs and sags shall be brushed out immediately, or the paint shall be removed and the surface repainted.

Areas inaccessible by spray such as cracks and crevices and the blind sides of rivets and bolts shall be painted by brush. Stripe coat material may be applied by spray, to speed up the transfer of material to the surface, but all stripe coating must be brushed in to ensure complete coverage.

Fluid tips shall be of proper orifice size and fan angle, and fluid control gun of proper construction as recommended by the manufacturers of both the material being sprayed and the equipment being used.

The air pressure to the paint pump shall be adjusted so that the paint pressure to the gun is proper for optimum spraying

effectiveness. The pump pressure shall be controlled by a suitable regulator with working gauges.

Pressures higher than that necessary to properly atomize the paint shall not be used.

The air compressor used shall be of sufficient size to maintain the regulated air pressure to the pump constant at all times irrespective of the amount of equipment being operated from the compressor.

Proper air line filters and dryers shall be provided to remove oil and condensed water from the air. The air from the spray gun impinging against the surface and into a clean rag or blotter paper shall show no condensed water or oils.

The trigger of the gun shall be pulled fully open and held fully open during all spraying to ensure proper application of paint. The trigger shall be released at the end of each stroke.

216.10.05 Mitt and Dauber Application – Mitt and dauber application shall be limited to situations where other coating application methods are poorly suited. Their use shall be at the sole discretion of the Ministry Representative.

Coating thickness shall be uniform over the entire surface of the object coated. Coating shall be free of pinholes or other holidays, sags, etc.

216.10.06 Powder Coatings – Powder coatings are finely ground coatings that are electrostatically sprayed onto the substrate, then melted into a consolidated layer by heat. Coatings may be thermoplastic or thermosetting. Cleaning is typically in a series of chemical baths.

Substrates contaminated with soluble salts shall be washed to remove those salts before being cleaned to meet specification. Acceptable levels of soluble salts shall be at the “designated NVC levels” as stated in SS 216.07.02(b).

The substrate to be coated shall be cleaned to the coating manufacturer’s specifications. The water test shall show a clean surface.

If required for adhesion, a conversion coating shall be applied to the substrate before powder coating.

Fresh and reclaimed powder shall be dry, free flowing, and free of lumps and contaminants.

Soaking oven temperatures and residence times shall be sufficient to completely melt the powder, but not enough to oxidize or discolour the coating.

Coating thickness and gloss shall be uniform over the entire surface of the object being coated. Coating shall be free of pinholes or other holidays.

216.11 Application Methods (Metallizing and Hot Dip Galvanizing)

216.11.01 Metallizing – Metallizing shall be applied in accordance with SSPC-CS 23.00/AWS C2.23M/NACE No. 12.

Metallizing includes the thermal spraying of zinc, aluminum or their alloys onto a properly prepared metal substrate.

The method of heating the spray metal is either by flame or by plasma arc. Spray metal may be in wire or powder form to fit the apparatus used to spray the metal. The application method shall ensure that a uniform, clean, well adhered, film of metal is applied to the steel surface.

Surface preparation shall be SSPC SP5/NACE No. 1. Profile shall be sharp, 50 to 75 µm. Cut metal edges may be harder and consequently have less profile. If that is the case, they shall be ground back to softer metal and reblasted to achieve the specified profile.

Metallizing shall be applied by qualified personnel with documented field experience metallizing steel structures such as bridges.

Metallizing shall be applied in a uniform layer at a thickness of 150 to 200 µm thickness. For special applications, the thickness may be specified outside this range.

When the metallizing is to be overcoated to form a duplex system, the newly metallized surface shall be protected from contamination until coated. If the metallized surface becomes contaminated, the steel substrate shall be re-cleaned to SSPC SP5/NACE No. 1 as above and re-metallized.

Metallizing is porous, and application of a coating over the surface may require a mist coat or other special procedures to avoid bubbling and subsequent problems caused by the interaction of the porous coat with the coating being applied. This cost, if any, shall be borne by the Contractor.

Metallizing may be applied over a much wider range of ambient conditions than normal paint. When application conditions deviate outside normal coating conditions, the Contractor shall consult the Ministry Representative for acceptance before proceeding.

216.11.02 Hot Dip Galvanizing – Hot dip galvanizing shall be applied in accordance with ASTM A123 or ASTM A153, as applicable.

216.12 Application of Coatings

216.12.01 General – Application of coatings on steelwork specified to be painted as part of a Contract shall meet the following requirements.

(a) **Welding Requirements** – Areas to be field welded shall have no paint applied for a distance of at least 100 mm back from the weld area.

If inorganic zinc primer is used, the set-back shall be a minimum 25 mm.

All welds shall be cleaned of weld residue including weld spatter and roughness on the welds. All porosity and pin holes in welds shall be repaired by welding.

Sharp edges shall be rounded to a minimum radius of 1 mm. Burrs around drilled holes shall be removed.

All welds shall receive a stripe coat of midcoat material over the primer prior to the midcoat application.

Welding done on existing coated surfaces shall have the coating removed, past the heat affected zone or 100 mm from the weld whichever is more, down to bare metal. Surface profile of the cleaned area shall be 50-75 µm (2-3 mils).

(b) Faying Surfaces – Slip-critical bolted connection locations shall be as identified on the Drawings, Special Provisions or as directed by the Ministry Representative.

In the parts of a structure where steelwork is specified to be painted, the following shall apply:

(i) General – Over thickness will not be accepted. If over thickness occurs, then the surface will be cleaned and recoated at the Contractor's expense.

Touch up coating and partial repairs of faying surfaces shall not be permitted.

(ii) For slip-critical connections:

(A) Faying surfaces shall be blast cleaned and shall receive one coat of inorganic zinc primer. No midcoat or topcoat shall be applied.

(B) Primer coatings shall meet the Class B coating requirements as specified in CSA-S6.

(C) The class of the coating shall be determined based on the results from testing done in accordance with the "Specifications for Structural Joints Using High-Strength Bolts" issued by the Research Council on Structural Connections.

(D) A copy of the certificate of testing, the test procedures and results, and the test specimen preparation procedures for the Class B coating shall be supplied to the Ministry Representative for each coating used on faying surfaces.

(E) Faying surfaces shall have the same blast cleaning and coating application as was used in the tests to determine the mean slip coefficient. Coated joints shall not be assembled before the coating has cured for the minimum time used in the tests to determine the mean slip coefficient.

(iii) For connections that are not slip-critical:

(A) Faying surfaces shall be blast cleaned to SSPC-SP 5/NACE No. 1 and receive one coat

of inorganic zinc primer. No mid or top coat shall be applied.

(B) Primers shall be applied and cured in accordance with SS 216.

(c) Surfaces Inaccessible After Erection – Surfaces inaccessible after erection shall receive the field coating in the shop, before shipment to the erection site. Prior to erection, handling damage shall be repaired to the satisfaction of the Ministry Representative.

(d) Application and Curing of Successive Layers of Coating – Each layer of coating shall be in a proper state of cure or dryness before the application of the succeeding coat. Unless otherwise specified a minimum of overnight drying shall be required. Under poor drying and curing conditions, this period will be extended.

Curing conditions shall be those given by the coating manufacturer in the product application instructions. Curing time will vary according to temperature and in some cases humidity. The required curing time is estimated at the ambient conditions during curing. This will normally be done from data supplied by the coating manufacturer.

Coatings that have a maximum overcoating window shall not be overcoated after that time without appropriate surface preparation as provided in writing by the coating manufacturer. The overcoat window will be calculated from data supplied by the coating manufacturer using the ambient conditions at the Site.

(e) Primer – Primer shall not be applied until the cleaned steel surface meets specification requirements. Once specification requirements are met for a substrate, the primer must be applied within 8 hours. The substrate if protected will be assumed to continue to meet the specification for 8 hours unless obvious contamination has occurred. Determination of this will be at the sole discretion of the Ministry Representative.

Primer shall be applied to specification requirements. Should the specification requirements be different than the coating manufacturer's application instructions then the more stringent specification shall apply unless otherwise approved by the Ministry Representative.

(f) Penetrating Sealers – Penetrating sealers or low viscosity products designed to wick into seams and seal them shall be applied according to the coating manufacturer's product data sheets unless the Contract calls for specific application parameters. Should that be the case, the sealer shall be applied to the Contract requirements.

Current industry thinking has these products applied on top of a single primer coat and under stripe and midcoats.

Under certain circumstances it may be necessary to use a penetrating sealer as a primer or as a first coat. This will be at the sole discretion of the Ministry Representative.

- (g) **Stripe Coat** – All areas such as crevices, corners, rivets, bolts, nuts, welds, edges and other protrusions shall be given a stripe coat to ensure that there is sufficient coating dry film thickness. Unless it is specifically excluded, it is the intent of the Ministry that all such areas shall receive a stripe coat. Coating may be applied by spray to save time, but it shall be brushed into all areas to be striped.

The stripe coat is a separate coat and shall be applied at the thicknesses given for the midcoat coating.

The stripe coat shall be applied to the area and at least 25 mm out from the area to be striped.

The stripe coat shall be a contrasting colour to the coats above and below it.

The stripe coat shall be cured before being overcoated.

- (h) **Midcoat** – Midcoat shall be applied as a full coat onto clean, cured, primer, sealer, and stripe coats. The stripe coat and or sealer shall be cured before being coated with a subsequent coat.

- (i) **Caulking** – Caulking shall be applied after the midcoat and prior to the topcoat.

Caulking shall be compatible with the coating system used.

Silicone Caulking shall not be used.

Suitable caulking materials shall be in accordance with SS_308 and as listed in the RPL under the sub heading System PCM Paint Caulking Materials.

Caulking shall be applied as specified in the Contract.

Caulking shall not be top coated until it cures according to the manufacturer's product data sheets. Lower temperatures may increase this time.

If there is a time window for top coating, the caulking shall be top coated before the window expires.

- (j) **Topcoat** – Once the undercoats and caulking if any have cured, the topcoat shall be applied.

- (k) **Lapping Existing Coatings** – Primer if applied shall not overlap existing coatings by more than 10-20 mm. Ministry preference is that it overlaps very slightly to seal the interface joint.

Midcoat shall overlap existing coatings by a minimum 50 mm.

Topcoat shall overlap existing coatings by a minimum 50 mm and shall hide all newly added midcoat material.

216.12.02 Shop Coating – Unless otherwise specified, the following procedure shall apply to all new steel prior to shipment to the site of erection:

Before blast cleaning, the steel shall be checked for contamination and if necessary the surface cleaned in accordance with SS 216.07.02.

Before coating, the steel shall be blast cleaned to SSPC-SP 10/NACE No. 2 Near white blast. Abrasive blast cleaning will be permitted prior to fabrication provided that any contamination incurred during fabrication is removed prior to coating, leaving an SSPC-SP 10/NACE No. 2 surface. Small areas of contamination occurring during fabrication shall be cleaned by manual cleaning to SSPC-SP 3 Power tool cleaning and/or SSPC SP WJ-4/NACE WJ_4 as necessary to remove contamination. Large areas of contamination occurring during fabrication shall be dry abrasive blast cleaned again to SSPC-SP 10/NACE No. 2 Near-white blast.

Surface profile for primers shall be 50 to 75 µm and sharp.

Prior to dry abrasive blasting, all welding slag, spatter and alkaline deposits in the vicinity of the welds shall be removed. Surfaces shall then be blast cleaned to SSPC-SP 10/NACE No. 2 Near-white blast.

When fabrication is completed and the surfaces cleaned as required, they shall be inspected for radius of corners, weld spatter, unwelded interfaces, porosity or pin holes in welding and burrs around drilled holes prior to the application of paint.

After passing inspection and before any contamination has occurred, the Contactor shall prime all steel surfaces with one coat of the selected shop coat primer from SS_308. Primer shall be applied to specification requirements. A dry abrasive blast cleaned surface shall be coated within 8 hours of cleaning and before rust blooming becomes apparent. If either condition is exceeded then the area must be re-blasted to specification.

Clean or partially coated steel shall be protected from contamination including, but not limited to dirt, oil, water, grease, soluble salts and overspray, while in the shop. Contaminated surfaces shall be re-cleaned at the Contractor's expense.

Surfaces that will be in contact with concrete shall be coated with the full system prior to erection. Coatings with known incompatibilities with high pH shall not be used in contact with the concrete.

Surfaces inaccessible after erection shall receive the field coats of paint in the shop, before shipment to the erection site.

Faying surfaces shall meet the requirements of SS 216.12.01(b).

If more than one coat is to be shop applied, the manufacturer's recommended recoat times for the ambient temperatures shall be followed.

Generally, a minimum of 16 hours drying/curing time shall be required between the application of any paint in the shop and the moving or handling of any piece so painted. This period may be increased in cases where poor drying conditions exist, such as shop areas open to the weather in winter months. Since curing times vary with different coating products, variations from this time may occur with specific products. Blocking used to support structures during the coating applications will be moved after the curing of applied paint is complete and the surface area of the original blocking locations shall be prepared and painted as required by this specification.

Quick Dry/Cure products or products with "winter" catalysts shall not be substituted for SS_308 products unless accepted by the Ministry Representative.

216.12.03 Transport of Coated Items – Transport of coated items shall be done in a manner that imparts minimum damage to the coating system.

The coatings shall be cured before handling and shipment.

Canvas or Fabric slings shall be used to move the items. Chains or other methods that are known to damage coatings shall not be used.

Coated items to be transported shall have sufficient dunnage to prevent coating damage. Transport tie-downs shall be padded to protect the coated surfaces and edges.

Transport damage shall be repaired at the Contractor's expense.

216.12.04 Field Repair of Damaged Shop Coating and Field Weld Areas – After the complete erection of shop coated steel members the following procedure shall apply:

Before coating, field weld areas shall be ground off to remove all welding slag, spatter and alkaline deposits in the vicinity of the welds. Washing will be required when the shop coat or steel has been contaminated with air borne dirt, dust, salt, chemicals, etc., unless the steel shows no visible contamination, and meets the "designated NVC levels" as stated in SS 216.07.02(b) for non-visible contaminants. Surfaces shall then be cleaned to SSPC-SP 11. The existing coating shall be feathered back 25 mm from damaged areas into good coating. Feathering shall continue until the existing coating is well adhered. This may be considerably more than the 25 mm feather back.

Care shall be taken not to damage surrounding areas.

Note: This cleaning shall include the 100 mm welding set back.

All field connections and all areas of the shop applied coating system damaged during handling, shipping and erection shall be cleaned to SSPC-SP 3 or SSPC SP 11 at

the discretion of the Ministry Representative. Low pressure water cleaning shall be used as necessary to remove dirt, salts and other deleterious contamination.

All areas that have been cleaned to bare metal as required shall be coated with an organic zinc field primer, applied by brush to ensure full coverage. If the area is too large to coat practically with a brush, then spray may be used, providing overspray does not contaminate the surrounding finished coating.

Lapping of new and existing coatings shall be as specified in SS 216.12.01(k).

If a partial system was applied in the shop, then the primed areas shall be coated with successive coats to the level of the surrounding coating and then the entire structure shall be top coated. This treatment shall include a brushed-in stripe coat.

If a coating system was completely applied in the shop and on erection some damage has occurred or there are still uncoated areas, then these areas shall be cleaned in accordance with this section and successively coated with each coat of the paint system up to the topcoat including a brushed-in stripe coat. Manufacturer's recoat times for the ambient temperatures shall be followed.

216.12.05 Field Coating – Remaining coats shall be applied as specified.

Faying surfaces shall meet the requirements of SS 216.12.01(b).

(a) Touch-up coating- This section deals with the repair of small areas of coating. The definition of small areas, will vary depending on the situation, but in general, will apply to areas that can be economically treated using the touch-up technique.

The coating used for touch up and repair shall be one of the SS 308 Field Overcoat (FO) systems unless otherwise specified.

The area to be repaired and coated shall be washed in accordance with SSPC SP WJ-4/NACE WJ-4, to achieve a surface cleanliness as per SS 216.07.02(b) before attempting further cleaning. If the area meets the "designated NVC levels" as per SS 216.07.02(b) and is free of dirt and detritus, washing may be omitted. Areas of the coating that have failed or rusted shall be cleaned to SSPC-SP 3 or SSPC-SP 11 using the appropriate tools.

The old coating shall be feathered back 25 mm from damaged areas into good coating. Feathering back shall continue until the coating remaining is well adhered. This may be considerably more than the 25 mm feather back.

Pack rusted crevices will require more thorough cleaning. The degree of cleaning required will be detailed in the Contract Documents for each specific

structure. After cleaning, they shall be coated with the prime coat and then a brush coat of penetrating sealer shall be applied to the crevice and allowed to soak in. Repeated applications of the penetrating primer may be necessary until the inside of the crevice is saturated. Where the coating system used does not include a specific primer coat, the penetrating sealer shall be applied as a first coat.

In general, coating used for touch up and repair shall be of the same type as those used initially. For example if the coating system to be touched up is an alkyd system, then alkyds are the coatings of choice. The coating to be used shall be subject to acceptance by the Ministry Representative or as specified in the Contract.

The selected primer shall be applied in the damaged areas. The primer shall be cured for the time recommended by the manufacturer prior to additional coating being applied

After the primer is cured, the midcoat(s) shall be applied to bring the surface of the coating to the same level as the surrounding sound coating. The midcoat shall be applied within the manufacturer's suggested range of thickness. The midcoat(s) shall be cured for the time(s) recommended by the manufacturer.

After the midcoat material is cured, the area shall be given a topcoat. Where practical, the topcoat shall be extended to a logical edge such as the edge of the beam or plate, to present a less patchy appearance.

Lapping of new and existing coatings shall be done in accordance with SS 216.12.01(k).

- (b) **Overcoating** – The object is to clean the surface, repair spot or area damage with coatings and to overcoat the entire area with a topcoat.

The coating used for touch up and repair shall be one of the SS 308 Field Overcoat (FO) systems.

The area to be repaired and coated shall be washed to the requirements of SSPC-SP, WJ-4/NACE WJ-4, with a surface cleanliness meeting the “designated NVC levels” as per SS 216.07.02(b) before attempting further cleaning. If the area meets the “designated NVC levels” as per SS 216.07.02(b) and is free of dirt and detritus, washing may be omitted. Areas of the coating that have failed or rusted shall be cleaned to SSPC-SP 3 or SSPC-SP 11. Larger failed areas shall be cleaned to SSPC-SP 6/NACE No. 3. In this case, surface profile should be 50-75 µm and sharp.

Debris contaminating the rest of the structure or the immediate environment due to cleaning operations shall be removed by the Contractor.

The old coating shall be feathered back 25 mm from damaged areas into tightly adhered coating. If the

coating cannot be removed with a dull putty knife, it is considered tightly adhered.

Pack rusted crevices shall be cleaned in accordance with SS 216.12.05(a).

Once all the existing damaged areas are built up to the level of the existing coating, the topcoat is applied over the entire area to seal the existing coating and all the repairs.

In general, coating used for overcoating shall be of the same type as those used initially. For example if the coating system to be touched up is an alkyd system, then alkyds are the coatings of choice. Use of an overwintered test patch is recommended to test compatibility. The coating to be used shall be subject to acceptance by the Ministry Representative or as specified in the Contract.

- (c) **Recoating** – The area to be recoated shall be washed to the requirements of SSPC-SP WJ-4/NACE WJ-4, to achieve a surface cleanliness meeting the “designated NVC levels” as per SS 216.07.02(b) before attempting further cleaning. The entire area shall be blast cleaned to SSPC-SP 6/NACE No. 3. Note that some specialty coatings or coatings in some difficult applications such as immersion may require SSPC-SP 10/NACE No. 2 or even SSPC-SP 5/NACE No. 1.

Surface profile should be 50 – 75 µm and sharp.

The Contractor shall provide full containment of debris generated by the cleaning, coating and curing operations.

Debris contaminating the rest of the structure or the immediate environment due to cleaning operations shall be removed by the Contractor.

Sharp edges shall be radiused to 2 mm.

- (d) **Zone Coating** – Zone coating may use any one of or a combination of touch-up, overcoating or recoating on limited areas of a structure.
- (e) **Field Curing of Coatings** – Each coating application shall be cured before it is coated over or exposed to weather or traffic.

Coatings inside enclosed spaces shall continue to be ventilated until dry or cured to ensure that they cure correctly.

Uncured coating shall be protected against damage from dust or other detrimental foreign matter. If necessary, coating shall be delayed until the cause of the dust, etc. has abated. Uncured coating shall be protected from the weather until cured. In some cases, this may include hoarding, heating, ventilation or dehumidification. Direct propane heaters shall not be used for heating during the painting or curing process. Coating contaminated or damaged by any of the above

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conditions shall be repaired at the Contractor's expense.

216.13 Damage to Adjacent Property – It is the intention of the Ministry that the coating work shall have no adverse effect on adjacent property. Repair of damage to adjacent property, vehicles, pedestrians and other portions of the structure due to the Contractor's coating operations is the sole responsibility of the Contractor.

COATING OF STEELWORK

216.14 Cleanup – All construction and painting debris must be removed from the site and the site environs. The site and surroundings shall be left in the same condition as prior to work being commenced.

216.15 Measurement and Payment – Measurement and Payment shall be as set out in the Contract Documents.

SECTION 218

CONCRETE FOR MINOR WORKS

SCOPE

218.01 General – SS 218 shall be used only where explicitly specified by the Ministry in the Contract.

Concrete produced under SS 218 is intended to be used only for minor concrete works for Ministry infrastructure such as:

- non-structural reinforced and non-reinforced cast-in-place concrete, in applications such as fence post and guardrail embedment, and thrust blocks
- Minor structures such as small culvert endwalls.

For infrastructure that will be owned by a third-party, such as a municipality, the specifications of that owner will normally apply (such as MMCD).

MATERIALS

218.10 Cement – Cement shall conform to the requirements of CSA A3000 and CSA A23.1.

218.11 Aggregate – Concrete aggregate shall be tested for sulphate content in accordance with CSA A23.2-3B.

218.12 Concrete – Concrete for minor works shall be in accordance with CSA A23.1 – Concrete materials and methods of concrete construction.

Unless otherwise specified, concrete shall meet the requirements of Table 218-A, Class A concrete, and the requirements of the sulphate exposure category identified in the contract.

218.13 Mix Design and Performance – The Contractor shall be responsible for the performance of the concrete, carrying out all testing to verify performance, quality control of all aspects of the concrete operations and for proportioning all concrete mix designs.

The mix shall be designed to be compatible with the concrete aggregate, as well as to meet the sulphate exposure identified in the Contract.

The Contractor shall submit documentation demonstrating that the concrete mix design and constituent components

meet the specified requirements and providing the details of the quality control work plan. The documentation shall be submitted to the Ministry Representative at least two (2) weeks in advance of the when concrete production is scheduled to commence.

No concrete shall be placed prior to the completion of the Ministry review and acceptance of the documentation.

Table 218-A: Classification of Concrete Mixtures:

Class of Concrete	Required Strength at 28 Days (MPa)	CSA A23.1 Class of Exposure
A	32	C-2*
B	32	C-2*
C	25	C-4
X	32	C-2*
Y	32	C-2*

*Class of exposure shall also be compatible with the sulphate exposure category identified. If no exposure category is identified the class of exposure shall also meet the requirements of S-2.

PAYMENT

218.90 Basis of Payment – Payment for Concrete for Minor Works is usually incidental to another pay Item.

Accordingly, no direct payment shall be made for Concrete for Minor Works unless the "Schedule of Approximate Quantities and Unit Prices" includes a specific pay Item for Concrete for Minor Works.

When there is a specific pay Item, payment shall be made at the price bid per cubic metre of concrete, measured in-place in and forming part of the finished Work. Payment shall be accepted as full compensation for everything furnished and done in connection therewith.

SECTION 220

SHOTCRETE AND SOIL ANCHOR WALLS

SECTION UNDER DEVELOPMENT

SECTION 232

METAL BIN-WALL

232.01 Scope – This Section covers the construction of metal bin type retaining walls (Metal Bin-Wall) in accordance with the general layout and details indicated on SS Drawings 1-SP232 through 3-SP232.

232.02 Provision of Metal Bin-Wall – Metal Bin-Walls of the design (depth to height ratio) called for shall be constructed at the locations and as shown on the Drawings with the materials, accessories, and necessary ancillary work all in accordance with the details indicated on the Drawings, Specifications, Special Provisions, the SS Drawings or as directed by the Ministry Representative. Any additional installation requirements specified by the Metal Bin-Wall supplier shall also be adhered to by the Contractor.

The Contractor shall provide the manufacturer's drawings (shop drawings) to the Ministry Representative for review at least two (2) weeks prior to components being delivered to Site. The shop drawings shall clearly show the following:

- height and depth of all bins;
- thickness of all stringers, spacers, verticals and other components;
- material specifications;
- assembly instructions; and
- any special details.

232.03 Materials – Metal Bin-Wall materials will be supplied by the Contractor in accordance with the Contract.

Galvanized steel sheet and galvanized fasteners shall be in accordance with CSA G401. Vertical connectors and base plates shall be in accordance with CSA G40.21 Grade 260W minimum and shall be galvanized in accordance with ASTM A123.

232.04 Construction

232.04.01 Construction – Construction shall be carried out with all labour, tools, equipment and incidentals necessary to complete all Metal Bin-Wall construction in accordance with good work practice.

232.04.02 Foundation – Foundation excavation shall be carried down to the depths shown on the Drawings or to such greater depths as the Ministry Representative may direct. In the case where rock, hardpan or other unyielding material is encountered, it shall be removed to a depth 300 mm below the design grade of the bin-wall footing and backfilled in accordance with SS 232.04.04.

232.04.03 Erection – The units shall be assembled as shown on the drawings. Components shall be handled

carefully and any which are damaged shall be removed and replaced with new components at the Contractor's expense.

Minor damage, in the opinion of the Ministry Representative, to the galvanized coating shall be repaired by coating with two heavy coats of zinc rich paint selected from the Ministry's Recognized Products List under the category of "Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers".

Before the assembly bolts are tightened, columns shall be checked for line and batter or plumbness, where applicable.

232.04.04 Backfill – Structure backfill material shall be in accordance with the material requirements for Bridge End Fill (BEF) as specified in SS 202.

Structure backfill material shall be placed as wall foundation, backfill inside the bins, and for 0.6 m outside the wall. The material shall be spread in layers not exceeding 150 mm in thickness and shall be compacted to a minimum 95% Standard Proctor Maximum Dry Density obtained by the current ASTM test method D698.

Backfill inside each bin shall progress simultaneously but not in advance of that outside the wall.

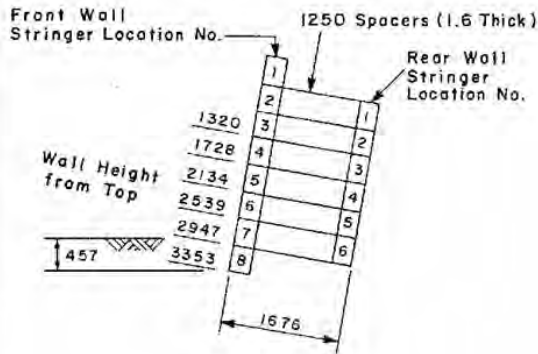
Note: Bin-wall may be backfilled as it is erected provided that the columns are held in correct position while the backfill is being placed.

232.05 Measurement and Payment – Metal Bin-Wall shall be paid for at the Unit Price bid per square metre of facial area of wall, measured on the batter and including the toe burial. Payment shall be full compensation for work required to supply and install the Metal Bin-Wall, including but not limited to, supply, hauling to the Site, and erection of all steel components and also supply, hauling, placement and compaction of the structure backfill.

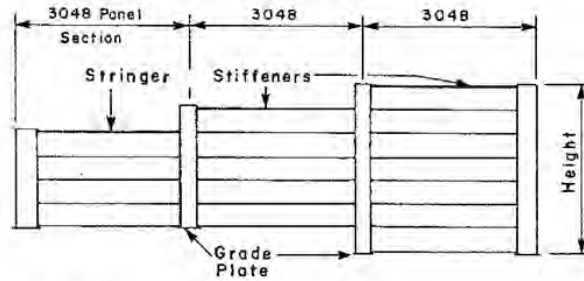
Excavation for foundation shall be measured and paid for in accordance with and at the rate bid for either "Foundation Excavation" in accordance with SS 407 or for "Roadway Drainage and Excavation" in accordance with SS 201, as specified in the Contract Documents.

Backfill placed beyond 0.6 m from the face of the Metal Bin-Wall shall be incidental to "Foundation Excavation" in accordance with SS 407 or "Roadway Drainage and Excavation" in accordance with SS 201.

DESIGN A

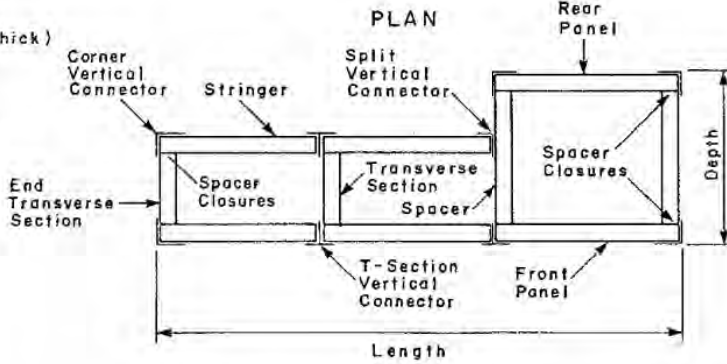
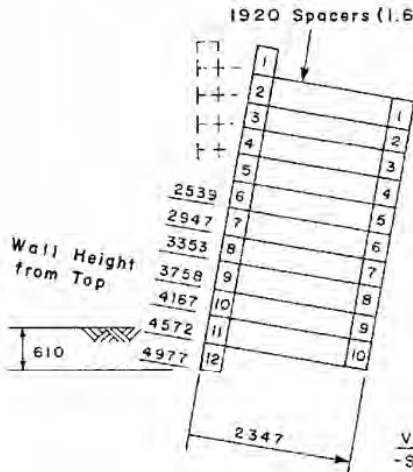


ELEVATION



Front Wall Stringer Location No.	Rear Wall Stringer Location No.	Thickness
1 through 8	1 through 6	1.6
9 through 12	7 through 10	2.0
13 through 16	11 through 14	2.8

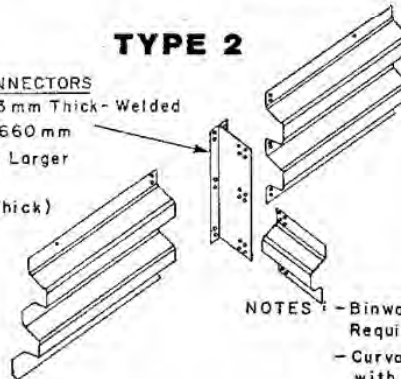
DESIGN B



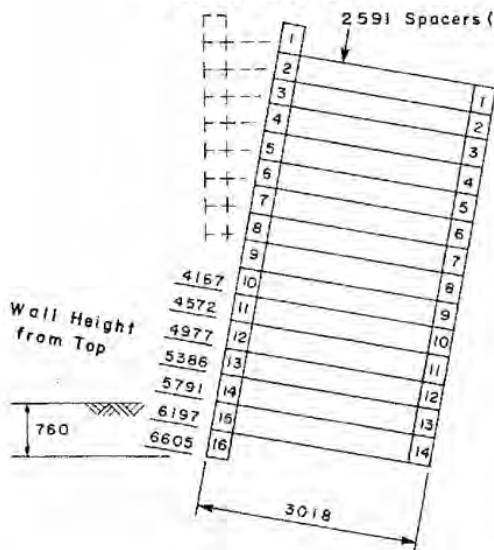
TYPE 2

VERTICAL CONNECTORS

- Steel Plate 6.3 mm Thick - Welded
- Max. Length 3660 mm
- Splice Plate if Larger



DESIGN C



- NOTES**
- Binwall Batter = 1:6 or Vertical as Required by Contract Drawings.
 - Curvature Detail in accordance with Suppliers Manual.
 - All measurements in millimetres.
 - For Type 1 see Dwg. 3-SP 232.

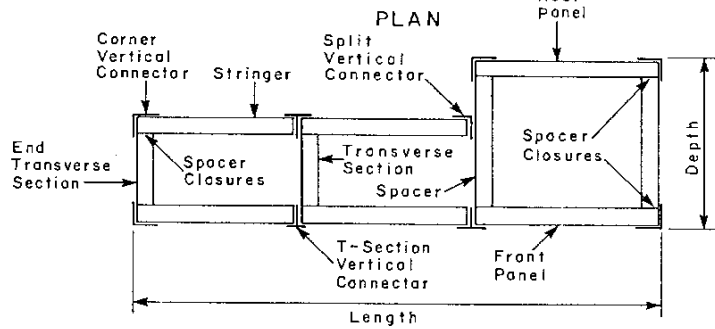
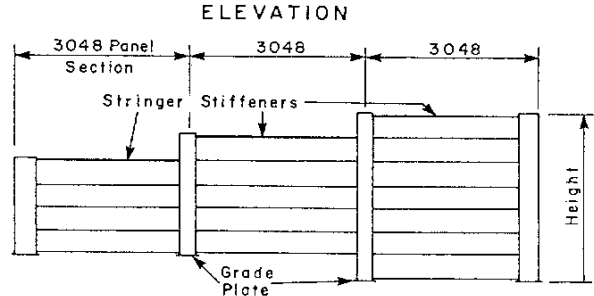
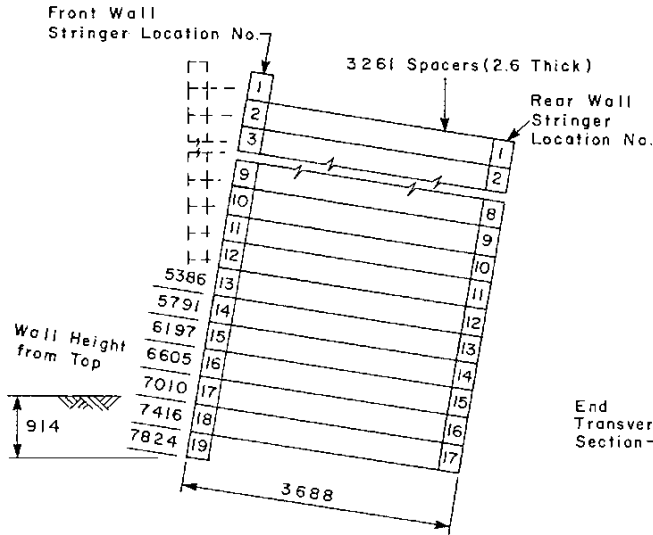


Province of British Columbia

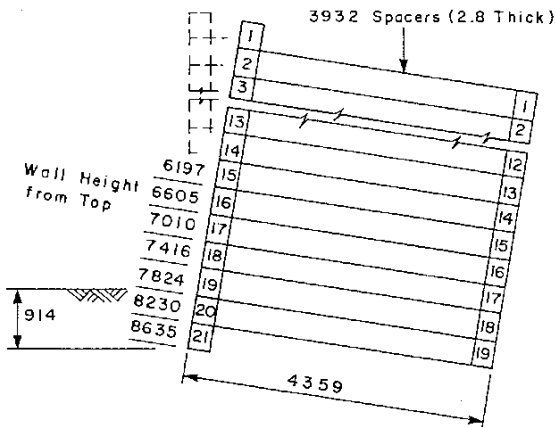
Ministry of Transportation and Highways

Revisions		TYPE 2 METAL BIN WALL DESIGNS A, B & C	
Date	1985	Approved	SPECIFICATION DRAWING No. 1-SP 232
By	MGE/Jan	Exec. Dir. of Engineering	

DESIGN D



DESIGN E

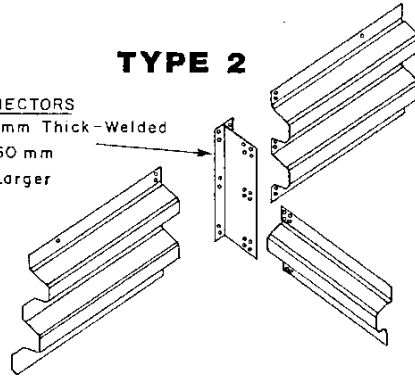


Front Wall Stringer Location No.	Rear Wall Stringer Location No.	Thickness
1 through 8	1 through 6	1.6
9 through 12	7 through 10	2.0
13 through 19	11 through 17	2.8
20 and 21	18 and 19	3.5

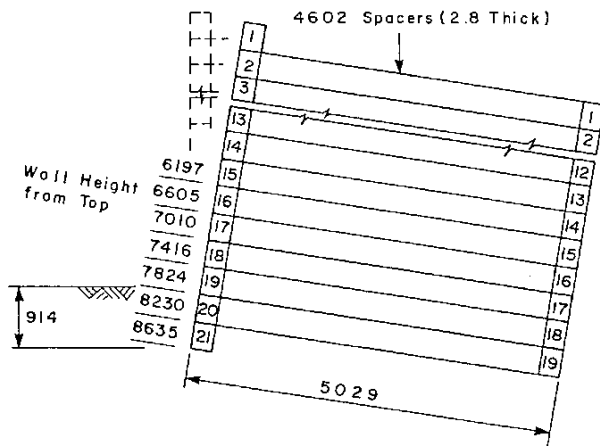
TYPE 2

VERTICAL CONNECTORS

- Steel Plate 6.3 mm Thick - Welded
- Max. Length 3660 mm
- Splice Plate if Larger



DESIGN F

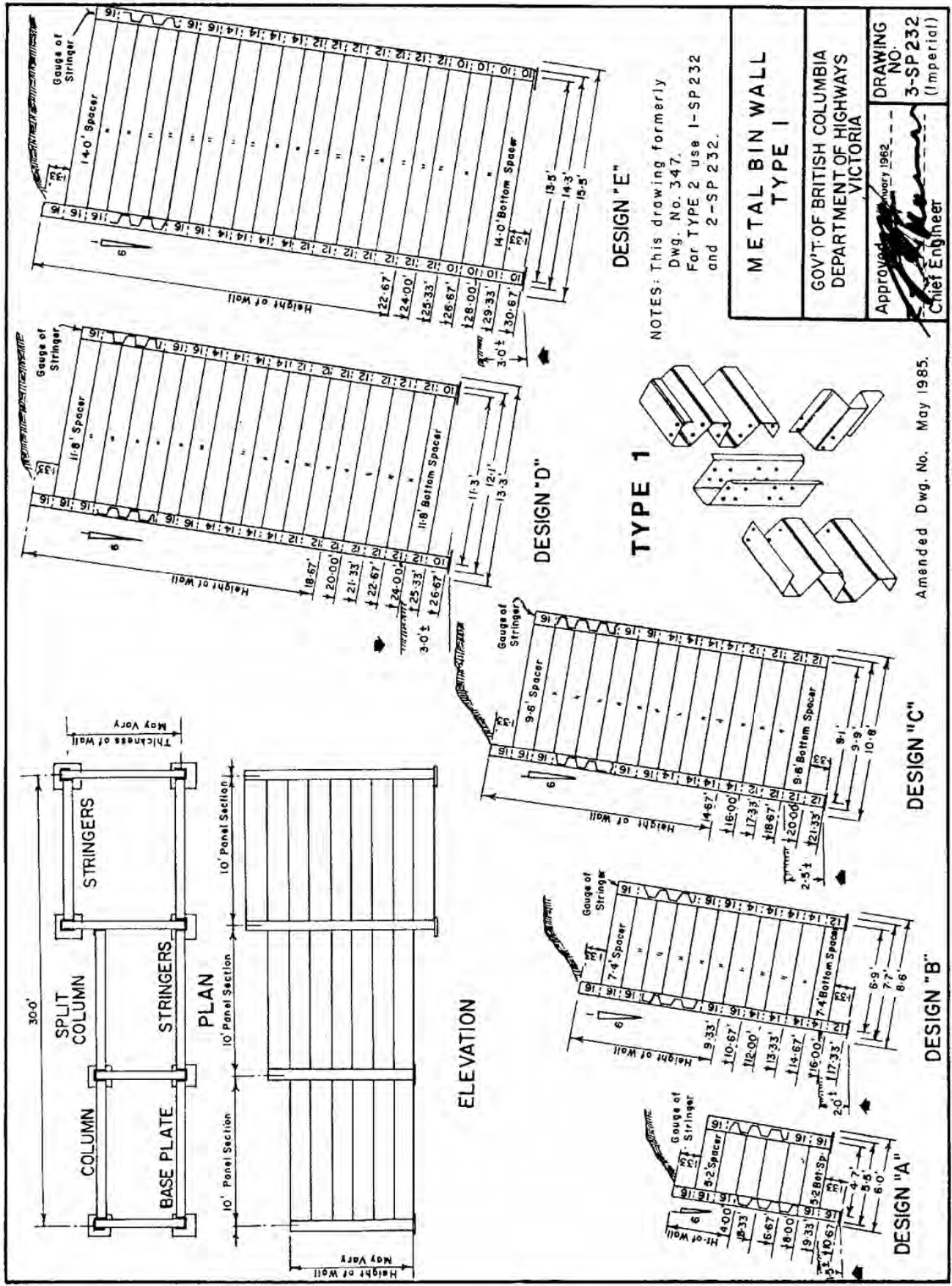


- NOTES :- Binwall Batter = 1:6 or Vertical as Required by Contract Drawings.
- Curvature Detail in accordance with Suppliers Manual.
- All Measurements in Millimetres.
- For Type 1 see Dwg. 3-SP232.



Province of British Columbia Ministry of Transportation and Highways

Revisions	TYPE 2 METAL BIN WALL DESIGNS D, E & F	
	Date 1585 11 06	Approved MGE/Estes
	Exec. Dir. of Engineering	
	SPECIFICATION DRAWING No. 2-SP 232	



SECTION 303

CULVERTS

SCOPE

303.01 Scope – This section covers the construction of pipe and box culverts and their associated end treatment. Culverts installed using trenchless techniques are not covered in these Standard Specifications.

303.01.01 Definitions – For the purposes of this Section, the following definitions apply:

(a) **Backfill Zones:** (See also Figure 303-1)

- (i) **Backfilling** means the operation of filling the embedment and backfill material.
- (ii) **Backfill Material** means fill material used above the embedment material and below the subgrade.
- (iii) **Bedding Material** means the material used to support the culvert.
- (iv) **Embedment Material** means material supporting the culvert, from the top of the bedding to the bottom of the backfill.
- (v) **Internal Streambed Material** means granular material laid inside a culvert to mimic a natural stream bed and aid in fish passage.
- (vi) **Subgrade** means the grade upon which the first layer of granular pavement structure is laid.

(b) **End Treatment Components:**

- (i) **Apron** means a horizontal slab in the channel immediately upstream or downstream of an endwall.
- (ii) **Cut-off Wall** means a buried vertical wall below the springline of a pipe to prevent water flow along the exterior of the pipe.
- (iii) **Endwall** means a vertical or sub-vertical wall surrounding the inlet or outlet end of a culvert. Also known as a “headwall”.
- (iv) **Low-Permeability End Seal** means installing a piping-resistant material near the end of a pipe to limit inflow of water.
- (v) **Wingwall** means a wall extending to the side of an endwall.

MATERIALS

303.10 Materials – The Contractor shall supply all material in accordance with SS 145.15 and the following:

303.10.01 Concrete Products – supplied in accordance with the Design and the following, as appropriate to the application:

- (a) **Concrete Pipe (CP)** – In accordance with CSA A257, ASTM C14, ASTM C76, ASTM C506, ASTM C507, or ASTM C655.
- (b) **Concrete Boxes (CBX)** – In accordance with ASTM C1433 or ASTM C1577.
- (c) **Gaskets** – Rubber gaskets in accordance with CSA A257.3, ASTM C443, or ASTM C1677.
- (d) **Flexible Joint Sealants** – Flexible joint sealants shall be in accordance with ASTM C990.

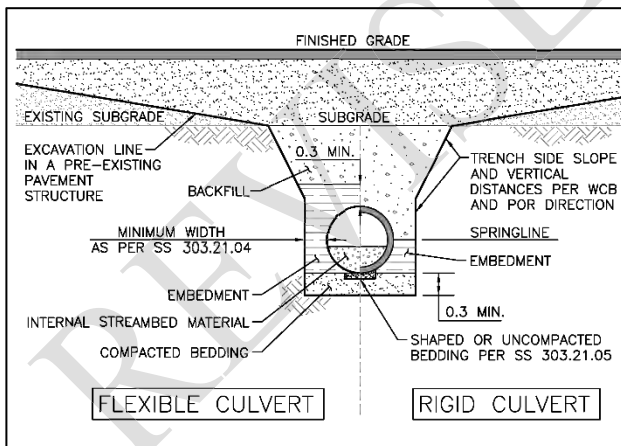
Note: Flexible joint sealants shall be used only where explicitly specified by the Designer and only for structures which do not convey water, such as for pedestrian/cyclist tunnels, farm access tunnels or wildlife tunnels.

- (e) **Concrete Sulphate Resistance** – Concrete shall meet the sulphate resistance requirements as stated in the Special Provisions or on the Drawings.

303.10.02 Corrugated Steel Pipe (CSP) – supplied in accordance with SS 320.

303.10.03 Structural Plate Corrugated Steel Pipe (SPCSP) – supplied in accordance with SS 320.

Figure 303-1: Culvert Backfill in Trench Installation



Notes:

- “POR” means “Professional of Record”; typically a geotechnical engineer in this case;
- “Rigid” culverts are concrete; all other material types are considered “flexible”.

303.10.04 Corrugated Polyethylene Pipe Plastic (PP) – supplied in accordance with SS 317.

303.10.05 PVC Pipe – supplied in accordance with SS 318.03.

303.11 Design Live Load - Culvert products shall be designed for the highway Design live load as follows:

- The Design live load in accordance with the [Ministry Supplement to the Canadian Highway Bridge Design Code \(CHBDC\) S6](#);
- AASHTO LRFD Bridge Design Specifications HL-93 live load; or
- AASHTO Standard Specifications for Highway Bridges H-20 or HS-20 live load.

303.12 Self-Consolidating Concrete – Self-Consolidating Concrete (SCC) may be used in the manufacture of concrete pipe (reinforced or plain) and box culverts, only under the following conditions.

303.12.01 Products – Only for products limited to the following sizes and specifications:

- (a) pipes and barrels smaller than 3.0 m in any internal cross-sectional dimension manufactured in accordance with CSA A257, ASTM C14, C76, C478, C506, C507, or C655.
- (b) boxes with both internal spans and rise less than 3.0 m manufactured in accordance with ASTM C1433 or C1577.

303.12.02 Documentation – the Contractor shall provide the following documentation for review upon request by the Ministry Representative:

- (a) a full copy of the applicable concrete mix design utilized in the manufacture of any product(s) intended to be incorporated into Ministry works, and
- (b) Documentation in accordance with either of the following:
 - (i) Details in accordance with CSA A23.2-24C documenting the proven history of successful performance for the required strength, durability, and other performance requirements of the proposed mix design for the manufacture of any product(s) intended to be incorporated into Ministry works, or
 - (ii) A full-scale test shall be used to verify the self-consolidating characteristics for placement and for the hardened concrete properties of the mix design for the manufacture of any product(s) intended to be incorporated into Ministry works. Documentation in accordance with CSA A23.2-24C shall be submitted demonstrating that the proposed mix design will achieve the required

strength, durability, and performance requirements.

303.12.03 SCC Mix Requirements – The self-consolidating concrete mix shall conform to the requirements of CSA A23.1, including Clause 8.6 and Table 22, and the appropriate CSA and ASTM standards listed above. The SCC shall meet the following requirements:

- (a) Minimum specified compressive strength of 35 MPa at 28 days
- (b) S1 class of exposure with the addition of chloride exposure
- (c) Exposed to cycles of freeze/thaw
- (d) Maximum water-to-cementitious materials ratio (W/C_m) of 0.40
- (e) Rapid chloride permeability test result (ASTM C1202) of less than 1000 coulombs within 91 days

303.13 – Manufacture of Precast Concrete Products

303.13.01 Certified Plants – For Contracts tendered after December 31, 2021, all concrete pipe, boxes, precast concrete endwalls and other precast concrete products shall be supplied from a manufacturer that is certified by an independent third-party certification agency to produce these products according to applicable CSA and ASTM manufacturing standards.

The quality assurance programs recognized specifically for certification of the manufacturer are as follows:

- (a) The Canadian Precast Concrete Quality Assurance ([CPCQA](#)) Certification Program.
- (b) The CSA Group Testing & Certification Inc. ([CSA](#)) Certification Program.
- (c) [QCAST](#) Certification Program by the American Concrete Pipe Association for Precast Products.
- (d) National Precast Concrete Association ([NPCA](#)) Plant Certification.
- (e) Other certification organizations acceptable to the Ministry Representative that are accredited by the Standards Council of Canada for the certification of manufacturers for these products.

303.15.02 Product Marking – All products shall be marked with the certification body's logo confirming that the production of the product is in accordance with the quality and requirements of the specified standards for the products.

CONSTRUCTION

303.20 Construction

303.20.01 Backfill Material – Construction of backfill material shall conform to SS 201 Roadway and Drainage Excavation.

303.20.02 Freezing Temperatures – When the air temperature is below 0°C, no backfilling is allowed unless otherwise accepted by the Ministry Representative. If acceptance is granted, all backfill materials shall be in a thawed state when placed and compacted. Frozen granular backfill materials will not be permitted. No backfill material will be permitted to be placed directly on frozen substrate.

303.20.03 Distortion – During construction of bedding, embedment, and backfilling, the culvert distortion shall be monitored and controlled so that:

- (a) the vertical distortion never exceeds 2% of the rise; and
- (b) the lateral distortion never exceeds 2% of the span.

303.20.04 Low-Permeability End Seal – Where a culvert has no endwall or cut-off wall, and bedding or embedment is 25 mm IGB or other permeable material, the upstream end of the embankment shall be sealed to minimize the potential for piping failure. Any such seal shall be considered as an End Treatment.

The seal shall be constructed by replacing the IGB/permeable material with 25 mm WGB, a clay seal, or other low permeability, erosion resistant material approved by the Ministry Representative, in the upstream end of the bedding and embedment to a horizontal distance of 1 m into the embankment (see Figure 303-2) and, where shown on the Drawings, may also extend upstream of the inlet.

If used for an end seal, clay shall have plasticity properties lying within the shaded area of Figure 303-3 or be approved by the Ministry Representative.

303.20.05 End Bevels – Any culvert 1000 mm or less in diameter that ends within the “clear zone”, as shown on the Drawings, and which is not protected by roadside barrier shall be beveled (mitered) to match the embankment slope, as approved by the Ministry Representative.

303.21 Concrete, Corrugated Steel, Polyethylene and PVC Pipes and Concrete Boxes (this subsection does not cover SPCSP).

303.21.01 Layout – All culverts shall be laid out and constructed in general accordance with the lines, grades, and locations specified in the Drawings, or as directed by the Ministry Representative.

Figure 303-2: Low Permeability End Seal

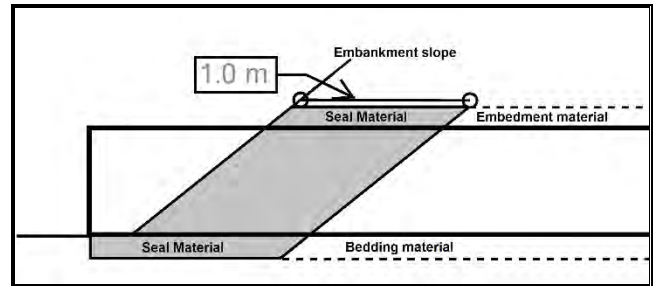
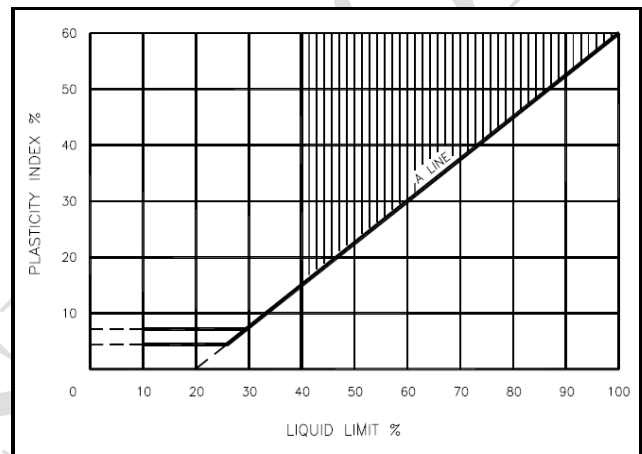


Figure 303-3: Clay End Seal Plasticity Chart



Culverts are to be field fit by the Contractor, to reflect actual conditions encountered on-Site, at a desirable grade of approximately 2% and minimum grade of 0.5%, all to be approved by the Ministry Representative prior to installation.

Such field fits will normally involve minor changes in location, elevation, grade, skew, depth and/or length resulting from actual local drainage course locations, post-stripping surface elevations, final ditch depths, and other factors. Where specified in the Contract or by the Ministry Representative, the field fit may also include adding camber to accommodate any anticipated post-construction consolidation of the embankment.

Some culverts may be designed to control drainage and are not to be field fit without the approval of the Ministry Representative. Any such culverts will be identified in advance for the Contractor, through a note on the Drawings, in the Special Provisions, or by the Ministry Representative.

303.21.02 Flow Obstructions – Any obstacles to flow, such as filter cloth used for siltation control, shall not be placed directly on the end of any culvert. Where practicable, there shall be a minimum spacing of 1 m between the end of the culvert and any obstacles to flow.

Any obstructed culvert shall be cleaned out in accordance with SS 165.10.07.

303.21.03 Inlet and Outlet Ditches – Inlet and outlet ditches to culverts shall be constructed to the lines and grades as shown in the Design and field fit as approved by the Ministry Representative.

303.21.04 Trenching – The trench and other preparatory work shall be approved by the Ministry Representative before actual placing starts.

- (a) A full trench condition shall be provided wherever possible; a minimum trench depth shall be 50% of the culvert diameter.
- (b) The trench width shall be as required to accommodate the Contractor's compaction method and equipment, but not less than 0.3 m on each side of the culvert.
- (c) If, in the opinion of the Ministry Representative, the material in the bottom of the excavation is of such a character as to cause unequal settlement along the length of the culvert, the trench shall be dug below the grade to such depth as ordered, backfilled with crushed base course (IGB or WGB) as per SS 202 or other suitable material approved by the Ministry Representative, and compacted as per SS 202 to ensure a firm and uniform foundation.
- (d) Where a culvert trench is to be excavated through an existing pavement structure:
 - (i) the cut slope through the sub-base and base gravels shall be 6H:1V, or as directed by the Ministry Representative, to minimize differential settlement. See Figure 303-1; and
 - (ii) the road base shall be reconstructed to match the pre-existing pavement structure in material types and thicknesses.

303.21.05 Bedding

- (a) **Bedding Material** – The bedding of all culverts shall consist of:
 - (i) 25 mm intermediate or well graded base course (IGB or WGB) as per SS 202; or
 - (ii) other suitable material approved by the Ministry Representative,
 and, except as noted in SS 303.21.05(b)(ii), shall be compacted as per SS 202 to 95% Standard Proctor maximum dry density as determined in accordance with ASTM D698, to ensure a firm and uniform foundation.

Where IGB is used, a low permeability end seal shall be installed per SS 303.20.04.

- (b) **Bedding Construction** – Bedding for round or other curved shaped culverts shall be installed to a minimum

depth of 300 mm below the culvert, a minimum of 300 mm each side of the culvert, and shall:

- (i) have its top surface shaped so that at least 25% of the circumference of the culvert is in contact with the prepared bedding for the whole of its length, or
- (ii) be left uncompacted below the middle 1/3 of the pipe, for a depth equal to
 - (A) for CSP and PP, twice the corrugation depth, or
 - (B) for PE, PVC, and concrete culverts, 75 mm or 1/24th of the pipe diameter, whichever is greater.

Bedding for Concrete Boxes shall be a minimum of 50 mm in depth (minimum 100 mm over a rock foundation), laid true to grade, and be compacted to a minimum of 95% Standard Proctor maximum dry density as determined in accordance with ASTM D698.

For any gasketed or joint sealed culvert, a transverse trench shall be dug across the bedding at each joint to prevent any granular material from contaminating the joint or impeding the joining process.

303.21.06 Placing – Concrete culvert (CP and CBX) shall be laid beginning at the downstream end with the spigot end pointing downstream. Culvert with elliptical reinforcement shall be laid with the minor axis of the reinforcement as marked by the manufacturer in a vertical position.

Concrete culvert joints shall be sealed using a rubber gasket, installed in accordance with the culvert manufacturer's recommendations.

CSP shall be laid beginning at the downstream end, with separate sections shall be firmly jointed together by a culvert collar.

Where practicable, the first length of culvert at the inlet and outlet shall be full length, to minimize the likelihood of uplift failure.

Corrugated Polyethylene Plastic Pipe with bell and spigot connections shall be laid in the direction with the bell end of the plastic pipe pointing in the opposite direction of water flow, or facing upstream.

Any cuts in CSP shall be coated with two heavy coats of zinc rich paint selected from the Ministry's [Recognized Products List](#) under the category of "Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers".

303.21.07 Embedment – Embedment Material shall consist of 25 mm intermediate or well graded base course (IGB or WGB) as per SS 202, or other mineral aggregate acceptable to the Ministry Representative.

Where IGB is used, a low permeability end seal shall be installed per SS 303.20.04.

Embedment material for embankment installation shall extend a minimum of two-thirds of the span or 0.9 m beyond the culvert span on each side, whichever is greater. Embedment material for trench installation shall extend a minimum of 0.3 m on either side.

Embedment material shall be placed, in layers not exceeding 150 mm in depth when compacted, to an elevation of:

- (a) for CSP, PP, PE or PVC culvert, 300 mm or the 1/6th of the pipe diameter, whichever is greater, above the culvert crown,
- (b) for CP culvert, the springline of the culvert (i.e. up to the widest point of the culvert), and
- (c) for CBX culvert, the crown of the culvert.

Embedment material shall be compacted to a minimum 95% (100% within 300 mm of subgrade elevation) of the Standard Proctor maximum dry density as determined in accordance with ASTM D698.

303.21.08 Backfilling – Backfilling material shall be placed and compacted outside and above the Embedment Material, up to subgrade elevation, and shall consist of material suitable for embankment construction in accordance with SS 201.

Backfilling shall be done symmetrically. The differential height of backfilling material on either side of the culvert shall not exceed 300 mm.

The material within 450 mm directly above the crown of the culvert shall be laid and compacted as one lift. For a culvert with crown within 300 mm of subgrade, installation methods shall be approved by the Ministry Representative.

303.22 Structural Plate Corrugated Steel Pipe/Pipe-Arch Culverts (SPCSP)

303.22.01 Excavation – Excavation shall conform to SS 407 Foundation Excavation. If a coarse boulder material or solid rock is encountered when preparing the bed of the SPCSP, the material shall be excavated to a minimum depth of 300 mm below the SPCSP invert and backfilled with granular material having a maximum size of 75 mm.

303.22.02 Assembling – All bolted plates shall be in contact for the full width and length of the seam lap. The bolts in the valley of each longitudinal seam shall be nearer to the visible edge of the plate than the bolts in the crest. The torque on the bolts prior to backfilling shall be in accordance with CSA S6 or as recommended by the manufacturer in the specific plate assembly drawings. A minimum of 5% randomly selected bolts shall be tested in each longitudinal and circumferential connection. The installation shall not be accepted if 10% or more of tested bolts do not meet the specified torque requirements.

Rotation of the pipe culvert and/or spiralling of the longitudinal seams shall not be permitted.

303.22.03 Bedding, Embedment and Backfilling – Bedding, embedment and backfilling shall be in accordance with SS 303.21 and SS 202 unless otherwise specified in the Drawings or the Contract.

For multiple SPCSP structures, structural backfill shall be provided between adjacent SPCSP structures. The minimum clear spacing between adjacent buried structures shall not be less than 1000 mm or one-tenth of the largest span, whichever is greatest, unless approved otherwise by the Designer and the Ministry Representative.

Backfilling shall be done symmetrically. The differential height of backfilling material on either side of the SPCSP at any transverse section shall not exceed 400 mm.

Only hand held compaction equipment is allowed within 1 m behind any endwall or wing wall.

End dumped or loose pushed material shall not be piled closer than 3 m from the culvert. Hauling equipment shall not be operated over the culvert until backfilling operations have completed a suitable cover approved by the Ministry Representative.

303.23 Culvert End Treatment – Such treatments may include endwalls, wingwalls, cut-off walls, aprons, traversable grates, up-lift prevention measures, or any combination thereof, and shall be constructed in accordance with the details shown on the applicable Drawings or SP Drawings.

Construction and materials for concrete and reinforcement shall conform to SS 211 and SS 412 and excavation shall conform to the requirements of SS 407.

Rip-rap end treatment is covered under SS 205 and is not included within the Work under this SS 303.

If specified on the Drawings or in the Contract, the Contractor may provide an alternative End Treatment product, as a Value Engineering Proposal in accordance with SS 125. Alternate products shall be designed by a professional engineer registered with Engineers and Geoscientists BC. Sealed engineered drawings of the alternate products shall be provided to the Ministry Representative for review and approval at least two (2) weeks prior to the start of the fabrication of the product.

MEASUREMENT

303.80 Culverts – Culverts will be measured by the METRE along their invert length as installed for each size and type of culvert.

303.81 End Treatments – End Treatments will be measured by EACH for each culvert End Treatment location. Each stand-alone endwall, cut-off wall, wing wall, apron, traversable grate, up-lift prevention measures, or any combination of those components at a single location, will be counted as a single unit.

PAYMENT

303.90 Culverts – Payment for excavation, culvert supply, assembling, placing, beveling within the clear zone, and backfilling shall be at the Contract Unit or Lump Sum Price bid for each of these items of work, or at the Contract Unit Price bid per metre of culvert.

303.91 Culvert End Treatment – Payment for CULVERT END TREATMENT will be at the Contract Unit or Lump Sum Price bid per size and scope of Culvert End Treatment required.

The Contract Unit Price shall include provision of everything necessary; endwall, (complete with cut-off-wall, wing wall, apron, traversable grate), or up-lift prevention measures, or any combination thereof; incremental costs of materials used as a Low Permeability

End Seal; formwork, reinforcement, air entrained concrete, placing and tamping, stripping and cleaning, curing, engineering design for alternate End Treatment products, and any other work necessary in connection therewith, including excavation of Type D material and backfilling, but excluding riprap and Type A excavation.

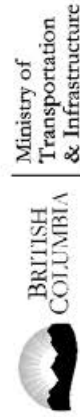
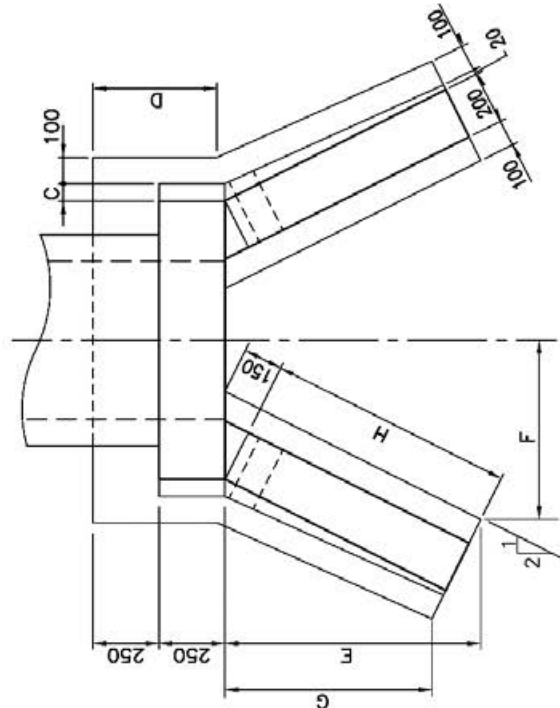
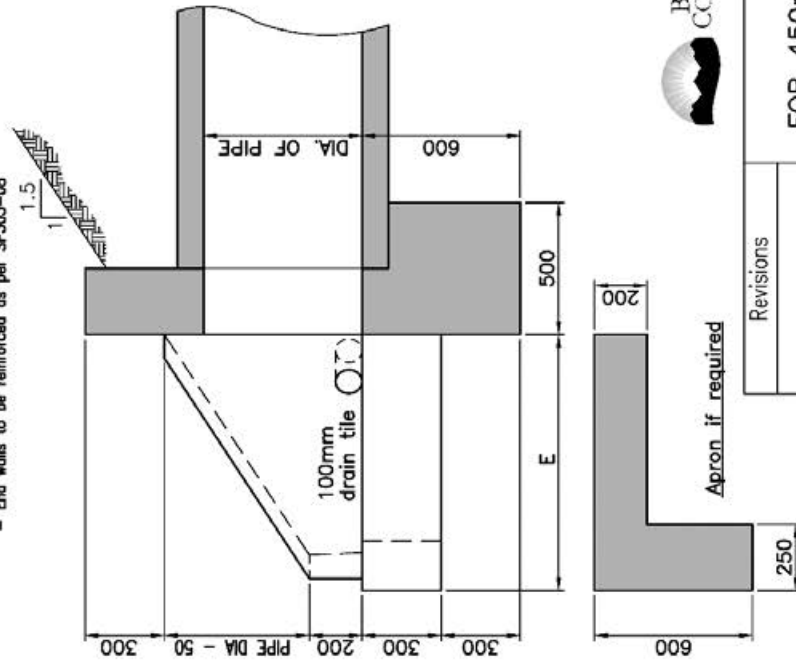
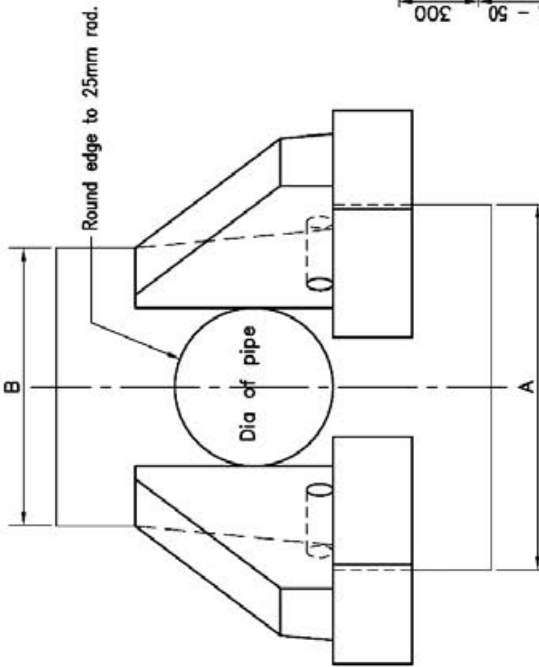
303.92 Full Compensation – The price(s) bid shall be full compensation for the cost of furnishing all labour, materials, equipment, tools and incidentals necessary to complete the work as specified in the Contract.

303.93 Type A Excavation – Excavation of Type A material encountered in the trench or Culvert End Treatment excavation will be paid at the Contract Unit Price for Type A, or where such an Item is not included in the Contract, as a Change to Work.

PIPE DIA.	A	B	C	D	E	F	G	H	m ³ of concrete without apron	m ³ of concrete with apron
450	1205	905	50	470	745	490	555	680	0.88	1.07
600	1385	1055	65	470	970	690	785	935	1.14	1.43
750	1560	1210	75	460	1200	870	1010	1190	1.43	1.87
900	1740	1360	90	460	1430	1060	1240	1445	1.78	2.38
1050	1915	1515	100	460	1655	1250	1470	1705	2.13	2.92

All dimensions are in millimetres

- Notes:
- See SP303-03 for formulas giving exact lengths and other factors governing the construction of end walls.
 - Chamfer exposed edges 20mm.
 - Carry footings to greater depth if necessary for good foundation.
 - Do not run pipe through to face of headwall unless pipe is bedded into concrete.
 - Concrete and reinforcing bar to be in accordance with SS211 and SS412 respectively.
 - End walls to be reinforced as per SP303-08



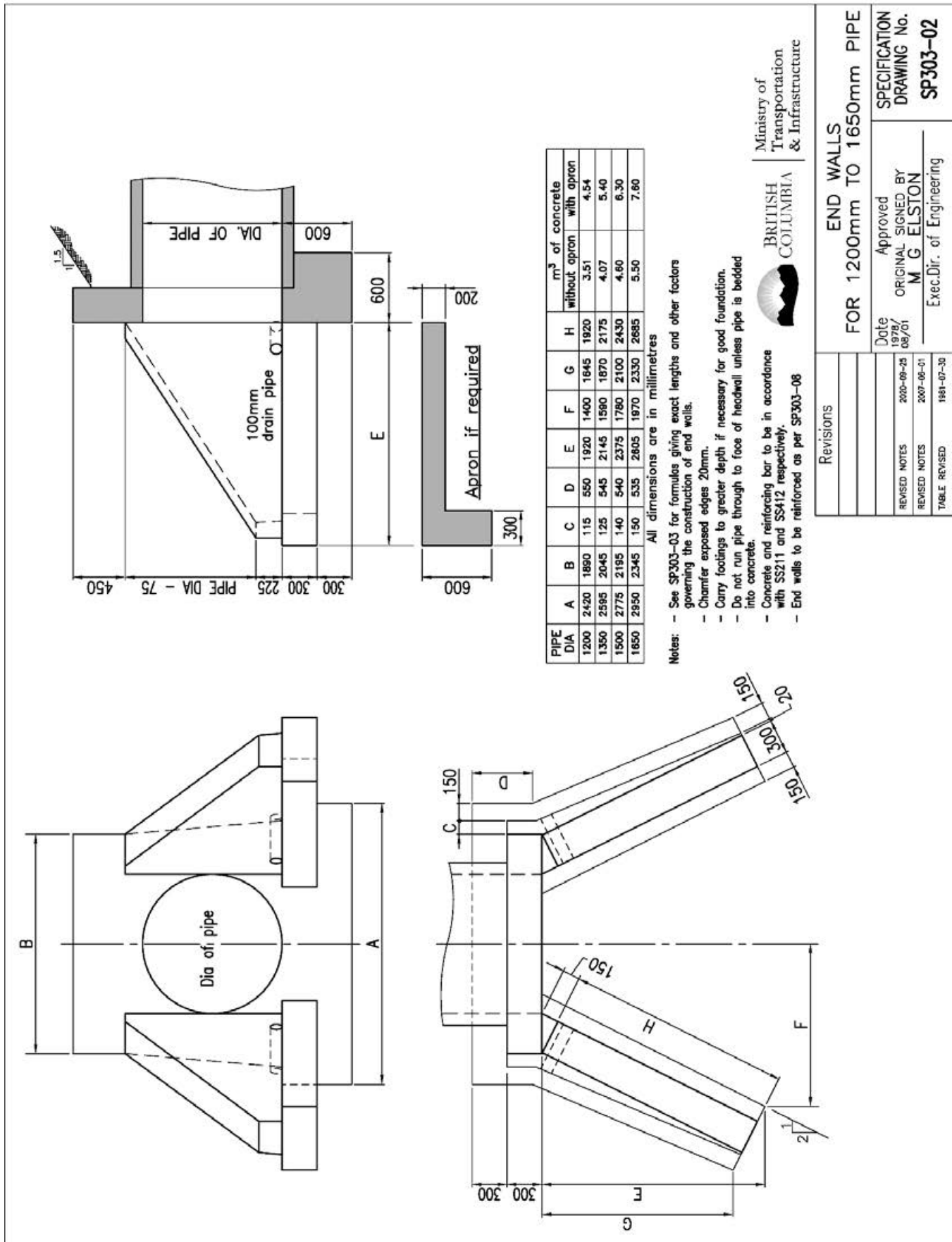
Ministry of Transportation & Infrastructure

Revisions	

Date	1978/08/01	Approved	
REVISED NOTES	2000-09-23	ORIGINAL SIGNED BY	M. G. ELSTON
REVISED NOTES	2007-06-01	Exec. Dir. of Engineering	
TABLE REVISED	1981-07-30		

END WALLS FOR 450mm TO 1050mm PIPE

SPECIFICATION DRAWING No. SP303-01



NOTE:
 in Spec. Dwg. SP303-01 - m = 85mm n = 200mm
 in Spec. Dwg. SP303-02 - m = 130mm n = 225mm



FIG. 2a
DOWNSTREAM END

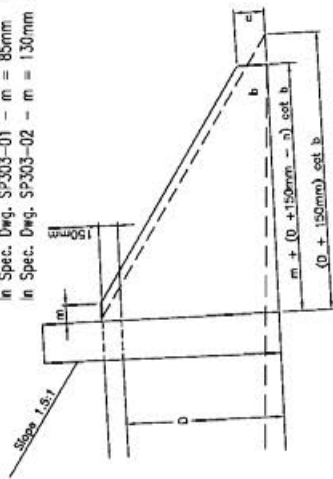


FIG. 2b
UPSTREAM END

DIAGRAMS FOR EXACT LENGTHS
OF WING AND CULVERT PIPE

NOTE:
 In Figs 1, 2a & 2b it is assumed that the headwalls are constructed normal to the grade of the culvert this is permissible where the grade does not exceed the recommended maximum of 1 in 30.

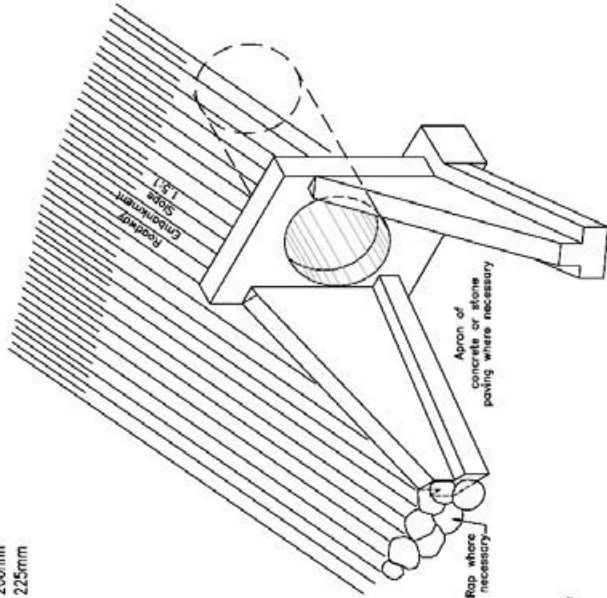


FIG. 3
GENERAL VIEW OF END
WALL IN PLACE

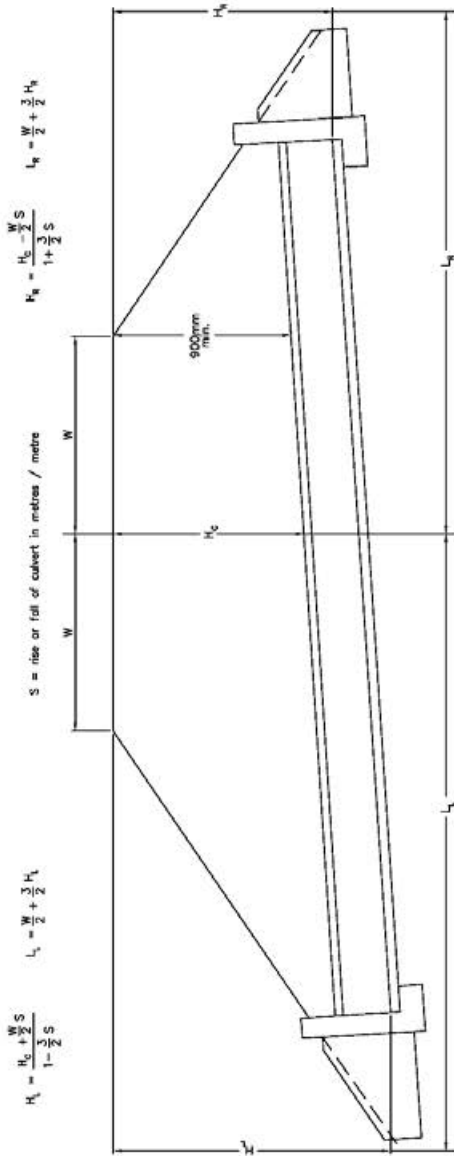


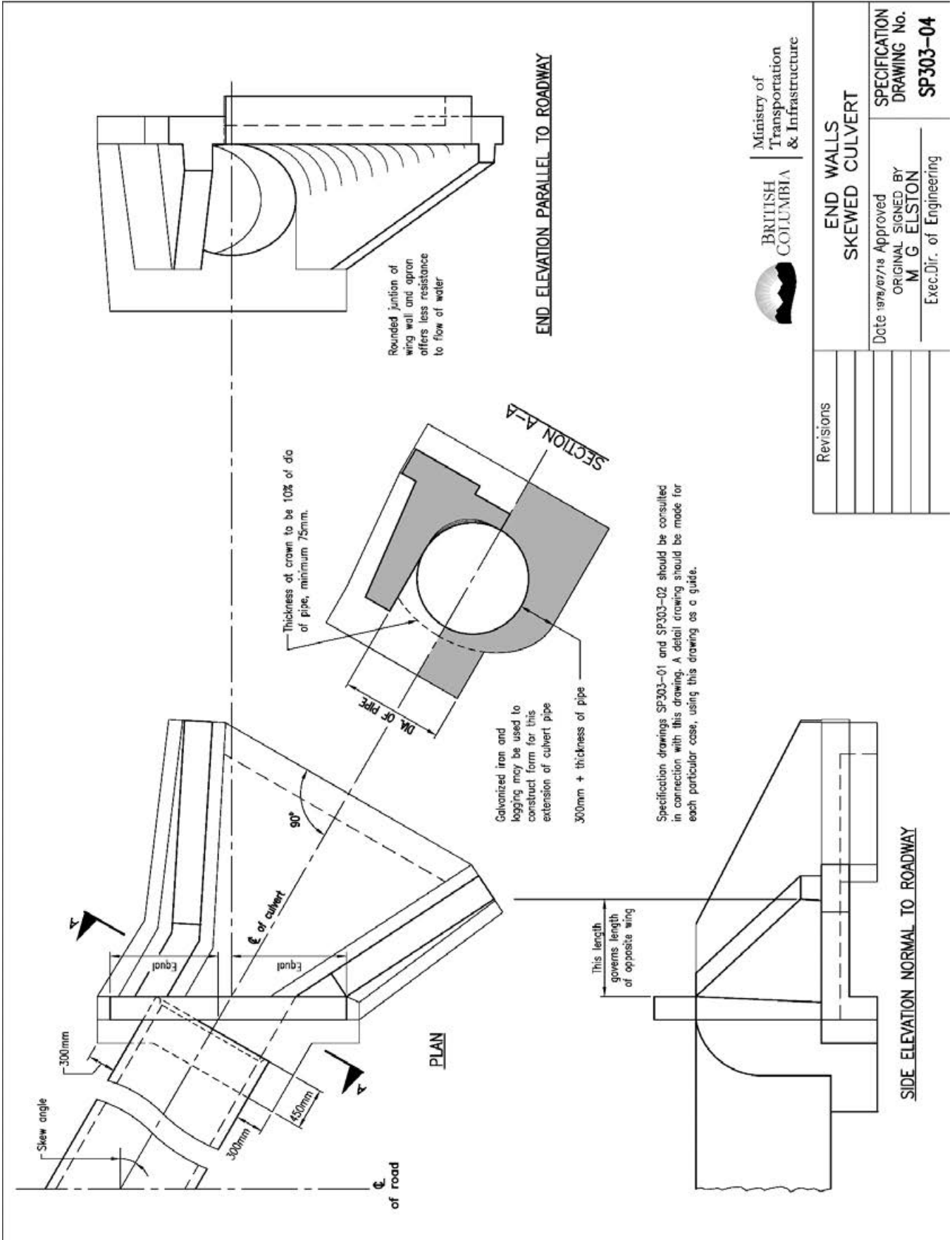
FIG. 1 LAYOUT FOR INTERSECTION OF SLOPES



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& Infrastructure

Revisions

Date 1976/08/01	Approved ORIGINAL SIGNED BY M G ELSTON	SPECIFICATION DRAWING No. SP303-03
Exec.Dir. of Engineering		



Ministry of Transportation & Infrastructure

Revisions	

END WALLS SKEWED CULVERT

Date 1978/07/18 Approved
 ORIGINAL SIGNED BY
M. G. ELSTON
 Exec. Dir. of Engineering

SPECIFICATION DRAWING No. SP303-04

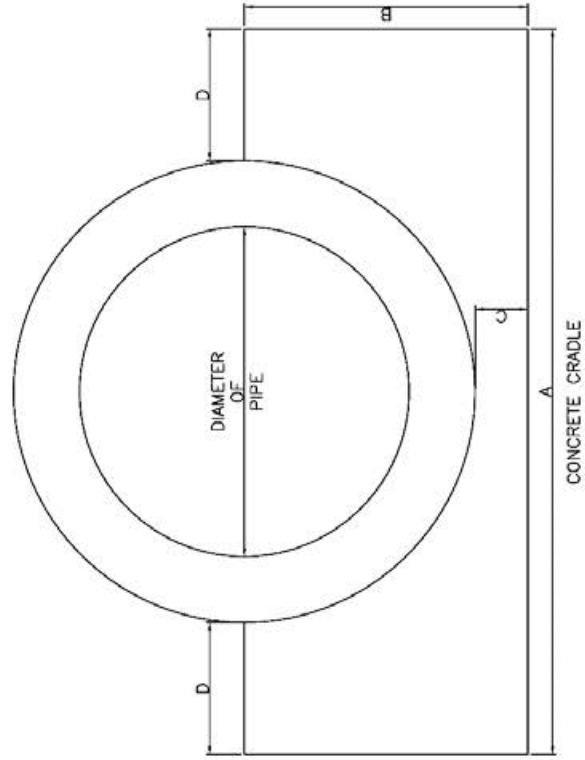
Notes:

Lower part of cradle to be a stiff mix of concrete, lay pipe to established line and grade. Follow immediately with concrete of a workable consistency to the required height. Cradles of these dimensions are for concrete pipes only.

Use concrete cradles under the following conditions.
High fill.
Steep culvert grade.
Rock formations.

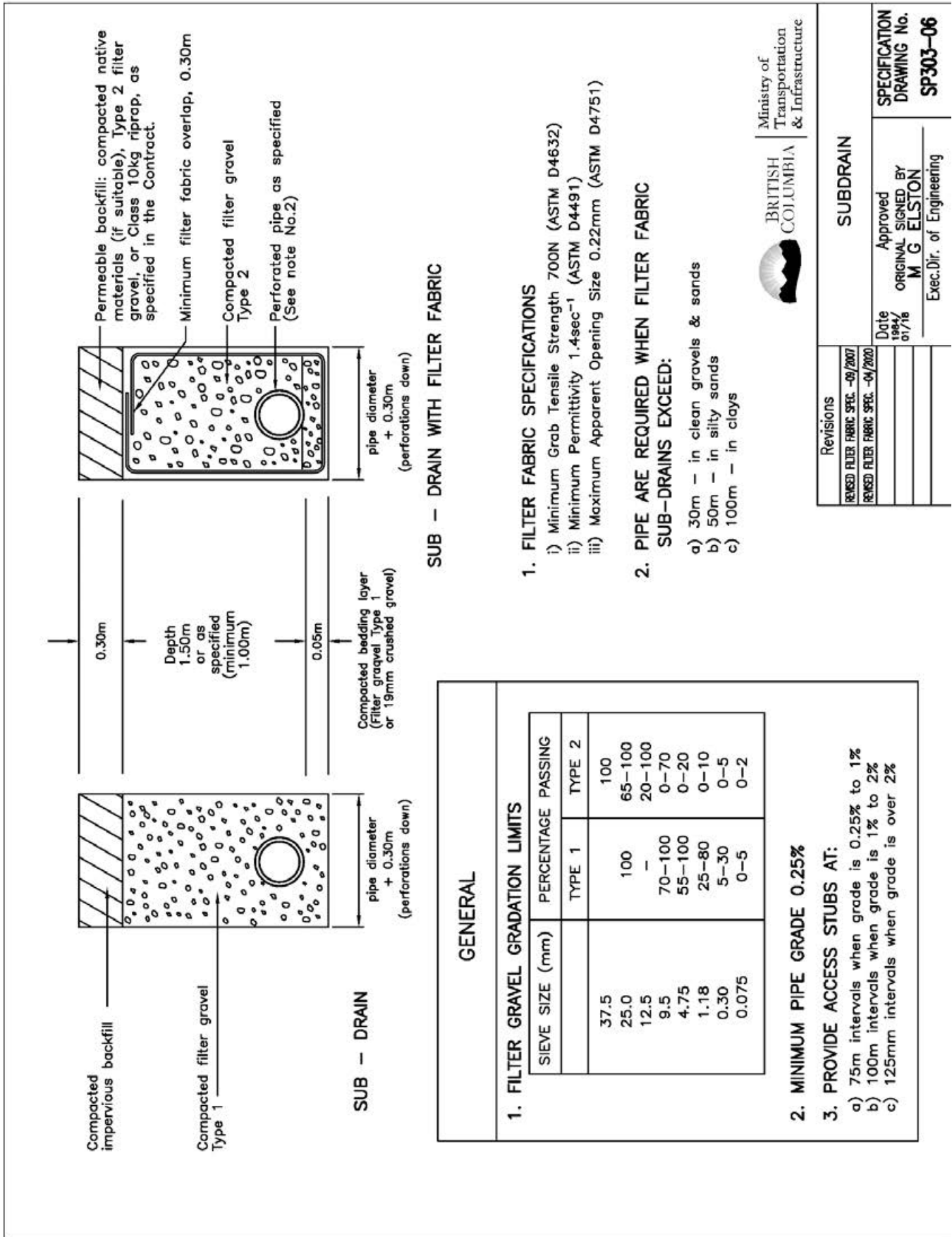
Concrete to be in accordance with SS211.

PIPE DIMENSIONS		CONCRETE CRADLES					m ³ per m
Inside Diameter mm	Wall Thickness mm	A mm	B mm	C mm	D mm		
600	95	1400	500	100	300	0.45	
750	108	1580	605	115	300	0.58	
900	121	1755	695	115	300	0.69	
1050	133	1935	795	130	300	0.84	
1200	146	2110	895	140	300	0.99	
1350	159	2290	995	150	300	1.16	
1500	172	2470	1100	165	300	1.35	
1650	184	2645	1200	180	300	1.53	
1800	197	2825	1300	190	300	1.73	



CONCRETE CRADLE FOR CULVERT PIPE

Revisions	Date 1978/ 07/18	Approved ORIGINAL SIGNED BY M. G. ELSTON	SPECIFICATION DRAWING No. SP303-05
REVISED NOTES	2020-09-25	Exec. Dir. of Engineering	
TABLE REVISED	1981-07-30		



SUB - DRAIN WITH FILTER FABRIC

1. FILTER FABRIC SPECIFICATIONS

- i) Minimum Grab Tensile Strength 700N (ASTM D4632)
- ii) Minimum Permittivity 1.4sec⁻¹ (ASTM D4491)
- iii) Maximum Apparent Opening Size 0.22mm (ASTM D4751)

2. PIPE ARE REQUIRED WHEN FILTER FABRIC SUB-DRAINS EXCEED:

- a) 30m - in clean gravels & sands
- b) 50m - in silty sands
- c) 100m - in clays

GENERAL		
SIEVE SIZE (mm)	PERCENTAGE PASSING	
	TYPE 1	TYPE 2
37.5	100	100
25.0	100	65-100
12.5	-	20-100
9.5	70-100	0-70
4.75	55-100	0-20
1.18	25-80	0-10
0.30	5-30	0-5
0.075	0-5	0-2

2. MINIMUM PIPE GRADE 0.25%

3. PROVIDE ACCESS STUBS AT:

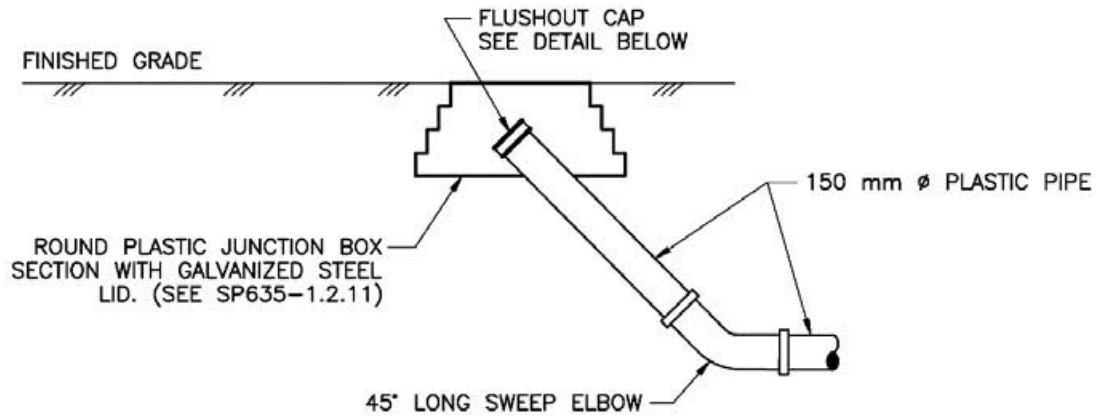
- a) 75m intervals when grade is 0.25% to 1%
- b) 100m intervals when grade is 1% to 2%
- c) 125m intervals when grade is over 2%



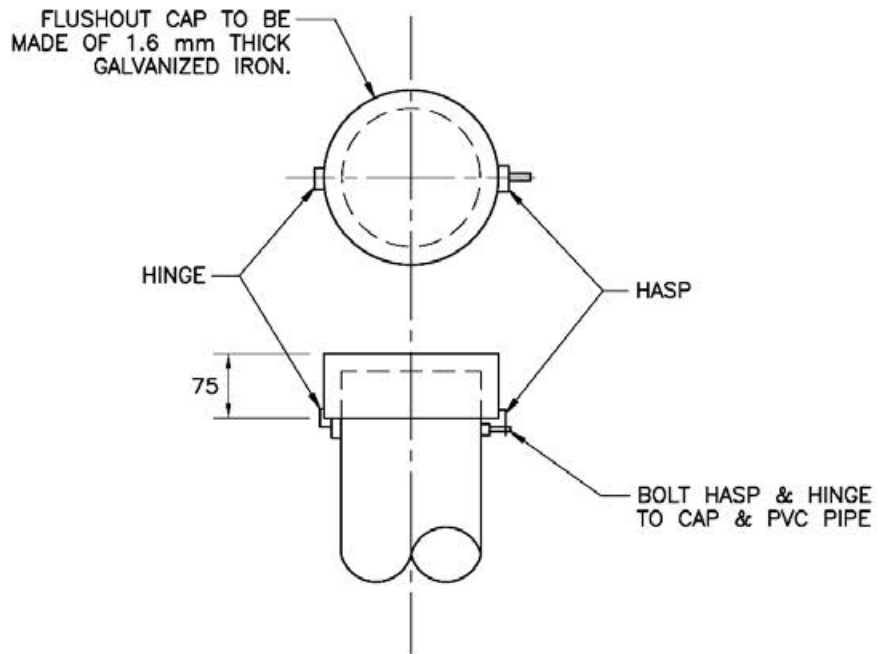
Revisions	SUBDRAIN	
REVISED FILTER FABRIC SPEC. -09/2007	Date	Approved
REVISED FILTER FABRIC SPEC. -09/2020	15/04/07/16	ORIGINAL SIGNED BY
		M G ELSTON
		Exec.Dir. of Engineering
		SPECIFICATION DRAWING No.
		SP303-06

SUBDRAIN FLUSHOUT

SP303-07



FLUSHOUT AND COVER DETAIL

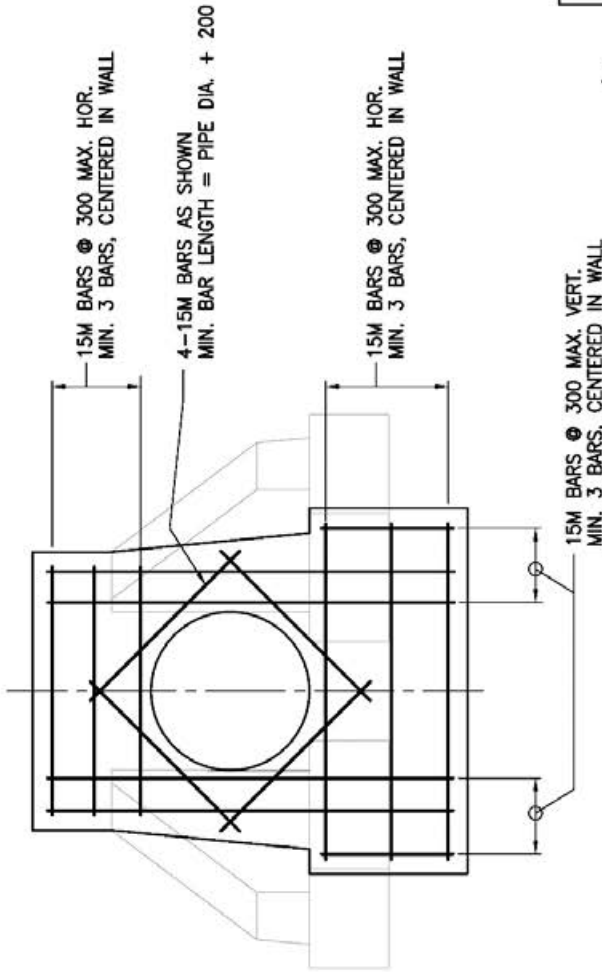


FLUSHOUT CAP DETAIL

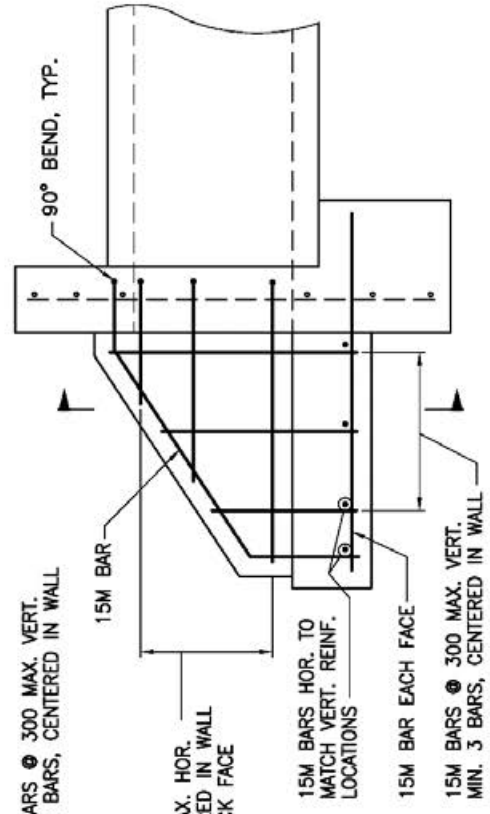
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



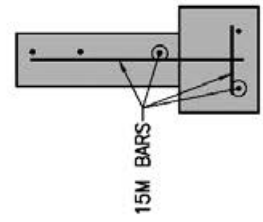
REINFORCING STEEL SHALL BE IN ACCORDANCE WITH C.S.A. SPECIFICATION G30.18M, GRADE 400R. SPlicing OF REINFORCING IS NOT PERMITTED. MINIMUM CONCRETE COVER: 75mm



TYPICAL HEAD WALL



TYPICAL WING WALL



SECTION



Revisions

END WALLS
REINFORCING DETAILS

Date 2008/07/18 Approved
ORIGINAL SIGNED BY
Chief Highway Engineer
SPECIFICATION
DRAWING No.
SP303-08

SECTION 308

COATING SYSTEMS

308.01 Scope – This Section defines the coating systems required for shop and field coating of structural steel or wood/concrete surfaces which may be exposed to exterior corrosive conditions.

308.02 Standards and Specifications

a) Specifying Bodies:

- **SSPC** – The Society for Protective Coatings, has a large number of coating and cleaning specifications.
- **ASTM** – The American Society for Testing and Materials, produces test procedures for a wide range of products and processes.
- **NACE** – The National Association of Corrosion Engineers, also produces specifications and test procedures for coatings and corrosion engineering.

All these bodies have a web presence available by searching the name above.

b) Colour Specifications using a five-digit number refer to SAE AMS-STD 595A colour numbers.

c) Measuring Units – The metric unit (μm) or imperial unit (mil) can be used interchangeably, at the conversion factor of:

1 mil = 25 μm (1 mil = 1 thousandth of an inch)

308.03 System Selection Table – The System Selection Table contained in the Coating Systems section of the Ministry [Recognized Products List \(RPL\)](#) summarizes the coating systems currently in use. The table lists application areas where specified coating systems may be used. It is intended to assist in the choice of a coating system for specific applications. Some systems may find use in more than one specific application or in applications not mentioned.

Where a system is specified in a Contract, only those coating products listed for that coating system may be used.

308.04 System Selection Criteria – The selection of a coating system is dictated by the location and environment of the structure to be coated. The coating system may be applied in a shop or in the field. The field location may necessitate a specific maintenance regimen, for example, a complete recoat or a single overcoat. The environment may be moderately or severely corrosive. In addition, there may be concerns with limiting overspray.

The following general comments apply to all coatings.

- Coatings shall not be mixed and matched between suppliers, or within a supplier's product line.
- All coatings shall bear the manufacturer's name and address, batch number, paint type, date of manufacture, DOH or [SAE AMS-STD 595A color code](#)¹, and mixing and application instructions. All labels shall contain the requisite WHMIS label information and be accompanied by a valid MSDS. All coatings are subject to inspection and testing before acceptance or at the time of application.

308.05 Coating Products – The Ministry [Recognized Products List \(RPL\)](#) lists coating products that have been reviewed and accepted for use on Ministry projects. The RPL can be reviewed at the following Ministry web site:

<https://www2.gov.bc.ca/gov/content/transportation/transportation-infrastructure/engineering-standards-guidelines/recognized-products-list>

Products not listed on the Ministry Recognized Products List shall not be used.

Products on the RPL shall be used in accordance with the introduction/definitions, descriptions, notes qualifiers and all other information given in the RPL.

The use of any product from the RPL by the Contractor shall in no way relieve the Contractor from its contractual obligations.

¹ Note: The Recognized Product List may still refer to these as “US Federal Specification 595B” colours. The colour numbers have not changed between the US Federal Specification 595B and the SAE-AMS-STD 595A specifications.

SECTION 312

STEEL TRAFFIC BARRIERS

312.01 Scope – This Section covers the quality and manufacture of steel beam guardrails, wood posts, steel posts, and accessory material for use as traffic barriers.

The layout and installation of materials shall be in accordance the Contract and the SS Drawings of the SP312 series unless otherwise specified.

SS 604 covers the installation of the material described by this Section together with the construction of ancillary work.

Note: SS Drawings numbers SP312-5, SP312-9, and SP312-10 are not used.

312.02 Standards of Reference – Standards of various authorities are referred to herein generally in sufficient detail for the purpose of this Section. For detailed testing and other requirements, copies of the Standards are obtainable as follows:

CSA: Canadian Standards Association:
Standards Sales
13799 Commerce Parkway
Richmond, BC V6V 2N9

<https://www.csagroup.org/codes-standards/>

ASTM: American Society for Testing and Materials,
1916 Race Street,
Philadelphia, PA 19103

<https://www.astm.org/Standard/standards-and-publications.html>

TF13: Task Force 13

<http://www.tf13.org/Publications.php>

AASHTO: American Association of State Highway and Transportation Officials
444 North Capitol Street N.W., Suite 225
Washington, DC 20001

<https://store.transportation.org/>

312.03 General – Steel traffic barriers shall consist of rail sections fabricated for installation to develop continuous beam strength with the necessary safety end assemblies.

End assemblies shall be “[Manual for Assessing Safety Hardware \(MASH\)](#)” certified.

All rail sections and other components shall match the design profiles, specifications and dimensions of the Task Force 13 requirements cited below, in part to ensure full interchangeability of similar components regardless of the source of manufacture.

Unless otherwise specified, guardrails and posts and accessories shall be in accordance with SS Drawings

SP312-1 SP312-2, SP312-3, SP312-4, SP312-6, SP312-7, SP312-8, and SP312-11. Design and part numbers are taken from the Task Force 13 “[Guide to Standardized Roadside Hardware](#)” and are reproduced on these drawings.

Task Force 13 information is available on-line at:

<http://www.tf13.org/Guides/index.php>

The name and/or trademark of the manufacturer and the metal thickness shall be clearly and permanently marked on each component clear of the splicing overlap and on the opposite face to the traffic side.

312.04 Steel Beam Guardrail – Guardrail shall be manufactured in accordance with AASHTO M 180 “Standard Specification for Corrugated Sheet Steel Beams for Highway Guardrail” and shall conform to the following detail requirements.

312.04.01 Beam Classification (According to the AASHTO Standard) – The shape, class and type, together with the Task Force 13 design and part numbers, shall be as in accordance with the Special Provisions, Contract Drawings and/or SS Drawings of the SP312 series.

(a) **Shape** – W-Beam are for SGR38 installations, and Thrie-Beam for SGR09 and SGM09 installations and both shall be accurately formed to profile, dimensions and tolerances of AASHTO M 180 with overall cross sections of 312 mm x 85 mm and 508 mm x 85 mm respectively (TF13 Designation [RWM01a to RWM22a](#) and [RTM02a to RTM06a](#)).

(b) **Class and Type** – Unless otherwise, beams shall be Class A, Type II.

(c) **Length** – Normally 3.81 m (12’ 6”) nominal length beams (4.19 m, +0 mm/-75 mm overall) will be required.

(d) **Punchings** – In addition to joining bolt holes, punchings may be designed as 7.62 m, 3.81 m, 1.905 m or 0.953 m (25’ 0”, 12’ 6”, 6’ 3”, 3’ 1½” respectively), see SS Drawings, SP312-1 and SP312-2 for requirements.

(e) **Curved Rails** – For curved rail data see SS Drawing SP312-11.

312.04.02 Metal

(a) **Properties** – Properties of the base metal for beams and any transition section shall conform to the following requirements:

- Yield point minimum: 345 MPa
- Tensile strength minimum: 483 MPa
- Elongation minimum in 50 mm: 12%

SECTION 312

STEEL TRAFFIC BARRIERS

Note: End and buffer sections shall have a minimum yield point of 227 MPa and a minimum tensile strength of 310 MPa.

- (b) **Sheet Thickness** – Nominal and finished thicknesses shall be in accordance with Table 312-A.
- (c) **Sheet Widths** – Sheet Widths shall be minimum 483 mm for W-Beams and 749 mm for Thrie-Beams both with a permissible tolerance of minus 3.2 mm.

312.04.03 Zinc Coating – The zinc used for the coating of Type I and II sections shall be as prescribed in ASTM B6, and shall be at least equal to the grade designated as “Prime Western.”

Steel galvanized before beam fabrication shall be coated in accordance with ASTM A653. Beams galvanized after fabrication shall conform to the requirements of ASTM A123. In either case, the coating shall be firmly bonded to the base metal after fabrication is complete.

The average mass of zinc coating Type I (AASHTO M 180) shall be 610 g/m² for three specimens but the minimum is 550 g/m² for any individual specimen. The average mass of zinc coating Type II (AASHTO M 180) shall be 1220 g/m² for three specimens but the minimum is 1100 g/m² for any individual specimen. The mass of coating is the total amount on both sides of the sheet or beam.

312.04.04 Quality of Work – Beams shall be punched for splice and post bolts in strict conformity with AASHTO M 180 to the designated number, and centre to centre spacing, of posts.

Upon delivery the beams shall be ready for assembly and installation.

Field cutting shall be avoided where practicable. Where necessary, field cutting shall be performed with a cut-off or hack saw; field hole cutting shall be performed with a drill and/or reamer. Flame or plasma cutting, or welding, shall not be permitted.

All field cuts or holes shall be deburred and treated with two heavy coats of zinc rich paint selected from the Ministry’s [Recognized Products List](#) under the category of “Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers”.

Warped or otherwise deformed beams, and those with injurious defects or excessive roughness of the zinc coating will be rejected.

Beams for concave and/or convex curved installations with a radius of 45 m or less shall be accurately and smoothly bent in the shop with minimum deformity. The radius of curvature shall be clearly stencilled on the back of each curved rail section.

312.04.05 Examination – Examination may include the following:

- (a) Dimension measurements for size and thickness all of which shall be within the tolerances specified by SS 312.04 and Table 312-A.

- (b) Coating generally shall be smooth, free of beading or sharp projections at edges. Coating adherence shall prevent the peeling of any portion of the zinc coating so as to expose the base metal by cutting or prying with a stout knife under considerable pressure.

Table 312-A: Beam Thickness by Class – Base Metal and Finished Galvanized

Class	Base Metal Nominal Thickness (mm)	Finished Galvanized Beam Thickness (mm)		Tolerance (mm)
		Type I	Type II	
A	2.67	2.74	2.82	minus 0.23
B	3.43	3.51	3.58	minus 0.25

Note: Tolerance given is for under specified thickness; there is no limit for over thickness.

312.04.06 Testing – When required by the Ministry Representative, galvanizing testing shall be carried out according to ASTM A90 or AASHTO T 65 to determine the mass of galvanizing in conformity with the requirements of SS 312.04.03. Alternatively, magnetic gauge determination in accordance with ASTM E376 may be permitted by the Ministry Representative.

The testing of the mechanical properties of the base metal may be carried out by the Ministry Representative for compliance with the requirements of SS 312.04.02.

312.04.07 Rejection and Retests – Should the beam tested fail to meet the testing requirements specified above, two additional beams as selected at random by the Ministry Representative shall both meet the test requirements in every respect, otherwise the complete lot represented by the samples will be subject to rejection.

In the event of complete lot rejection and prior to any further testing by the Ministry, the intended replacement lot shall be satisfactorily tested by a mutually acceptable independent testing agency at no expense to the Ministry.

Rejected material shall be replaced at the Supplier’s or Contractor’s expense, including shipping charges and removal of rejected material at the construction site, if applicable.

312.05 Guardrail Accessories

312.05.01 Transition Section – Transition sections for W-Beam to Thrie-Beam shall be of matching metal thickness and finish and be fabricated to the Task Force 13 design profile (TF13 Designation [RWT01a-b](#) or [RWT02a-b](#)) to provide a smooth and uniform transition of beam widths and corrugations.

312.05.02 Terminal Connectors – For connections to bridge parapets, abutments and the like, TF13 Designation [RWE02b](#) or [RTE01b](#) terminal connectors shall be used and the finish shall be Type II.

312.05.03 Back-up Plates – Back-up Plates are required where steel beams between splices are to be installed to any steel posts (standard 305 mm lengths) and shall be fabricated of the stipulated lengths from the standard beam section (as per TF13 Designation [RWB01a](#) or [RTB01a](#)).

312.05.04 Fasteners and fittings – All fasteners and fittings shall be in accordance with TF13 and shall be galvanized in accordance with ASTM A153 or ASTM A123 as appropriate.

312.05.05 Cable Assembly – Cable Assembly for Breakaway Cable Terminal (BCT) installation shall consist of a swaged fitting, 25 mm diameter threaded stud, locking pin, nut and washer at each end of a cable of 19 mm nominal diameter, 6 x 19 wire strand core independent wire rope core, galvanized, right regular lay wire rope with a minimum breaking strength of 200 000 N (45,000 lb) and conforming to AASHTO M 30 with the overall assembly length of 1980 mm developing a similar breaking strength (TF13 Designation [FCA01](#), [FWC24a](#), and complete with plate washers, [FPB01](#)).

312.05.06 Anchor Plate – Anchor Plates for the breakaway cable attachment to the steel beam for BCT assemblies shall be fabricated to the TF13 design profile (TF13 Designation [FPA01](#)) from steel plate conforming to ASTM A36 and galvanized according to ASTM A123 after forming and drilling for and including eight 16 mm diameter hexagonal-head bolts 38 mm long each with nut and 3.0 mm thick plain washer 44 mm outside diameter (TF13 Designation [FBX16a](#) and [FWC16a](#)) complying with SS 312.05.04.

312.05.07 Anchor Inserts – Anchor inserts for fixing terminal connector end sections to existing concrete shall be as shown on the Drawings or as directed by the Ministry Representative.

312.05.08 Steel Posts

(a) **Concrete Installation** – Installation of steel posts on concrete components and the transitions of the barrier system to the concrete components shall be as shown on the Drawings or as directed by the Ministry Representative.

(b) **Soil Embedment Installation** – Unless otherwise specified, details shall be in accordance with SS Drawings SP 312-2, SP 312-3 and SP 312-4. Steel posts shall be fabricated from standard structural steel H-beam that conforms to the latest version of any of the following standards:

- AASHTO M 270 (ASTM A709), Grade 36 [250] or Grade 50S [345S] without the supplementary requirements;

- ASTM A36; or
- ASTM A992.

After the section is cut-to-length and all holes are drilled or punched, each guardrail post shall be hot dip galvanized in accordance with ASTM A123.

Any areas where the galvanizing is damaged during installation shall be touched up with two heavy coats of zinc rich paint selected from the Ministry's [Recognized Products List](#) under the category of "Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers".

312.05.09 Steel Blockouts – Steel blockouts for modified Thrie-beam guardrail shall be manufactured to any of the following standards:

- AASHTO M 270 (ASTM A709), Grade 36 [250] or Grade 50S [345S] without the supplementary requirements;
- ASTM A36; or,
- ASTM A992

After the section is cut and all holes are drilled or punched, the components shall be hot dip galvanized in accordance with ASTM A123.

Any areas where the galvanizing is damaged during installation shall be touched up with two heavy coats of zinc rich paint selected from the Ministry's [Recognized Products List](#) under the category of "Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers".

Unless otherwise specified, the blockout type, size and length shall be in accordance with TF13 Designation [PWB03](#), complete with connecting bolts and nuts ([FBX16a](#)).

312.06 Treated Wood Posts and Offset Blocks

312.06.01 Rectangular Posts – Unless otherwise specified, details shall be in accordance with SS Drawings SP 312-1, SP 312-2 and SP 312-3. Rectangular posts and offset blocks shall be Douglas Fir/Hemlock "No. 1", Structural Posts and Timber, graded in conformity with the requirements of SS 909.

Wanes on any face shall not exceed the following width, being the minimum permissible post width less the portion entirely free of wane:

- Above grade (including blocks): 25 mm
- Below grade: 60 mm

Posts and blocks shall be supplied in the exact lengths specified and shall be rough-sawn or S4S dressed, nominal 200 mm x 150 mm, with dressed minimum dimension of 189 mm x 138 mm and a tolerance of plus 3 mm.

312.06.02 Preservation Treatment – All required wood components shall be pressure treated in accordance with SS 908.

The preservatives and retention of preservatives for all posts and blocks shall conform to SS 908 from the classification use table under the heading of “Roadside guardrail posts and other guardrail components”.

Treated wood shall also meet the requirements of SS 604 - Steel Traffic Barrier Construction.

Ideally, all posts and blocks shall be pressure treated after being cut to the specified length. However, the buried end of any post must be pressure treated in a treatment facility, but the rest of the cuts may be field treated in accordance with SS 908.07.

312.07 Quality Control – The Contractor shall be fully responsible for all Quality Control testing, inspection and documentation to achieve compliance with the Contract.

The Contractor shall provide documented test results for the materials to the Ministry Representative for review in accordance with the Contract, and upon request.

312.08 Quality Assurance – The Ministry may implement a Quality Assurance program to confirm compliance with the Contract. Without limitation, the QA activities may include auditing the Contractor’s and fabricator’s Quality Control program, inspection, and testing, all at the Ministry’s discretion.

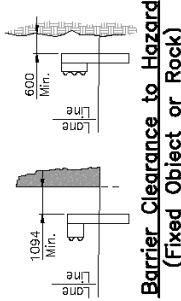
All materials may be inspected by the Ministry before shipment to the construction site, or at the construction site, or both. The supplier or manufacturer shall provide the necessary facilities to enable the Ministry Representative to expeditiously examine selected pieces or take specimen samples for testing from the material lot(s) clearly identified for intended supply to the Ministry. Identification shall be such that after inspection and testing the lot may be subsequently identified for acceptance or rejection.

Inspection of material before shipment shall not preclude its subsequent rejection at the construction site if found to fail any requirements of this specification.

W-Beam Barrier on Timber Posts **SGR38**

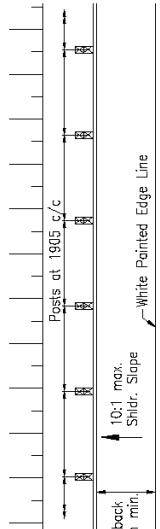
Thrie Beam Barrier on Timber Posts **SGR09**

SP312-1

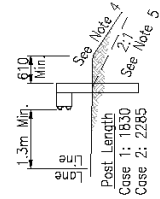


Barrier Clearance to Hazard (Fixed Object or Rock)

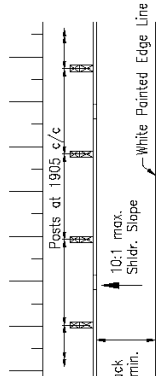
NOTE: The above clearances are based on a standard post spacing of 1905



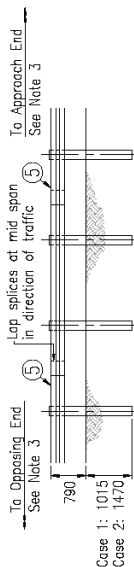
Plan



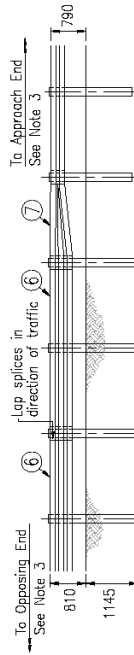
Barrier Placement Near Embankment



Plan



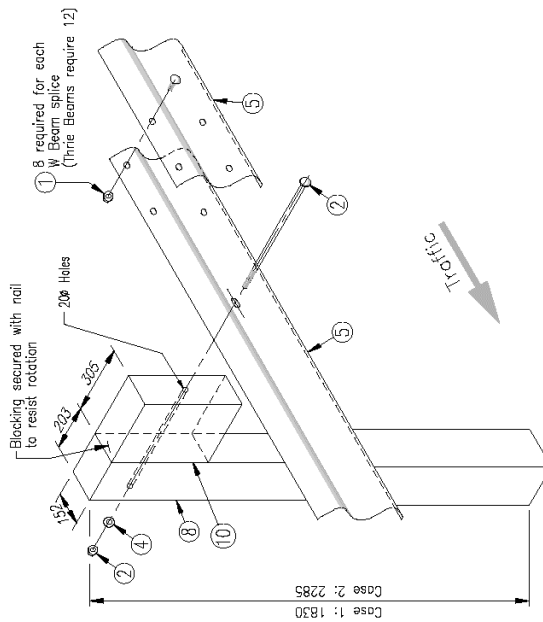
Elevation W-Beam Guardrail



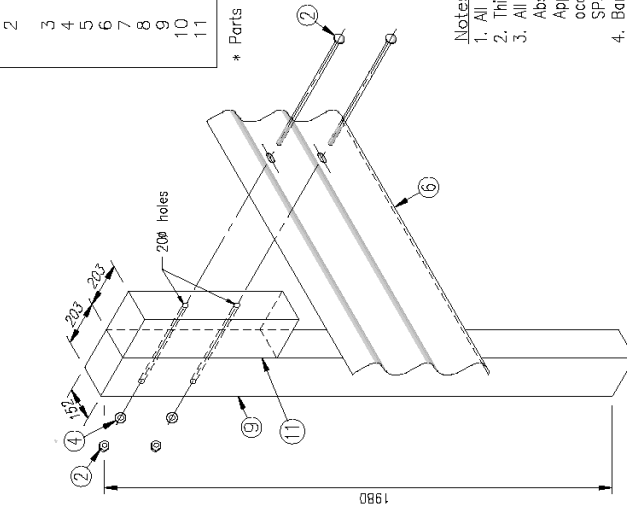
Elevation Thrie Beam Guardrail (with Transition to W-Beam)

Part No.	* TF13 Part No.	Description
1	FBB01	16mm (5/8") Button head bolt (35mm Lg.) & Recess nut
2	FBB07	16mm (5/8") Button head bolt (530mm Lg.) & Recess nut
3	---	Washer for 16mm (5/8") Bolt
4	FWC16a	SGR38b/d 4-Space W-Beam
5	RWM04a-b	SGR09c Thrie Beam
6	RTM02a-b	W-Thrie Beam Transition Section
7	RWT02a-b	152x203 Timber post
8	PDE02/PDE18	152x305 Timber post
9	PDE03	152x305 Timber post
10	PDB11a-b	152x203 Timber post
11	PDB02a	152x203 Timber post

* Parts list as per Task Force 13.



Exploded View of W-Beam Splice/Post Assembly (Meets MASH 2016 TL-3)



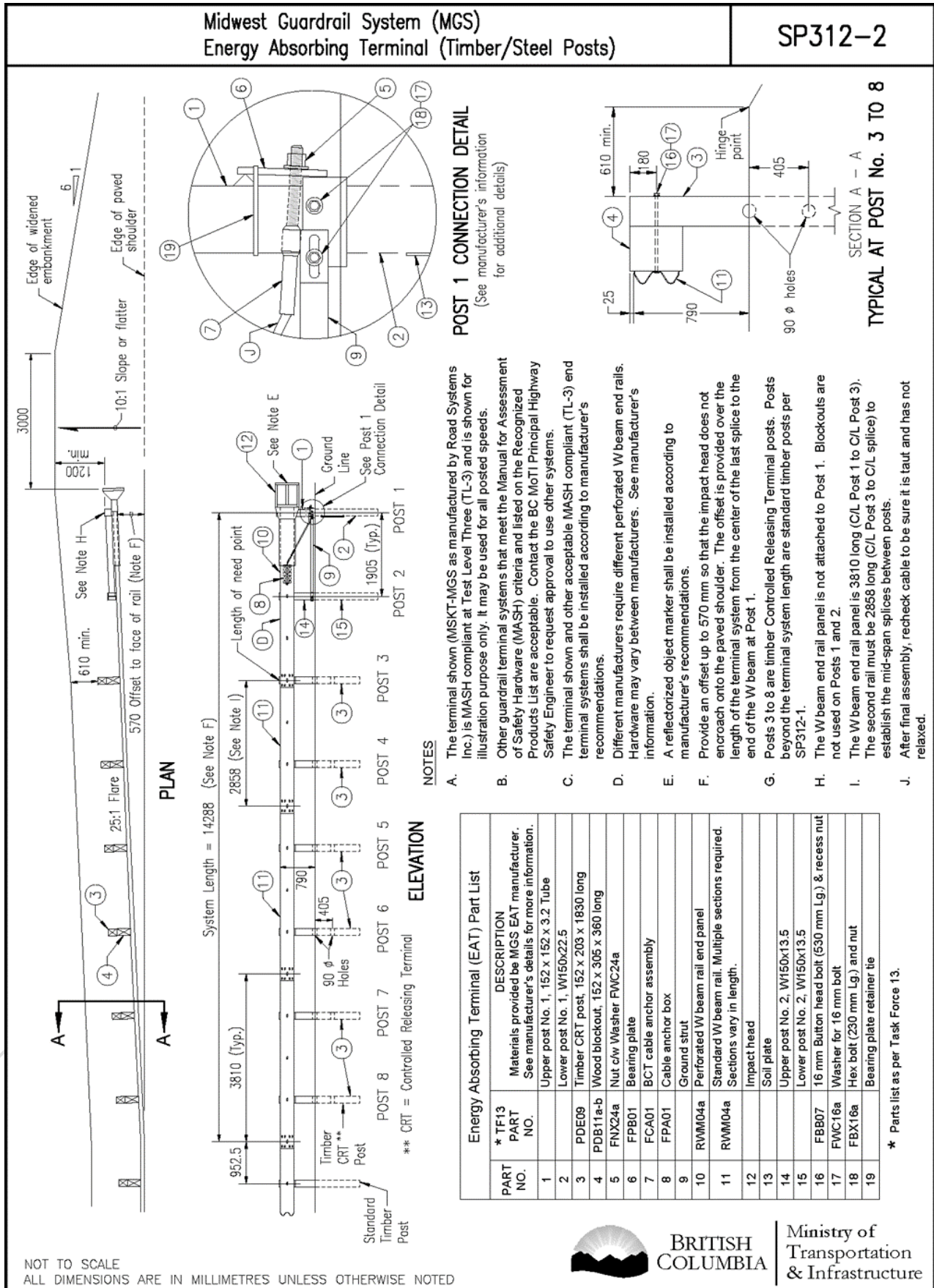
Exploded View of Thrie Beam Splice/Post Assembly (Meets NCHRP 350 TL-3)

- Notes:**
- All dims. are in mm unless otherwise noted.
 - This drawing to be used with Specification 312.
 - All traffic barrier installations will have an Energy Absorbing Terminal or buried end treatment at both the Approach and Opposing ends to reduce vehicle occupant hazard and provide anchorage. See Dwg. SP312-2 for details.
 - Barrier Placement Case 1: 1830 long W beam post - use when there is 0.61 m or greater shoulder width behind the post.
 - Barrier Placement Case 2: 2285 long W beam post - if side slope is 2:1 or flatter, posts may be placed at the shoulder break point.

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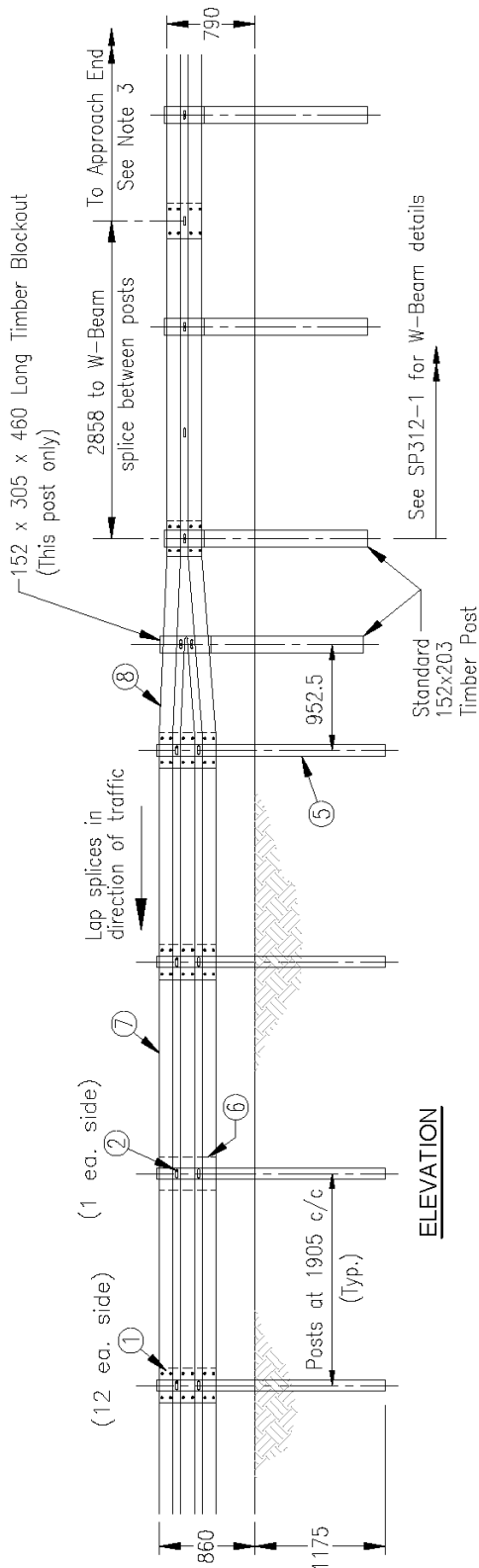


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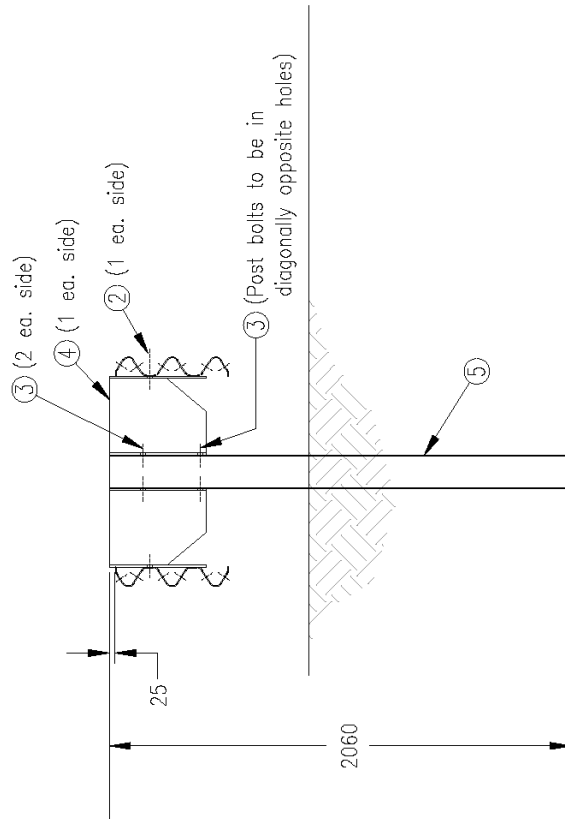
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Modified Thrie Beam Barrier on Steel Posts SGM09b

SP312-3



ELEVATION



SIDE VIEW

(Meets NCHRP 350 TL-4)

Part No.	* IF13 Part No.	Description
1	FBB01	16mm (5/8") Button head bolt (35mm Lg.) & Recess nut
2	FBB02	16mm (5/8") Button head bolt (50mm Lg.) & Recess nut
3	FBX16a	16mm (5/8") Hex head bolt (40mm Lg.) & nut
4	PWB03	Modified Thrie Beam Blackout
5	PWEO4	Steel Guardrail Post W150x13.5
6	RTB01a	Thrie Beam Backup Plate
7	RTM02a	SGM09b Thrie Beam
8	RWTO1a	W - Thrie Beam Transition Section

* Parts list as per Task Force 13.

Notes:

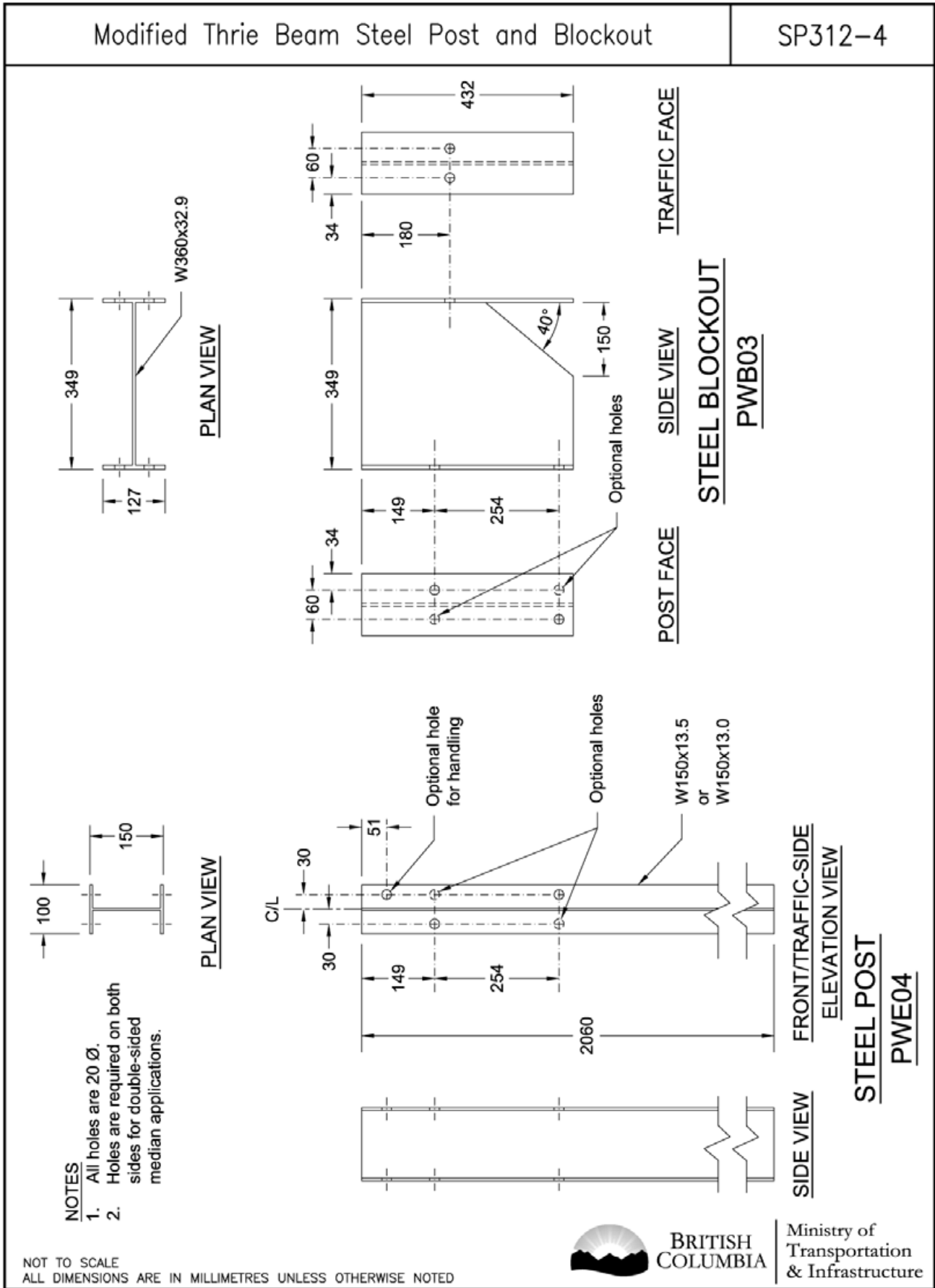
1. All dims. are in mm unless otherwise noted.
2. This drawing to be used with Specification 312.
3. All traffic barrier installations will have Energy Absorbing Terminal end treatments at both the Approach and Opposing ends to reduce vehicle occupant hazard and provide anchorage. See Dwg. SP312-2 for details.

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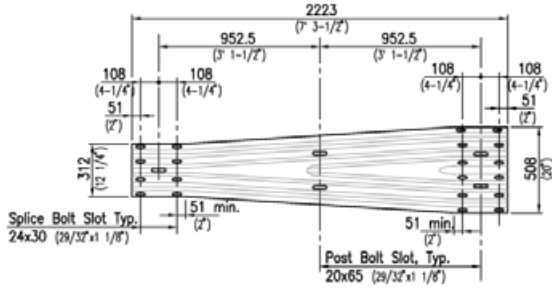
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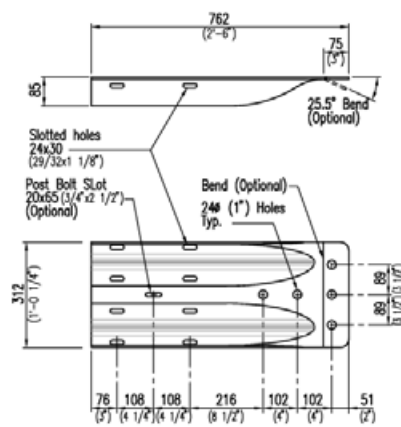
Steel Guardrail Beam Parts		SP312-6																										
<p style="text-align: center;">W Beam RWM01a - 22a</p> <p style="text-align: center;">Thrie Beam RTM02a - 06a</p>	<p style="text-align: center;">Back-up Plate (non-splice posts) RTB01a-b</p>																											
<p style="text-align: center;">16mm (5/8") Button Head Bolt and Recess Nut FBB01-05</p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th>DESIGNATOR</th> <th>L</th> <th>T (Min.)</th> </tr> </thead> <tbody> <tr> <td>FBB01</td> <td>35</td> <td>30</td> </tr> <tr> <td>FBB02</td> <td>50</td> <td>45</td> </tr> <tr> <td>FBB03</td> <td>255</td> <td>100</td> </tr> <tr> <td>FBB04</td> <td>460</td> <td>100</td> </tr> <tr> <td>FBB05</td> <td>640</td> <td>100</td> </tr> </tbody> </table>	DESIGNATOR	L	T (Min.)	FBB01	35	30	FBB02	50	45	FBB03	255	100	FBB04	460	100	FBB05	640	100	<p style="text-align: center;">16mm (5/8") Button Head Bolt and Recess Nut FBB06-07</p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th>DESIGNATOR</th> <th>L</th> <th>T (Min.)</th> </tr> </thead> <tbody> <tr> <td>FBB06</td> <td>360</td> <td>100</td> </tr> <tr> <td>FBB07</td> <td>530</td> <td>100</td> </tr> </tbody> </table>	DESIGNATOR	L	T (Min.)	FBB06	360	100	FBB07	530	100
DESIGNATOR	L	T (Min.)																										
FBB01	35	30																										
FBB02	50	45																										
FBB03	255	100																										
FBB04	460	100																										
FBB05	640	100																										
DESIGNATOR	L	T (Min.)																										
FBB06	360	100																										
FBB07	530	100																										
<p style="text-align: center;">Washer for 16mm (5/8") Bolt FWC16a</p>	<p style="text-align: center;">16mm (5/8") Hex Bolt and Nut FBX16a</p>																											
<p>NOT TO SCALE ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED</p>		<div style="display: inline-block; vertical-align: middle; margin-left: 10px;"> <p>BRITISH COLUMBIA</p> <p>Ministry of Transportation & Infrastructure</p> </div>																										

Steel Guardrail Beam Parts	SP312-7
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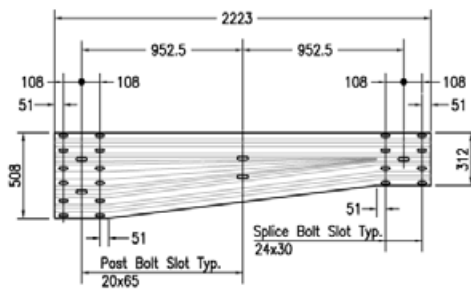
**W – Thrie Beam Transition Section
RWT01a-b**



**W Beam Terminal Connector
RWF02b**



**W – Thrie Beam Transition Section
RWT02a-b**



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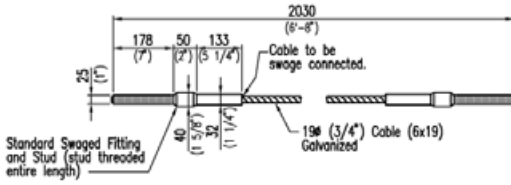
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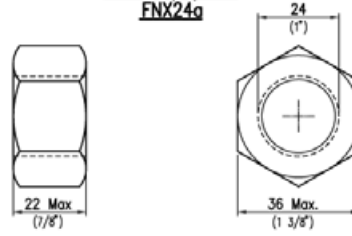
Steel Guardrail – B.C.T. End Assembly Parts

SP312-8

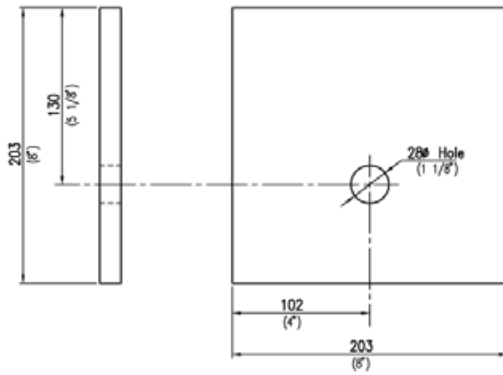
**BCT Cable Anchor Assembly
FCA01**



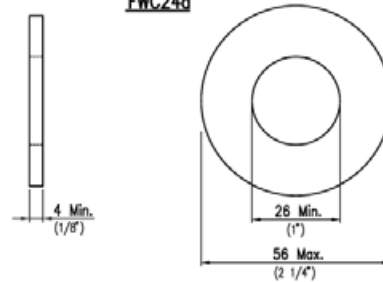
**24mm (1") Nut
FNX24g**



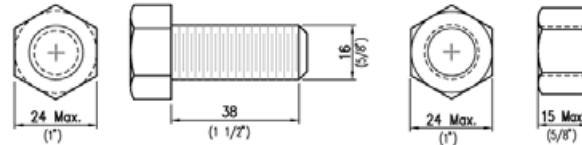
**BCT (Timber) Bearing Plate
FPB01**



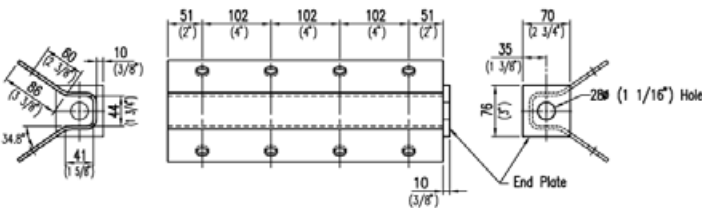
**24mm (1") Washer
FWC24g**



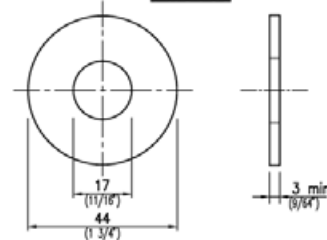
**16mm (5/8") Hex Bolt and Nut
FBX16g**



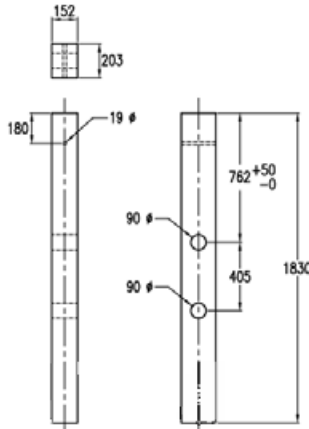
**BCT Anchor Plate
FPA01**



**Washer for 16mm (5/8") Bolt
FWC16g**



CRT Post



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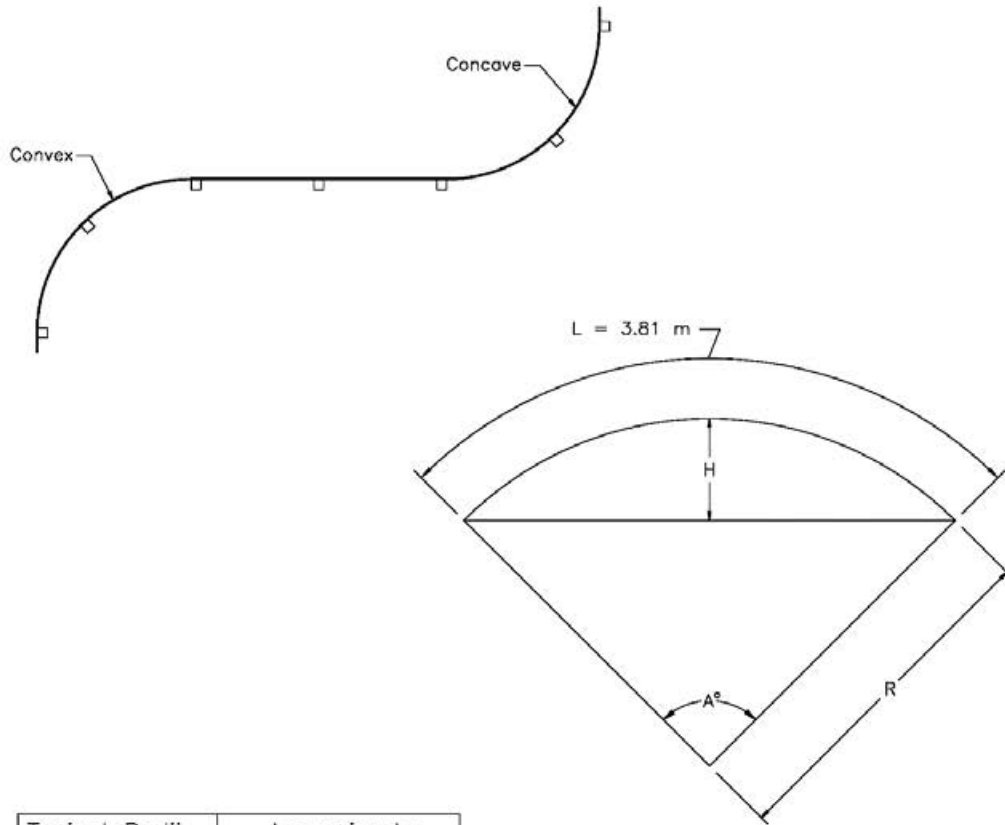


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Curved Rails

SP312-11



Typical Radii R (m)	Approximate A (deg) H (mm)	
1.5 (min. convex)	145	1055
3.0 (min. concave)	72	585
5.0	43	360
7.5	29	240
10.0	21	180
12.5	17	145
15.0	14	120
18.0	12	100
21.0	10	85
25.0	8	70
30.0	7	60
35.0	6	50
40.0	5	45
45.0	4.5	40

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SECTION 316

STEEL AND WIRE FENCE MATERIALS

316.01 Scope – This Section covers the quality and manufacture of wire fencing material, steel posts, gates and hardware, and various accessory materials and the fabrication of steel tubular fencing for pedestrian and bicyclist applications.

Treated wood fence posts are specified by SS 909.

For convenience, chain link fabric necessary for protection on rock slopes is included in this Section.

The uses of materials specified in this Section are generally indicated on Drawings of the SP741 series for construction of fences and SS 207 for rock protection.

The types of Standard Wire fencing and Chain Link fabric for fencing and protection on rock slopes covered by this Section are designated as follows:

- **Type A** Special Wire Fabric Fence for use only on railway rights-of-way

- **Type B** Standard Wire Fabric Fence
- **Type C** Standard Barbed Wire Fence
- **Type D** Chain Link Fence.

Chain link fence protection on rock slopes and tentative requirements for High-tensile Smooth-wire Fences are also included.

316.02 Applicable Documents – See Table 316-A.

Note: Standards referred to in this Section are obtainable from:

CGSB: Publishing and Depository Services
Public Works and Government Services
Canada
Ottawa ON K1A 0S5
<https://www.tpsgc-pwgsc.gc.ca/ongc-cgsb/boutique-store-eng.html>

Table 316-A: Documents Applicable for Reference

<u>ASTM STANDARD (unless otherwise noted)</u>	<u>Specification title</u>
<u>CAN/CGSB138.1-M</u>	<u>Fabric for Chain Link Fence</u>
<u>CSA C83</u>	<u>Communication and Power Line Hardware [for Strain Age Embrittlement test]</u>
<u>A53</u>	<u>Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated Welded and Seamless</u>
<u>A90</u>	<u>Standard Test Method for Weight [Mass] of Coating on Iron or Steel Articles with Zinc or Zinc-Alloy Coatings</u>
<u>A116</u>	<u>Standard Specification for Metallic-Coated Carbon Steel Woven Wire Fence Fabric</u>
<u>A121</u>	<u>Standard Specification for Metallic-Coated Carbon Steel Barbed Wire</u>
<u>A123</u>	<u>Standard Specification for Zinc (Hot-Dipped Galvanized) Coatings on Iron and Steel Products</u>
<u>A153</u>	<u>Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware</u>
<u>A370</u>	<u>Standard Test Methods and Definitions for Mechanical Testing of Steel Products</u>
<u>A641</u>	<u>Standard Specification for Zinc-Coated (Galvanized) Carbon Steel Wire</u>
<u>A1011</u>	<u>Standard Specification for Steel, Sheet and Strip, Hot-Rolled, Carbon, Structural, High-Strength Low-Alloy, High-Strength Low-Alloy with Improved Formability, and Ultra-High Strength</u>
<u>B6</u>	<u>Standard Specification for Zinc (Slab Zinc)</u>
<u>B117</u>	<u>Standard Practice for Operating Salt Spray (Fog) Apparatus</u>
<u>B211</u>	<u>Specification for Aluminum-Alloy Bar, Rod, and Wire</u>
<u>D2247</u>	<u>Standard Practice for Testing Water Resistance of Coatings in 100 % Relative Humidity</u>
<u>E4</u>	<u>Standard Practices for Force Verification of Testing Machines</u>
<u>F1043</u>	<u>Standard Specification for Strength and Protective Coatings on Steel Industrial Fence Framework</u>
<u>G152</u>	<u>Standard Practice for Operating Open Flame Carbon Arc Light Apparatus for Exposure of Nonmetallic Materials</u>
<u>G155</u>	<u>Standard Practice for Operating Xenon Arc Light Apparatus for Exposure of Non-Metallic Materials</u>

ASTM: American Society for Testing and Materials
 100 Barr Harbor Drive
 West Conshohocken, PA
 19428-2959
 USA
<http://www.astm.org/>

316.03 Type A & B Fence Fabrics – Fence fabric for railway and highway rights-of-way shall be manufactured from good commercial quality galvanized steel wire in general accordance with ASTM A116, but shall conform to the requirements following and those listed in Table 316-B.

316.03.01 Styles – See Table 316-B. Length of fabric rolls shall be 101 m unless otherwise ordered.

Note: Other standard and specialty fabrics when specified shall meet the following general specifications.

316.03.02 Wire Size and Strength – All wire used in the fabric shall have a minimum galvanized coated nominal diameter of 2.50 mm (12 1/2 ga.). The variation permitted from the measured nominal diameter shall be ± 0.1 mm. The minimum breaking force shall be 4250 N.

316.03.03 Zinc Coating – The zinc used for coating shall be any grade of zinc conforming to ASTM B6. All wire shall have a uniform coating of zinc which shall adhere firmly to the base metal. The average mass of zinc coating shall be 90 g/m² of uncoated wire surface for three specimens but minimum 80 g/m² for any individual specimen.

316.03.04 Quality of Work – Splicing of individual line wires by means of a wrap joint, electric butt weld or galvanized sleeve is permitted.

The maximum number of line wire joints shall not exceed half of the number of line wires in any roll of fencing.

All joints shall be made in accordance with good work practice.

All stay wires shall be properly spaced and substantially perpendicular to the line wires.

The locks or fastenings at the intersections of the longitudinal and vertical wires of the woven wire fencing shall be of such construction as will prevent them from slipping, either longitudinally or vertically. All locks and fastenings shall be effectively galvanized.

Horizontal wires of all woven wire fencing shall be provided with tension curves to accommodate temperature changes after erection.

Table 316-B: Styles of Fence

Fence Types	Fabric Style No.	Number of Horizontal Wires	Fabric Height (mm)	Stay Wire Spacing (mm)	Wire Diameter (mm)
A	1047-6	10	1195	150	2.50 min.
B	939-6	9	990	150	2.50 min.

316.03.05 Testing

- (a) **Samples:** Sufficient length of fabric to include at least three vertical stay wires shall be cut from each roll to be tested as required by SS 316.15 from which line wire and stay wire specimens shall be taken for testing.
- (b) **Diameter of coated wire:** The average of two readings taken at right angles to each other at random on uniformly coated areas shall all be within the tolerance measurements specified in SS 316.03.02.
- (c) **Strength:** The tensile strength of three line wire specimens shall be tested in accordance with ASTM A370 on a machine calibrated to ASTM E4 standards. The tensile strengths shall meet the requirements of SS 316.03.02.
- (d) **Coating adhesion:** Three line wire specimens shall be wrapped in a close helix at least twice around a cylindrical mandrel having a diameter four times the nominal diameter of the wire: wrapping speed shall not exceed four seconds per turn. The coating shall adhere to the wire without flaking or being removed by rubbing with bare fingers.
- (e) **Coating mass:** Three line wire specimens (from each roll), each more than 400 mm in length, and three stay wire specimens of short lengths between knots, etc., but each specimen totalling more than 400 mm shall be tested in accordance with ASTM A90 for conformity with the requirements of SS 316.03.03.
- (f) **Rejection and Retest:** Refer to SS 316.16.

316.04 Type D Fence Chain Link Fabric – Chain link fence fabric shall be manufactured from good commercial quality galvanized steel wire (or other specified finish) in general accordance with CAN/CGSB-138.1M and shall conform to the requirements listed in SS 316.04.01 through SS 316.04.04.

316.04.01 Fabric Classification (according to CGSB Standard) – The type, class, style and grade of chain link fence fabric shall be as specified on the Purchase Order, Work Order or Special Provisions in accordance with the Drawings and/or SS Drawings SP741-05.01 and SP741-05.02, selected from the following or as may be otherwise specified or approved.

- (a) **Type/Class 1A** – Zinc coated (galvanized) steel wire fabric normally will be specified for all heights of chain link fences as indicated on Drawing SP741-05.01, unless Type/Class 1B Vinyl coated steel is required for authorized special installations.

(b) **Style** – nominal diameter of zinc coated wire:

- Heavy, for highway protection fence from rolling rock
- Medium, for Standard and Security height fences
- Light, for Residential height fences (see SS Drawing SP741-05.01)
- See TABLE 316-C and Table 316-D.

(c) **Grade** – mass of zinc coating per unit area of uncoated wire: Grade 1, average 490 g/m² for three specimens, minimum 440 g/m² for any individual specimen. The zinc used for coating shall be any grade of zinc conforming to ASTM B6. All wire shall have a uniform coating of zinc which shall adhere firmly to the base material.

Note: When specified, Grade 2 heavy coating (average 610 g/m², individual 550 g/m² minimums) for corrosive environments and the zinc coating (average 90 g/m², individual 80 g/m² minimums) for vinyl coated wire and the vinyl coating requirements shall be in accordance with CAN/CGSB-138.1M.

316.04.02 Fabric Height, Length and Mesh – Wire shall be woven throughout to a uniform diamond mesh 50 mm ± 3.0 mm clear between parallel mesh wires: fabric height as specified ± 25 mm with both selvage edges twisted and/or knuckled:

- fabric 1.5 m high and under: knuckled both edges
- fabric 1.8 m high and over: knuckled one edge, and twisted the other
- Residential height fence light style fabric 1.0 m and 1.2 m high: knuckled both edges.

Length of rolls with slack removed by nominal tensioning shall be 100 m unless otherwise ordered.

Note: Typical (normal) ordering designation from above: Type 1, Class A, Grade 1, Medium Style with roll width (height), length, mesh size and quantity.

316.04.03 Quality of Work – Fabric shall be of uniform quality and condition, free from all defects and imperfections affecting serviceability and appearance.

316.04.04 Testing

(a) **Samples:** Three full height pickets shall be provided from each roll as required by SS 316.15.

(b) **Diameter of coated wire:** The average of two readings taken at right angles to each other at random on uniformly coated portions of each pair of pickets between bends shall be within the tolerance measurement specified in SS 316.04.01(b).

(c) **Strength:** The tensile strength of three line wire specimens shall be tested in accordance to ASTM A370 on a machine calibrated to ASTM E4 standards. The tension applied to the undeformed portion between two adjacent bends shall meet the requirements of SS 316.04.01(b).

(d) **Coating adhesion:** Three specimens of undeformed portions between two adjacent bends shall be wrapped in a close helix once around a cylindrical mandrel having a diameter four times the nominal diameter of the wire: wrapping speed shall not exceed four seconds per turn The coating shall adhere to the wire without flaking or being removed by rubbing with bare fingers.

(e) **Coating mass:** Three specimens (from each roll) each more than 400 mm long excluding knuckled or twisted ends shall be tested in accordance with ASTM A90 for conformity with the requirements of SS 316.04.01(c).

(f) **Rejection and Retest:** Refer to SS 316.16.

316.05 Chain Link Fabric and Accessories on Rock Slopes – Chain link fabric for protection on rock slopes shall

Table 316-C: Zinc Coated Wire

Type 1A	Style	Zinc Coated Wire Nominal Diameter	Breaking Force Minimum (N)
	Heavy	5.0 mm ± 0.1 mm	10 000
	Medium	3.5 mm ± 0.1 mm	5 000
	Light	3.0 mm ± 0.1 mm	3 600

Table 316-D: Vinyl Coated Zinc-Coated Wire (When Permitted)

Type 1B	Style	Vinyl Coated Zinc-Coated Wire Nominal Diameter	Breaking Force Minimum (N)
	Heavy	5.0 mm ± 0.1 mm	6 000
	Medium	3.5 mm ± 0.1 mm	3 000
	Light	3.0 mm ± 0.1 mm	2 600

SECTION 316

be manufactured and tested in general conformity with SS 316.04 and to the following requirements:

- Fabric shall be to CAN/CGSB-138.1M designation of wire woven to 75 mm diamond mesh in rolls 3.0 m wide, knuckled on both selvage edges, and 15 m long. See [Table 316-E](#)

Note: For corrosive conditions and environment Grade 2 – heavy galvanizing may be specified as a special order with possible extended lead time required for delivery.

Table 316-E: Fabric Designation

<u>Type 1</u>	<u>Class A Grade 1</u>	<u>Style</u>	<u>Nominal Diameter</u>
<u>Steel</u>	<u>zinc coated (galvanized)</u>	<u>Light</u>	<u>3.0 mm</u>
		<u>Medium</u>	<u>3.5 mm</u>

316.06 Barbed Wire – Barbed wire for Type B & C fences and for security height Type D fences shall be manufactured from good quality galvanized steel wire in general accordance with ASTM A121 but shall conform to the following detail requirements:

- (a) Wire Size and Strength:** The wire shall be continuously twisted double strand of 2.5 mm nominal diameter (12 1/2 ga.) and to have a minimum breaking force of 4000 N.
- (b) Barbs:** Four point barbs shall be of not less than 2.0 mm nominal diameter (14 ga.) and regular 150 mm maximum spacings.
- (c) Zinc Coating Class (according to ASTM A121):** The zinc used for coating shall be any grade of zinc conforming to ASTM B6. All wire, including barbs, shall have a uniform coating of zinc which shall adhere firmly to the base metal.

The average mass of zinc coating per unit area of uncoated wire for three specimens and the minimum for any individual specimens shall be in accordance with the class of coating as follows:

- (i) Class 1** for use in the construction of Type C fences and for the top strand of Type B fences (see Drawings SP741-01.01 and SP741-02.01); the coating shall be average 90 g/m² and 80 g/m² individual minimums.

STEEL AND WIRE FENCE MATERIALS

- (ii) Class 3** for use with Security height Type D Chain Link fences (see Drawing SP741-05.01); coating shall be average 240 g/m² and 215 g/m² individual minimums.

- (d) Length:** Spools shall be 400 m unless otherwise ordered.
- (e) Quality of Work:** Splicing of individual strand wires by means of a wrap joint or an electric butt weld is permitted. Not more than three splices or joints shall exist in any 400 m spool of barbed wire. Such splices or joints shall be made in accordance with good work practice.

The twist in the double strand barbed wire shall be uniform and continuous in one direction to lock the barbs in place. The barbs shall be sharp and well formed.

Wire strands showing nicks or indentures likely to cause the wire to break under flexure are not acceptable.

- (f) Testing**
 - (i) Samples:** Sufficient length of barbed wire shall be cut from each spool to be tested as required by SS 316.15.
 - (ii) Strength:** The tensile strength of three line wire specimens shall be tested in accordance to ASTM A370 on a machine calibrated to ASTM E4 standards. The tensile strength shall meet the requirements of SS 316.06(a).
 - (iii) Coating Mass:** Three single wire specimens (from each spool), each aggregating more than 400 mm in length shall be tested in accordance with ASTM A90 for conformity with the requirements of SS 316.06(c).
 - (iv) Rejection and Retest:** Refer to SS 316.16.

Note: Smooth (barbless) galvanized wire, not specified to be "high-tensile," shall be double strand meeting all the above requirements except those for barbs.

316.07 High-tensile Wire – Requirements for galvanized wire for high-tensile smooth-wire (no barbs) fencing are shown in [Table 316-F](#).

Table 316-F: Requirements for Galvanized Wire for High Tensile Smooth-Wire (No Barbs) Fencing

Diameter:	2.5 mm nominal diameter (12 ½ ga.)
Tensile Strength:	between 1170 MPa and 1590 MPa (170,000 to 230,000 psi)
Breaking Force:	in excess of 5800 N (1300 lbf)
Zinc Coating (Class 3):	minimum 240 g/m ²
Coil Length:	in excess of 1200 m (approx. 100 lb.)

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STEEL AND WIRE FENCE MATERIALS

316.08 Brace, Tension and Tie Wire

316.08.01 Brace Wires – Brace wire shall be manufactured of good commercial quality soft temper steel wire 3.5 mm nominal diameter gauge and zinc coated (galvanized) normally minimum 80 g/m² (Class 1 according to ASTM A641) or as otherwise specified or approved.

Brace wire for high-tensile smooth-wire fencing may be fence wire specified in SS 316.07.

316.08.02 Tension Wire – Tension wire to secure bottom of chain link fencing and at top where specified shall be manufactured of good commercial quality coil spring crimped steel wire 5.0 mm nominal diameter gauge and zinc-coated (galvanized) normally 400 g/m² (Class B according to ASTM A641) for Standard and Security height fences, or as otherwise specified or approved. Residential height fences tension wire shall be 3.5 mm nominal diameter.

316.08.03 Tie Wire and Wire Hog Ring Clips – Tie wire and wire hog ring clips shall be of steel of the nominal diameter gauge specified and zinc coated equivalent to that specified for the relevant fencing fabric: Aluminum tie wire shall be of specified gauge and according to ASTM B211.

316.09 Steel Fence Posts on Rock (Excluding Chain Link Fence) – Steel fence posts and braces for Type A, B and C fences where specified or required on exposed rock or rock with minimum over-burden shall be of good commercial quality steel conforming to either Category A or Category B as specified in SS 316.10 with the following exceptions:

- Line post mass shall be not less than 1.9 kg/m and angles 4.75 kg/m minimum, both drilled for attaching barbed wire or a substantial number of fence fabric line wires with wire ties or clips.
- Channel braces shall be 1.5 kg/m minimum.

316.10 Steel Pipe Posts, Rails and Braces – Chain Link Fencing

316.10.01 Category and Sizes – All posts, rails and braces shall be of Category 1 or Category 2 at the discretion of the supplier and shall be supplied in the nominal outside diameter as specified on the Purchase Order, Work Order or Special Provisions in accordance with the Drawings and/or Drawing SP741-05.02 and selected from Table 316-G or Table 316-H or as may be otherwise specified or approved.

Note: Specifications or Purchase Order may permit thin wall tubing and consequent lower zinc coating for proprietary galvanized and vinyl coated residential fence posts and gates.

Bolts and nuts for fastening braces to posts shall be galvanized Class D (305 g/m² individual pipe posts, rails and braces for chain link Type D fences shall be manufactured in general accordance with ASTM A53.

- (a) **Category A** – Category A posts, braces and rails shall be steel butt weld pipe, Schedule 40, as specified by ASTM A53, except that the hydrostatic testing requirement is waived, and the minimum weight shall be no less than 95% of the nominal weight. Pipe shall be zinc-coated in accordance with ASTM A53. This

Table 316-G: Requirements for Pipe Components

Actual Outside Post Diameter (mm)	26.7	33.4	42.1	48.3	60.3	73.0	88.9
Wall Thickness (mm)	2.87	3.38	3.56	3.68	3.91	5.16	5.49
Mass (kg/m)	1.7	2.5	3.4	4.0	5.4	8.6	11.3
Tolerances:	Diameter ± 1%			Mass ± 5%			

Table 316-H: Requirements for Pipe Components for Chain Link Fencing – Category A

Pipe Sizes	Nominal Outside Diameter (mm)		
	Residential	Standard	Security
Line Posts	42	48	60
End, Corner & Straining Posts	48	60	73
Gate Posts, gates 2.5 m and less	48	60	73
Gate Posts, gates over 2.5 m	48	73	89
Top Rails	33	33	42
Brace Rails	--	33	42

ASTM Standard requires, among other things, an average zinc coating of 550 g/m² and a minimum zinc coating of 490 g/m².

- (b) **Category B** – Category B posts, braces and rails shall be made from steel pipes. The pipe shall be formed from steel strip conforming to ASTM A1011. The steel in the formed pipe shall have a minimum yield strength of 350 MPa (50,000 psi). The product of the yield strength and section modulus of the pipe shall be not less than that of pipe meeting the requirements for Category A posts and rails.

The outer surface of Category B posts, braces and rails shall have all of the following protective coatings, applied in the following order:

- (i) Hot dip zinc coating, conforming to ASTM B6, High Grade or Special High Grade, applied at an average weight of 305 g/m² (1.0 oz/ft²) and at a minimum weight of 275 g/m² (0.9 oz/ft²), as determined by the test method in ASTM A90.
- (ii) Chromate conversion coating, with a weight of 30 micrograms/sq. in. ±15 micrograms/sq.in., as determined by a strip and weight method for the zinc and chromate and by determining the percentage of each by atomic absorption spectrophotometer.
- (iii) Clear acrylic polyurethane coating with a thickness of 0.100 mm ± 0.025 mm (0.4 ± 0.1 mils).

The combined outer coating shall have an ability to resist:

- 1000 hours of exposure to salt fog with a maximum of 5% red rust when tested in accordance with ASTM B117.
- 500 hours of exposure to 100% relative humidity without blistering and peeling when tested in accordance with ASTM D2247; and
- 500 hours of exposure in a weatherometer without film cracking of the clear coat when tested in accordance with ASTM G152, Type E or EH carbon arc, or ASTM G155, Type_B or BH xenon arc.

The inner surface of Category B, posts, braces and rails shall have either a zinc coating as per ASTM F1043, Type_B, applied at an average rate of 305 g/m² and a minimum rate of 275 g/m²; or an ASTM F1043, Type D zinc coating with a minimum zinc powder loading of 91% by weight and a minimum thickness of 0.075 mm (0.3 mils). The inner coating shall have an ability to resist 650 hours of exposure to salt fog with a maximum 5% red rust when tested in accordance with ASTM B117.

316.10.02 Quality of Work – The finished pipes shall be reasonably free from injurious defects. All burrs at ends of pipes shall be removed.

The zinc or zinc/chromate/acrylic coating shall be free from injurious defects and excessive roughness.

316.10.03 Testing

- (a) **Samples:** A 100 mm specimen shall be cut from each end of one pipe in every lot of 500 or fraction thereof.

Note: Specimens may be taken from the extreme ends of pipe that has been cut to length after galvanizing; otherwise, specimens shall be taken at least 200 mm clear of each galvanized end build-up.

- (b) **Dimensions:** Specimens shall be measured for diameter and wall thickness/mass and all shall be within the tolerance measurements specified by SS 316.10.01.

- (c) **Mass Coating:** Specimens shall be tested for the mass of zinc or composite coating in accordance with SS 316.10.01 and ASTM A90.

- (d) **Rejection and Retest:** Refer to SS 316.16.

316.11 Gates and Hardware – Gates shall be supplied with hardware ready for installation as specified by the Purchase Order, Work Order or Special Provisions in accordance with the Drawings and/or Drawings SP741-04.04, SP741-04.05, SP741-05.03 and SP741-05.04 or as may be otherwise specified or ordered.

Gates may be specified by the types as follows:

- **Type I** Pressed sheet steel frame, in-fill and brace members
- **Type II** Pipe frame and horizontal pipe in-fill members
- **Type III** Pipe frame, bracing and wire fabric in-fill
- **Type IV** Pipe frame, bracing and chain link fabric in-fill
- **Type V** Slip wire range gates barbed wire and wood droppers

with the general requirements of material, finish, hardware and quality of work specified in SS 316.11.01 through SS 316.11.06.

316.11.01 General – Gate sizes shall be the fence height or as otherwise stipulated and the nominal width designated by the clear opening width between gate posts with allowance made for hinges and latches, etc.

Gates shall be fabricated of galvanized pipe, tubing or formed sheet metal sections properly joined, braced and with in-fill all as specified by type below.

All pipe joints shall be cut to fit closely together and fully welded, vertical stiles neatly capped or mitred to horizontals

with drilled holes to vent and drain closed end pipe members. Bends where permitted at corners shall be made without deforming tube or pipe section.

All welds, conforming to the best commercial practice, shall be smooth, and with adjoining and other marred galvanized pipe surfaces, shall be painted with two heavy coats of zinc rich paint selected from the Ministry's Recognized Products List under the category of "Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers".

All hardware and required fittings and fasteners shall be of suitable aluminum alloy or of steel or malleable or ductile cast iron, galvanized in accordance with ASTM A153. Hinges generally shall be so designed as to permit the gate to swing back against the fence and to prevent the easy removal of gate and hinges. Hinges and other fittings shall all be of adequate strength for fastening securely without distortion and shall be sound and free of defects. Chain and/or latching gate catch for padlocking together with keepers to hold the gate open and centre drop bolt and sleeve for double swing gates shall be provided as and where specified or ordered.

All galvanizing and other finished surfaces shall be smooth and free of sharp dangerous protrusions.

316.11.02 Type I Gate (sheet metal members) – Formed heavy duty galvanized sheet metal sections with rolled edges as horizontal rails and vertical stiles, braced vertically and/or diagonally with similar members as necessary shall be rigidly riveted together to form a rugged non-sagging prefabricated gate.

Nominal sizes:

- 1220, 2440, 3050, 3660, 4270 and 4880 mm wide; and
- 1220 mm high.

Gates to be supplied completely assembled with required hardware for wood (or metal) gate posts all as specified or ordered.

316.11.03 Type II Gates (all pipe) – Galvanized pipe 42 mm OD, 3.4 kg/m mass with zinc coating minimum 490 g/m² (see SS 316.10.01 and SS 316.10.03) shall be welded together as gate frame and horizontal pipe in-fill members at 305 mm on centres. Vertical centre bracing of 5 mm x 19 mm flat bar on both sides of all horizontals is not required for gates 3050 mm and 3660 mm nominal widths. Gussets shall be welded to opposite corners of the gate as shown on Drawing SP741-04.05.

Nominal sizes:

- 3050, 3660, 4270 and 4880 mm wide; and
- 1220 mm high.

Gates shall be supplied completely assembled with required hardware for wood (or metal) gate posts all as specified or ordered.

316.11.04 Type III Gates (pipe and fabric) – Galvanized pipe 33 mm OD, 2.5 kg/m mass with zinc coating minimum 490 g/m² (see SS 316.10.01 and SS 316.10.03) shall be formed with even radius bent corners or neatly welded corners and vertical bracing at 1/3 points in the width of 27 mm OD, 1.7 kg/m pipe. Cleats or gussets shall be welded to opposite corners of gate frame/panel(s) as indicated on SS Drawing SP741-04.05, holed for securing under tension a diagonal brace consisting of 5.0 mm nominal diameter galvanized coil spring wire (see SS 316.08) with or without forged turn-buckle as and when specified or ordered.

In-fill shall be wire fabric all of 3.5 mm galvanized diameter wire with seven or more horizontal line wires, and stay wires not more than 600 mm apart. Fabric shall be securely fixed taut without sag to gate frame all round and to vertical brace in an approved manner.

Nominal sizes:

- 3050, 3660, 4270, 4880 and 5490 mm wide; and
- 1070 and 1220 mm high.

Gates shall be supplied completely assembled with required hardware for wood (or metal) gate posts all as specified or ordered.

316.11.05 Type IV Gates (pipe and chain link) – Gate frame and required vertical bracing and/or diagonal bracing to cleats all welded as generally indicated on SS Drawing SP741-05.04 for hinged swing gates shall be fabricated to the designated size(s) complete with chain link in-fill and hardware for fixing to metal posts all as detailed, specified or ordered.

Pipe shall be in accordance with SS 316.10 and welding with SS 316.11.01. In-fill of chain link fabric as specified for fencing (see SS 316.04), shall be attached to gate frame stiles with tension bars and bands (see SS 316.12), and to horizontal members and vertical bracing with tie wire (see SS 316.08.03), all taut and sag free.

Sliding gates may be specified for gate openings over 4.0 m and shall be fabricated as generally indicated for swing gates but with diagonal tension rod bracing to support centre from both ends. The rolling hardware shall hold the gate vertical and parallel to the gate posts and adjoining fence at all times to allow free operation from any position while not retained by latches or keepers.

Roller assembly sheaves and wheels with bearings, grease fittings, necessary clamps, supports and fasteners, and chain and/or latching gate catch for padlocking, all of approved design, shall be provided as and when specified or ordered.

316.11.06 Type V Gates (range slip wire) – Barbed wire continued across standard width gate opening (for possible future farm gate) with 50 mm diameter full fence height treated wood droppers securely figure of eight wire tied and stapled to each line wire shall be provided with a galvanized

steel strap loop at bottom and either similar loop at top with an intermediate chain (for padlocking) or, when specified, a Princeton or other approved cinch-up lockable latch device all for securely attaching to fence or gate posts to hold open-end dropper.

316.12 Accessory Material – All required fittings, fasteners, hardware and accessories shall be of suitable aluminum alloy or steel, malleable or ductile cast iron, and galvanized in accordance with ASTM A153, or as may be otherwise specified and/or ordered.

- (a) Staples for securing wire and fabric to treated wood fence posts shall be nominal 3.5 mm diameter steel wire with a driving length of at least 45 mm heavy galvanized finish.
- (b) Hog ring clips for securing wire fabric and/or barbed wire of Type A, B and C Fences to steel posts and to tension wire of Type_D Fences see SS 316.08.03.
- (c) Dowel, Spikes, Bolts and Nuts shall be of good commercial quality, galvanized where specified or ordered.
- (d) Metal Droppers for high-tensile fencing will be prefabricated "Waratah" or other acceptable galvanized formed sheet metal droppers, 1070 mm for 5-wire and 1170 mm long for 6-wire fences, with preformed wire clips: such metal droppers or other acceptable manufacture may be specified or permitted as an alternative to the wood droppers specified by SS 909.08 for barbed wire (Type C) fences.

Note: Galvanized twisted double wire fence stays are permitted only for the extension of normal fence heights to deer height and the like.

- (e) All terminal and tensioning posts, for Type D chain link fences, shall be provided with aluminum or galvanized weather tight caps to fit and fasten securely and with extended arms for engaging barbed wire of security fences.
- (f) Caps for line posts for Type D chain link fences, shall carry top rail. Rail and brace end caps shall be provided for securing to rails and bolting with steel bands to posts.
- (g) Tension Bars for securing chain link fencing to terminal and tensioning posts and to vertical stiles of gates of Type D Fences shall be merchant quality mild steel strip 5 mm x 19 mm in minimum lengths of 50 mm less than full height of the chain link fabric, hot dip galvanized.
- (h) Tension and Brace Bands for securing brace and top rail caps and tension bars to posts shall be formed of minimum 2.5 mm x 19 mm mild steel strip, hot dip galvanized complete with bolt and nut.

- (i) Tensioning Devices for individual in-line high-tensile wire will be "Hayes" or other acceptable manufacture and operation.
- (j) Mechanical Splices and Fasteners shall be of "Nicopress FW2-3" or other acceptable manufacture applicable to the respective diameter and tensile strengths of wires.

316.13 Packaging and Marking – Each roll of fencing fabric or chain link mesh and spool of wire shall be tightly rolled and firmly tied to prevent loosening during shipment and handling.

Posts shall be securely fastened in bundles or sets, and accessories packaged in cartons or other suitable containers, to prevent loss or damage during shipment and handling.

Each product, roll, bundle or container etc., shall be clearly marked with the name, brand and/or trade mark of the manufacturer; the heat or lot number and other information sufficient to identify the style, type, class, grade, finish, height and/or length, nominal or outside diameter, gauge and the applicable Standard to which the product conforms.

316.14 Inspection – No material shall be shipped prior to inspection or until a release for shipment has been issued by the Ministry Representative.

All material may be inspected before shipment to the construction site or at the construction site, or both. The Supplier or manufacturer shall provide the necessary facilities to enable the Ministry Representative to expeditiously examine selected pieces from the material lot(s) clearly identified for intended supply to the Ministry. Identification shall be such that after inspection and testing the lot(s) may be subsequently identified for acceptance or rejection.

Inspection of material before shipment shall not exclude its subsequent rejection at the construction site if found to fail any requirements of this specification.

316.15 Sampling – One roll of fabric or spool of wire, in every lot of 50 or fraction thereof, in the shipment may be selected at random for testing but in no case will less than two samples be tested.

Specimens for testing will be required from the outside ends of the selected sample roll or spool material and both ends of one post in every 500 or fraction thereof.

Sufficient length of each roll, spool or pipe shall be taken to provide the number of specimens for each of the testing requirements specified.

Wire used for knots, knuckling, twisting and wraps and wire containing welds shall be excluded from tests.

See respective material tests for pertinent sample lengths.

316.16 Rejection and Retests – If any tested specimen fails to meet the testing requirements specified above, the roll, spool or pipe represented by the specimens shall be replaced

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with two additional rolls, spools or pipes selected by the Ministry Representative and both shall meet the test requirements in every respect, otherwise the complete lot represented by the samples will be subject to rejection.

In the event of complete lot rejection and prior to any further testing by the Ministry, the intended replacement lot(s) shall be satisfactorily tested by a mutually acceptable independent testing agency at no expense to the Ministry: where the time delay for retesting is unacceptable to the Ministry the order or part thereof will be subject to cancellation.

Rejected material shall be replaced at the Supplier's or Contractor's expense including shipping charges and removal of rejected material at the construction site, if applicable.

316.17 Steel Sidewalk Fence – Steel pedestrian and bicyclist sidewalk fence material shall be installed in

STEEL AND WIRE FENCE MATERIALS

accordance with the requirements of the BC Supplement to TAC Geometric Design Guide, the Drawings and the details provided on SP741-07.01 and SP741-07.02. All steelwork shall be fabricated in accordance with SS 422. All field welded railing connections shall conform to the requirements of SS 422.

If the 'swaged' detail shown on SP741-07.03 is proposed as an alternative, the Contractor shall provide the Ministry Representative with satisfactory test results from an accredited testing agency for the following test procedures:

- Flattening test in accordance with ASTM A370
- Whole body tensile test in accordance with ASTM A370
- Strain age embrittlement test in accordance with CSA C83 @-45°C

SECTION 317

P.E. PLASTIC DRAINAGE PIPE

DESCRIPTION

317.01 Scope – This specification covers the requirements for polyethylene plastic pipe products to be used for storm sewers, pipe culverts, and subdrains. This specification covers non-pressure polyethylene plastic pipe with nominal pipe diameters of 100 mm through to 900 mm. Larger pipe from 900 mm up to 1500 mm will be engineered (type, location application, backfill) as specified in the Special Provisions.

317.02 Applicable Documents – This specification refers to the following Standards, specifications or publications:

Canadian Standards Association (CSA):

B182.8: Profile Polyethylene (PE) Storm Sewer and Drainage Pipe and Fittings. (For nominal pipe diameters of 100 mm through to 900 mm.)

(Available only as part of the CSA B1800 Thermoplastic Non-Pressure Piping Compendium)

American Society for Testing Materials (ASTM):

D883: Standard Terminology Relating to Plastics

D2122: Standard Test Method for Determining Dimensions of Thermoplastic Pipe and Fittings

D2412: Standard Test Method for Determination of External Loading Characteristics of Plastic Pipe by Parallel-Plate Loading

D2444: Standard Test Method for the Determination of Impact Resistance of Thermoplastic Pipe and Fittings by means of a Tup (Falling Weight)

D3212: Specifications for Joints for Drain and sewer plastic Pipes Using Flexible Elastomeric Seals

D3350: Standard Specification for Polyethylene Plastic Pipe and Fittings Materials

F667: Standard Specification for 3 Through 24 in. Corrugated Polyethylene Pipe and Fittings

American Association of State Highway and Transportation Officials (AASHTO):

M 252: Standard Specification for Corrugated Polyethylene Drainage Pipe. For nominal pipe diameter from 75 mm through to 250 mm.

M 294: Standard Specification for Corrugated Polyethylene Pipe. 300 to 1500 mm (12 to 60-in.) Diameter.

317.03 Definitions: For the purpose of this specification the following definitions apply.

317.03.01 Polyethylene Plastic – A plastic based on polymers made with ethylene as essentially the sole monomer.

317.03.02 Corrugated Inner Wall Pipe – A single walled pipe product with a corrugated inner waterway where the wall is formed into a series of alternating ridges and grooves.

317.03.03 Profile – A pipe wall construction that presents an essentially smooth surface in the waterway but includes ribs or other shapes, which can be either solid or hollow, that help brace the pipe against diametrical deformation.

317.03.04 Open Profile Pipe – A pipe product that has an essentially smooth waterway braced circumferentially or helically with outside corrugations, forming an open profile pipe.

317.03.05 Closed Profile Pipe – A pipe that has a smooth waterway wall braced circumferentially or helically with corrugations, ribs, or other shapes that are joined integrally by an essentially smooth outer wall.

317.03.06 Certification Body – An independent third party agency, accredited by the Standards Council of Canada to confirm that a pipe manufacturer:

- (a) Produces pipe products to the quality and requirements of an accepted CSA standard; and
- (b) Has the mandate to certify the pipe products produced.

317.03.07 Certified – Pipe products that have been marked with a certification body's logo confirming that the production of the pipe product is in accordance with the quality and requirements of CSA B182.8.

317.04 Pipe Types and Materials – The pipe, fittings and couplers shall be manufactured from virgin polyethylene plastic, in accordance with the specifications listed in Table 317-A for the types and profiles of pipe.

317.05 Joining Systems

317.05.01 Flexible Subdrain Tubing – Couplings shall conform to the requirements of ASTM F667.

317.05.02 Flexible Culvert – Couplings shall conform to AASHTO M 252 or AASHTO M 294 Type C or CP.

317.05.03 Smooth Inner Wall Culvert or Storm Sewer – Joining system for open profile pipe shall conform to the requirements of CSA B182.8. Couplings shall be specified as non-gasketed corrugated external split couplings, or integral bell and gasket. Non-gasketed double bell snap

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P.E. PLASTIC DRAINAGE PIPE

type couplings are only permitted for 100 mm to 200 mm diameters.

Integral bell and gasket joint shall be comprised of an elastomeric vulcanized rubber gasket provided on the spigot end of the pipe residing in the corrugation valley or crest of the corrugation. Joint shall be certified by CSA to meet leakage requirements of ASTM D3212.

Foam type gaskets shall not be permitted.

Table 317-A: Polyethylene (P.E.) Plastic Drain Pipe & Fittings Specifications

Type	Profile	Specification (See Note 1)
Flexible Subdrain Tubing	Corrugated Inner Wall	ASTM F667.
Flexible Culvert	Corrugated Inner Wall	AASHTO M 252 or AASHTO M 294.
Smooth Inner Wall Culvert or Storm Sewer	Open or Closed	CSA B182.8 for profile pipe.

Note (1): Clean, rework PE material generated from the manufacturer's own production of pipe, fittings, or accessories shall be permitted to be recycled by the manufacturer in the respective product, provided that the pipe, fittings, or accessories produced meet all of the requirements of the respective standards. Post-consumer recycled plastic resin shall not be used as a component of the HDPE compound during pipe manufacture.

317.06 Requirements

317.06.01 Quality of Work – The pipe shall be homogenous throughout and free from visible cracks, flaws, foreign inclusions or other injurious defects. The pipe shall be uniform in colour, opacity and other physical properties

317.06.02 Pipe Diameter

(a) **Nominal Diameter** – The average inside diameter rounded to the nearest whole number (mm).

(b) **Inside Diameter** – The permissible inside diameter tolerances shall be +3% and -1.5%. Measurements shall be taken in accordance with ASTM D2122.

317.06.03 Length – Laying length for flexible culvert; smooth inner wall culvert; or storm sewer shall be 6 m for all diameters otherwise specified on the Purchase Order, or Work Order or Drawings. A tolerance of ± 25 mm on the nominal laying length will be permitted.

317.06.04 Pipe Stiffness – The minimum pipe stiffness shall be:

(a) **Flexible Subdrain Tubing** – 210 kPa at 5% when tested in accordance with ASTM D2412.

(b) **Flexible Culvert** – 210 kPa at 5% when tested in accordance with ASTM D2412.

(c) **Smooth Inner Wall Culvert or Storm Sewer** – 320 kPa at 5% when tested in accordance with ASTM D2412. Pipe stiffness shall be documented on the Plant Certificate by a CSA accredited body for the appropriate pipe diameters.

317.07 Markings – All pipes shall be clearly marked in accordance with CSA B182.8, at intervals of no more than 1.5 m with 5 mm or larger letters with the following information:

- Manufacturer's name or trademark
- Nominal diameter
- Material designation and cell class: 322420C
- The words "Drainage" or similar
- Pipe stiffness (i.e. 320 kPa)
- The applicable Specification designation (i.e. CSA B182.8)
- Date of manufacture and plant designation
- Logo of accepted CSA third party Certified Agency

For Smooth Inner Wall Culvert or Storm Sewer, the pipe shall be embossed with the CSA Trademark of accepted third party confirming the product is certified by the Canadian Standards Council (CSC).

317.08 Quality Assurance Testing

317.08.01 Supplier Quality Control – The supplier shall develop and maintain an effective Quality Control system to ensure that adequate inspection coverage is maintained throughout the manufacturing process. Evidence of such inspection shall be available to the authorized inspector of the Ministry prior to shipment from the place of manufacture or the supplier's storage facility.

317.08.02 Ministry Surveillance – The Ministry reserves the right to maintain surveillance over the Supplier's Quality Control system to ensure conformance with this specification.

317.09 Physical Properties – The physical properties of corrugated HDPE pipe such as impact strength, pipe flattening and other tests shall meet these requirements for:

317.09.01 Flexible Subdrain Tubing – ASTM F667

317.09.02 Flexible Culvert – AASHTO M 252 or AASHTO M 294

317.09.03 Smooth Inner Wall Culvert or Storm Sewer – Certified to CSA B182.8 for profile pipe.

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P.E. PLASTIC DRAINAGE PIPE

317.10 Certificate

317.10.01 Flexible Subdrain Tubing and Flexible Culvert – Suppliers shall provide a certificate upon request from the Ministry Representative to indicate that the product was produced and tested according to the appropriate specification requirements.

317.10.02 Smooth Inner Wall Culvert or Storm Sewer – The supplier shall provide, upon request from the Ministry Representative, a copy of the CSA certificate of compliance issued by a CSA accredited certification body confirming that the manufacturer produces certified polyethylene plastic pipe products and indicating plant locations, pipe sizes, pipe stiffness and pipe joining systems.

317.11 Inspection

317.11.01 Ministry Access – The Ministry Representative shall have access to the fabrication plant for inspection and every facility shall be extended for this purpose.

317.11.02 Inspection – Inspection may include checks on physical dimensions and general quality of work.

317.11.03 Sampling – For the purpose of sampling for SS 317.06 and SS 317.09, one piece from each lot of 50 pipes or fraction thereof in a shipment shall be selected for sampling. All test specimens shall be cut from the selected pipe(s).

317.11.04 Failed Sample – If the test sample fails to meet the requirements of SS 317.06 when tested in accordance with SS 317.09, that batch will be rejected and shall be replaced by the supplier with a new batch that conforms to this Specification at no additional cost to the Ministry.

317.11.05 Acceptance – Pipes shall be made available for inspection and acceptance at the point and time of incorporation into the Work.

SECTION 318

PVC PLASTIC DRAINAGE PIPE

DESCRIPTION

318.01 Scope – This Section covers the requirements and methods for testing for:

- Type PSM* Poly Vinyl Chloride Drainage Pipe
- Profile Poly Vinyl Chloride Drainage Pipe

and the perforation requirements of type PSM pipe up to a nominal diameter of 375 mm.

Note: The term PSM is not an abbreviation but rather an arbitrary designation for a product having certain outside dimensions.*

318.02 Applicable Documents – This Section refers to the following standards, specifications and publications.

Canadian Standards Association (CSA):

B182.2 PSM Type Polyvinylchloride (PVC) Sewer Pipe and Fittings

B182.4: Profile Polyvinylchloride (PVC) Sewer Pipe and Fittings

(The above are available only as part of the CSA B1800 Thermoplastic Non-Pressure Piping Compendium)

American Society for Testing Materials (ASTM):

D883: Standard Terminology Relating to Plastics

D1784: Standard Specification for Rigid Poly(Vinyl Chloride) (PVC) Compounds and Chlorinated Poly(Vinyl Chloride) (CPVC)

D2122: Standard Method of Determining Dimensions of Thermoplastic Pipe and Fittings

D2152: Standard Test Method for Adequacy of Fusion of Extruded Poly(Vinyl Chloride) (PVC) Pipe and Molded Fittings by Acetone Immersion

D2412: Standard Test Method for Determination of External Loading Characteristics of Plastic Pipe by Parallel-Plate Loading

D2444: Standard Test Practice for Determination of the Impact Resistance of Thermo-plastic Pipe and Fittings by Means of a Tup (Falling Weight)

D3034: Standard Specification for Type PSM Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings

D3212: Standard Specification for Joints for Drain and Sewer Plastic Pipes Using Flexible Elastomeric Seals

F412: Standard Terminology Relating to Plastic Piping Systems

F477: Standard Specification for Elastomeric Seals (Gaskets) for Joining Plastic Pipe

F794: Standard Specification for Poly(Vinyl Chloride) (PVC) Profile Gravity Sewer Pipe and Fittings Based on Controlled Inside Diameter.

MATERIALS

318.03 Materials – The pipe shall be made of PVC plastic having a cell classification of 12454, or 12364, as defined in ASTM D1784.

The fittings shall be made of PVC plastic and shall have a cell classification of 12454, or 13343, as defined in ASTM D1784.

Elastomeric gaskets shall comply with the requirements of ASTM F477.

318.04 Quality of Work – The pipe shall be homogenous throughout and free from visible cracks, flaws, foreign inclusion, or other injurious defects. The pipe shall be uniform in colour, opacity and other physical properties.

318.05 Joining System – All sizes of pipe shall be supplied with Elastomeric Gasket Joints providing a watertight seal in accordance with ASTM D3034 and ASTM D3212.

318.05.01 Gasket Compression – Integral Bell Gasketed Joints shall be designed so that when assembled, the gasket shall be compressed radially on the pipe spigot or in the bell to form a watertight seal.

318.05.02 Gasket Displacement – The joint shall be designed to avoid displacement of the gasket, when assembled in accordance with the manufacturer's recommendation.

318.05.02 Joint Assembly – Joint assembly shall be in accordance with the pipe manufacturer's recommendations.

Note: Manufacturers wishing to supply pipe joining systems other than those listed under this specification shall submit details to the Ministry Representative for approval prior to the awarding of the contract.

318.06 Requirements

318.06.01 Length – Laying length shall be 4 m or 6 m for all diameters unless otherwise specified by the Purchase Order, Work Order or Drawings. A tolerance of ± 25 mm on the nominal laying length will be permitted.

318.06.02 Type PSM, PVC Drainage Pipe Dimensions – The average outside diameter and the minimum wall thickness of Type PSM PVC drainage pipe shall be as specified in Table 318-A when measured in accordance with ASTM D2122.

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318.06.03 Profile PVC Drainage Pipe Dimensions – The average inside diameter and minimum wall thickness of Profile PVC drainage pipe shall be as specified in Table 318-B when measured in accordance with ASTM D2122.

318.06.04 Perforations – Perforated pipe up to the nominal diameter of 37.5 mm shall be type PSM and perforated in accordance with Figure 318-1 and Table 318-C.

The perforations shall be 8 mm to 10 mm in diameter, circular, and cleanly cut. The gasketed bell and spigot end of the pipe shall be unperforated for a length equal to the depth of the socket and/or shoulder.

Table 318-A: Pipe Dimensions – Type PSM

Outside Diameter (mm)					
Nominal Diameter (mm)	Average	Tolerance	Minimum	Maximum	Minimum Wall Thickness
100	107.06	± 0.22	106.84	107.28	3.06
150	159.38	± 0.28	159.10	159.66	4.55
200	213.36	± 0.30	213.06	213.66	6.10
250	266.70	± 0.38	266.32	267.08	7.62
300	317.50	± 0.46	317.04	317.96	9.07
375	388.62	± 0.58	388.04	389.20	11.10
450	475.00	± 0.70	474.30	475.70	13.57
525	560.00	± 0.85	559.15	560.85	16.00
600	630.00	± 0.95	629.05	630.95	18.00
675	710.00	± 1.05	708.95	711.05	20.29

Table 318-B: Pipe Dimensions – Profile

Inside Diameter (mm)						
Nominal Diameter (mm)	Average	Tolerance	Minimum	Maximum	Dual Wall	Minimum Waterway Wall Thickness
450	448.31	± 1.41	446.90	449.72	2.13	3.30
525	527.05	± 1.50	525.55	528.55	2.41	4.06
600	596.90	± 1.78	595.12	598.68	2.79	4.58
675	673.10	± 2.00	671.10	675.10	3.05	5.20
750	749.30	± 2.30	747.00	751.60	3.3	5.96
900	901.70	± 2.70	899.00	904.40	3.94	7.36
1000	1003.30	± 3.00	1000.30	1006.30	5.08	8.26
1200	1206.50	± 3.55	1202.95	1210.95	5.08	10.16

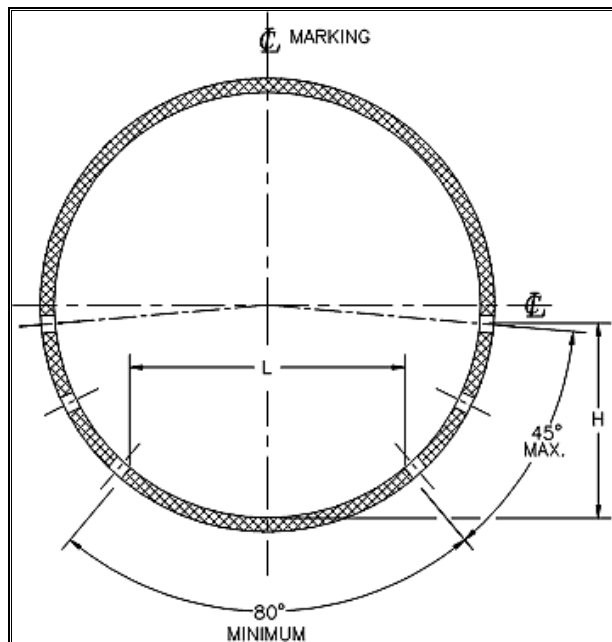
Table 318-C: Perforation Requirements

Nominal Diameter (mm)	Minimum Rows of Perforations	Horizontal Hole Spacing (mm)	H* (mm)*	L** (mm)
100	2	75 ± 5	30	65
150	4	75 ± 5	70	73
200	4	75 ± 5	93	130
250	4	75 ± 5	115	163
300	6	150 ± 5	140	195
375 - 1200	6	150 ± 5	175	240

*H: maximum height of the centreline of the uppermost perforations above the Invert.

**L: minimum length of the unperforated inner surface at the bottom of the pipe.

Figure 318-1: Perforated Pipe Cross Section



318.07 Service Connections (and or Branches)

318.07.01 Type PSM

- (a) Injection Molded PVC Gasketed Fittings meeting ASTM D3034 and certified to CSA B182.2.
- (b) Fabricated PVC Gasketed Fittings meeting ASTM D3034 and certified to CSA B182.2.
- (c) Insertable Tees complete with rubber boot.

318.07.02 Profile

- (a) Fabricated PVC Gasketed Fitting meeting ASTM F794 and Certified to CSA B182.4
- (b) Insertable Tees complete with rubber boot.

318.08 Physical Properties

318.08.01 Impact Strength – The minimum drop weight impact strength for pipe shall meet the requirements of Table 318-D when tested in accordance with SS 318.10.01.

318.08.02 Pipe Stiffness – The minimum pipe stiffness when tested in accordance with SS 318.10.02 and ASTM D2412 shall be 320 kPa.

318.08.03 Pipe Flattening – There shall be no evidence of splitting, cracking or breaking when the pipe is tested in accordance with SS 318.10.03.

318.08.04 Extrusion Quality – The pipe shall not flake or disintegrate when tested in accordance with ASTM D2152.

Table 318-D: Impact Strength at 0°C

Nominal Diameter (mm)	joules	ft. / lb.
100	135	100
150	165	122
200	175	129
250	190	140
300	205	151
375	230	170
450 and larger	250	184

318.09 Marking – All pipe shall be clearly marked at intervals of no more than 1.5 m with 5 mm or larger letters with the following information:

- Manufacturer's name or trademark
- Nominal diameter
- Material designation and cell class: PVC-12454 or 12364

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- The word "Drainage" or similar
- The class: PS320
- The applicable specification designation (ASTM D3034 or ASTM F794)
- Date of manufacture and plant designation

The marking on perforated pipe shall be 180° from a point equidistant between the bottom row of holes as shown in and Table 318-C.

318.10 Test Methods

318.10.01 Impact Resistance – Five specimens, each nominally 150 mm long for the PSM pipe and 300 mm long for the Profile pipe, shall be tested in accordance with ASTM D2444, using a 15 kg Tup "A" and the flat plate holder "B". The height of the drop shall not be less than 600 mm or more than 3 m. The test shall be performed at 0°C (± 0.5°C), allowing no more than 15 seconds to complete the test after removing the sample from the conditioning medium. Perforated samples shall be positioned as shown in .

Any cracking or other signs of material fatigue evident after the test shall constitute a failure.

If all five (5) specimens pass, the lot is acceptable.

If one specimen fails, five more specimens shall be tested.

If two or more specimens in the initial five (5) fail, or if any specimen in the second five (5) fails, the entire lot is rejected.

318.10.02 Pipe Stiffness – The pipe stiffness shall be determined at 5% deflection of the initial inside diameter in accordance with ASTM D2412. Three specimens, each with a length equal to one pipe diameter shall be tested; all shall meet the required stiffness. Perforated samples shall be positioned as shown in Figure 1.

318.10.03 Pipe Flattening – Three specimens, each nominally 150 mm long, shall be flattened between parallel plates in a suitable press until the distance between the plates is 40% of the original nominal diameter of the pipe. The loading rate shall be uniform and such that the

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compression is completed within two to five minutes. Remove the load and examine the specimen. All three specimens shall meet the requirements.

318.10.04 Pipe Dimensions – Measurements shall be taken in accordance with ASTM D2122.

318.11 Inspection

318.11.01 Access – The Ministry shall have access to the fabrication plant for inspection and every facility shall be extended for this purpose. For materials manufactured outside of British Columbia, SS 145.15.02 also applies.

318.11.02 Inspection – Inspection may include checks on physical dimensions and the quality of work.

318.11.03 Sampling – For the purpose of sampling for SS 318.08 and SS 318.10, one pipe from each lot of 50 pipes or fraction thereof in a shipment shall be selected for sampling. All test specimens shall be cut from the selected pipe(s).

318.11.04 Failed Sample – If a test sample fails to meet the requirements of SS 318.10 when tested according to SS 318.10, that batch will be rejected and shall be replaced by the Supplier with a new batch that conforms to this specification at no additional cost to the Ministry.

318.11.05 Acceptance – Pipes shall be made available for inspection and acceptance at the point and time of incorporation into the Work.

318.12 Quality Assurance

318.12.01 Supplier Quality Control – The Supplier shall develop and maintain an effective Quality Control system to ensure that adequate inspection coverage is maintained throughout the manufacturing process. Evidence of such inspection shall be available to the authorized inspector of the Ministry prior to shipment from the place of manufacture or from the Supplier's storage facility.

318.12.02 Ministry Surveillance – The Ministry reserves the right to maintain surveillance over the Supplier's Quality Control system to ensure conformance with this Specification.

SECTION 320

CORRUGATED STEEL PIPE

DESCRIPTION

320.01 General – This Section covers the material and fabrication requirements for corrugated steel pipe, spiral rib pipe, and structural plate corrugated steel pipe products for applications such as culverts, storm sewers, sanitary sewers, subdrains, ground recharge systems, well casings, underpasses, stream enclosures, shelters, and tunnels.

Abbreviations for the various types of steel pipe are as follows:

CSP Corrugated Steel Pipe means Galvanized, Aluminized Type II and Polymer Laminated corrugated steel pipe with helical corrugations, fabricated from coiled steel sheet, with continuous helical seam.

SPCSP Structural Plate Corrugated Steel Pipe means hot-rolled sheets or plates that are corrugated, curved to radius, custom hot-dip galvanized or has a thermoplastic copolymer coating, assembled, and bolted together to form pipes, pipe-arches, and other shapes. This includes DCSP Deep Corrugated Structural Plate (Type I, II, III).

320.02 Supply and Fabrication

320.02.01 Standards – The supply and fabrication of Corrugated Steel Pipe (CSP) and Structural Plate Corrugated Steel Pipe (SPCSP) including couplers and appurtenances shall be in accordance with CSA G401.

320.02.02 Plant Certification – All CSP and SPCSP shall be supplied from a manufacturing plant certified to CSA G401.

The certification shall be performed by a 3rd party agency accredited by the Standards Council of Canada. Certified CSP and SPCSP shall be marked according to CSA G401 markings, along with the logo of the 3rd party certification body.

The manufacturer's plant certificate documentation shall be made available to the Ministry upon request. This specification is available from CSA.

320.06 Quality of Work

320.06.01 Ministry Inspection – Products shall be made available for inspection and acceptance by the Ministry at the point and time of incorporation into the Work.

320.06.02 Quality Management – The manufacturer shall establish the Quality Control plan and demonstrate its implementation. The Contractor's Quality Management Plan shall incorporate Quality Assurance testing measures to assure the quality of the materials supplied by the manufacturer meet this Specification.

320.06.03 Contractor Quality Assurance Testing – Minimum Quality Assurance (QA) tests and inspection, to be performed by the Contractor, shall be per Table 320-A (next page).

Table 320-A: Contractor Quality Assurance Testing

Element	Description	Minimum Contractor Quality Assurance Testing / Inspection Frequencies	CSA G401 Clause No.
Manufacturer's QC	Demonstration of effectiveness	For each Designated Supplier (for over \$200,000): Once per Contract	7
Material	Mill Certificate	One (1) per Contract	4.2.3; 4.3.3; 9.3; 4.1.2
	Mill Markings	One (1) per Contract and one additional per load of material delivered	4.5.1.2.4; 4.5.2.2.4; 4.5.3.2.4 4.2.3, 4.3.3
Fabrication	Thickness	One (1) per Contract per product	CSP: Table 3 SPCSP: Table 4
	Corrugation profile	Visual each piece, plus measurements of one (1) per Contract and one (1) additional per load of material delivered	CSP: 5.1.1.2.1, 5.1.1.2.3, Figure 4 and Table 6 SPCSP: 5.2.2.2 and Figure 12
	Circumference	One (1) per Contract per product	CSP: Tables <u>6, 7, 10, 11</u>
	Lock seam strength	For Contracts with 10 or more pieces supplied: One (1) per Contract	5.1.3.4
	Perforation size	One (1) per shipment	5.1.5
Coating	Type	Visual, each pipe or plate	
	Thickness	For each material coating specified: One (1) per Contract and one additional per load of material delivered	4.5
	Staining or Damage	Visual, each piece	8.2
	Damage Repair	Each	6
Couplers	Type	Visual, each piece	5.1.7
SPCSP	Drawing and Plate Identification	Each installation	5.2.7

SECTION 321

TRAFFIC MARKING PAINT AND SPECIAL MARKINGS

DESCRIPTION

321.01 Scope – This Section applies to the supply and application of Traffic Marking Paint and Special Markings on concrete or asphalt roadway surfaces under the jurisdiction of the Ministry within the Province of British Columbia. Traffic Marking Paint (Paint) and Special Markings (Special Markings) shall be used with accepted overlay glass reflectorized beads.

321.02 Reference Specifications – The test methods, practices, and specifications listed in Table 321-A are applicable to the Work:

Table 321-A: Applicable Specifications

ASTM Test Designation	Title of Test
<u>D562</u>	<u>Standard Test Method for Consistency of Paints Measuring Krebs Unit (KU) Viscosity Using a Stormer-Type Viscometer</u>
<u>D711</u>	<u>Standard Test Method for No-Pick-Up Time of Traffic Paint</u>
<u>D868</u>	<u>Standard Practice for Determination of Degree of Bleeding of Traffic Paint</u>
<u>D913</u>	<u>Standard Practice for Evaluating Degree of Traffic Marking Line Wear</u>
<u>D969</u>	<u>Standard Test Method for Laboratory Determination of Degree of Bleeding of Traffic Paint</u>
<u>D4061</u>	<u>Standard Test Method for Retroreflectance of Horizontal Coatings</u>
<u>D6628</u>	<u>Standard Specification for Color of Pavement Marking Materials</u>
<u>D7585</u>	<u>Standard Practice for Evaluating Retroreflective Pavement Markings Using Portable Hand-Operated Instruments</u>
<u>E1710</u>	<u>Standard Test Method for Measurement of Retroreflective Pavement Marking Materials with CEN-Prescribed Geometry Using a Portable Retroreflectometer</u>
SAE Test Designation	Title of Test
<u>SAE AMS-STD-595A</u>	<u>Standard Colors Used in U.S. Government Procurement</u>

321.03 General Requirements – The composition and consistency of the Paint will be at the discretion of the manufacturer. Paint shall be supplied in white or yellow colours accepted by the Ministry without any "pre-mix" beads. Application of Paint shall include an overlay of glass reflectorized beads.

Special Markings shall have reflective characteristics to meet Ministry requirements. Paint and Special Markings products shall meet Environment Canada's Federal Regulations on VOC's limits.

Special Markings material and application requirements shall be identified in the Special Provisions.

Traffic Marking Paint shall be supplied from traffic paint products listed on the BC Ministry of Transportation and Infrastructure Recognized Products List, or as identified in the Special Provisions.

Special Marking Materials shall be supplied from the BC Ministry of Transportation and Infrastructure Recognized Products List or as identified in the Special Provisions.

The BC Ministry of Transportation and Infrastructure Recognized Product List is available on-line at:

http://www.th.gov.bc.ca/publications/eng_publications/geotech/Recognized_Products_Book.pdf

MATERIALS

321.04 Material Requirements – Paint and glass beads shall comply with the following:

321.04.01 Consistency – 85 ± 5 Krebs units in accordance with test method ASTM D562.

321.04.02 Drying Time – Maximum 30 minutes at 21°C at 50-60% relative humidity in accordance with test method ASTM D711.

321.04.03 Bleeding – The Paint shall have a degree of resistance to bleeding of seven or greater in accordance with test method ASTM D868 or ASTM D969.

321.04.04 Road Service Life – Paint shall have acceptable properties for general appearance, durability and night visibility. Acceptable properties are defined as being equal to or better than those obtained by Paints currently "Qualified or Accepted" by the Ministry through Ministry product testing or other jurisdiction testing acceptable by the Ministry Representative.

321.04.05 Night-time Retroreflectance – Paint shall meet retroreflectance as per ASTM D7585 with the exceptions as identified within this Specification and the Ministry’s Pavement Marking Contract. Retroreflectance shall be measured in accordance to ASTM E1710.

321.04.06 Application Characteristics

- (a) The degree of settling shall be such that the Paint can be readily re-mixed to a uniform consistency and there shall be no skinning. Paint shall not have marked thixotropic properties.
- (b) Paint shall be capable of being sufficiently atomized to produce a uniformly applied line 100 mm in width with absence of side splatter, overspray, or cobwebbing within the limits imposed by the application equipment used on the Contractor’s line striping machines.
- (c) Paint shall be capable of being successfully applied to at least 2 km of dash line without the necessity of making adjustments to any machine settings and without showing any evidence of distortion of the spray pattern or appreciable build up of paint in the spray gun tips.

321.04.07 Colour – In accordance to ASTM D6628:

- (a) White Paint shall comply with SAE AMS-STD-595A White 17886.
- (b) Yellow Paint shall comply with SAE AMS-STD-595A Yellow 33538
- (c) Yellow Paint shall conform to the a-b colour box for traffic Paint using the L*a*b colour coordinate system.

321.04.08 Retroreflectance – Retroreflectance shall be in accordance with ASTM D4061.

321.04.09 Glass Beads – Glass reflectorized beads shall be added during the spraying of Paint in accordance with the glass beads manufacturer’s recommended application requirements.

Glass beads shall be supplied from glass bead products listed on BC Ministry of Transportation and Infrastructure Recognized Products List.

321.05 Traffic Markings: Paint shall be applied in the manner as required under the Work requirements. Types of Paint lines shall be identified by the Ministry in accordance to the “Manual of Standard Traffic Signs and Pavement Markings,” Fig. 7.1. Ministry publications are available on-line at:

http://www.th.gov.bc.ca/publications/eng_publications/eng_pubs.htm#signing

Special Markings shall be laid out and identified in the Special Provisions.

PERFORMANCE SPECIFICATIONS

321.06 Performance Specifications

321.06.01 Longitudinal Pavement Markings – The Contractor shall:

- (a) Layout all, with no additional payment, Longitudinal Pavement Markings as identified on the Drawings or existing pavement markings in accordance with the Ministry’s “Manual of Standard Traffic Signs and Pavement Markings” or as instructed by the Ministry Representative.
- (b) Paint Longitudinal Pavement Markings on New Pavement Projects as follows:
 - (i) Apply an initial application of Longitudinal Pavement Markings at a wet thickness of not less than 10 mils; then after curing;
 - (ii) Apply a second application, with no additional payment, in accordance with the paint manufacturer’s suggested application interval. The wet thickness shall not be less than 15 mils regardless of product used; and
 - (iii) The wet thickness of all Longitudinal Pavement Markings will be applied uniformly.
- (c) Ensure all Longitudinal Pavement Markings have well defined edges and are free of tire tracking, with no splatter, excessive overspray or other defects;
- (d) Ensure Longitudinal Pavement Markings are straight, or of uniform curvature, and conform with the alignment;
- (e) Ensure the following dimensional criteria are met:
 - (i) Longitudinal Pavement Markings do not exceed a dimensional width of 110 mm for specified 100 mm wide line. No tolerance below 100 mm is allowed for the specified 100 mm line;
 - (ii) Longitudinal Pavement Markings do not exceed a dimensional width of 210 mm for specified 200 mm wide line. No tolerance below 200 mm is allowed for the specified 200 mm line;
 - (iii) The distance between the lines for Double Solid and Simultaneous Solid and Broken do not exceed a dimensional separation of 110 mm. No tolerance below 100 mm is allowed;
 - (iv) Longitudinal Pavement Marking Broken Lines do not exceed a maximum dimensional length deviation of ±100 mm for the specified length; and
 - (v) No spaces between Broken Lines exceed a maximum dimensional length deviation of ±100 mm from the specified length of space.
- (f) Apply glass beads immediately following the final Paint application. Glass beads are to be applied on all

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TRAFFIC MARKING PAINT AND SPECIAL MARKINGS

Longitudinal Pavement Markings at a uniform application rate as required by the glass bead Manufacture.

(g) Ensure:

- (i) Retroreflectance properties of not less than 200 millicandela·m⁻²·lux⁻¹ for White Paint under dry conditions are achieved on all Longitudinal Pavement Markings for at least 30 days from the time of application;
- (ii) Retroreflectance properties of not less than 150 millicandela·m⁻²·lux⁻¹ for Yellow Paint under dry conditions are achieved on all Longitudinal Pavement Markings for at least 30 days from the time of application;
- (iii) Retroreflectance properties of not less than 150 millicandela·m⁻²·lux⁻¹ for White Paint under dry conditions are achieved from time of application to October 15th of the calendar year in which the line was painted;
- (iv) Retroreflectance properties of not less than 100 millicandela·m⁻²·lux⁻¹ for Yellow Paint under dry conditions are achieved from time of application to October 15th of the calendar year in which the line was painted;
- (v) All durability testing is in accordance with ASTM D913;
- (vi) The condition of Longitudinal Pavement Markings meet the minimum of photographic reference standard No. 8 (Film 97 percent Intact) Chipping ASTM D913 from time of application to October 15th of the calendar year in which the line was painted. Longitudinal Pavement Markings that do not meet this criteria must be repainted immediately upon detection or as directed by the Ministry Representative; and
- (vii) The condition of Longitudinal Pavement Markings must meet the minimum of photographic reference standard of No. 4 (Film 77 percent Intact) Chipping ASTM D913 on January 15th of the subsequent year to which the line was painted.

321.06.02 Transverse, Chevron and Crosshatch Pavement Markings – The Contractor shall:

- (a) Repaint the Transverse, Chevron and Crosshatch Pavement Markings, with no additional payment, as identified on the Drawings or existing pavement markings to conform to the Ministry's "[Manual of Standard Traffic Signs and Pavement Markings](#)." The Ministry Representative shall be contacted about any existing Transverse, Chevron and Crosshatch Pavement Markings that do not conform to the Ministry's "[Manual of Standard Traffic Signs and Pavement Markings](#)" prior to proceeding with the repainting;

- (b) Layout and Paint all new Transverse, Chevron and Crosshatch Pavement Markings in accordance with the Ministry's "[Manual of Standard Traffic Signs and Pavement Markings](#)" or as instructed by the Ministry Representative;

- (c) Apply Paint to Transverse, Chevron and Crosshatch Pavement Markings at a wet thickness of not less than 16 mils. This may be done in several coats as long as underlying coats are cured prior to overcoating;

- (d) Ensure all Transverse, Chevron and Crosshatch Pavement Markings are free of tire tracking, with no splatter, excessive overspray or other defects;

- (e) Ensure the ends of all Chevrons and Crosshatching are within 50 mm of the centre of the intersecting longitudinal line;

- (f) Ensure all Transverse, Chevron and Crosshatch Pavement Markings have well defined edges and are free of horizontal fluctuations;

- (g) Apply glass beads immediately following the final Paint application. Glass beads are to be applied on all Transverse, Chevron and Crosshatch Pavement Markings at a uniform application rate as required by the glass bead manufacturer; and

(h) Ensure:

- (i) Retroreflectance properties of not less than 200 millicandela·m⁻²·lux⁻¹ for White Paint under dry conditions are achieved on all Transverse, Chevron and Crosshatch Pavement Markings for at least 30 days from the time of application;

- (ii) Retroreflectance properties of not less than 150 millicandela·m⁻²·lux⁻¹ for Yellow Paint under dry conditions are achieved on all Transverse, Chevron and Crosshatch Pavement Markings for at least 30 days from the time of application,

- (iii) All durability testing is in accordance with ASTM D913;

- (iv) The condition of Transverse, Chevron and Crosshatch Pavement Markings meet the minimum of photographic reference standard No. 8 (Film 97 per cent Intact) Chipping ASTM D913 from time of application to October 15th of the calendar year in which the marking was painted. Transverse, Chevron and Crosshatch Pavement Markings that do not meet these criteria must be repainted immediately upon detection or as directed by the Ministry Representative; and

- (v) The condition of Transverse, Chevron and Crosshatch Pavement Markings meet the minimum of photographic reference standard No. 4 (Film 77 percent Intact) Chipping ASTM D913 from time of application to June 15th

of the subsequent year in which the marking was painted.

321.07 Quality Assurance – This section establishes how the pavement markings will be accepted or not accepted for conformance to the Specifications.

Once per day that pavement markings are being laid, a minimum of two test sites are to be selected. The selected test sites must be representative of the line type sprayed on the pavement surface of that day (e.g., if 90% of the accomplishment of the day was centreline, then the expectation is that the test sites will be on centreline; if 50% of the accomplishment was on centreline and 50% was on lane line, then the expectation would be that there would be a test site on each line type).

321.07.01 Testing – The following tests must be performed and recorded per test site for that day:

- (a) **Paint Colour** – Testing for conformance shall be made by visual comparison to a standard colour card provided by the Paint manufacturer that certifies colour compliance with this specification.
- (b) **Dimensions** – Longitudinal Pavement Markings shall be measured and documented for conformance with the Pavement Marking Specifications.
- (c) **Night-time Retroreflectance** – Retroreflectance will be measured as per ASTM D7585 and ASTM E1710, notwithstanding the following exceptions contrary to ASTM D7585:
 - (i) Contrary to Section 6.2.1.4 and 6.2.2.4 of the ASTM D7585, checkpoint areas will be as per test sites;
 - (ii) Contrary to Section 6.2.1.4 and 6.2.2.4 of the ASTM D7585, all measurements made within a single checkpoint area may be averaged and recorded as an average. Any average of the readings that do not meet these Pavement Marking Specifications will be determined as unacceptable work; and,
 - (iii) Contrary to Section 5.1 of the ASTM D7585, testing for Retroreflectance will be carried out only when the newly painted surface is clean, dry, free of all excess beads, and after 24 hours of Paint being applied.
- (d) **Longitudinal Pavement Markings** – shall have a minimum initial coefficient of retroreflective luminance as indicated in these Pavement Marking Specifications.

321.07.02 Night-time Retroreflectance Measurements – All measurements for night-time retroreflectance will be made using calibrated equipment, capable of measuring retroreflectance in accordance with the applicable standards, as approved in writing by the Ministry. Measurements will be made using the retroreflectometer

manufacturer's instructions for operation and procedures and will be made only by competent staff.

Sampling will be made using a sample size of 20 test point measurements at 5 m intervals on a single line, tabulated as an average with standard deviation and percent relative standard deviation, and minimum/maximum values.

All measurements and related data will be retained by the Contractor in an electronic format approved by the Ministry Representative and will be submitted to the Ministry Representative on request.

321.07.03 Test Report – A test report shall be produced for each checkpoint area and will include the following information:

- (a) Test date;
- (b) Average of the readings at each test location, expressed as millicandelas per square metre per lux ($\text{mcd}\cdot\text{m}^{-2}\cdot\text{lx}^{-1}$); the average of the readings will be reported for each traffic direction for centrelines;
- (c) Geographical location of the test site, referenced by offsets from LKI/RFI landmarks;
- (d) Identification of the pavement marking material tested: type, colour, date painted, and transverse location on road (line type);
- (e) Identification of the instrument used;
- (f) Value and date of standardization of the instrument standard panel used;
- (g) Remarks concerning the overall condition of the line, such as rubber skid marks, carryover of asphalt, snow plow damage, and other factors that may affect the retroreflectance measurement;
- (h) Ambient temperature; and
- (i) Operator's name.

321.07.04 Thickness and Consistency – Longitudinal, Transverse, Chevron, and Crosshatch Pavement Markings shall have a sufficiently thick cross section throughout their entire length and width, and completely cover the intended area being marked. Pavement markings will be tested as per ASTM D913. Newly applied pavement markings must exceed the photographic reference standard of 97%.

321.07.05 Daytime visibility – When viewed dry or wet in the daytime, the pavement markings shall be readily visible for a forward distance of 150 m, or as far forward as possible until obstructed by the road geometry if less than 150 m.

Daytime visibility will be assessed visually. Where it is not clear that the specification is met, the distance will be measured.

321.07.06 Durability – Longitudinal, Transverse, Chevron, and Crosshatch Pavement Markings shall be visually

SECTION 321

TRAFFIC MARKING PAINT AND SPECIAL MARKINGS

assessed for conformance with this Specification using ASTM D913.

321.07.07 Test Site and Acceptance or Rejection – A test site is defined as a site where the above six tests have been performed in a single location. If two or more line types are applied per day that pavement markings are being laid, it may be necessary to perform relevant tests for each line type at more than one location.

A failure of any one of the six above components will constitute a failure for that site and will be considered unacceptable work.

321.07.08 Quality Control – All Quality Control records shall be retained by the Contractor and made available to the Ministry Representative upon request.

The Ministry may conduct tests as deemed necessary to identify the Paint supplied is the same as the Paint submitted for qualification.

PAYMENT

321.08 General – Payment for Traffic Marking Paint and Special Markings will be made at the Traffic Marking Pavement and Special Markings contract price or per line km as itemized in the Special Provisions Schedule 7, inclusive for supply and application of Traffic Marking Paint, layout of traffic lines, glass beads, and Special Markings, costs associated with quality control, material supply, application, related material manufacturer's requirements and costs for surface preparation.

SECTION 407

FOUNDATION EXCAVATION

407.01 Scope – This Section covers foundation excavation for structures including, but not limited to, excavation for abutments, piers, walls, and bridge end fills, as specified herein, and shall apply generally to all structures other than those where it distinctly states that the prices bid are inclusive of excavation.

407.02 Excavation – All materials shall be removed as necessary for the construction of foundations or other works. Foundation excavations shall not be larger than is reasonably necessary. Excavations and adjacent highways and other facilities shall be protected as necessary by barricades, shoring, dykes and/or cofferdams.

Excavations shall be constructed in compliance with the [Workers Compensation Act](#) and the [Occupational Health and Safety Regulation](#).

407.03 Description of Material Types – Material types will be as defined in SS 201.11.

No distinction shall be made between wet and dry excavation.

407.04 Preparation of Foundations – Care shall be taken not to disturb the bottom of the excavation. If the bottom of the excavation is disturbed in material other than rock, the Contractor shall remove and dispose of all disturbed material and shall replace it with material meeting the material, placement and compaction requirements of SS 201.40 Bridge End Fill. If the bottom of the excavation is disturbed in rock, the Contractor shall remove and dispose of all disturbed material and shall replace it with a concrete sub-footing, as directed by the Ministry Representative.

Where concrete is to be placed on rock, the rock surfaces shall be clean and free from any loose materials.

The bottom of the excavation shall be capable of providing a competent foundation and may require confirmation by the Ministry Representative prior to further works. Where, in the opinion of the Ministry Representative, the bottom of an excavation is not competent, the Ministry Representative may direct the Contractor to excavate deeper. The Ministry Representative may direct replacement of the incompetent material with material meeting the material, placement and compaction requirements of SS 201.40 Bridge End Fill or with a concrete sub-footing.

Unless underwater concreting is approved by the Ministry, excavations for concrete structures shall be dewatered, if necessary, so that concrete is placed in the dry.

407.05 Backfilling – After the structures are sufficiently built, excavations shall be backfilled to the original ground contours with excavated material, as directed by the Ministry Representative, unless this material is designated

as unsuitable. In the case where backfill is to be with Bridge End Fill, the provisions of SS 202.23 shall apply.

Drainage course material shall be installed as shown on the Drawings. The gradation of the drainage course materials shall be as specified in [Table 407-A](#).

Table 407-A: Drainage Course Material Gradation

SIEVE SIZE (mm)	% PASSING BY MASS OF TOTAL SAMPLE
40	100
20	0 – 100
10	0

407.06 Measurement and Payment – Payment for foundation excavation will be made at the Unit Price per cubic meter bid for the material types as identified in SS 407.03. Payment shall include excavation, shoring, barricades, backfilling to the original ground contours with suitable material, compaction of the material, compaction tests and [Quality Control](#). No payment will be made for removal and replacement of material disturbed by the Contractor below the required depth of excavation. Payment shall cover the supply and placement of gravel drainage courses, filter cloth and drain pipes, and joint waterproofing membranes if shown on the Drawings. If applicable, payment shall also cover restoration of roadway gravel and pavement damaged by the Contractor's operations. Where there is an excess of excavated materials or excavated materials are deemed unsuitable by the Ministry Representative, payment shall cover loading, hauling and disposal of such materials.

Excavated materials, which the Ministry Representative deems unsuitable for backfill, will be replaced with suitable material by the Contractor and paid for as Extra Work.

Excavation volumes for footings and box culverts shall be calculated based on the bottom of the excavations as shown on the Drawings or directed by the Ministry Representative and vertical planes 600 mm beyond the edges of the structures. No payment will be made for material removed outside of these boundaries.

No allowance shall be made for excavation of manholes or catch basins.

In the case of excavation for bridge end fill, payment will be made for material removed to the neat lines as shown on the Drawings and shall cover loading, hauling and disposal of such material.

SECTION 407

FOUNDATION EXCAVATION

Progress payments will be made as indicated in Table 407-B. Table 407-B: Progress Payment Breakdown

Table 407-B: Progress Payment Breakdown

Work	<u>Type A</u>	<u>Type D</u>
Excavation	80%	60%
Backfilling <u>and</u> Disposal	20%	40%

SECTION 412

CONCRETE REINFORCEMENT

DESCRIPTION

412.01 Scope – This Section describes the supply, fabrication and installation of reinforcing for concrete structures.

The requirements of CSA-S6, “Canadian Highway Bridge Design Code” and the Ministry “Bridge Standards and Procedure Manual – Supplement to CHBDC S6” shall apply unless specified otherwise herein or in the Contract.

For items not covered by CSA-S6, “Canadian Highway Bridge Design Code” and the Ministry “Bridge Standards and Procedure Manual – Supplement to CHBDC S6” or the Contract, CSA A23.1 shall apply.

References to standards shall be to the current editions at time of tendering.

MATERIALS

412.11 General – All reinforcing bars shall be deformed unless indicated otherwise on the Drawings.

The type and grade required shall be as specified on the Purchase Order, Work Order, Drawings or Special Provisions.

Unless specified otherwise, wire ties for reinforcing steel shall be minimum 1.6 mm diameter.

Two copies of mill certificates giving chemical and physical properties of the reinforcing steel shall be furnished to the Ministry Representative.

The identification of reinforcing bars shall be maintained throughout the fabrication, coating (if required) and shipping processes to the installation on the job.

The Contractor shall, as part of the Quality Control program, have random samples (selected by the Ministry Representative) of reinforcing steel, of all material types and surface treatments, tested for bending and tension. Two samples shall be taken of each material type, size and grade of bar used in the Work. Testing shall be in accordance with CSA G30.18 unless otherwise specified. Testing shall be performed by an approved testing agency.

412.11.01 Uncoated Reinforcing Steel – Concrete reinforcement shall conform to the requirements of the ASTM and CSA Standards shown in Table 412-A, as applicable.

Wire ties for uncoated reinforcing steel shall be annealed steel wire.

Table 412-A: Requirements for Uncoated Reinforcing Steel

<u>Test Designation</u>	<u>Standard Test Method / Procedure</u>
<u>ASTM A1064</u>	<u>Standard Specification for Carbon-Steel Wire and Welded Wire Reinforcement, Plain and Deformed, for Concrete</u>
<u>CSA G30.18</u>	<u>Carbon Steel Bars for Concrete Reinforcement</u>

412.11.02 Epoxy-Coated Reinforcing Steel – Epoxy-coated reinforcing steel bars shall conform to the requirements of ASTM A775 and D3963. Application plants shall be certified under the Concrete Reinforcing Steel Institute (CRSI) Voluntary Certification Program for Fusion-Bonded Epoxy-Coating Applicator Plants, and proof of certification shall be submitted to the Ministry Representative. All epoxy-coated bars shall be clearly labelled with the name of the manufacturer, the primer and the coating system used and the date of production. The method of labelling shall be determined by the applicator plants and test certificates of performance of epoxy coated bars shall be submitted to the Ministry Representative.

Further to paragraph 9.2 of ASTM D3963, the identification marks on all epoxy coated reinforcing bars shall be maintained from the point of delivery until they are incorporated into the structure.

The Ministry Representative may require samples of epoxy resin and epoxy patching material for acceptance testing as called for in ASTM A775. The Ministry Representative may require tests of coated bars as called for in ASTM A775.

Wire ties for epoxy coated reinforcing bars shall be epoxy coated .

412.11.03 Galvanized Reinforcing Steel – Hot dipped galvanized reinforcing bars shall conform to the requirements of ASTM A767M – Class II Coating.

Only CSA G30.18 Grade W bars shall be galvanized.

All bent bars shall be heat treated for stress relief prior to galvanizing. Stress relief shall be at a temperature from 480°C to 560°C for 1 hour per 25 mm of bar diameter as per ASTM A-767M.

Galvanized reinforcing bars shall not be placed in contact with uncoated reinforcing bars.

Wire ties for galvanized reinforcing steel shall be zinc coated or stainless steel.

412.11.04 Fibre Reinforced Polymer (FRP) – FRP bars, grids and tendons shall be manufactured and qualified in accordance with CSA S807.

The FRP supplier shall provide test data from an independent third party testing lab that demonstrates the material conforms to the specification.

Ties for FRP bar shall be plastic or nylon.

412.11.05 Low-carbon, Chromium, Steel Reinforcing Bar – Low carbon, chromium reinforcing bar and spirals shall conform to ASTM A1035, Type CS, grade 100 (690 MPa). Nominal dimensions, unit masses and deformation requirements for Imperial bar sizes shall conform to ASTM A1035.

Unless otherwise specified, only one type or grade of low-carbon, chromium, reinforcing steel shall be supplied for use throughout the project.

Wire ties for low carbon, chromium reinforcing bar shall meet the requirements for tie wire for stainless steel reinforcing bar.

412.11.06 Stainless Steel Reinforcing Bar– Stainless steel reinforcing bars and spirals shall conform to ASTM A276 and ASTM A955, minimum Grade 420, except as noted. Nominal dimensions, unit masses and deformation requirements for Imperial bar sizes shall conform to ASTM A955.

Stainless steel reinforcing bars shall be of a stainless steel type specified in Table 412-B.

Unless otherwise specified, only one type of stainless reinforcing steel shall be supplied for use throughout the Work.

Stainless steel reinforcing bars shall be hot-rolled, de-scaled and pickled to the required mechanical properties and dimensions.

Stainless steel reinforcing bars shall be free of deposits of iron and non-stainless steel.

Fabrication of stainless steel reinforcing bars shall be such that the bar surfaces are not contaminated with deposits of iron and non stainless steel and that the surfaces are not damaged due to straightening from coil.

Wire ties for stainless steel reinforcing bars shall be stainless steel wire of a type listed in Table 412-B except that tie wire used to tie stainless steel reinforcing bars to epoxy coated reinforcing steel bars shall be epoxy coated wire.

412.11.07 Order Lists and Bending Diagrams – All order lists and bending diagrams shall be submitted to the Ministry Representative for review before material is ordered.

The review of order lists and bending diagrams by the Ministry Representative shall not relieve the Contractor or Suppliers, as applicable, of responsibility for the correctness thereof.

Table 412-B: Types of Stainless Steel

Type	ASTM Designation	Unified Numbering System (UNS) Designation
316 LN	A955	S31653
DUPLEX 2205	A955	S31803
DUPLEX 2304	*	S32304

* Chemical composition shall be within the ranges identified in Table 1 (Typical Composition %) of and in conformance to the requirements of ASTM A276. The dimensioning and mechanical properties shall be in conformance with ASTM A955.

CONSTRUCTION

412.31 Bending – Reinforcement bars shall be cut and bent to the shapes shown on the Drawings. Bending shall be sufficiently accurate that the placing tolerances (specified in SS 412.33.01) can be met. All bars shall be bent cold, unless otherwise permitted. Bars partially embedded in concrete shall not be field bent except as shown on the Drawings or where specifically permitted.

Field bending of galvanized steel reinforcing bar and fibre reinforced polymer reinforcing bar is not allowed.

Field bending of stainless steel reinforcing bar is only allowed upon prior written approval from the Ministry's Representative. Stainless steel reinforcement shall only be bent using equipment specifically designed for that purpose.

412.31.01 Hooks and Bend Dimensions – Unless otherwise shown on the Drawings, hooks shall have the following dimensions:

- 180° bend plus extension of at least 4 bar diameters, but not less than 60 mm.
- 90° bend plus extension of at least 12 bar diameters.
- for stirrups and ties only, either a 90° or a 135° bend plus extension of at least 6 bar diameters at the free end of the bar.
- Minimum inside diameters of bends shall be as shown in Table 412-C. For stainless steel reinforcing bars #8 and smaller, the minimum bend diameters shall be six times the bar diameter. Minimum bend diameters for #9 through #12 stainless steel reinforcing shall be eight times the bar diameter. Minimum bend diameters for #13 and larger stainless steel reinforcing shall be ten times the bar diameter.

Exceptions:

- Minimum inside diameters of bends and 90° and 135° hooks for stirrups and ties shall be 4 bar diameters for

uncoated and stainless steel bars and 8 bar diameters for epoxy-coated bars.

- Minimum inside diameters of bends in welded wire fabric, plain or deformed, for stirrups and ties shall not be less than 4 wire diameters for deformed wire larger than 7 mm and two wire diameters for all other wires, except that bends with an inside diameter of less than 8 wire diameters shall be not less than 4 wire diameters from the nearest welded intersection.

Note: Galvanized reinforcing bars shall be pre-bent before galvanizing.

Table 412-C: Minimum Inside Diameters of Bends

Bar Size	Uncoated (mm)			Epoxy coated, FRP, and Galvanized (mm)	Stainless Steel* (mm)
	300R	400R or 500R	400W or 500W		
<u>10M</u>	<u>60</u>	<u>70</u>	<u>60</u>	<u>80</u>	<u>60</u>
<u>15M</u>	<u>90</u>	<u>100</u>	<u>90</u>	<u>120</u>	<u>90</u>
<u>20M</u>	<u>-</u>	<u>120</u>	<u>100</u>	<u>160</u>	<u>120</u>
<u>25M</u>	<u>-</u>	<u>150</u>	<u>150</u>	<u>200</u>	<u>150</u>
<u>30M</u>	<u>-</u>	<u>250</u>	<u>200</u>	<u>240</u>	<u>240</u>
<u>35M</u>	<u>-</u>	<u>300</u>	<u>250</u>	<u>350</u>	<u>280</u>
<u>45M</u>	<u>-</u>	<u>450</u>	<u>400</u>	<u>450</u>	<u>450</u>
<u>55M</u>	<u>-</u>	<u>600</u>	<u>550</u>	<u>550</u>	<u>550</u>

*for imperial stainless steel bar sizes see text in SS 412.31.01

412.32 Handling and Storage – Reinforcing bars shall be stored on platforms, skids or other suitable supports clear of the ground and shall be protected as practicable from mechanical injury and surface deterioration caused by exposure to conditions producing rust. When placed in the Work, reinforcement shall be free from dirt, loose rust or scale, mortar, paint, grease, oil, or other materials that would reduce bond.

412.32.01 Epoxy-Coated Reinforcing Steel – Epoxy-coated reinforcing bars shall be handled and stored so as to minimize damage to the coating. Equipment for handling the bars shall have protected contact areas. Nylon slings or padded wire rope slings shall be used. Suitable bundling bands shall be used to prevent damage. Bundles of coated bars shall be lifted at multiple pick-up points to prevent bar-to-bar abrasion from sags in the bundles of bars. The bars or bundles shall not be dropped or dragged.

Epoxy coated steel reinforcing bars shall be stored off the ground on protective cribbing, and timbers placed between

bundles when stacking is necessary. Supports shall be placed sufficiently close to prevent sags in the bundles.

Long term storage of epoxy-coated bars shall be minimized.

If circumstances require storing epoxy-coated steel reinforcement outdoors for more than two months, protective storage measures shall be implemented to protect the material from sunlight, salt spray and weather exposure. Epoxy-coated steel reinforcing bars stored in corrosive environments will require protection sooner. Epoxy-coated steel reinforcing bars or bundles shall be covered with opaque polyethylene sheeting or other suitable opaque protective material.

Epoxy-coated bars shall not be cut or bent on Site without the permission of the Ministry Representative. In any case, the flame-cutting of coated bars will not be permitted.

When immersion-type vibrators are used to consolidate concrete around epoxy-coated steel reinforcing bars, the vibrators shall be equipped with rubber or non-metallic vibrator heads.

Any bar with coating damage exceeding 2% of the surface area of the coated steel reinforcing bar in any 300 mm length, shall be rejected. When the extent of the damage does not exceed 2% of the surface area in any 300 mm length, all damaged coating shall be repaired with patching material in accordance with ASTM A775, Section 12. Patching material shall be applied in strict accordance with the manufacturer's written instructions. Prior to application of the patching material, rust shall be removed from the damaged areas. The patching material shall be allowed to cure before placing concrete over the epoxy-coated steel reinforcing bars.

Compliance with the "Guidelines for Jobsite Practices", contained as an Appendix of ASTM A775 shall be mandatory.

Where on-site inspection or testing by the Ministry Representative demonstrates non-compliant coating thickness or integrity, for any shipment of bars, such bars may be rejected, and in any case shall not be placed in the structure until the Ministry Representative is satisfied as to their acceptability.

The Contractor shall make available to the Ministry Representative, for inspection and acceptance, all repairs performed to protective coatings on any reinforcing steel. Repairs of epoxy coatings shall not be performed when the epoxy-coated bar or ambient air temperature is 5°C or less, or when moisture is present on the bar. Patching material shall not be applied if precipitation is expected within four hours of the procedure. If precipitation does occur within four hours, the Ministry Representative shall determine as to whether the material as applied is acceptable. If the placed epoxy-coated bars have been exposed to salt spray or road salts, the bars shall be rinsed with fresh water to remove any chloride contamination prior to placing concrete.

412.32.02 Stainless Steel Reinforcing Bar – Stainless steel reinforcing bars shall be stored clear of the ground on timbers or other suitable protective cribbing spaced to prevent sags in the bundles. Stacks of bundles of straight bars shall have adequate blocking to prevent contact between the layers of bundles. Stainless steel reinforcing bars shall be stored separately from black reinforcing steel bars, with the bar tags maintained and clearly visible until ready for placing.

412.33 Placing and Fastening – Before any concrete is placed, the placing and securing of reinforcing steel including dowels, within the area of concrete placement shall be complete. Tying in place of all dowels projecting from the area of concrete placement shall be subject to acceptance by the Ministry Representative before any concrete is placed. The reinforcing steel shall be free from dirt, detrimental rust, loose scale, paint, oil or other foreign material. Steel reinforcement - other than stainless steel reinforcing bars - with rust, mill scale, or a combination of both shall be acceptable, provided the minimum physical properties including height of deformations and mass of a wire brushed test specimen are not less than the applicable specification requirements.

Reinforcement shall be placed in the positions shown on the Drawings, within the tolerances specified below, adequately supported and secured against displacement. All splices of adjacent bars shall be securely tied together. Tying 100% of the bar intersections is required where the bar spacing centres are 300 mm or greater. Tying of 50% of the bar intersections is required when the bar spacing centres are less than 300 mm.

The locations of the top reinforcing steel in bridge decks shall be checked by running a full deck-width template along the longitudinal screeds. The lower edge of the template shall be set at the cover dimension (as specified or noted in the Drawings) plus any anticipated deflection of the screed rail support system below the level of the deck surface. No steel shall touch the template nor be more than 6 mm distant from it. Any discrepancies outside of these limits shall be corrected by the Contractor to the satisfaction of the Ministry Representative.

The location of reinforcing steel near deck joint anchors shall be adjusted so that there will be no interference with the deck joint anchors.

Tack welding of reinforcement for cage assembly or securing of reinforcement will be permitted only in the following circumstances:

- (a) on bars provided by the Contractor that are additional to design requirements, except that no additional shear stirrups or tensile reinforcement bars will be permitted, as they may adversely affect structural performance.
- (b) at the ends of bars where the Contractor provides additional length that is in excess of design requirements and this additional length is detailed on bar bend sheets or shop drawings, with notes indicating

the purpose of the additional length. Such bar bend sheets or shop drawings shall have written acceptance by the Ministry Representative prior to welding taking place.

- (c) with the written acceptance of the Ministry Representative, in which case, welding shall be in accordance with SS 412.34.02.

412.33.01 Tolerances – Tolerances for placing reinforcement shall, unless otherwise specified, be as shown in Table 412-D

Table 412-D: Tolerances for Placing Reinforcement

<u>Element</u>	<u>Tolerance</u>
Concrete cover to top reinforcing steel in bridge decks	+ 6 mm – 0 mm
Concrete cover to reinforcement, other than top reinforcing steel in bridge decks	± 8 mm
Bar location, except cover, when depth of a flexural member, thickness of a wall or smallest dimension of a column is 200 mm or less	± 8 mm
Bar location, except cover, when depth of a flexural member, thickness of a wall or smallest dimension of a column is larger than 200 mm but less than 600 mm	± 12 mm
Bar location, except cover, when depth of a flexural member, thickness of a wall or smallest dimension of a column is 600 mm or larger	± 20 mm
Longitudinal location, except cover, of bends and ends of bars	± 50 mm
Longitudinal location, except cover, of bends and ends of bars at discontinuous ends of members	± 20 mm

412.33.02 Bar Supports and Spacers – Bar supports and spacers shall be adequate to ensure concrete cover and bar spacings are maintained within the specified tolerances.

Bar supports and spacers shall be sufficient in number and strength to support the reinforcement and prevent displacement by workers or equipment before and during concreting and shall be adequately spaced to ensure that any sagging between supports does not intrude on the specified concrete cover.

Bar supports and spacers shall be of a type and material that will not cause rust spots, blemishes or spalling of concrete surfaces.

Bar supports and spacers shall be precast concrete, plastic, hot-dip galvanized steel, epoxy-coated steel or stainless steel, except that, plastic supports or plastic spacers over 200 mm in height shall not be allowed.

Precast concrete supports shall have a compressive strength and quality not less than that of the concrete in which they are embedded. For finished surfaces, the face of the support in contact with the forms shall not exceed 50 mm in any dimension and shall have a colour and texture to match that of the finished concrete surface.

Bar supports and spacers for approach slabs and formed horizontal slabs such as bridge decks, bridge sidewalks, and top slabs of culverts shall meet the following additional requirements:

- (a) Supports or spacers up to 200 mm in height shall be either precast concrete or plastic.
- (b) Plastic supports or spacers shall have an individual minimum breaking strength of 4.5 kN (1000 lbs) and shall be capable of maintaining strength and dimensional properties for the range of temperatures encountered on Site.
- (c) Maximum spacing between supports, for each mat of bars, shall be 1200 mm, except that the maximum spacing between plastic supports, for each mat of bars, shall be 1000 mm.
- (d) Each support shall carry the load from not more than one mat of reinforcing steel, except as provided below:
 - (i) Where a support is used that is specifically designed by the manufacturer to carry two mats at two separate positions, or
 - (ii) Where the top mat of reinforcing steel is more than 200 mm above the slab soffit, the top mat may be supported by reinforcing steel spreader bars, matching the material of the top mat. Spreader bars shall be secured between the top and bottom reinforcing mats and the bottom reinforcing mat shall be supported on precast concrete chairs adequately spaced to carry the additional weight of the top mat of reinforcing steel.

412.33.03 Concrete Cover – Concrete cover for reinforcing steel shall be in accordance with the Ministry “Bridge Standards and Procedure Manual – Supplement to CHBDC S6”.

412.33.04 Spirals – Spirals shall be held in place and to line by vertical spacers.

For spiral rods less than 15M, spacing shall be as follows:

- two spacers per loop for spirals less than 500 mm in diameter.
- three spacers per loop for spirals 500 mm to 800 mm in diameter.

- four spacers per loop for spirals over 800 mm in diameter.

For spiral rods 15M and larger, spacing shall be as follows:

- three spacers per loop for spirals up to 600 mm in diameter.
- four spacers per loop for spirals over 600 mm in diameter.

If vertical reinforcements are to serve as spacers, spirals shall be securely tied to vertical reinforcement at the spacing required for spacers.

Stainless spirals shall be provided with stainless steel spacers in conformance with ASTM A276.

412.34 Splicing of Bars – Special requirements for splicing, such as particular locations for splices, use of overlength bars or special lap lengths, shall be as shown on the Drawings.

Splices in bars larger than 35M shall be mechanical coupler splices or welded. The detail of such splices shall be subject to acceptance by the Ministry Representative.

412.34.01 Lapped Splices

- (a) Horizontal bars shall not be spliced unless greater than standard mill lengths – 12 m for 10M bars and 18 m for larger bars – would otherwise be required.
- (b) Splices to be staggered so that no more than one third of the reinforcing steel in a member shall be spliced in any transverse section that is within the required lap length.
- (c) If not shown on the Drawings, the length of lapped splices shall be in accordance with CSA-S6, Clause 8.15.10.1.
- (d) Adjacent reinforcing bars shall not be spliced at the same locations unless shown on the Drawings or authorized by the Ministry Representative.
- (e) Column spirals shall be spliced in accordance with CSA-S6, Clause 8.14.4.2.
- (f) Lap splice bars shall be of the same material as the bars being connected.

412.34.02 Welded Splices – Welded splices shall be used only if detailed on the Drawings. Welding shall conform to the requirements of CSA W186, and shall be performed by a company certified by the Canadian Welding Bureau to the requirements of CSA Standard W186.

Welded splices shall not be used on epoxy coated bars or stainless steel bars, with the exception of stainless steel spirals.

412.34.03 Mechanical Coupler Splices – Mechanical couplers shall be used for splices only if pre-approved or detailed on the Drawings or authorized in writing by the Ministry Representative. Such couplers shall develop in tension or compression, as required, at least 120% of the

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specified yield strength of the bars, but not less than 110% of the mean yield strength, representative of the bars to be used, in the test of the mechanical connection.

Wedge couplers shall not be used in bars greater than 15 mm diameter. When mechanical couplers are used, the Contractor shall ensure that the minimum concrete cover as specified is maintained. Where mechanical couplers are to be used to couple stainless steel reinforcing bars, the mechanical coupler shall be stainless steel of a type specified in [Table 412-B](#).

The total slip of the reinforcing bars within the splice sleeve of the connector after loading in tension to $0.5f_y$ and relaxing it to $0.05f_y$ shall not exceed the following measured displacements between gauge points straddling the splice sleeve:

- Up to and including 45M bars 0.25 mm
- 55M bars 0.75 mm

MEASUREMENT

412.81 General – When required, reinforcing steel will be measured by KILOGRAMS for the total mass in each part of the structure(s) as computed using [Table 412-E](#). The mass of clips, separators, tie wires, chairs and other fastening devices, and epoxy or other coating shall not be added into the calculations of mass of reinforcing steel.

PAYMENT

412.91 General – Payment for REINFORCING STEEL will be made at the Contract Lump Sum Prices for reinforcing steel in each part of the structure(s), for the supply, cutting, bending, coating (if required), shipping, placing and securing of reinforcing steel. Payment shall also include all costs associated with [Quality Control](#). Payment for increases or reductions in quantities of uncoated and coated reinforcing steel ordered by the

Ministry Representative will be made at the following Unit Prices per kilogram.

- Uncoated reinforcing steel \$3.00
- Epoxy-coated reinforcing steel \$4.00
- Galvanized reinforcing steel \$5.00
- Stainless reinforcing steel \$8.00

Table 412-E: Nominal Linear Mass for Reinforcing Steel

Bar Designation	Mass (kg/m)
10M	0.785
15M	1.570
20M	2.355
25M	3.925
30M	5.495
35M	7.850
45M	11.775
55M	19.625
#3 (stainless steel)	0.556
#4 (stainless steel)	1.011
#5 (stainless steel)	1.559
#6 (stainless steel)	2.225
#7 (stainless steel)	3.032
#8 (stainless steel)	3.995
#9 (stainless steel)	5.053
#10 (stainless steel)	6.416
#11 (stainless steel)	7.880

SECTION 413

BRIDGE DECKS AND CONCRETE OVERLAYS

DESCRIPTION

413.01 General – This Section describes requirements for the construction of Bridge Decks, and for the construction of Concrete Overlays on previously constructed (old) bridge decks and on newly constructed (new) bridge decks. The requirements of SS 211 shall apply except as otherwise specified in this Section. Unless otherwise specified in this Section, bridge deck concrete shall include diaphragms, deck slabs, bridge parapets and medians, bridge sidewalks, approach slabs and overlay concrete.

413.31 Bridge Decks

413.31.01 General – Deck slab construction shall be full-width and continuous between the joints as shown on the Drawings, and shall be placed in the sequence as shown on the Drawings. Any variation from this requirement shall be subject to acceptance by the Ministry Representative.

(a) **Camber Survey** – Prior to commencing deck construction and subsequent to the completion of the erection of girders, with the girders in a thermally neutral condition and with no dead load other than the dead load of the girders and necessary fall protection, the Contractor shall profile all the girders. Camber elevations are to be taken at 1000 mm centres maximum (or as indicated on the Drawings or Special Provisions), centrelines of all bearings and ends of girders. The camber elevation locations are to be laid out to an accuracy of 25 mm horizontally and camber elevations to 1 mm vertically. Within 14 days of the Contractor supplying the camber elevations to the Ministry Representative, the Contractor will receive haunch heights for setting the deck slab soffit, (for suspended slab type structures only) and the design finished grade elevations for the deck, complete with anticipated dead load deflections at 1000 mm stations along the centreline of roadway, to be used by the Contractor in establishing the required screed rail settings.

In the event that actual girder camber values vary significantly from the design values, the Contractor will be required to modify the original design finished grade as directed by the Ministry Representative.

(b) **Method, Equipment & Personnel** – The Contractor shall submit to the Ministry Representative for review, four weeks prior to the scheduled start of deck slab concrete placement, a Deck Placement Plan which provides a complete description of the method, equipment, material and personnel, including previous deck placement experience of the personnel to be employed. This shall include the equipment for mixing, transporting, placing, vibrating and curing the

concrete and shall also include details of the finishing equipment and support rails. The submittal shall clearly demonstrate that the proposed methods, equipment and personnel have the capability to carry out the Work in accordance with SS 413. All review concerns raised shall be addressed to the satisfaction of the Ministry Representative. All equipment shall be subject to inspection and acceptance by the Ministry Representative. Equipment may be rejected during operation if it is deemed unsatisfactory.

Two weeks prior to the scheduled start of deck slab concrete placement, the Contractor shall meet with the concrete supplier and the Ministry Representative to review deck concrete operations with respect to concrete mix design and performance, concrete delivery, placing, finishing and curing equipment and methods.

(c) **Haunch Setting, Screed Rails & Reinforcing Cover** – The Contractor shall set the deck slab soffit formwork to within 2 mm of the haunch heights as determined in accordance with SS 413.31.01(a).

After installation of the deck slab reinforcing, the Contractor shall set the screed rails for the deck finishing machine. Screed rails shall be installed to an accuracy of ± 2 mm and have a maximum deflection between supports of 1 mm. Allowance for dead load deflection of the structure shall be made in the screed rail setting as well as any anticipated deflection due to the loading of any formwork that may be supporting the screed rail system. The screed rails shall be adequately supported outside the area to be concreted and shall be completely installed before concrete is placed, unless otherwise accepted by the Ministry Representative.

Prior to the placement of deck slab concrete and subsequent to the installation of deck slab reinforcing and screed rails, the locations of the top reinforcing steel in bridge decks shall be checked by running a full width deck template along the longitudinal screeds. The lower edge of the template shall be set at the cover dimension (as specified or noted in the Drawings) plus any anticipated deflection of the screed rail support system below the level of the deck surface. No steel shall touch the template nor be more than 6 mm distant from it. Any discrepancies outside of these limits shall be corrected by the Contractor to the satisfaction of the Ministry Representative.

413.31.02 Concrete Placement

(a) **Concrete Temperature at Placement** – Concrete designed for bridge decks and overlay including any bridge deck concrete containing silica fume shall not

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have a temperature in excess of 17°C at time of placement. Concrete used in elements having a minimum section thickness greater than 1000 mm shall have temperature controls as per CSA A23.1 Clause 5.2.5.4. Methods used for maintaining temperature controls shall not diminish the quality of the concrete, and shall not exceed the specified W/C_m ratio.

When the atmospheric temperature is 15°C or higher, SS 211.18 – Hot Weather Concreting, shall apply except as modified herein.

Concrete for bridge decks and overlays shall not have a temperature greater than 17°C when placed. Prior to the placement of bridge deck and overlay concrete, when the ambient air temperature is at or above 15°C, or when there is a probability of its rising to 20°C or above during the placement period (as forecast by the nearest official meteorological office), the Contractor shall submit to the Ministry Representative for acceptance, the proposed placing operations complying with the requirements for hot weather concreting.

When the ambient air temperature is at or above 15°C, or when there is a probability of its rising to 20°C or above during the placement period (as forecast by the nearest official meteorological office), bridge decks and overlays shall be cast when daily air temperatures are forecast to be a minimum (normally between 12:00 midnight and 9:00 AM).

- (b) **Time of Placement** – All bridge deck and overlay concrete shall be fully discharged and placed in its final location within 45 minutes after the water and cement have been combined. Under conditions contributing to rapid stiffening of concrete, the Contractor shall adjust its placing operations to suit reduced handling time.

Time of placement extensions will be considered on a case-by-case basis and may require the use of Hydration Stabilizing Admixtures (HSAs) conforming to the requirements of ASTM C494 Type B, Retarding or Type D, Water-Reducing and Retarding Admixtures. If accepted by the Ministry Representative, guidelines for the use of HSAs shall be as follows:

- (i) HSA modified concrete for bridge decks shall be fully discharged and placed within 90 minutes after water and cement have been combined.

(A) When HSA's are used, these time extensions are subject to preconstruction trials being conducted by the Contractor, to establish the appropriate HSA dosage to provide suitable extended slump life of concrete, without increasing the water/cementitious ratio of the concrete above that which would be required if HSA's were not used.

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(B) The use of HSA's shall in no instance modify the maximum concrete temperature required at time of placement.

- (ii) A one-time only addition of HSA will be allowed. This will be during initial batching of the concrete or immediately at completion of batching as recommended by the admixture manufacturer. Addition of HSA at any other time will be cause for rejection of the concrete.

- (c) **Concrete Placement-Diaphragms** – Unless otherwise specified, diaphragms shall be placed either:

- (i) not more than two hours before the adjacent deck concrete is placed. In this case, diaphragm concrete shall have a slump less than 40 mm and shall be retarded so that it sets at the same time as the deck concrete, or
- (ii) a sufficient length of time ahead of the deck concrete, so that the diaphragm concrete attains a compressive strength of 15 MPa before the deck slab is cast.

Sequence for the placing of concrete for diaphragms at ends of girders on continuous and integral type spans shall be as shown on the Drawings.

- (d) **Concrete Placement - Deck Slab** – The top surfaces of concrete stringers and deck panels, if applicable, shall be prepared in accordance with SS 211.12.03 and the surfaces shall be in a saturated surface dry condition immediately before the deck is cast.

Concrete shall be placed at a rate of not less than 6 m/hr. The concrete front and finishing operation shall be kept parallel to the substructure. Placing shall be started at such time as to permit finishing during daylight hours, except if ambient temperatures are as outlined in SS 413.31.02(a), then adequate lighting is to be provided in accordance with [Workers Compensation Act and the Occupational Health and Safety Regulation](#), to allow for concrete operations under reduced light conditions.

In the event of unexpected rain, concrete placing shall cease and the surface shall be protected immediately. Surface concrete which has been rained on shall immediately be removed to the top of reinforcing steel.

- (e) **Finishing** – The deck surface shall be finished by a drum-type deck finishing machine on support rails. Acceptable finishing machines are Terex Bid-Well models 2450, 3600 or 4800, Gomaco models C-450, C-750, Allen Models 4836B, 6036 B 6048 B or alternate acceptable to the Ministry Representative.

There shall be no disturbance of the concrete after it has been consolidated and finished by the deck finishing machine. Two work bridges shall be provided to carry out the final deck finishing operations and for preparations to cure the concrete deck surface. No

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inserts will be permitted in the finished concrete roadway surface.

The surface behind the finisher shall be floated transversely to eliminate ridges and open texture. Areas, which cannot be machine-finished, shall be hand screeded and floated.

Prior to final surface finishing or texturing, the deck surfaces shall be repeatedly checked for accuracy of deck slab thickness and alignment. The Contractor shall measure and record the deck slab thickness at a frequency of at least one measurement per interior bay and overhang bay, at a maximum of 3000 mm centers longitudinally. Adjustments as acceptable to the Ministry Representative shall be made to the deck-finishing machine to maintain the minimum deck slab thickness and the required concrete cover on the deck reinforcing. The thickness readings of the deck slabs shall be supplied to the Ministry Representative at the conclusion of each placement section.

For bridge decks that will not have a waterproofing membrane, the final surface texturing shall be tined. The tining shall create transverse grooves 3 mm wide by 1.5 mm to 3 mm deep at 20 mm centre-to-centre spacing.

For bridge decks that will have a waterproofing membrane, the final concrete surface shall be floated and troweled as necessary to provide a smooth surface.

413.31.03 Repairs – Areas of the deck not meeting the tolerances as given in SS 211.16, shall be corrected as follows:

- (a) where the required alignment can be obtained with a maximum cut of 6 mm, the bumps shall be cut with a bump cutter which will provide an acceptable longitudinal grooved texture;
- (b) where the required alignment cannot be obtained with a maximum cut of 6 mm, the concrete shall be broken out with vertical edges to a minimum depth of 40 mm and shall be replaced with new concrete of the required quality.

Any physical damage to the deck surface shall be cut out and replaced. Any areas of open texture or plucked aggregates shall be repaired to the satisfaction of the Ministry Representative.

413.31.04 Interim Strength Requirements – Deck concrete shall attain strength of 15 MPa before parapets are placed and 25 MPa before heavy loads, such as concrete trucks are allowed on the bridge.

413.31.05 Curing

- (a) **General** – Freshly deposited concrete shall be protected from freezing, abnormally high temperatures or temperature differentials, premature drying, excessive moisture, moisture loss, heavy shocks,

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excessive vibrations and high stresses, for the period of time necessary to develop the required concrete properties.

- (i) The Contractor shall submit a detailed Deck Curing Plan for curing the deck and overlay concrete. The plan shall contain details such as water source, the distribution system, the collection and run-off control system, person(s) responsible for Quality Control, and staff to maintain the system. The curing system shall be capable of maintaining a free film of water on the areas of concrete requiring curing, for a minimum of 8 unattended hours and this capability shall be demonstrated prior to concrete placement. It shall be the responsibility of the Contractor to ensure that the system of curing and protection is properly planned, constructed and maintained throughout the entire curing period.
- (ii) Burlap shall be pre-soaked by immersing it in water for a period of at least 24 hours immediately prior to placing.
- (iii) Two layers of burlap shall be applied immediately after finishing of the concrete surface within 2 to 4 m of the final surface texturing or finishing operations (no more than 20 minutes) and shall be maintained in a saturated condition in a manner, which does not damage the finished concrete surface. Strips must overlap 150 mm and must be held in place without marring the surface of the concrete. The burlap shall be maintained in a continuously wet condition throughout the curing period and a continuous film of free water on the surfaces of the areas noted herein shall be maintained by means of soaker hoses, sprinklers, etc.
- (iv) Concrete for diaphragms, monolithic decks, concrete overlays, approach slabs, parapets, and sidewalks shall be cured for a minimum of seven consecutive uninterrupted twenty-four hour periods at a minimum temperature of 10°C and for the time necessary to attain 80% of the specified compressive strength of the concrete. The compressive strength tests for verifying completion of the curing period shall be determined using field cured test cylinders cured in accordance with Clause 9.3.2 of CSA A23.2-3C. The curing period will begin following the completed placement of the concrete in the section cast.
- (v) The temperature of the concrete being cured must be monitored to ensure maximum and minimum temperature requirements in accordance with SS 211.15.02 are met.
- (vi) Regardless of ambient temperature, moist curing with burlap and water must be provided at all

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times. During cold weather, burlap must be prevented from freezing.

- (vii) At completion of the curing period, the curing shall be discontinued in a manner so as not to subject the concrete to undue stresses due to rapid moisture loss in the concrete.
- (viii) When the air temperature is at or below 5°C or when there is a probability of it falling below 5°C within the next 5 days after the curing period, then the following shall apply:

- (A) At completion of the curing period, the addition of free water is to be discontinued and followed by an additional five (5) consecutive uninterrupted twenty-four hour periods of drying with the concrete temperature maintained at a minimum temperature of 5°C.

- (b) **Additional Curing Requirements for Concrete Containing Silica Fume or Fly Ash, With or Without Fibres** – A fog mist shall be applied continuously from the time of screeding until the concrete is covered with burlap, in such a way as to maintain high relative humidity above the concrete and prevent drying of the concrete surface. Water must not be allowed to drip, flow, or puddle on the concrete surface during fog misting, when placing the burlap, or at any time before the concrete has achieved final set.

Fogging equipment will consist of pressure washers (minimum output pressure of 28 MPa (4000 psi)) with suitable tips for producing a fine water mist or fog. A minimum of two pressure washers shall be used for this process and the adequacy of the range and coverage shall be demonstrated prior to placing any concrete.

When the air temperature is at or below 5°C or when there is a probability of it falling below 5°C within the next ten (10) days after the curing period, then the following shall apply:

- (i) At completion of the curing period, the addition of free water is to be discontinued and followed by additional ten (10) consecutive uninterrupted twenty-four hour periods of drying with the concrete temperature maintained at a minimum temperature of 10°C.

Modifications to the curing process may be required if it is anticipated that the deck may be subjected to the application of de-icing chemicals within the first month after the curing period.

413.31.06 Payment – for bridge deck FORMWORK and CONCRETE will be made as outlined in SS 211.21.01 and SS 211.21.02 respectively and payment for bridge deck REINFORCING STEEL will be made as outlined in SS 412.91 unless otherwise provided for in SS 413.

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413.32 Concrete Overlays

413.32.01 General – This Subsection describes requirements for repair and resurfacing of existing bridge decks (old) and newly constructed bridge decks (new) with a concrete overlay, describing the construction practices, materials and equipment required to place acceptable concrete overlay. This work is usually carried out on one-half of the deck at a time, while traffic is maintained on the other half. The requirements of SS 413.31 shall apply except as otherwise specified in this Subsection.

413.32.02 Graseline – The Contractor shall survey and reference profiles of the existing bridge deck and approaches prior to scarification as follows.

(a) Profile Surveys

- (i) Longitudinally the profiles shall be taken at 2 to 3 m stations for the entire length of the structure, including 21 m beyond each end, with additional shots taken at the back of ballast walls and all existing or proposed new joint locations.
- (ii) Transversely the number of profiles required will be determined by:
 - (A) A maximum spacing between elevation points of 3 m.
 - (B) Profiles are required 500 mm from each curb or parapet face.
 - (C) Profiles are required 300 mm (inside area to be concreted) from longitudinal overlay construction joint lines requiring adjustable screed rails. In general longitudinal overlay construction joints shall be located at the center of a lane or between lanes. The final location of the longitudinal construction joints is subject to the acceptance of the Ministry Representative.
 - (D) A profile is required at hinge point of crown.
- (iii) Elevation points are to be laid out in cross section ninety degrees to centerline of roadway on straight structures and radial to centerline of roadway on curved or spiraled structures.
- (iv) Profile elevation points shall be laid out to an accuracy of ± 25 mm horizontally and elevations taken to an accuracy of ± 3 mm vertically and be referenced such that they can be re-established within ± 25 mm from their original location.

- (b) The Contractor shall provide the Ministry Representative with the gradeline profiles plotted at a scale of 1:10 vertical and 1:50 horizontal and a spreadsheet showing actual original elevations for all points surveyed.

- (c) In the case of old decks, subsequent to scarifying and cleaning of the deck surface, the Contractor shall re-

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establish the original elevation points, take new elevations for the points to the nearest millimeter and submit these elevations along with plotted profiles to the Ministry Representative.

- (d) The Ministry will provide the Contractor with design finished grade profiles and elevations for the survey points as originally laid out by the Contractor. These profiles and elevations shall be used by the Contractor to set the screed rails for the deck-finishing machine, checking of overlay thickness during the dry run of the deck machine and for calculating the volume of overlay concrete required.
- (e) The Contractor is responsible for properly setting the screed rails to match the design gradeline provided by the Ministry Representative. Depressions creating ponded water, or localized high spots in the concrete surface, resulting from deficient finishing procedures shall be repaired at the Contractor's expense.
- (f) At completion of the deck overlay, the Contractor shall re-establish the original elevation points, take as built elevations for the points to the nearest millimetre, and submit these elevations along with plotted profiles to the Ministry Representative.
- (g) No specific payment shall be made for this work as all costs associated with the production of the gradelines and profiles are deemed incidental.

413.32.03 Removal of Existing Concrete Deck Surface –

The existing concrete surface shall be removed by use of scarification (roto-milling), hydro-demolition or alternate methods acceptable to the Ministry Representative, over the full length and width of the bridge deck. If methods other than scarification are proposed by the Contractor, the Contractor shall submit all required details of the proposed work to the Ministry Representative and shall obtain the acceptance of the Ministry Representative at least two weeks prior to starting the work. Alternate methods shall meet the requirements specified for scarification.

- (a) **Scarification** – If scarified, equipment shall be capable of concrete removal to an accuracy of ± 5 mm. The scarifying equipment shall produce a surface that does not exceed 10 mm in amplitude for roughness. The maximum cut will be to within 10 mm of the reinforcing steel. Depth of scarification will be specified by the Ministry.

The Contractor shall monitor the total depth of concrete cover to the top mat of reinforcing steel using a pachometer or alternate methods acceptable to the Ministry Representative. Readings shall be taken at regular intervals ahead of the scarification. The Contractor shall adjust the rate and depth of concrete removal to ensure that the reinforcing steel is not damaged and the desired scarified profile is achieved. All reinforcing steel damaged by the Contractor's operations will be replaced at the Contractor's expense.

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Care must be taken not to damage the existing deck drains. The Contractor will be required to repair or replace any damaged drains at the Contractor's expense. Drains are to be plugged to insure containment of debris.

All areas inaccessible to the scarification equipment shall be prepared by hydro-demolition, grinders or by jackhammers with weight limited to a maximum 18 kg. Preparation shall minimize damage to the sound substrate concrete. Larger jackhammers may be used if accepted by the Ministry Representative and if performance acceptable to the Ministry Representative can be demonstrated.

In the case of previously overlaid decks, concrete shall be removed to below the old bond line. Where the bond line is within 10 mm of the reinforcing steel, the remaining concrete and the old bonding material shall be removed by hydro-demolition or pneumatic tools.

Upon completion of scarification of the deck, prior to opening the scarified deck lane to traffic and at the direction of the Ministry Representative, the Contractor shall place temporary asphalt tapers (minimum 40:1) using asphalt patch material (cold mix) with tar paper underneath at all deck joints and bridge ends as required to maintain adequate traffic movement. The asphalt tapers shall be maintained until traffic is no longer using that lane or the final paving is complete. The temporary tapers shall be in accordance with SS 502.

- (b) **Payment** – Payment for removal of the existing concrete deck surface and asphalt roadway approaches will be made at the Lump Sum price bid. Payment for removal shall include all work for the removal of the concrete and asphalt roadway approaches including but not limited to sweeping, washing and /or blowing of debris, power hand tool removal of all concrete inaccessible to the scarification process, containment and the removal and disposal of all debris from the site. Payment shall also include all labour, material and equipment for the supply, installation and maintenance of temporary let downs, and disposal of all asphalt material from the Site.

413.32.04 Removal of Concrete for Partial and Full Depth Repairs –

The Contractor, in the presence of the Ministry Representative, shall carry out a detailed visual inspection of the deck surface for patch concrete. A chain drag method shall be used to examine the entire deck surface for evidence of additional delaminated and deteriorated concrete. These areas of unsound concrete and patch concrete shall be removed to sound concrete or removed full depth when considered necessary by the Ministry Representative.

Removal and repair of a specific area will not be completed until the Contractor and the Ministry Representative have inspected the affected areas requiring repair, and have agreed on the extent of repair required.

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The Contractor shall supply the necessary labour and supplies to mark out the repair areas.

(a) **Repair Procedures** – Partial and full depth concrete deck repairs shall proceed as follows:

- (i) The Contractor shall keep repair boundaries square or rectangular and avoid abrupt changes in width of a given repair area. The edge of the repair area shall be maintained vertical. No debris shall be permitted to fall from the bridge. Saw cut the perimeter of the repair areas to a minimum depth of 25 mm to prevent feather edges. Do not cut the existing reinforcing steel.
- (ii) Remove deteriorated, delaminated and patch concrete to sound concrete. Maximum jack-hammer weight shall be 18 kg. Care shall be taken to avoid damage to the reinforcement and remaining concrete. Jackhammers are to be operated at angles less than 45° as measured from the surface of the deck to the hand tool. The Contractor shall use wedge type chisel bits. Moyles and round point chisels are not permitted. Adhering concrete shall be removed from the reinforcement using hammers not larger than 7 kg.
- (iii) Remove all concrete to allow a 25 mm space around all exposed reinforcing bar within partial depth repairs. Expose all corroded reinforcement at edges of partial and full depth repair area.
- (iv) Full depth repairs will only apply when the soffit concrete is removed. For full depth repair areas, the Contractor shall install all necessary formwork prior to cleaning of reinforcing and concrete surfaces. The finished soffit repair concrete shall be flush with the surrounding soffit concrete. Areas for full depth repair exceeding 1 m² shall be patched prior to placement of the overlay. Before placement of the concrete patch, the surface of the adjoining concrete should be saturated with water for 30 minutes minimum and coated with a bonding agent immediately ahead of the fresh concrete.
- (v) Mildly corroded reinforcement shall be blast cleaned to commercial blast finish SSPC-SP6. Severely corroded and damaged reinforcement shall be replaced. Mechanical couplers acceptable to the Ministry Representative or laps in accordance with CSA-S6, “Canadian Highway Bridge Design Code” Clause 8.15.10.1 shall be used to attach new bars to the existing bars. Mechanical couplers shall develop at least 125% of the specified yield strength of the bar and shall be in accordance with CSA-S6.
- (vi) Prior to the placement of the partial and full depth repair area infill concrete the surfaces of the sub deck within the repair area shall be abrasive blast

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or high-pressure water blast (minimum 35 MPa / 5000 psi) cleaned to remove all bruised and fractured concrete and foreign materials such as dirt, dust, laitance, sand, grease, oil, concrete slurry etc. to the satisfaction of the Ministry Representative.

(vii) The Contractor shall collect and remove all dirt, dust, and blasting sand from the repair areas of the sub deck, by vacuum or similar method, and dispose of the waste materials away from the Site. Disposal shall be in accordance with all applicable environmental regulations.

(b) **Payment** – Payment for removal of Concrete for Partial Depth Concrete Repair of the sub deck will be made at the Unit Price per square metre bid. Payment shall be for all labour, materials and equipment to break out the concrete, including; inspections, sounding, cleaning of mildly corroded reinforcing, abrasive blast cleaning of the existing concrete surfaces within the repair areas, saw cutting, and containment and disposal of all debris.

Payment for Removal of Concrete for Full Depth Concrete Repair of the sub deck will be made at the Unit Price per square metre bid. Payment shall be for all labour, materials and equipment to break out the concrete, including inspections, sounding, cleaning of mildly corroded reinforcing, abrasive blast cleaning of the existing concrete surfaces within the repair areas, saw cutting, and containment and disposal of all debris. Payment shall also include the supply, installation and removal of all required formwork.

413.32.05 Reinforcing Steel – Reinforcing steel shall be supplied and installed in accordance with SS 412. Welding of reinforcing bars shall not be permitted.

Severely corroded reinforcing steel in the existing deck, more than 25% section loss as determined by the Ministry Representative, shall be removed and replacement bars spliced to the existing reinforcement using mechanical couplers acceptable to the Ministry or lapped splices. Mechanical couplers and lap length shall be in accordance with CSA S6.

Reinforcing steel damaged by the Contractor’s operations shall be replaced at the Contractor’s expense.

(a) **Payment** – Payment for replacement of severely corroded reinforcing steel will be made at the Unit Price per kilogram shown for increases and reductions in SS 412.91 and will be paid out of the Provisional Sum Item for Site Modifications. Payment shall be for Quality Control, supply, cutting, bending, shipping, placing and securing of reinforcing steel.

Payment for mechanical couplers will be made on a Force Account Basis under the Provisional Sum Item for Site Modifications.

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413.32.06 Removal and Replacement of Existing Deck Joints

- (a) **Expansion joint system** – The Contractor is responsible for demolition and removal of existing concrete and expansion joints.

The steel portions of the replacement deck joint shall be supplied and fabricated in accordance with SS 422.

The steel portions of the deck joint shall be galvanized and the joint armoring shall be installed 5 mm below the finished concrete nosing surface. Galvanized surfaces damaged by welding shall be touched up with two coats of an approved zinc rich coating.

Each joint seal shall be supplied in a single length, without splices. Before the joint seal is installed, the joint shall be thoroughly cleaned with a wire brush and all moisture removed from the joint. The seal shall be installed in accordance with the Manufacturer's recommendations and demonstrated to be watertight.

- (i) **Payment** – Payment for deck joints will be made at the Price bid for each joint. Payment shall be for all Work as shown on the Drawings and Special Provisions. Payment shall include but not be limited to demolition and removal of the existing deck and curb concrete, removal of the existing joints seal, preparation of shop drawings; the supply, fabrication, galvanizing, and installation of the joint armoring, anchors, forming, reinforcing and supply and installation of joint seal. Payment shall also include the removal of the joint curb seal section and replacement with a joint sealant within the curbs section.

- (b) **Preparation of Filled Joints** – The Contractor shall cut a transverse line, directly above the filled joint location, during casting of the deck overlay concrete using an edge cutter attached to a bull float. The line shall be neat and straight.

The Contractor shall further saw cut the filled joint along the transverse line within 48 hours of casting the deck. The saw cut will be achieved using a 4 mm wide diamond blade to a depth of 30 mm below the top of the deck.

The Contractor shall fill the saw-cut with Sikaflex 1C SL, a one-component, self-leveling, polyurethane sealant or an alternate permanent filler, impervious to water and resistant to ultra violet radiation, acceptable to the Ministry Representative. Sealants shall be installed in accordance with the manufacturers' recommendations. The saw cut shall be clean and dry before placing the sealer.

- (i) **Payment** – Payment for preparation of filled joints will be made at the price bid for each joint. Payment shall be for all labour, materials and equipment required to produce the filled joints

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413.32.07 Preparation of the Sub Deck – The Contractor shall high pressure (minimum 110 MPa/15000 psi using a rotating head) water blast the surface of the entire concrete subdeck including both parapet and curb faces 50 mm up from original deck elevation and abutment ballast walls, no earlier than 2 days before placing the overlay concrete.

The water blasting equipment shall effectively remove laitance, loose materials, bruised concrete, dust, slurry, oil, or other contaminants (as applicable) that are detrimental to the concrete overlay bond and shall leave behind a sound concrete surface, thoroughly cleaned and roughened to partially expose the coarse aggregate. The Contractor shall provide full containment and disposal of all debris, contaminants, etc., and shall comply with all applicable environmental, Workers Compensation Act, and Occupational Health and Safety Regulation requirements. The Contractor shall provide adequate guards to contain over spray and flying debris during the scarification, water blasting, deck washing and chipping process. The Contractor shall take all means necessary to reduce and contain the amount of dust produced throughout the entire project.

For the period between water blasting and placement of overlay concrete, the sub-deck shall be protected from contaminants.

Water blasting equipment shall travel downhill, keeping all waste material ahead of the equipment and preventing the waste material from rehydrating on the previously cleaned surfaces.

- (a) **Payment** – Payment for high pressure water blasting the entire deck surface, including both parapet faces and abutment ballast walls, to clean and roughen the sound concrete surface, will be made at the Lump Sum price bid.

Payment shall be for all labour, materials and equipment required to produce the specified surface roughness. Payment shall also be for the containment and treatment of the contaminated water and the removal and disposal of any debris from the Site.

413.32.08 Concrete Overlay Placement – All concrete works will be governed by the requirements of SS 211 "Portland Cement Concrete" and SS 413.31, except as otherwise specified in this Section.

The Contractor shall be responsible for the design of all concrete mixes. The Contractor shall be responsible for Quality Control of each component of the concreting operation, including aggregate and component quality, batching, mixing, transporting, placing, consolidating, finishing, curing and testing.

The Contractor shall submit a detailed Overlay Placement and Curing plan of equipment and manpower to be used for placement and curing of the concrete overlay a minimum of 14 days prior to the scheduled placement dates for the acceptance of the Ministry Representative. The plan shall

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also contain details such as: water source, the distribution system, the collection and run-off control system, person(s) responsible for Quality Control, and staff to maintain the system. All review concerns raised by the Ministry Representative shall be addressed to the satisfaction of the Ministry Representative.

The Contractor will undertake a test placement of the proposed deck mix design as indicated by the requirements of SS 211.03.05.

(a) Materials & Equipment – Acceptable types of overlay are High Density concrete and Silica fume modified concrete.

For High Density concrete overlays, acceptable finishing machines are Bid-Well model OF400, Bid-Well model OF500 or alternate (low slump pan-type overlay finishers), acceptable to the Ministry Representative. Concrete shall be produced on the Site using volumetric concrete mobile mixers. All equipment shall be provided in good working condition.

For High Density concrete overlays for structures over 50 m long or structures over 20 m long with a grade greater than 3%, two finishing machines shall be used in tandem. The speed of the trailing machine shall be such that its motion is continuous. Any open texture remaining after two finishing machine passes shall be repaired by hand.

For Silica fume modified concrete overlays, acceptable finishing machines are Terex Bid-Well models 2450, 3600 or 4800, Gomaco models C-A450, C-750, Allen Models 4836B, 6036 B 6048 B or alternate acceptable to the Ministry Representative.

Concrete shall be produced either by a qualified concrete batch plant or at the bridge site using pre-bagged concrete mix and mixer trucks. All equipment shall be provided in good working condition.

Finishing machines shall be capable of forward and reverse motion under positive control and with provision for raising screeds to clear previously screeded surfaces while traveling in reverse.

The Contractor shall provide steel rail mounted mechanical deck concrete finishing equipment of adequate size and design to permit the complete placement and finishing of a single lane section from abutment to abutment with the prescribed surface finish, without forming any cold joints.

The Contractor is responsible for properly setting the screed rails to ensure longitudinal and transverse drainage from the deck without ponded areas or “bird baths”.

Sufficient screed guide rails will be set out for the full length of the anticipated section to be cast plus 6 m length at each end for run in and run out of the deck

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machine, adjusted for height and accepted by the Ministry Representative prior to the pour as follows:

- (i) Screed guide rails, upon which the finished machine will be placed outside the area to be concreted, will be horizontally and vertically stable. Rails shall be installed with fully adjustable supports at 350 mm maximum spacing
- (ii) The finishing machine and guide rails will be adjusted so that the height of the screed above the existing subdeck at each point meets the Ministry Representative’s requirements. To confirm the adjustment of the machine and guide rails, the screed will be “dry-run” and clearance measurements taken at 3 m intervals and provided to the Ministry Representative for acceptance. The minimum overlay thickness shall be 50 mm. Resetting of the machine and/or guide rails will be done as necessary to obtain an acceptable “dry-run”. Adjustments to the machine or the rails will not be permitted after an acceptable “dry-run” is complete.

Work bridges shall be provided to facilitate bull floating, edge finishing with hand tools, correction of defects in machine finished concrete and tining and fog curing.

Rigid forms made of wood or steel shall be installed at longitudinal bulkheads in a manner that will maintain the desired shape during concrete placement, consolidation and finishing. The longitudinal bulkhead will be installed at the centerline or lane lines of the bridge deck. The top of the longitudinal bulkhead will be set at the finished deck elevation. Any honeycombing or poorly consolidated concrete after stripping the longitudinal bulkhead will be cut back into the new overlay a minimum of 150 mm.

The overlay shall be placed in strips with longitudinal joints near lane markings. Each strip shall be placed continuously between joints; adjacent strips shall not be cast for 24 hours.

- (b) Bonding Agent** – The Contractor shall apply a pre-approved cementitious bonding slurry to the prepared sub-deck and repair concrete. The prepared concrete sub deck shall be in a saturated surface dry (SSD) state prior to slurry placement. The Contractor shall supply the necessary equipment such as water hoses and vacuum or oil free compressed air.

The cementitious bonding slurry is to consist of type GU or GUL portland cement mixed with a water/cement ratio not exceeding 0.38 by mass. The slurry must not exceed 3 mm in depth and be scrubbed into the substrate surface with coarse brooms; in particular, no pooling will be permitted. The bonding slurry is to be applied immediately ahead of the overlay concrete placement and in a timely fashion so as not

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allow drying or setting up of the slurry prior to the placement of overlay concrete. Any areas to which the slurry has been applied, that in the opinion of the Ministry Representative exhibit drying or setting shall be cleaned of the bonding slurry and re-coated prior to the placement of deck overlay concrete.

The vertical faces at the edge of the scarified sub-deck surface; at construction joints; and at concrete/deck-joint interfaces for old bridge decks and the vertical faces at sub-deck and curb or parapet interface for new decks shall be coated with an epoxy-bonding agent conforming to ASTM C881, Type V, and Grade 2. The Class of bonding agent shall be appropriate for the temperature at the time of application. The bonding agent shall be applied in accordance with the manufacturer's instructions.

BRIDGE DECKS AND CONCRETE OVERLAYS

(c) **Interim Partial Strength** – Overlay shall attain a minimum strength of at least 30 MPa before removal of the burlap and opening to traffic. All other requirements of seven day curing shall be adhered to. The interim strength tests shall be determined using field cured test cylinders cured in accordance with Clause 9.3.2 of CSA A23.2-3C.

(d) **Payment** – Payment for placement of the concrete overlay and concrete for partial and full depth repairs will be made at the Unit Price per cubic metre bid. Payment shall be for all labour, materials and equipment for the final preparation and cleaning of the existing prepared concrete sub deck, supply and application of the cementitious bonding agent and the epoxy bonding agent, the supply of cement, silica fume, superplasticizer, all other constituent concrete materials and the supply and placing and curing of the concrete and containment and disposal of all debris.

SECTION 415

MANUFACTURE AND ERECTION OF PRECAST AND PRECAST PRESTRESSED CONCRETE MEMBERS

DESCRIPTION

415.01 Scope – This Section covers the manufacture and erection of precast and precast prestressed concrete members.

415.02 General

415.02.01 General Requirements – The manufacture and erection of precast and precast prestressed concrete members shall conform to the requirements of these specifications and details as shown on the Drawings and in the Special Provisions.

415.02.02 Plant Certification – Precast concrete elements shall be manufactured in plants certified to the current CSA A23.4 requirements for the product groups corresponding to the element types being fabricated. Certification shall be by the Canadian Precast Concrete Quality Assurance (CPCQA) Certification Program or by a certification organization accredited by the Standards Council of Canada (i.e. Canadian Standards Association (CSA)). Certification shall be in effect prior to the beginning of Work, and maintained throughout the period of manufacture.

415.02.03 Materials and Construction – Materials and construction shall conform to the current CSA A23.4 and the current PCI “Manual for Quality Control for Plants and Production of Structural Precast Concrete Products”. Where there is a discrepancy between the requirements of these two standards, the more stringent shall apply. In cases where it may be unclear as to which standard is more stringent, the Ministry Representative in consultation with the Ministry Chief Bridge Engineer will have the final say.

415.03 Prefabrication Meeting – The Ministry will at its discretion, convene a prefabrication meeting with the Contractor to review issues such as, but not limited to, lines of communication, fabricator and sub-fabricator scope of work, location of all work, procedures on Quality Control, plant certification, concrete source and mix designs, use of Hydration Stabilization Admixtures (if applicable), mill certificates, alternate details and procedures, and any other specific requirements of this specification as it relates to the Work.

415.04 Working Drawings – Working drawings shall consist of the following:

- Shop drawings,
- Transportation details, and
- Erection drawings

Transportation details and erection drawings shall be prepared and sealed by a professional engineer registered with the Association of Professional Engineers and

Geoscientists of the Province of British Columbia (APEGBC).

Shop drawings shall be prepared and sealed by a professional engineer registered with APEGBC when the Contractor is responsible for the design of items that are detailed on the shop drawings.

Working drawings shall be in the same system of units as the design drawings.

Lettering for notes and dimensions shall be at least 2.5 mm and 4 mm for headings. Drawings shall be legible when printed on 11” x 17” sheets.

415.04.01 Shop Drawings – Shop drawings shall show all information and details needed for the fabrication of the members including, but not limited to, such items as member shapes and dimensions, mark numbers and general arrangement of member locations, mass, prestressed and non-prestressed reinforcement, embedments, openings, block outs, chamfers, recesses, finishes, concrete mix design, stressing details, special tolerances, special handling instructions, lifting details and lifting locations.

A copy of the shop drawings shall be available at all times at the location where the components shown on the drawings are being fabricated. Changes to the precast from what is shown on the reviewed shop drawings, or repairs made during fabrication and/or construction, shall be indicated by the Contractor on a marked-up set of shop drawings and submitted in digital format to the Ministry Representative at the completion of the Work.

415.04.02 Transportation Details – Transportation details shall include such items as:

- Description of hauling and handling equipment,
- Weight of members,
- Length and height of loads,
- Location and method of member support, and engineering calculations for situations where members are to be supported during transportation further from their ends than outlined in SS 415.48,
- Details for handling, storing, and loading of members.

415.04.03 Erection Drawings – Erection drawings shall show in detail the method of erection including, but not limited to, the following:

- Erection procedures
- Procedures for off-loading of members upon delivery
- Details for temporary storage and support of members on site prior to erection

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- Equipment to be used
- Layout or general arrangement drawing showing the layout of the members, equipment positioning, and access roads
- Crane make, model, and capacity charts, boom length(s), crane placement, and access for transporting of members to crane(s)
- Radii and loads for crane lifts
- Rigging details
- Mass of members, rigging and special installation equipment
- Details for installation and removal of all falsework, temporary supports, temporary bearings, bracing, guys, dead-men, and lifting devices
- Attachments to the bridge members and bridge structure for temporary support and special launching equipment
- Detailed description of sequence of operations
- Details for special installation equipment such as a launching truss, launching nose, head frames, spreader beams and rollers
- Details for installation of members onto the permanent bearings
- Traffic control plan for roadway and rail traffic
- Details for protection of existing utilities affected by the erection procedures
- Layout and detail of fall protection and their sequence of installation.

The Contractor shall be responsible for the lateral stability of members and shall design and provide bracing as necessary until completion of the Work.

The erection drawings shall be complete in detail for all anticipated phases and conditions during erection and during the temporary support of members. The Contractor shall submit calculations upon request, to the Ministry Representative that demonstrate that allowable stresses are not exceeded in members, falsework, temporary bracing and temporary supports and that member capacities and final geometry will be correct. The calculations shall be sealed by the professional engineer that sealed the erection drawings.

Falsework, temporary supports and temporary bracing shall meet the requirements of CSA S269.1, "Falsework and formwork" and shall also meet all the requirements for falsework given in Clauses [20.17 to 20.26 inclusive](#) of the [WCB Occupational Health and Safety Regulation](#).

A professional engineer registered with APEGBC shall be responsible for any field designs and any changes made to the erection procedures. Field designs and changes to the

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erection procedures must be documented and sealed by the responsible professional engineer and must be available at the Site prior to the affected erection work being carried out.

Immediately before placement of loading on falsework, the Contractor must ensure that the falsework is inspected and a sealed engineering certificate is issued by a professional engineer registered with APEGBC which:

- Indicates the specific areas inspected, and
- Certifies that the falsework has been erected in accordance with the latest approved erection drawings and supplementary instructions.

Prestressed concrete stringers shall not be set on their final bearings, nor shall the bridge deck be cast, until 30_days after the casting of the stringers. If it is necessary to erect the stringers earlier, they shall be placed on temporary supports. Prestressed concrete twin cell box stringers up to a maximum length of 20_metres may be shipped and erected after 10 days if the specified 28_day compressive strength has been attained.

415.04.04 Submittals – The Contractor shall submit working drawings in digital format to the Ministry Representative at least 14 days prior to commencement of fabrication. These working drawings shall have been reviewed and approved by the Contractor. By this review and approval, the Contractor represents that it has determined and verified all field measurements, field construction criteria, materials, and similar data, and that it has checked and coordinated each working drawing with the requirements of the work and the contract documents. The Contractor shall indicate its review and approval by including on each drawing the date and signature of a person designated by the Contractor as being responsible for the Work. Working drawings shall be accompanied by a transmittal listing each of the drawings submitted. At the time of submission, the Contractor must notify the Ministry Representative in writing of any deviations in the shop drawings from the requirements of the contract documents. Any Work done or materials ordered prior to the review of the working drawings shall be at the Contractor's risk. The Ministry Representative will review the drawings for general compliance with the contract requirements.

If modifications to the drawings are required, the Ministry Representative will return one set of drawings, marked up, to the Contractor. The Contractor shall re-submit one complete set of revised drawings in digital format to the Ministry Representative. Any drawing that has changed from the version originally submitted shall be identified as such on a transmittal accompanying the revised drawing set.

If no exceptions are taken to the drawings, the Ministry Representative will return one set of reviewed drawings to the Contractor. Shop drawings will not be reviewed without the transportation and erection drawings applicable to the members in question.

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Any work done on materials ordered prior to the review of working drawings shall be at the Contractor's risk. Erection will not be allowed to proceed without the Ministry Representative's review of the method proposed.

Review of working drawings shall not relieve the Contractor of any responsibility for dimensions or detail or for carrying out the work in full accordance with the drawings and specifications.

If so agreed to in advance by the Ministry Representative, working drawings may be submitted in paper format. The reference to working drawing submittal copies shall be increased to four paper copies in this case.

At least 14 days before fabrication is to commence, or as otherwise requested by the Ministry, the fabricator shall submit a schedule of fabrication to the Ministry Representative in the form of a Gantt Chart. At the discretion of the Ministry, the schedule shall be updated on no less than a monthly basis. The schedule shall be provided to the Fabricator's Quality Control organization and to the Ministry Representative for reference and planning of inspections and progress reporting. The Contractor shall report any interim variations to the schedule.

415.04.05 Working Drawing Format - The Contractor shall transmit working drawings through attachments to e-mail. Unless otherwise agreed to by the Ministry Representative, electronic attachments to an email must total no more than 7 MB and must be submitted unzipped. Drawing files shall be submitted in PDF format to print out on 11" x 17" size pages. PDF sets shall be created by "distilling" CAD sheets rather than by scanning paper plan sets. Unless otherwise agreed to by the Ministry Representative, electronic attachments greater than 7 MB in size shall be sent in two parts by separate emails, denoting "1 of 2" and "2 of 2" in the subject lines after other required subject-line information.

If agreed to by the Ministry Representative, the Contractor may employ a document and data management service such as SharePoint® to transmit working drawings. If this process is used, the limitation on drawing file size is waived. The Contractor shall be responsible for setting up the appropriate folders within the document and data management software and for providing access to these folders to the fabricator, Ministry Representative and the design engineer. Email notifications shall be sent to all parties whenever updates are made.

The resolution of drawings shall be such that the finest detail must be legible at full scale on a monitor without zooming in (1-in. width on an 11 x 17-in. sheet is 1_in. on the monitor).

Drawings shall be black images on a white background.

All PDF sheets within a single file shall be assembled ensuring that all sheets are rotated to a "ready to read"

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orientation within the PDF file set. Generally, 11" x 17" plan sheets in landscape, 8½" x 11" note sheets in portrait, so that the majority of text is vertical. PDF sheets shall be ready to print out on appropriately sized paper sheets with no additional formatting required by the viewer.

Drawings of large or complicated pieces, where it is not practical to show all details on an 11" x 17" sheet, may be submitted on full size paper drawings, in which case the Contractor shall submit four copies of the drawing set. When revisions to full size paper drawings are required, the Contractor shall supply four complete sets to the Ministry Representative.

415.05 Alternative Details – All details shall, in general, conform to those shown on the Drawings. Any proposed variation shall be submitted in accordance with the provisions of SS 125 – Value Engineering Proposals. The submission of drawings showing alternative details shall be prepared and sealed by a professional engineer registered with APEGBC.

Alternative details must meet the requirements of the Contract and shall require the acceptance of the Ministry Representative prior to implementation.

Any consideration or implementation of a submitted VEP by the Ministry will also be in accordance with SS 125.

415.06 Quality Control – The Contractor shall implement a quality control program to meet the Contract requirements. The Quality Control plan shall be made available to the Ministry Representative for review.

415.07 Quality Assurance – The Ministry will implement a Quality Assurance program by auditing the Contractor's Quality Control program and by inspection at its discretion.

The Contractor shall notify the Ministry Representative at least 14 days before fabrication is to commence. In addition, the Contractor shall provide a minimum of 48 hours' notice to the Ministry Representative that a product will be available for inspection and/or certification by the Ministry's Inspector. If the schedule is subsequently changed, the Contractor shall provide the 48hour notice from the time that the Ministry is notified of this change. If the product is not available or is not sufficiently complete for inspection/certification as notified, at the sole discretion of the Ministry Representative, the Contractor shall be charged stand-by costs for the Ministry's Inspector. The Contractor shall allow the Ministry's representatives safe access to all parts of work, and shall supply such information and assistance as is required. When the Ministry requests, the Contractor shall provide samples of any materials. Inspection by the Ministry Representative shall not relieve the Contractor from obligation to perform the work in accordance with the Contract.

Concrete tests shall be in accordance with the current CSA A23.2 and with concrete sampled as close to the point

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of final deposit into the form as is practicable. All testing personnel shall be certified under CSA A283 or certified as an ACI Concrete Field Testing Technician – Grade 1 (minimum). The Contractor shall make available at the plant a sheltered, heated space for the casting and storage of test cylinders, with curing tanks and a concrete cylinder testing machine.

The Ministry may reject any items which, in its opinion, do not comply with the requirements of this specification. The Ministry in its sole discretion may back charge all inspection costs for the rejected material to the Contractor.

Each unit shall be certified as acceptable by a Ministry Inspector before it is shipped from the shop. This certification shall not relieve the Contractor of responsibility for subsequent damage or for defects which become apparent before the work is finally accepted by the Ministry. The Contractor shall provide 5 days notice to the Ministry Representative regarding intent to ship a unit or product and the product shall be made available for inspection prior to loading and shipping.

MATERIALS

415.11 Materials Supplied by the Contractor

415.11.01 Cement – Portland cement shall conform to the requirements of CSA A3000.

Unless specified otherwise, the Contractor shall use Type GU (General Use) or HE (High Early) Portland Cement. GU cement shall have a C₃A content of greater than 4.0% (by standard Bogue calculation) or 5% by mill certificate and a Blaine Fineness of less than 480 m²/kg.

415.11.02 Aggregates – Fine and Coarse Aggregates shall conform to CSA A23.1. Coarse aggregates shall not exceed 20 mm nominal size.

415.11.03 Water – Water shall conform to CSA A23.1. The mortars and tests shall be made in accordance with CSA A23.2.

415.11.04 Reinforcing Steel – All reinforcing steel shall be in accordance with SS 412.

All reinforcing shall be of the lengths called for on the Drawings and all bends shall be made in accordance with SS 412.

All bending schedules shall be furnished by the Contractor for the Ministry's review and any material ordered before such review shall be at the Contractor's risk. The review of bending diagrams by the Ministry shall in no way relieve the Contractor of responsibility for the correctness thereof.

Reinforcing steel material/coating type is designated on the Drawings using the following abbreviations:

- ME epoxy-coated;
- MG galvanized;

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- MC low carbon, chromium steel; and
- MS stainless steel;

415.11.05 Prestressing Steel – Pre-tensioning steel shall consist of seven-wire, uncoated, low relaxation strands as called for on the Drawings and complying with the requirements of ASTM A416M.

All strands for each project shall be obtained from one mill, and preferably strands from one heat shall be used for the Work. Members shall use strands from one heat. When multiple heats are used in a project, the modulus shall be matched in all members. Each reel of strand shall be identified by a secure tag showing the size of strand, CSA or ASTM Designation number, heat number, and name or mark of the manufacturer.

The Contractor shall supply to the Ministry Representative, for each heat number, one set of the standard mill test reports. Strands shall be protected at all times to prevent corrosion and shall be dry and clean (free from scale, rust, oil, soap, grease, and other deleterious materials) immediately before the concrete is placed. No strand which has previously been tensioned or gripped by strand chucks shall be incorporated into the members.

415.11.06 Admixtures – Chemical admixtures shall conform to ASTM C494. Air-entraining admixtures shall conform to ASTM C260.

Water reducing admixtures shall be of a lignosulfonic acid or salt, or hydroxylated carboxylic acid or salt.

Type B, Retarding or Type D, Water-Reducing and Retarding (Hydration Stabilizing) Admixtures conforming to the requirements of ASTM C494 shall not be incorporated into the mix design and/or added to the concrete without the acceptance of the Ministry Representative. When accepted by the Ministry Representative, the guidelines given in SS 415.37, SS 415.40.01 and SS 415.42.01 shall apply.

415.11.07 Mix Design Submittals – The Contractor shall submit a report outlining the proposed mix design for each classification of concrete to the Ministry Representative for review and acceptance in accordance with SS 211.03.04.

415.12 Storage of Materials – All materials shall be stored in accordance with the requirements of CSA A23.1. Cement shipments shall be used in the order in which they are received.

415.13 Premixed Aggregates – Pre-mixed aggregates in which the fine and coarse aggregates are combined in definite proportions will be permitted provided that the aggregates are delivered in batches directly to the hopper of the concrete mixer. Pre-mixed aggregates shall be transported and delivered in batches, each containing the correct quantity for one batch of concrete. At no time after the fine and coarse aggregates are combined shall any batch come in contact with or intermingle with any other batch.

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415.14 Forms – All exterior forms shall be of steel and shall be mortar-tight and of sufficient rigidity to prevent distortion due to incidental loadings during construction. Forms for skewed ends may be of rigid steel-reinforced plywood.

Wood forms will be permitted for the side walls of ballast wall pieces and for side walls of the deck panel pieces only. Wood formwork shall be designed, supplied and installed in accordance with CSA S269.1. Wood formwork shall render a true smooth surface free from fins and projections. Only new plywood shall be permitted for plywood formwork.

Forms shall produce 20 mm chamfers and fillets as shown on the Drawings.

415.15 Ministry's Office – Except for "on site" precasting work, where an office is already provided under the general contract, the Contractor shall provide at the point of manufacture a suitable lock-up office for the sole use of the Ministry.

The office shall be weatherproof and reasonably soundproof and provided with ample window area and ventilation. Location of the office shall be acceptable to the Ministry Representative. The office shall measure not less than 3 m x 4 m. In the event that the office is to be used concurrently with other inspection agencies, the minimum area required for Ministry's use shall be 3 m x 4 m. The Contractor shall install satisfactory heat and lights and provide a telephone, high-speed internet access, and a draughting table 1 m x 2.5 m in size with a plywood top, desk 0.75 m x 1 m, two chairs, drafting stool and one square metre of shelves.

Long distance calls made by Ministry personnel will be to the Ministry's account.

The Contractor shall, during the life of the work, regularly clean and properly maintain, heat and light to the office.

The office and contents shall be for the use of the Ministry Representative for the duration of the Work and may, if necessary, be used concurrently with other inspection agencies.

CONSTRUCTION

415.31 Reinforcing Steel – All reinforcing bars shall be accurately placed in the positions shown on the Drawings and firmly held during placing and setting of the concrete. Installation shall be in accordance with SS 412 and this specification. Tolerances shall be as specified by CSA A23.4.

When placed in the work the reinforcing steel shall be free from dirt, rust, loose scale, paint, oil, or other foreign materials. Bars shall be tied at all intersections except that where spacing is less than 300 mm, alternate intersections shall be tied. Form clearance shall be maintained by stays, ties, hangers or other approved supports. Before placing

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concrete, the placing and securing of reinforcing in the forms shall be inspected by the Ministry Representative.

All reinforcing shall be placed in the full lengths called for on the Drawings. Splicing of bars will not be permitted unless acceptable to the Ministry Representative. Splices shall be staggered. Unless otherwise shown or called for on the Drawings, splices shall be made by lapping in accordance with the requirements of SS 412. The bars shall be placed so as to maintain the minimum specified clearances between bars and the concrete surface. Welding of reinforcing steel shall be done only if called for on the Drawings.

Tack welding of reinforcing steel, if permitted, shall be in accordance with CSA W186-M.

415.32 Stressing – The dials of gauges shall be at least 150 mm in diameter; digital gauges are an acceptable alternative. Gauges shall be calibrated with the hoses and jacks with which they are to be used, in accordance with CSA A23.4 clause 28.2.3.2. The calibration must have been performed within the previous 6 months of use or 12 months maximum. Before stressing commences, certified calibration curves shall be furnished to the Ministry Representative. All strand chucks shall be cleaned and carefully inspected before each use.

At the sole discretion of the Ministry, the stressing operation may be inspected by the Ministry Representative. The Contractor shall provide a minimum 48 hours notice to the Ministry Representative prior to the stressing operation to allow for inspection. If during the work any gauges give erratic results, the tension system or tension device shall be recalibrated.

The Contractor shall schedule the work so that placement of concrete follows tensioning of strands within 24 hours. In cases of unavoidable delay, a maximum period of 80 hours will be allowed, provided that the strands do not show unacceptable corrosion (see current PCI "Manual for Quality Control for Plants and Production of Structural Precast Concrete Products"). The reason for the unavoidable delay shall be documented in the Contractor's Quality Control documentation along with documentation that strand condition is still acceptable. This information shall be made available to the Ministry Representative upon request.

Before tensioning, an initial force shall be applied to each strand to equalize stresses and eliminate slack. This force shall be between 5 and 25% of the final prestress force. After the initial tension forces have been applied, each strand shall be marked at the outer end of each chuck as a reference line from which the elongation of the strand can be measured.

The method of tensioning shall be such that the specified tension is maintained in the strands until the forces in the strands are transferred to the concrete member(s). The

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maximum jacking load in each strand shall be 80% of the ultimate strength of the strand.

A tensioning force shall be determined both by reading the jacking load on the calibrated gauge and by measuring the elongation of the strand. When the difference between the two methods is greater than 5%, the cause of the discrepancy shall be ascertained and corrected.

Elongations of strands shall be calculated and shown on the working drawings. Calculations for elongations shall include appropriate allowances for possible slippage or relaxation of the anchorages, friction at hold down points, temperature difference, and strand relaxation.

415.33 Forms – The design of the forms shall take into account the effect of vibration on concrete as it is placed.

All exterior forms shall be set and maintained true to the designated lines until the concrete has hardened. When forms are unsatisfactory in any way, either before or during the placing of concrete, the Ministry Representative may order the work stopped until the defects have been corrected.

The shape, strength, rigidity, mortar-tightness and surface smoothness of re-used forms shall be maintained at all times. Any warped or bulged formwork must be realigned before being re-used. Forms which are unsatisfactory in any respect shall not be reused.

Void forms shall be of an approved product and shall be anchored firmly by a method acceptable to the Ministry Representative to prevent displacement during the placing of the concrete. Any member with a void out of position in excess of the specified tolerance will be rejected. Exterior forms shall be treated in moderation with form oil before placing the concrete. Any material which will adhere to or discolour the concrete shall not be used.

Side forms and any attachments which would obstruct strain of the members shall be removed before transfer of stress.

415.34 Admixtures – The Contractor shall add sufficient air-entraining agent to ensure an air-void system in accordance with CSA A23.1 or as called for on the Drawings, and as determined by an air-meter used in accordance with CSA A23.2.

The use of other admixtures, such as water-reducing agents or superplasticizer, shall be subject to the acceptance of the Ministry Representative.

Type B, Retarding or Type D, Water-Reducing and Retarding (Hydration Stabilizing) Admixtures shall not be incorporated into the mix design and/or added to the concrete without the acceptance of the Ministry Representative. When accepted by the Ministry Representative, the guidelines given in SS 415.37, SS 415.40.01 and SS 415.42.01 shall apply.

415.35 Measurement of Materials – Measurement of materials shall conform to CSA A23.1. The Contractor

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shall produce a certificate or report of inspection from a weights and measures approved, independent testing authority, and which is not more than 6 months old. The certificate or report will be accepted as proof of accuracy for the batching equipment. Cement in standard packages need not be weighed.

The surface moisture shall be determined in accordance with CSA A23.2 and the batch weights of aggregates and water adjusted accordingly.

415.36 Mix Requirements – Minimum compressive strength at time of transfer of stress to the concrete and at 28 days as determined by CSA A23.2 shall be as called for on the Drawings. In any case, the force in the prestressing tendons shall not be transferred to the concrete until the compressive strength of the concrete is at least 25 MPa for pretensioned components and 20 MPa for post-tensioned components.

Slump measurement shall be determined by CSA A23.2. The mix design shall specify the slump and the tolerances in the measured slump shall be ± 20 mm for superplasticized concrete. Slump shall not exceed 200 mm for superplasticized concrete and shall be measured within 5 minutes after discharge from the mixer.

In order to minimize shrinkage and creep, the cement/aggregate ratio shall be kept as low as is consistent with strength requirements.

415.36.01 Self Consolidating Concrete (SCC) – SCC meeting the requirements of A23.1 may be considered for use on a project-specific basis and its use is subject to the approval of the Ministry Representative.

For the use of SCC to be considered, the Contractor shall submit the proposed mix design and detailed documentation for ministry review, a minimum of 28 days in advance of fabrication, which shall include as a minimum the following:

(a) Details in accordance with CSA A23.2-24C documenting the proven history of successful performance for the required strength, durability, and other performance requirements of the proposed mix design in the fabrication of products similar to those proposed for the use of SCC in the Work; or

(b) A full-scale test shall be used to verify the self-consolidating characteristics for placement and for the hardened concrete properties of the mix design for the proposed application. Documentation in accordance with CSA A23.2-24C shall be submitted demonstrating that the proposed mix design will achieve the required strength, durability, and performance requirements.

The Ministry Representative may also request additional documentation be submitted to assist in the ministry review. If, in the sole discretion of the Ministry Representative, the

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submitted documentation does not demonstrate acceptable performance of the SCC, then SCC shall not be used.

415.37 Batching and Mixing – The batching plant and mixing of concrete shall conform to CSA A23.1.

Mixing shall begin within 15 minutes after the cement has been added either to the water or aggregate.

Within 30 minutes after the introduction of the mixing water to the cement and aggregate, or the cement to the aggregate, the concrete shall be placed in its final position in the forms. In hot weather or under other conditions contributing to quick stiffening of the concrete, the maximum allowable time may be reduced by the Ministry Representative.

Time of placement extensions will be considered on a case by case basis and may require the use of Hydration Stabilizing Admixtures (HSA's) conforming to the requirements of ASTM C494 Type B, Retarding or Type D, Water-Reducing and Retarding Admixtures. If accepted by the Ministry Representative, guidelines for the use of HSA's shall be as follows:

- HSA modified concrete when used shall be fully discharged and placed in its final position within 90 minutes after the introduction of water to the cement and aggregate.
- All concrete in a given precast element shall be placed in its final position before 90 minutes.
- When HSA's are used, these time extensions are subject to preconstruction trials being conducted by the Contractor, to establish the appropriate HSA dosage to provide suitable extended slump life of concrete, without increasing the water/cementitious ratio of the concrete above that which would be required if HSA's were not used.
- The use of HSA's shall in no instance modify the maximum concrete temperature required at time of placement.
- A one time only addition of HSA will be allowed. This will be during initial batching of the concrete or immediately at completion of batching as recommended by the admixture manufacturer. Addition of HSA at any other time will be cause for rejection of the concrete.

Concrete mixing and transporting equipment shall be capable of placing concrete in the forms at the rate of 20 cubic metres per hour. Batch delivery shall be uniform and at a maximum interval of 10 minutes.

The maximum size of each batch shall not exceed the maximum rated capacity of the mixer as stated by the manufacturer of the mixer.

Concrete when placed in the form shall have a temperature not less than 10°C nor greater than 30°C, except for High-

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performance concrete which shall be limited to a maximum temperature of 25°C.

415.38 Handling and Placing Concrete – In preparation for the placing of concrete, all construction debris and extraneous matter shall be removed from the interior of forms. Struts, stays and braces inside the forms shall be kept to a minimum and entirely removed when the concrete placing has reached an elevation rendering their service unnecessary.

Concrete shall be handled and placed in accordance with the requirements of CSA A23.1 and shall be placed in the shortest possible time after mixing is completed. When rectangular void forms are used, concrete under voids shall be placed and compacted before void forms are placed in position.

Concrete may be deposited a maximum of 2 m without the use of pipes, provided, when required, suitable measures are taken to prevent segregation and premature coating of upper reinforcing steel. When pipes are used, they shall, as far as practicable, be kept full of concrete during placing and their lower ends shall be kept buried in the newly placed concrete.

After initial set of the concrete, the forms shall not be jarred and no stress shall be placed on projecting reinforcing bars.

415.39 Vibration – Concrete, during and immediately after depositing, shall be thoroughly compacted. The compaction shall be accomplished by mechanical vibration subject to the following provisions:

- (a) Vibration shall be internal for all accessible parts and shall be external for inaccessible parts.
- (b) Vibrators shall be of a type and design acceptable to the Ministry. They shall be capable of transmitting vibration to the concrete at frequencies recommended in CSA A23.1.
- (c) The intensity of vibration shall be such as to visibly affect a mass of concrete of 50 mm slump over a radius of at least 500 mm.
- (d) The Contractor shall provide a sufficient number of vibrators to properly compact each batch immediately after concrete is placed in the forms.
- (e) Vibrators shall be manipulated so as to thoroughly work the concrete around the reinforcing and embedded fixtures and into the corners and angles of the forms.
- (f) Vibration shall be applied at the point of deposit and in the area of freshly deposited concrete. The vibrators shall be inserted into and withdrawn from the concrete slowly. The vibration shall be of sufficient duration and intensity to thoroughly compact the concrete, but shall not be continued so as to cause segregation.

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- (g) Vibration shall not be continued at any one point to the extent that localized areas of grout are formed.
- (h) Application of vibrators shall be at points uniformly spaced and not farther apart than twice the radius over which the vibration is visibly effective.
- (i) Vibration shall not be applied directly or through the reinforcement to sections or layers of concrete which have hardened to the degree that the concrete ceases to be plastic under vibration. It shall not be used to make concrete flow in the forms over distances so great as to cause segregation, and vibrators shall not be used to transport concrete in the forms.
- (j) Vibration shall be supplemented by such spading as is necessary to ensure a smooth surface and dense concrete along form surfaces and in corners and locations impossible to reach with the vibrators.
- (k) Vibrator heads shall be smaller than the minimum clear distance between prestressing strands.

415.40 Test Cylinders – The Contractor shall make available, as may be required by the Ministry Representative, samples of the concrete used in casting the members.

Further, the Contractor shall take samples and make test cylinders in the presence of the Ministry Representative and in accordance with CSA A23.2-1C, -3C and -9C. At least 8 cylinders per casting bed for prestressed girders, and 4 cylinders per casting bed for non-prestressed components shall be made for testing. At the discretion of the Ministry Representative, depending on the quantity and type of element being cast, the number of cylinders may be reduced. Cylinders shall be representative of all stages of concrete placement.

415.40.01 Release Strength – Two cylinders from each end of the casting bed shall be cured in the same manner as the members and tested by the Contractor under the observation of the Ministry Representative at the Contractor's plant to assess the strength of the concrete at the time of transfer of force to the concrete. Only one cylinder from each end need be tested if the average strength of the two cylinders is equal to or greater than the required release strength at transfer and no test cylinder is more than 1.4 MPa below the release strength. No transfer of force shall occur without the acceptance of the Ministry Representative. At the discretion of the Ministry Representative, depending on the quantity and type of element being cast, the number of cylinders may be reduced.

If all release cylinders are tested and the release strength has not been attained, further testing with an impact hammer shall be allowed, provided a written procedure acceptable to the Ministry Representative is provided by the Contractor. The procedure shall correlate readings from areas of a

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prestressed girder where the release strength has been determined by concrete test cylinders with similar areas on the girder in question. If this situation occurs, extra pairs of release cylinders shall be made on future casts until confidence in determining the release strength is re-established.

When HSA's are incorporated into the mix design, pre-construction tests shall be conducted to determine the rate of early strength development for the selected concrete mix design. Tests shall be repeated when there is a change in the HSA dosage rate.

415.40.02 28 Day Strength – Two cylinders from each end of the casting bed shall be identified as "acceptance cylinders" and shall be cured in the Contractor's curing tanks. These cylinders shall be tested by the Contractor under the discretionary observation of the Ministry Representative to determine the twenty-eight day strengths. The strength of the concrete in the member shall be taken as the average of the compressive strength of the "acceptance cylinders", however, no individual cylinder shall be more than 3.5 MPa below the specified strength and the average strength shall equal or exceed the 28_{day} strength.

415.40.03 Additional Cylinders – For special cases, extra cylinders shall be made and tested if required by the Ministry Representative. In the case of ready mix, and plant mixed concrete with no established records, extra cylinders, in addition to the release strength or 28_{day} strength cylinders, shall be made from each load.

415.41 Surface Finish – Throughout the work, special precautions shall be employed in formwork and concrete placing to produce a smooth and dense concrete. Exposed surfaces shall be as free as possible from form marks, fins, honey-combing and pock marking. Only a minimum amount of pointing will be allowed. Poor surface finish upon the removal of the forms shall be sufficient cause for the rejection of any member. All exposed edges shall be chamfered unless otherwise noted on the Drawings.

Roadway and surfaces to be bonded to shall be rough screeded by wood floating following initial strike off.

Surfaces to be bonded to (e.g. keyways in box beams, recesses at lifting devices) shall be sandblasted or exposed with chemical retarder and water blasted to remove all laitance and foreign matter and to roughen the surface. The amplitude of the surface roughness shall be 5 mm. Loose sand and debris shall be washed or blown off. Water, cement or proprietary topping material shall not be added to the surface during finishing. The surface roughness amplitude at keyways in box beams and recesses at the lifting devices shall be approximately 1 mm.

Walkway surfaces shall be given a coarse transverse broomed finish following initial screeding and floating. Roadway surfaces not requiring any wearing surface shall be given a tined surface in accordance with

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SS 413.31.02(e). Roadways receiving a membrane shall be finished in accordance with SS 419.33 and SS 413.31.02(e).

The outer surfaces of all outside stringers shall be given a Class 2 finish, and any other surfaces with a different surface finish as indicated on the Drawings shall be finished in accordance with SS 211.17.

415.42 Curing – Precast or precast prestressed members may be cured by steam or heat as outlined herein.

415.42.01 Steam Curing – The concrete shall attain its initial set before application of steam (generally 2 to 4 hours after the final placement of concrete or as justified by previous experience or in accordance with ASTM C403/C403M Standard Test Method for Time of Setting of Concrete Mixtures by Penetration Resistance, all to the acceptance of the Ministry Representative.

If HSA's are used in the mix design, time to attain initial set after concrete has been placed may exceed that required for conventional concrete.

After the placement of concrete, and until the concrete has attained initial set, sufficient steam shall be applied to hold the air surrounding the member at a temperature not less than 13°C.

Steam curing shall be done within a suitable enclosure to contain the steam and minimize moisture and heat losses. The enclosure shall allow free circulation of steam around the sides and tops of members and shall not be in contact with the member.

Steam at the outlets shall be at 100% relative humidity and low pressure, to achieve 95% relative humidity in the enclosure. Application of the steam shall not be directly on the concrete or test cylinders and shall be from outlets distributed uniformly along the whole length of the member.

Temperatures within the concrete shall be measured and recorded in accordance with CSA A23.4 clause 23.2.3. Recording of temperatures shall continue until the end of the cooling-off period. If requested, temperature measurement records shall be made available to the Ministry Representative. During application of the steam, the temperature within the concrete shall increase at a rate not to exceed 20°C per hour until an optimum temperature of 55°C is reached. In no case shall the temperature within the concrete exceed 70°C. The maximum temperature shall be held until the concrete has reached the transfer strength. In discontinuing the steam, the concrete temperature shall not decrease at a rate greater than 15°C per hour until a temperature has been reached 20°C above the temperature of the air to which the concrete will be exposed..

415.42.02 Curing With Radiant Heat and Moisture – Curing with radiant heat shall be done by heating the casting area in combination with measures to protect concrete surfaces from moisture loss. Heat shall be applied by means of pipes circulating steam, hot oil or hot water or by electric

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heating elements. Pipes shall not be in contact with concrete, form surfaces or test cylinders.

Protection against moisture loss shall be by a continuous sprinkling to a cover of burlap, cotton matting covering the surface with tarpaulins or plastic sheeting, using a water spray, or ensuring that 100% relative humidity is present in the curing enclosure.

An auxiliary cover in addition to the moisture retaining cover shall be used above the entire bed to retain the heat. This cover shall be supported at a sufficient distance above the member to allow circulation of the heat.

The cycle of application of heat, temperature limitations, reduction of temperatures and use of recording thermometers shall be similar to the cycle specified for steam curing.

415.42.03 Additional Curing Requirements – Precast non-prestressed members shall be cured in accordance with the requirements of CSA A23.1 Section 7.8. Full depth and partial depth precast deck panels, as well as precast wing walls, parapets, abutments and pier caps, shall be cured in accordance with curing type 2 described in CSA A23.1 Table 19. The extended curing requirements of CSA A23.4 clause 23.3 shall be applied to these members. However, components containing silica fume concrete shall be cured in accordance with curing type 3.

415.43 Draped Strands – Where called for on the Drawings strands shall be draped in accordance with CSA A23.4.

415.44 Transfer of Force to Concrete Members – The transfer of the forces in the strands to the concrete member(s) shall be in accordance with CSA A23.4 (section on Detensioning).

415.45 Protective Coating for Ends of Strands – After completion of concrete curing, the ends of tensile strands shall be ground flush with the concrete surface. Where the ends of the members are to be embedded in concrete, the ends of the strands shall be painted with two coats of an organic zinc rich paint acceptable to the Ministry. Where the ends of the members are not to be embedded in concrete, the ends of the members shall be covered with thixotropic epoxy to provide at least 3 mm cover for a band width of 50 mm on each side of all strands. This material is to be applied in accordance with the manufacturer's recommendations.

415.46 Tolerances – For bridge stringers, the tolerances specified in the PCI "Manual for Quality Control for Plants and Production of Structural Precast Concrete Products" shall be modified so that the length of each member, measured parallel to the longitudinal axis, to extreme corners, at 20°C, does not exceed the specified length by more than 10 mm / 10 m (25 mm maximum).

The Contractor shall employ an adequate method of checking tolerances both before and after placement of concrete. Review of the method of checking tolerances by

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the Ministry Representative will not limit the Contractor's responsibility for the accuracy of the members.

415.47 Repairs to Members – Defective areas should be repaired immediately following removal of forms and before detensioning of strands, except where observation of the defect is not possible before detensioning.

If a defect is deemed to be a structural repair and meets the criteria stated in CSA A23.4 Section 33.3, notification shall be provided to the Ministry Representative regarding the nature and extent of the defect. The Contractor shall follow the repair procedures described in CSA A23.4 Section 33.3 and submit the proposed repair procedure to the Ministry Representative for review and acceptance prior to any repair work being undertaken.

Holes, honeycomb spots, etc., shall be treated as for Class 2 finish in accordance with SS 211.17. The method of repair for cracks, broken corners and edges, bulges and other defects shall be submitted to the Ministry Representative for review and acceptance. In any case, the Ministry Representative/Inspector shall be notified regarding all repairs to precast and precast prestressed members.

415.48 Handling, Storing and Loading of Members – Members shall at all times be kept in their normal upright position during handling, storage and transportation. They shall be picked up by means of vertical forces or forces inclined towards the opposite ends of the members by no more than 30° from the vertical, applied to lifting devices near the ends of the members. I-beams shall be adequately stiffened during lifting and transportation.

Stored beams shall rest on unyielding level supports near the ends of the beams.

When loaded for transportation, members shall be supported so that they will not be overstressed. In general, beams with straight strands may be supported up to one-tenth of their lengths from each end, whereas beams with draped strands should be supported at their bearings. If the Contractor believes that it is possible to safely support members further from their ends, the Contractor shall submit to the Ministry Representative, sketch(es), sealed by a registered professional engineer, showing the proposed support arrangement. Adequacy of support shall be based on the following assumptions:

- 100% impact on cantilevering ends, when checking stresses at supports,
- 50% negative impact on the whole member, when checking stress near midspan,
- 50% of prestress losses at time of shipping (unless circumstances dictate otherwise),
- allowable tensile stress = $0.5\sqrt{f_c}$
- allowable compressive stress = $0.6f_c$

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- where f_c = estimated strength of concrete at time of shipping.

Such sketch(es) shall be submitted at the time of submittal of the working drawings (see SS 415.04). Members transported on supports further from their ends than outlined above, without the submittal of sketch(es) acceptable to the Ministry, will be rejected.

Piles shall be lifted in the horizontal position at the pick-up positions shown on the drawings and at no other points. When stacking for storage or for delivery, piles shall be supported at the pick up points. They shall be handled carefully, avoiding dropping and jarring and avoiding surface abrasions or other damage.

Any member damaged so as to be, in the opinion of the Ministry Representative, unfit for the use for which it is intended shall be replaced.

415.49 Placement of Concrete in Keyways – Keyways between box stringers shall be clean and shall be pre-wet and saturated dry surfaces, before being filled with concrete.

Concrete in keyways shall attain a strength of at least 15 MPa before light traffic such as one-tonne trucks are allowed on the bridge, and at least 25 MPa before heavy loads such as concrete trucks are allowed on the bridge.

415.50 Post-Tensioning – All post-tensioning shall be done in accordance with the SS 416 – "Manufacture and Installation of Post-Tensioned Concrete Members".

QUALITY CONTROL PROGRAM

415.61 Quality Control Program – These Subsections describe the general Quality Control Program required by the Ministry of Transportation for any Fabricator undertaking the fabrication of permanent precast concrete or precast prestressed concrete bridge components. The Quality Control Program shall be part of the overall Quality Management Plan for the project.

The term "Quality Control" defines those activities that the Fabricator performs to conform to the Contract.

The term "Quality Assurance" defines those activities that the Ministry performs to ensure conformance to the Contract.

The term "Quality Audit" defines those activities that the Ministry performs to provide confidence that the QC and QA processes and resulting products satisfy the Contract requirements.

These Subsections contain statements of the Quality Objectives and Policies that the Ministry considers essential for successful and economical quality management. They also outline the Procedures and Documentation to implement and confirm that the Objectives are met.

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The provisions of the Quality Control Program set forth in these Subsections shall apply to all precast concrete bridge components.

These Subsections require the establishment of a Quality Management Team with the responsibility for the successful and timely implementation of all necessary Quality Control and Quality Assurance activities. Some positions shown in the Suggested Organization Chart (see Figure 1) may be held by the same individual. For example, the General Manager may also be the Contract Administrative Manager and the Purchasing Manager. The Plant Superintendent may also be the Receiver and the Structural Supervisor.

415.62 Quality Objectives and Policies

415.62.01 Quality Objectives – The Quality Objectives of the Ministry cover all precast concrete bridge components produced under the Contract, as follows:

- (a) Completed products shall conform fully to the governing Codes and Specifications stipulated in the Contract.
- (b) The Quality Control Program shall be fully integrated into the ongoing manufacturing activities of the Fabricator.
- (c) The operations of the Quality Control Program shall protect the interests of the Ministry with respect to scheduled delivery date and contracted price.

415.62.02 Quality Policy – It is Ministry policy that the interests of the Contractor and the Ministry coincide when a product meets the Quality requirements of the Contract, is delivered on time, and is produced in a cost-effective manner.

The Quality Control Program is an essential part of this policy. It is based on the fact that the best way to do any job is to do it right the first time. This applies to all stages of the design, drafting, procurement, manufacturing and testing of the product.

The Quality Control Program involves the Quality Management Team, Ministry Representative and Fabricator personnel. Their activities must be consistent with the Quality Objectives of the Ministry.

A Quality Control Manager shall be appointed with defined responsibilities in resolving quality matters and shall report to a senior management level. At each hold point, the Quality Control Manager shall:

- Document the successful completion of each stage as it progresses through fabrication and erection.
- Identify and report nonconforming components.
- Initiate or recommend disposition of nonconforming components.
- Verify corrections.

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Any persons assigned to perform Quality Control inspections shall be other than those performing or directly supervising the work and they shall not report directly to immediate supervisors responsible for producing the work.

The Quality Control Program is not subordinate to any design, drafting, procurement, manufacturing and testing activities, it is an essential part of them.

415.63 Scope of the Quality Control Programs – This Quality Control Program governs the fabrication of precast concrete bridge components for the Work. The Suggested Organization Chart is shown in Figure 1.

The General Manager shall be responsible for:

- adhering to this Quality Control Program in all respects.
- ensuring that completed structural components shall conform fully to the applicable design and fabrication requirements stipulated in the Contract.
- ensuring that all required documentation is produced according to this Quality Control Program.

415.64 Range of Capability – Within the Company, there shall be the necessary knowledge, skill (in-house or on a contract basis) and equipment to do the following work on precast concrete structural components:

- Design of lifting and erection devices not shown on drawings
- Preparation of shop fabrication and working drawings
- Preparation of bills of material
- Preparation of material requisitions and purchase orders
- Receiving, checking and storing materials for bridges
- Layout, form set-up, reinforcing steel, prestressing and post-tensioning
- Mix designs, batching, mixing, placing and curing of concrete
- Sampling, testing, making and breaking of concrete test cylinders
- Dimensional checking and verification
- Resolution of non-conformances
- Documentation of all stages of work with capability of tracing all major components
- Finishing, patching, storing and shipping
- Erection of bridge components

415.65 Drawings and Specifications – The Chief Design Engineer shall be responsible for:

- the design and location of lifting devices and the preparation of stressing sheets for prestressing or post-

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tensioning; and for the design of the erection procedures and any special erection equipment needed.

The Chief Draftsperson shall be responsible for:

- obtaining the latest revision of the design Drawings and specifications for the work.
- submitting working drawings, shop drawings and erection drawings to the Ministry for acceptance before commencement of the work.
- preparing material requisitions containing a full description of the material sizes, material specifications, and certifications required for conformance to the Contract.
- delivering the material requisitions to the Purchasing Manager in ample time to permit ordering, delivery and documentation and not delay the progress of the work.
- issuing requisitions for all sub-contracted drafting work and shall ensure that all conditions of the Contract are part of such sub-contracts.

415.66 Material Control – All materials for structural components shall be ordered by the Purchasing Manager in full conformance with the material requisitions provided by the Chief Draftsperson.

The Purchase Orders shall contain all information necessary to ensure that materials purchased will comply fully with the terms of the Contract. Where mill certificates and test reports are required, it shall be so stated on the Purchase Order. Instructions shall state when the certificates and reports are to be delivered to the Fabricator.

If a supplier proposes a substitute for any material, the Purchasing Manager shall refer the proposed substitution to the Chief Design Engineer for review. If the substitute is acceptable to the Ministry, the Chief Draftsperson shall amend all drawings and requisitions, withdraw old issues and issue the new versions.

The Receiver shall:

- inspect all materials on arrival for conformance with the Purchase Orders.
- confirm that mill certificates and test reports are provided and that they correctly identify the materials delivered.
- arrange with the Plant Superintendent to store all materials for the Contract in segregated areas.
- ensure all materials are clearly identified with the Contract.
- issue a non-conformance report covering overage, shortage or damage to the materials. Copies of the report shall be provided to the Purchasing Manager and the Quality Control Manager.

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The Purchasing Manager shall deliver all documentation to the Quality Control Manager for inclusion in the Quality Control file for the Contract.

415.67 Pre-Pour Inspection and Reporting – The Quality Control Manager shall ensure only documented materials are used for the work.

Forms used in the casting of members shall be inspected prior to use and shall meet all the requirements of CSA A23.4.

Reinforcement shall be fabricated and placed to satisfy SS 412 and CSA A23.4 and the detailed shop drawings for quantity and spacing. Any reinforcement used to anchor cast in hardware by being welded to it will require the Fabricator to be certified to CSA W186-M.

All cast in hardware shall be fabricated and placed according to CSA A23.4. Any hardware requiring welding, including stud welding, will require the fabricator to be certified under CSA W47.1 Division 2 and have approved procedures for each weld.

All pre-tensioning operations shall be done according to CSA A23.4. Care shall be taken to properly execute, supervise and record the operations in a clear and legible manner.

The Quality Control Manager shall file a written report of verification with the General Manager and report any non-conformance that may exist on any of the above items in SS 415.67.

415.68 Concrete Control – All materials used in concrete mixes such as cement, fine and coarse aggregates, and admixtures, shall have representative certificates that can verify their quality.

Production of concrete shall be done according to the Contract. Records shall be kept of the mix type and quantity used in each pour.

Handling, placing and vibration of freshly mixed concrete shall be done as per Contract specifications. Any variations and interruptions in the process shall be recorded.

The sampling and testing of freshly mixed concrete shall be in accordance with the specifications and procedures laid out in CSA A23.2. Concrete test cylinders shall be made, cured and tested in accordance with the Contract specifications.

Reports for all concrete testing shall be kept on file and all non-conformities reported.

415.69 Post Pour Inspection and Reporting – Dimensions of all members shall be verified to ensure tolerances are being maintained as per the Contract specifications.

Verification shall be made of finishes, blockouts, keyways, projecting rebar, sole plates and protection of the cut off strands.

Camber shall be monitored at release and 28 days.

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Members shall be stored according to the fabricator's documented procedures.

Members shall only be allowed to be shipped out for erection after they have been approved by both Quality Control and accepted by the Ministry Quality Assurance representative. A certificate of compliance may be issued by the Ministry Quality Assurance representative to acknowledge acceptance.

Records shall be kept on file for all items in this section.

415.70 Correction of Non-Conformities – When a non-conformance is encountered, the Quality Control Manager shall determine a recommended disposition and submit such disposition to the Ministry Representative for acceptance as quickly as possible.

If there is non-conformance to the material specification stipulated in the Purchase Order, the Purchasing Manager shall immediately find out the reasons for the delivery of non-conforming materials. If the material is of a grade superior to that ordered, the Quality Control Manager and the Chief Engineer shall be notified. They shall verify that the material is an acceptable alternative in all respects. This verification shall include consultation with the Ministry. If the material is of a grade inferior to that ordered, it shall be rejected and the correct material or a superior material shall be obtained.

If there is non-conformance of material delivery that will delay production, the Contract Administrative Manager and Plant Superintendent shall be notified immediately. They shall be given revised delivery dates for the adjustment of production scheduling. The General Manager shall determine the alternatives available and shall notify the Ministry.

If there are pre-pour non-conformities in form set-up, prestressing, rebar placement, hardware and blockout size and location, the Quality Control Manager and Structural Supervisor shall immediately review the non-conformance and notify the Chief Design Engineer who may require further investigation prior to submitting corrective action to the Ministry for acceptance. If the necessary corrective action will result in delay to production, the General Manager shall be notified for adjustment to the production schedule. The Plant Superintendent shall inform the Quality Control Manager when the corrective actions are being done so that conformance can be verified and the non-conformance report cancelled.

If there are post-pour non-conformities such as honeycombs or large voids after form removal, before de-tensioning the Quality Control Manager shall notify the Chief Design Engineer who will investigate and submit corrective action to the Ministry for acceptance. Other non-conformities such as low concrete strength, dimensional errors due to faulty forms, variations in camber from design and missing hardware shall all be reported by the Quality Control Manager to the Ministry Representative.

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415.71 Welding – All welding on structural components shall conform to the Welding Standards of the Contractor as approved by the Canadian Welding Bureau.

All welding consumables shall conform to the approved Welding Standards and shall be received, stored and conditioned according to the applicable Welding Standards.

All welders and welding operators on the Contract shall be qualified under the requirements of the current CSA W47.1 division 2 or better.

All welding of reinforcing steel, when acceptable to the Ministry, shall be done by fabricators certified under CSA W186.

415.72 Calibration of Measurement and Test Equipment – All measurement and testing equipment owned by the Fabricator and used in the Quality Control Program shall be calibrated and re-calibrated at the intervals and in the manner stipulated in CSA A23.4.

When the concrete testing services are contracted to a Qualified testing firm the testing firm shall meet the requirements of CSA A283. The testing firm shall confirm in writing to the Ministry Representative that all their measurement and test equipment used in the testing is properly calibrated. If requested, they shall furnish copies of the calibration test data for inclusion in the contract documentation.

415.73 Records Retention – The Fabricator's Record File for each Contract shall contain the pertinent drawings, purchase orders, bills of material, material mill certificates, test reports, Quality Control documents, and certificates of compliance.

The Fabricator's Record File shall be made available to the Ministry's Inspectors upon request.

Items in the Fabricator's Record File shall be retained as per company policy.

The Quality Control Manager shall ensure that each file is complete in all respects before it is placed in the Company archives.

415.74 Hold Points – In planning the work flow, it is essential to coordinate with the Ministry Representative to decide "hold" points (as defined in SS 145.12) for inspection. A list of hold points shall be drawn up by the Fabricator and the Ministry at a prefabrication meeting. The Ministry should be continuously informed of progress so that delays are minimized.

The "hold" points will typically include, but not be limited to, the following:

- verification of form size, quality and layout
- after stressing, rebar and hardware placement
- prior to form close up
- sampling and testing of concrete

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- release cylinders
- after patching and finishing
- 28 day cylinder results
- shipping arrangements.

It must be understood that work will not proceed past a "hold" point until it has been signed off by Quality Control and Quality Assurance. Reports must be completed promptly.

(Refer to the current edition of the PCI Manual – Publication MNL-116 for "Sample Record Forms")

MEASUREMENT

415.81 Precast Concrete Members – Precast Concrete members will be measured by EACH of the type specified.

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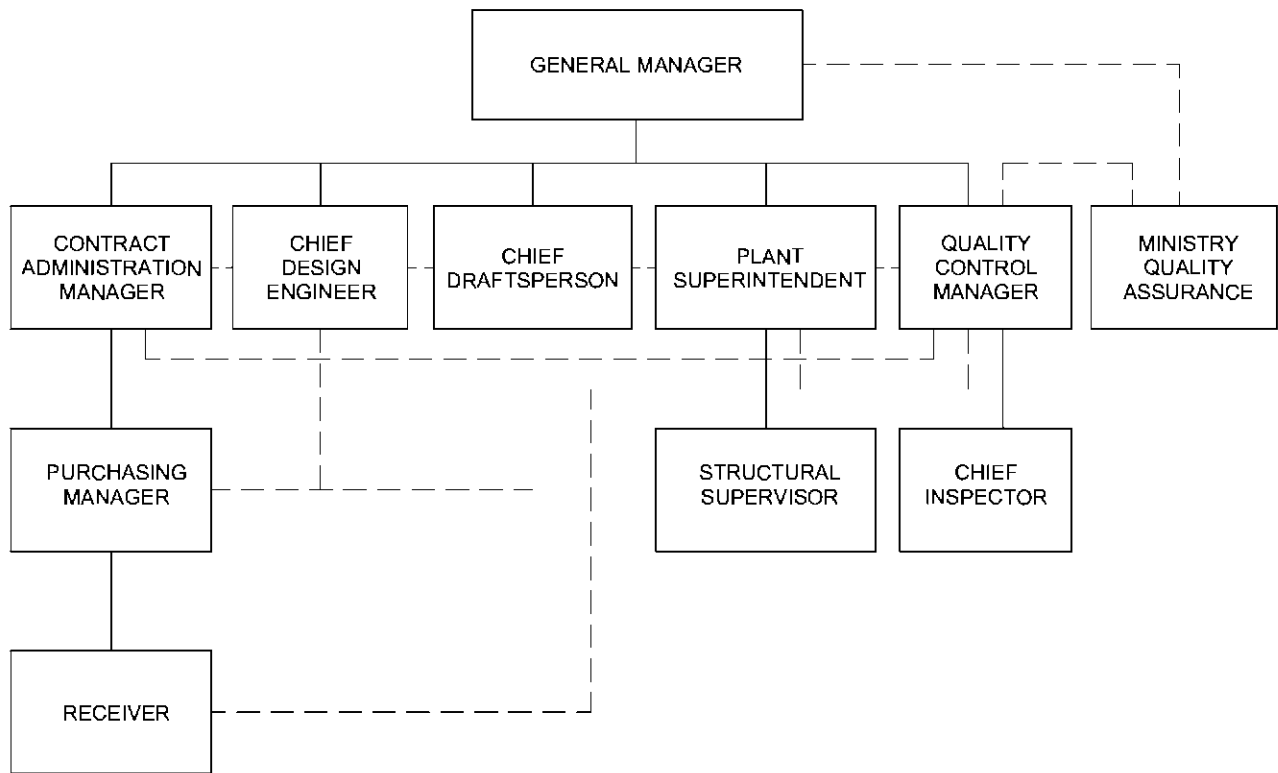
PAYMENT

415.91 Supply and Fabrication – Payment for supply and fabrication of PRECAST CONCRETE MEMBERS will be made at the lump sum price bid. Payment shall be for Quality Control, working drawings, supply and placing of concrete, formwork, reinforcing steel, prestressing strands and any required inserts. Payment shall also cover tensioning, curing, release, stripping of forms, patching as necessary and storage of members.

415.92 Shipping and Erection – Payment for shipping and erection of PRECAST CONCRETE MEMBERS will be made at the lump sum price bid. Payment shall be for Quality Control loading, shipping and unloading of members. Payment shall also cover falsework and erection of members. For box stringers, payment shall also cover the supply and placement of sealing strips between stringers.

FIGURE 1

SUGGESTED ORGANIZATION CHART



Direct Functional Reporting _____

Quality Control Reporting _____

Signed _____
General Manager

Date _____

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MANUFACTURE AND INSTALLATION OF POST-TENSIONED CONCRETE MEMBERS

DESCRIPTION

416.01 Scope – This Section covers the manufacture and installation of post-tensioned concrete members.

416.02 General

416.02.01 General Requirements – The requirements of CSA-S6, the Ministry Supplement to CSA-S6 and SS 415 shall apply unless specified otherwise herein or in the Contract.

For items not covered in CSA-S6, the Ministry Supplement to CSA-S6 SS 415 or the Contract, then CSA A23.1, Subsection 6.8, shall apply.

416.02.02 Post-Tensioning Procedure – Three weeks before post-tensioning, the Contractor shall submit to the Ministry Representative for review, complete details of the proposed procedure, including jacking sequence and loads, procedure for testing grout cubes, and a list of equipment. This procedure shall be prepared and sealed by a professional engineer registered with the Association of Professional Engineers and Geoscientists of the Province of British Columbia (APEGBC).

MATERIALS

416.11 Anchorages – Anchorages tested in an unbonded state shall develop at least 95% of the minimum specified ultimate strength of the prestressing tendons, without exceeding anticipated set and without slippage. Test results or certificates meeting these requirements shall be supplied by the Contractor.

Anchorages shall be able to sustain the loads applied to them without slippage, distortion or other changes that will permit loss of prestress.

The anchorages shall be of such a nature that they will not kink, neck down or otherwise damage the prestressing steel.

416.12 Ducts – The ducts shall be provided with vents at all high points and at the anchorages, so as to facilitate grouting. Drains may be provided at low points. In any case, in cold weather, care shall be taken to prevent water from collecting and freezing in the ducts. The locations of the vents and drains shall be as shown on the Drawings.

Cored and external ducts shall be watertight when tested under an internal water pressure of 0.35 MPa.

The vents and drains shall be 13 mm minimum diameter flexible tubes. The connections to the ducts shall be made with metallic structural fasteners, or an alternate approved by the Ministry Representative.

416.12.01 Internal Tendons – The sheaths for post-tensioning ducts shall be made of galvanized steel or plastic. Plastic sheaths shall meet the requirements of SS 416.12.02. The sheaths shall be corrugated, and chemically non-reactive with concrete, tendons, or grout. The inside cross-sectional area of the sheaths shall be at least two times the area of the prestressing steel.

Rigid steel sheaths shall have a wall thickness of at least 0.60 mm and shall be capable of bending to an inside radius of 9 m without distress. Semi-rigid steel sheaths shall have a wall thickness of at least 0.25 mm and shall be capable of bending to an inside radius of 3.50 m without distress.

The ducts shall be spliced with telescopic sleeves. The joint lines between the ducts and the sleeves shall be wrapped with tape or heat-shrink material to make the connections mortar-tight.

416.12.02 External Tendons – Unless otherwise specified in the Contract, sheaths for external post-tensioning shall be made of plastic. Plastic sheaths, including their splices shall be made of high density polyethylene conforming to ASTM D3350, cell classification 334420C. The polyethylene sheath shall be manufactured in accordance with ASTM D2239.

Plastic sheaths shall not be used when the radius of curvature of the tendon is less than 10 m. Sheaths and their splices for external post-tensioning shall be smooth, seamless and capable of withstanding a grouting pressure of at least 1 MPa.

416.13 Condition – Tendons, ducts, anchor assemblies and other accessories shall be free of grease, oil and other foreign matter. A light coat of rust will be permissible provided any loose rust is removed and the surface of the steel is not pitted.

416.14 Epoxy – Epoxy applied to match-cast segment ends shall be as specified by AASHTO LRFD Bridge Construction Specifications, Section 8.13.7.

416.15 Grout – Cement, water and admixtures for grout shall conform to the requirements of CSA A23.1, Clause 4.2. Unless otherwise specified, grout for post-tensioning shall have a compressive strength of at least 20 MPa at 7 days and 35 MPa at 28 days. The Contractor shall propose a design mix for the grout and submit it for review and acceptance by the Ministry Representative before grouting may begin.

The Contractor shall supply all materials for the grout.

The grout shall have a maximum water/cement ratio of 0.45. The grout shall consist of a mixture of Type GU (general

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use) cement, water and an approved admixture to control expansion and pumpability. The admixture shall be an aluminum powder base premixed material. Type HE (high early strength) cement may be substituted for Type GU during low temperature periods. The Contractor shall determine the dosage of the admixture by using volume change and bleeding tests conducted before grouting commences. The Contractor shall make test grout cubes and deliver them to a testing agency acceptable to the Ministry Representative.

EQUIPMENT

416.21 Stressing Equipment

416.21.01 Supervision – The Contractor shall provide a supervisor skilled in the use of the system of post-tensioning to be used.

416.21.02 Gauges and Calibration – The tensioning system shall be equipped with analog or digital gauges that have been calibrated for the specific jacks and controls by an approved testing agency within the last 6 months. The dials of the analog gauges shall be at least 150 mm in diameter. Before stressing commences, certified calibration curves shall be furnished by the Contractor.

The tensioning system or tensioning devices shall be recalibrated if during the work any gauges give erratic results.

416.22 Grout – The grout shall be mixed in a shear type (colloidal) mixer with sufficient capacity to guarantee a continuous supply of grout to the pump. The accessory equipment to batch all materials shall accurately measure all materials.

The pump shall be a positive-displacement type and shall produce an outlet pressure of at least 1 MPa. The pump shall have seals to prevent introduction of oil, air or other foreign substance into the grout and to prevent loss of grout.

The grouting equipment shall utilize gravity feed to the pump inlet from a hopper attached to and directly over it. The hopper must be kept partially full of grout to prevent air from being drawn into the ducts.

The grouting equipment shall contain a screen having clear openings of 2.5 mm maximum to screen the grout prior to its introduction into the grout pump. A screen opening of 5 mm is satisfactory if a grout with a thixotropic additive is used. This screen shall be readily accessible for inspection and cleaning.

The Contractor shall have on site a pressure gauge with a full-scale of no greater than 2 MPa. The Ministry Representative may request the Contractor to install the gauge in the grout line between the pump outlet and the duct inlets to confirm pumping pressures.

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The grouting equipment shall be capable of continuously grouting the largest tendon on the project in 20 minutes.

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416.31 Anchorages – The spacing between anchorages shall be sufficient to allow the operation of the stressing jacks to be unimpeded by the adjacent tendons.

The axis of the tendons shall be in line with the anchorages for a minimum of 400 mm.

The load from the anchoring devices shall be effectively distributed to the concrete.

Anchorages shall be rigidly held in position when the concrete in the section is placed. Anchorages shall hold the tensioned tendons without slip of more than 6 mm; otherwise detensioning will be required.

416.32 Ducts – Vents and drains in the ducts shall be mortar-tight, taped as necessary and shall provide means for injection of grout and for sealing. The ends of vents and drains shall be removed 25 mm below the concrete surfaces after grouting has been completed. If drains are used, they shall be vented downward and shall remain open until grouting commences to permit the removal of any water that may be present in the duct.

416.33 Placement and Protection of the Tendons and Other Accessories – The plan and profile location of the ducts shall be as shown on the Drawings. Ducts shall be securely held at intervals of 1000 mm or less to prevent displacement or flotation during the placement of concrete. They shall be placed with a tolerance of ± 6 mm in both the vertical and the horizontal directions. The location of the ducts shall permit the post-tensioning steel to follow the prescribed profiles.

The anchorage assemblies shall be placed so that the bearing surfaces are concentric with and perpendicular to the tendons within $\pm 1^\circ$.

Immediately before concrete is placed, the post-tensioning steel profile and alignment shall be checked and corrected where necessary. The ducts, grout fittings, vents and drains shall be inspected, and damage shall be repaired to prevent concrete from leaking into the ducts.

The grout fittings and the ducts shall be adequately protected from collapse and other damage. Grout openings, vents and drains must be securely anchored to prevent displacement during concrete placing operations.

The ends of the ducts shall be covered to prevent the entry of water or debris after the installation in the forms.

The ducts shall be blown out with compressed, oil-free air immediately after the completion of the concrete pour. The compressed air shall be used to the extent necessary to break up and remove any mortar in the ducts before it hardens.

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The ducts shall be blown out clear of any water or debris again prior to the placing of the tendons.

Duct ends at match-cast segment ends shall be fitted with gaskets or other sealing devices to prevent infiltration of epoxy during assembly. A uniform pressure of approximately 0.3 MPa shall be applied to the joint during the epoxy curing period. The pressure may be supplied by stressing a cable to a predetermined level, or by other means.

Segmental section joints of 10 mm to 70 mm wide shall be filled with a sand-cement mortar. Joints over 70 mm wide shall be filled with concrete. The strength of the mortar or concrete used in joints shall be at least equal to the parent concrete. The Contractor shall colour the mortar or concrete to match the parent concrete. The Contractor shall submit a trial mix with a proportion of white Portland cement, if necessary, to the Ministry Representative for review.

The concrete shall be vibrated with particular care at each cast-in-place end block (push-through diaphragm) and post-tensioning anchorage location. This will ensure adequate consolidation in the anchorage zones.

If the post-tensioning steel is installed prior to placing of concrete in the cast in place elements, then the prestressing steel in each tendon shall be pulled immediately after concreting, to ensure that it is free to move and is not bonded by concrete or grout leakage.

416.34 Stressing – The structure shall be stressed only after the concrete has reached its transfer strength as shown on the Drawings. No tendons shall be stressed until authorized by the Ministry Representative.

Tendons shall be stressed from the end as shown on the Drawings.

The supports under the segmental sections shall be able to accommodate horizontal movements caused by the stressing.

The multiple strands composing a tendon in a common duct shall be tensioned simultaneously. Tendons shall be tensioned in sequence as specified in the reviewed procedure. The tendon force at lock-off shall be as shown in the procedure.

Before commencing the stressing operation an initial tensioning force shall be applied to each tendon to equalize stresses and to eliminate slack. This force shall not exceed 20% of maximum jacking force.

After the initial tension forces have been applied, reference points shall be established for each tendon from which elongation by final tensioning forces will be measured.

The specified tensioning force shall be maintained steady in the strands until they are anchored. The sequence of tensioning shall be such that no undue tension stresses occur in the concrete near the ends of the members.

The tensioning force shall be determined by the calibrated gauge and by the elongation of the strands. When there is a difference of over 5% between the two means of calculation, the cause of the discrepancy shall be ascertained and corrected.

Tendons shall not be subjected to jacking forces in excess of 85% of their specified ultimate tensile strength. Any tendon subjected to such forces will be rejected.

The required tendon elongation and jacking force shall be computed, based on the required prestressing force and the estimated prestress losses.

The Contractor shall mark the ends of each strand adjacent to the chuck wedge after stressing is complete.

The safety of the stressing procedure shall be in accordance with [the Occupational Health and Safety Regulation](#).

Stressing records shall be kept by the Contractor. Stressing records shall clearly identify the tendons that were stressed, the sequence, the final jacking loads, the tendon elongations, the times, the temperatures and the dates. Stressing records shall be signed by the person supervising the stressing and submitted to the Ministry Representative.

416.35 Grouting – The temperature of grout shall be controlled by water temperature. Grout shall not be warmer than 30°C or colder than 5°C at the pump. The grout shall be mixed for a minimum of 2 minutes. The materials shall be batched in this order: water, cement and admixture. The admixture shall be added in accordance with the manufacturer's instructions.

The grout shall be mixed to a uniform consistency of thick cream without lumps or undispersed cement. The grout shall be agitated continuously and shall be recirculated when not injecting from the line.

Water shall not be added to the grout to increase the flowability that was decreased by delayed use of the grout. The grout shall be discarded when flowability has so decreased it cannot be pumped satisfactorily.

The time between the tensioning and grouting of a tendon shall not exceed 4 days.

Cored ducts with concrete walls shall be flushed with clean water to ensure that concrete is thoroughly wetted prior to grout injection. The ducts for external tendons shall be checked for leakage and blockage by flushing with clean water under pressure followed by oil-free compressed air immediately prior to grouting. Metal sheaths for internal ducts shall be flushed to clean out foreign materials, or oil free compressed air shall be used to check for blockages.

All grout and high point vent openings shall be open when grouting starts. Grout shall be allowed to flow from the first vent until any residual flushing water or entrapped air has been removed. The vent shall be capped or otherwise closed

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at that time. The remaining vents shall be closed in sequence in the same manner.

The pumping pressure at the tendon inlets shall be at least 0.7 MPa and shall not exceed 1.5 MPa.

If the pumping pressure exceeds the maximum allowable, grout may be injected at any grouted vent, as long as a one-way flow of grout is maintained. If this procedure is used the vents that are to be used for injection shall be fitted with positive shutoffs.

The grout shall be pumped through the ducts and continuously wasted at the outlet pipes until no visible slugs of water or air are ejected. The efflux rate of the ejected grout shall not be less than the influx rate of the injected grout.

To ensure that the tendons remain filled with grout, the outlets shall be closed, and the pressure held for 10 seconds. The grouting ends shall then be plugged. Plugs, caps or valves shall not be removed or opened until the grout has set. Alternatively, the outlets and inlets shall be kept under a 1000 mm head of hydrostatic pressure. The 1000 mm grout head shall be maintained until the grout has hardened.

If the grouting operation is interrupted or it cannot be completed due to mechanical failure or some other situation, then the grout shall be immediately flushed out of the ducts with water when one-way flow of grout cannot be maintained. Standby water flushing equipment, with its own source of power, shall be available in addition to the grouting equipment. It shall be capable of developing a pressure of 2 MPa, and sufficient capacity to flush out any partially grouted enclosures. After flushing, the ducts shall be blown out with oil-free compressed air and then checked for blockages. The grouting procedure shall be repeated if there are no blockages.

The ducts shall be completely filled with grout. The presence of ungrouted lengths of duct shall be cause for nonacceptance of the members. Members with partially ungrouted duct(s) shall be replaced or repaired to the satisfaction of the Ministry at the Contractor's expense.

Great care shall be taken to prevent blockages of the ducts due to grouting of nearby ducts or other causes.

All costs for investigations and for remedial measures due to blockages shall be at the Contractor's expense.

The grouting shall not be done when air temperatures are above 32°C or below 5°C, nor when freezing temperatures are predicted within the next 24 hours.

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The temperature of the concrete shall be 2°C or higher from the time of grouting until site-cured cubes of grout reach a compressive strength of 6 MPa.

The grouted ducts may be subject to non-destructive testing inspection by the Ministry to ensure that all ducts are fully grouted. The cost of the inspection will be borne by the Ministry if the inspection indicates that ducts are completely filled with grout. Otherwise, the costs of inspection and the cost of reinspection shall borne by the Contractor.

416.36 Cropping – The cropping of post-tensioning strand shall be by a cold-cutting procedure. It shall not take place until authorized by the Ministry. The strand marks placed at end of stressing shall be inspected to ensure that no slippage of strand has occurred prior to cropping. Under no circumstances shall heat be applied to post-tensioning strand from cropping or other operations. The cropped strand shall be provided with a minimum concrete cover of 25 mm.

416.37 Anchorage Recesses – The anchorage recesses shall be cleaned and sandblasted after completion of post-tensioning and grouting. The surfaces shall be coated with an epoxy bonding agent conforming to ASTM C881, Type V, Grade 2. The Class of bonding agent shall be appropriate for the temperature at time of application. The bonding agent shall be applied in accordance with the manufacturer's applications. Wire mesh acceptable to the Ministry representative shall be placed before concreting in recesses larger than 300 mm square.

The Contractor shall fill the recesses flush with adjoining surfaces. All parts of anchorages shall have a minimum cover of 25 mm. All recesses exposed to view shall be filled with concrete coloured to match the parent concrete. If requested, the Contractor shall submit a trial mix with a proportion of white Portland cement to the Ministry Representative for review.

PAYMENT

416.90 Payment – Payment for Manufacture and Installation of Post-Tensioned Concrete Members will be made at the Lump Sum Price bid. Payment shall include supply and pumping of grout into the post-tensioning ducts, patching of the anchorage recesses, supply and placing of elastomeric pads and strips and grout between stringers, and supplying, placing and tensioning of strands. Payment shall also include splicing of precast segments (if required) and the supply of all material and labour.

SECTION 418

SILANE TREATMENT OF NEW CONCRETE BRIDGE SURFACES

418.01 Scope – This Section describes the treatment of new concrete bridge surfaces with a silane protective solution. The Specification describes surface preparation and application.

Surfaces to be treated shall be as specified on the Drawings or in the Special Provisions.

418.02 Silane Solution – The silane solution shall be carried in a solution to meet the Canadian Environmental Protection Act, Volatile Organic Compound (VOC) Limits for Architectural Coatings Regulations, SOR/2009-264.

418.02.01 Delivery and Storage of Silane Solution – The silane solution shall be delivered to the Site in unopened, sealed containers with original seals intact, also with a manufacturer's label identifying the product (including a batch number). The containers holding silane solution shall be stored in dry, enclosed structures to protect the contents from misuse, weather and excessive heat.

418.02.02 Quality Assurance – Only silane solutions listed in the Recognized Products List shall be permitted for use in the Work.

The Recognized Products List may be found on-line at:

<https://www2.gov.bc.ca/gov/content/transportation/transportation-infrastructure/engineering-standards-guidelines/recognized-products-list>

Certification shall be required for each batch shipment of silane solution, prior to its use.

418.03 Contractor's Qualification – The applicator shall have a minimum of three years of successful and documented experience in application of liquid water repellents to concrete or masonry substrates, or else the applicator must provide documentation certifying that training approved by the manufacturer for applying silane solution was received. Certification shall be provided to the Ministry Representative upon request.

418.04 Site Conditions

418.04.01 Substrate Condition – Prior to silane treatment:

- (a) New concrete shall be cured for a minimum of 28 days.
- (b) All concrete surfaces shall be fully dry.

418.04.02 Weather Limitations – Application of silane solution shall be undertaken only during periods when rain or snow has not occurred during the previous two days and no rain or snow is forecast by the local weather office for the next two days.

Application of silane solution shall proceed only when the concrete surface temperature is above 4°C and below 32°C, and windspeed is less than 24 km/hr.

418.05 Surface Preparation – All concrete surfaces prepared for treatment shall be thoroughly cleaned prior to application of the silane solution. The method of cleaning shall be capable of removing loose concrete, curing compounds, laitance, dirt, dust, salt, oil, asphalt or other foreign materials. The cleaning process shall expose concrete pores and capillaries to readily accommodate silane penetration.

The cleaning process shall not damage the concrete surface, remove or alter the existing surface finish, or expose the coarse aggregate of the concrete. The method of cleaning shall be performed in such a manner as to provide a uniform appearing surface colour.

418.05.01 Pre-Surface Cleaning – If necessary, pre-surface cleaning preparations may include the use of solvent and hand tools to remove materials detrimental to the silane treatment of the concrete surface.

418.05.02 Surface Cleaning – Suitable methods to clean the concrete surface are:

- Sand blasting
- Steel shotblasting
- Hydro blasting
- Steam cleaning
- Water pressure washing using a minimum of 3.5 MPa (500 psi) nozzle pressure.

The cleaning method used shall be capable of cleaning concrete surfaces, as specified.

418.06 Silane Application – Silane application shall proceed only if the condition of the cleaned concrete surface is accepted by the Ministry Representative.

418.06.01 Surface Condition – All concrete surfaces shall be fully dry prior to silane solution application. Concrete surfaces previously steam cleaned or water washed must be allowed to dry prior to application of silane solution for:

- (a) Minimum one day during windy or sunny days when air temperatures are over 20°C.
- (b) Minimum two days during calm or cold days when air temperatures are between 4°C to 20°C.

418.06.02 Silane Solution – The silane solution shall be used as supplied by the manufacturer. It shall not be diluted or altered in any way. Water shall not be allowed to contaminate the silane solution.

418.06.03 Application Equipment – The silane solution shall be sprayed on the concrete surface by using low pressure 0.1 – 0.2 MPa (15 – 30 psi) positive displacement airless spray equipment. Suitable equipment is a pressurized garden

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sprayer or drum mounted pump. Large deck areas over 400 m² shall be sprayed using a spraybar type unit.

All spray equipment shall be fitted with fan spray nozzles and the spray adjusted to a "wet spray" condition (i.e., minimum atomization of silane solution).

- (a) **Cleaning of Application Equipment** – All application equipment must be clean and dry prior to use. If equipment is wet, it shall be rinsed with a small amount of methyl hydrate (methanol) prior to commencement of the actual application.

418.06.04 Rate of Application

- (a) **Surfaces Subject to Traffic Wear** – Bridge decks and other surfaces subject to traffic wear shall be treated with silane solution at a rate of 0.40 L/m².
- (b) **Surfaces not Subject to Traffic Wear** – Surfaces not subject to traffic wear (parapets, pier caps, sidewalks, etc.) shall be treated with silane solution at a rate of 0.23 L/m².

418.06.05 Method of Application – Silane solution shall be applied at a coverage rate according to this Specification. Horizontal surfaces shall be uniformly sprayed to saturate/flood the surface. Vertical surfaces should be treated from the bottom up and uniformly sprayed to saturate the surface.

If it is not possible to complete the silane treatment in one application, the boundary between the treated and untreated areas shall be clearly marked.

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418.06.06 Curing – Following the application, the surface of silane treated concrete shall be cured by mist spraying with water after:

- (a) 4-6 hrs, during days with temperatures between 25°C to 32°C; or
- (b) 6-12 hrs, during days with temperatures between 15°C to 25°C; or
- (c) 12-24 hrs, during days with temperatures between 4°C to 15°C.

The traffic may be allowed on silane treated concrete one hour after the water wetting operation.

418.07 Safety and Health Hazards – Personnel exposed to silane solution and methyl hydrate shall wear personal protective equipment such as coveralls, goggles, rubber gloves and proper respirators.

No smoking, open flames, sources of heat or ignition shall be permitted during the construction operation.

Special precautions must be taken during construction to prevent any over spray or spillage of materials on plants and vegetation or into fish-habitat water bodies and work shall be in accordance with SS 165, Protection of the Environment.

Manufacturer's material safety data and handling instructions, including disposal of containers, shall be followed.

418.08 Measurement and Payment – Payment for the concrete deck sealant will be made at the Unit Price bid per square metre for the preparation of the concrete surfaces and the supply, application and curing of the deck sealant.

SECTION 419

CONCRETE BRIDGE DECK WATERPROOFING SYSTEM PREFABRICATED MEMBRANE

DESCRIPTION

419.01 Scope – This Section describes the protection of concrete bridge decks with Rubberized Asphalt Prefabricated Membrane (Prefabricated Membrane) and an overlay of hot applied asphalt pavement.

The area to be covered with Prefabricated Membrane shall be as shown on the Drawings or as described in the Special Provisions.

This Section describes supply, installation, materials, surface preparation, application, and other related requirements.

NOTE: Prefabricated Membrane shall not be used on Ministry concrete bridge decks contaminated with acid-soluble chloride ion (Cl⁻) in excess of 0.10% by mass of cement.

419.02 Applicator's Qualification – Only applicator(s) trained and certified by the Prefabricated Membrane supplier/manufacturer, whose product is used in the Work, will be permitted to install waterproofing membrane and accessories. Certification documentation shall be provided to the Ministry Representative upon request.

Special Provisions may identify alternate acceptable documentation for remote locations and special requirements.

419.03 Quality Control – Installation and quality of the Prefabricated Membrane shall be inspected and corrected by the Contractor in accordance with SS 419 and the supplier/manufacture'r's requirements. If there is a discrepancy between the two, the more stringent requirement shall apply. In cases where it may be unclear as to which requirement is more stringent, the Ministry Representative will have the final say.

The Contractor shall provide a copy of the supplier/manufacture'r's quality control certification for each lot of material prior to its use in the Works.

419.04 Quality Assurance – The Ministry will implement a quality assurance program by auditing the Contractor's quality control program and by inspection at its discretion.

The Contractor shall notify the Ministry Representative at least 48 hours before installation of the Prefabricated Membrane. Inspection by the Ministry shall not relieve the Contractor from obligation to perform the work in accordance with the Contract.

MATERIALS

419.11 General – Materials shall be provided as follows.

419.11.01 Prefabricated Membrane

- (a) **Selection** – The Contractor shall use a Prefabricated Membrane selected from the Recognized Products List (RPL) under the category "Prefabricated Membrane for Concrete Decks".
- (b) **Supplier** – The Prefabricated Membrane shall be supplied by the supplier/manufacture'r of the prefabricated membrane as a complete package.
- (c) **Package** – The Prefabricated Membrane package shall be comprised of the following materials: primer, prefabricated membrane detailing strip, full width prefabricated membrane and mastic.
- (d) **Information Requirements** – Two weeks prior to the application of the Prefabricated Membrane, the Contractor shall provide the Ministry Representative with a list of the supplier/manufacture'r's materials for the selected Prefabricated Membrane and also provide detailed specifications, installation procedures, technical data sheets, safety data sheets and quality control procedures.

419.11.02 Primer – The Primer used in conjunction with the Prefabricated Membrane shall be a rubberized asphalt or polymer dispersed in organic solvent with minimum solids of 45% (by mass), and maximum drying time of one hour.

419.11.03 Prefabricated Membrane Detailing Strip – Prefabricated Membrane Detailing Strip (Membrane Detailing Strip), shall be a self-adhering internally reinforced sheet of rubberized asphalt and shall have thickness between 1.50 and 1.75 mm (60 to 70 mils) and shall be supplied in rolls of 300 mm width.

419.11.04 Full Width Prefabricated Membrane – Full Width Prefabricated Membrane shall be a self-adhering internally reinforced sheet of rubberized asphalt. Prefabricated Membrane shall have a minimum thickness of 1.5 mm (60 mils) and shall be supplied in rolls of width between 900 mm and 1500 mm.

419.11.05 Mastic – Mastic shall be a single component material, consisting of rubberized asphalt, organic solvent and fillers, supplied in caulking tubes.

419.12 Delivery and Storage – Prefabricated Membrane and accessory materials shall be delivered to the Site in unopened packages and containers with manufacturer's label identifying the product and a batch number.

The packages and containers shall be stored in a dry secured place protected from weather and excessive heat. Specific manufacturer's instructions for handling and storage shall be followed.

CONSTRUCTION

419.31 Traffic Restrictions - All traffic, other than construction equipment directly associated with the installation of the Prefabricated Membrane and placement of asphalt pavement shall be prevented from using areas of the deck that have received surface preparation until paving of that area is completed.

419.32 Job Site Conditions - In addition to the supplier/manufacturer's specifications, the following conditions shall apply.

419.32.01 Weather Limitations - Application of Primer and installation of Prefabricated Membrane and pavement overlay shall be undertaken only during stable weather when precipitation has not occurred during the previous 48 hours and is not imminent and when the minimum ambient and concrete surface temperature is at least 5°C and rising.

419.32.02 Substrate Conditions - Prior to the application of Prefabricated Membrane, new concrete shall be cured for a minimum of 28 days or as otherwise required by the Ministry Representative to ensure the extent of substrate moisture from the hydration process is not trapped beneath the Prefabricated Membrane nor restricting the penetration of the Primer.

419.33 Surface Preparation - All concrete surfaces for application of a Prefabricated Membrane shall be sound, smoothed to a profile of less than 2 mm, clean, and free from any contamination.

419.33.01 Preparation Conditions

- (a) All defects in concrete surfaces, cracks, delamination and spalled areas shall be repaired with a suitable patching material. All patching shall be cured to the acceptance of the Ministry Representative.
- (b) All protrusions shall be removed, and all edges shall be rounded to a minimum radius of 10 mm.
- (c) Concrete bridge decks and concrete-filled shear keys between prestressed concrete box stringers shall have smooth finish. Broom finishes are not acceptable.
- (d) All loose aggregates, laitance, dust, soil, oil and other debris and contaminants shall be removed from the concrete surface by one or more of a combination of the following methods:
 - (i) high pressure clean dry air sweeping;
 - (ii) sand blasting;
 - (iii) steel shot blasting;

(iv) hydro blasting using a minimum of 17.5 MPa (2500 psi) nozzle pressure; and

(v) steam cleaning.

The method used shall be capable of removing concrete laitance and cleaning concrete surfaces as specified to meet the Prefabricated Membrane manufacturer's requirements for a clean and smooth surface.

419.34 Priming - All concrete deck surfaces that are to receive Prefabricated Membrane must be treated with a Primer. Primer shall be used as supplied by the manufacturer. It shall not be diluted or altered in any way.

419.34.01 Concrete Surfaces - All concrete surfaces shall be clean and fully dry prior to application of Primer. Concrete surfaces previously steam cleaned, or water washed must be allowed to dry prior to application of Primer for:

- (a) minimum 24 hours during windy or sunny days when air temperatures are over 20°C.
- (b) minimum 48 hours during calm or cold days when air temperatures are between 5° to 20°C.

419.34.02 Primer Application - Primer application shall proceed only if the condition of cleaned and repaired concrete surfaces is approved by the Ministry Representative.

All concrete surfaces to be covered with Prefabricated Membrane shall be uniformly treated with Primer at the rate recommended by the supplier/manufacturer (typical rates are 0.15 to 0.25 L/m², depending on the porosity of the concrete).

The Primer shall be uniformly applied to the concrete deck surface by roller or brush.

419.34.03 Primer Curing - Only as much surface as will be covered by Prefabricated Membrane in the workday shall be primed. Primed concrete surfaces not covered by Prefabricated Membrane within the workday must be reprimed immediately prior to Prefabricated Membrane installation.

All primed areas must be tack free prior to application of the Prefabricated Membrane.

419.35 Prefabricated Membrane Installation

419.35.01 Detail Work - Extra attention must be given to all detail work which shall be performed prior to final installation of the full-width membrane.

The work shall be completed in accordance with SS Drawings SP 419-01 through -08, other related Special Provisions and as follows:

- (a) **Curb or Parapet** - inside corners at curbs or parapets shall be covered using a Membrane Detailing Strip. The uppermost edge of the Membrane Detailing Strip

shall be placed and extended up the face of curb or parapet to 6 mm (1/4") below the finished height of the asphalt pavement overlay and terminated in a 12 mm deep saw cut in the face. The Membrane Detailing Strip shall be installed in such a manner to ensure it is smooth and that it adheres strongly to the face of curb or parapet, and bridge deck without any visible defects ("tenting", "fish mouth" or "bubbles"). The first sheet of the full width of the Prefabricated Membrane shall then be applied as close as possible to the curb or parapet.

- (b) **Bridge Deck Edge** – on bridge decks without curbs or parapets, the full width Prefabricated Membrane or Membrane Detailing Strip shall terminate 6 mm (1/4") from the outer edges of the deck. The Prefabricated Membrane shall be heated at the edge of the deck with light flame from a torch or with a hot air gun, followed by pressing the Prefabricated Membrane down with a roller.
- (c) **Drains** – areas around drains, basins and any other protrusions shall be double covered with Prefabricated Membrane. First, flashing pieces of Membrane Detailing Strip are applied, followed by the application of the full width Prefabricated Membrane. The Prefabricated Membrane shall be installed in such a way as not to obstruct openings of drains and basins.
- (d) **Weep Holes** – do not require a Membrane Detailing Strip. The diameter of weep holes should be 25 mm (1"). The Prefabricated Membrane around weep holes shall be heated with light flame from a torch or hot air gun, and pressed down to expose the opening of the hole. Where the Drawings do not indicate weep holes, the Ministry Representative may identify locations for weep holes at approximately every 3m along the edge of the curb or deck and the work to install these additional weep holes will be paid for as Extra Work.
- (e) **Shear Keys, Joints and Cracks** – All shear keys, joints with a gap less than 6 mm, and all filled cracks wider than 3 mm shall be covered with a 300 mm wide Membrane Detailing Strip prior to application of a full width Prefabricated Membrane. Joints wider than 6 mm shall be treated as a standard expansion joint and Prefabricated Membrane shall be terminated at the expansion joint assembly.
- (f) **Bridge Ends** – A 150 mm wide Membrane Detailing Strip shall be installed over the joint between the existing pavement and the bridge deck. At each end of the bridge deck, a full-width Prefabricated Membrane shall extend at least 900 mm over the paved bridge approaches. In the case of Prefabricated Membrane application on an existing structure with gravel approaches, the Membrane Detailing Strip of Prefabricated Membrane shall be folded 150 mm over the deck edge and covered with a full width

Prefabricated Membrane. See SS Drawings SP419-05 to SP419-07.

419.35.02 Installation of Prefabricated Membrane – The full width Prefabricated Membrane shall be applied in a longitudinal direction from the low point to the high point so that laps shed water. The area along the lower edge of the deck shall be covered first.

The full width Prefabricated Membrane shall be laid flat, avoiding wrinkles and entrapped air. Each succeeding sheet of Prefabricated Membrane shall be applied overlapping the side laps a minimum 65 mm and the end laps a minimum 150 mm. The entire surface of the Prefabricated Membrane, especially laps, and all terminations shall then be rolled with sufficient pressure to affect a bond between the Prefabricated Membrane and the concrete surface.

419.35.03 Mastic Application – All Prefabricated Membrane terminations shall be sealed with a trowelled application of mastic 12 to 25 mm wide and 1.5 to 3.0 mm thick. Mastic shall be applied along the edge of the Prefabricated Membrane at the face of the curb or parapet, along both sides of the deck joints, around drains and weep holes and at termination of the Prefabricated Membrane on both ends of bridge approaches.

At the end of each working day, or when application is interrupted by a change in construction scheduling or weather, all Prefabricated Membrane terminations shall be sealed with the mastic.

419.35.04 Inspection and Repairs – Prior to paving over the Prefabricated Membrane, any defective areas of the Prefabricated Membrane shall be repaired to the satisfaction of the Ministry Representative, including:

- (a) tears, damaged areas and inadequately lapped seams shall be patched;
- (b) fish mouth shall be slit and repaired;
- (c) large blisters shall be punctured and repaired; and
- (d) if water migrates underneath the installed Prefabricated Membrane, then the Prefabricated Membrane covering the affected area shall be removed and replaced.

419.35.05 Traffic on Prefabricated Membrane – Traffic shall not be permitted on the Prefabricated Membrane prior to installation of the first lift of asphalt pavement with the exception of vehicles and equipment engaged in installing the asphalt pavement. Only vehicles or equipment provided with pneumatic tires or with flat tracks with rubber bonded pads, shall be permitted on the Prefabricated Membrane.

Any turning or sudden braking on the Prefabricated Membrane shall be avoided. Any damage to the Prefabricated Membrane resulting from the movement of vehicles or equipment shall be repaired at the Contractor's expense.

419.36 Paving – Asphalt overlay construction shall proceed only when the final condition of the installed waterproofing Prefabricated Membrane is accepted by the Ministry Representative.

Prior to commencing asphalt pavement construction, the weight of equipment, including loaded trucks, paver and rollers, and their maximum combined weight must be accepted by the Ministry's responsible Bridge Engineer. Only this accepted equipment and equipment combination shall be permitted on the bridge structure at the time of construction.

Asphalt materials, mix design, mix production and hot mix asphalt pavement construction shall meet the requirements of SS 502 and SS 952. Special Provisions will identify any changes to SS 502 bonus/penalty requirements for the bridge application.

419.36.01 Asphalt Mix and Pavement Design – Asphalt mix shall meet the requirements for Pavement Class_1, Medium Mix and appropriate asphalt binder. The asphalt pavement mix shall be paver laid and compacted in two nominal 50_mm lifts.

419.36.02 Placing the Asphalt Mix – In addition to standard paving practice:

- (a) paving shall only proceed over dry Prefabricated Membrane;
 - (b) no asphalt primer shall contaminate the Prefabricated Membrane surface;
 - (c) paving operation shall commence as soon as possible after the waterproofing Prefabricated Membrane is installed;
 - (d) paving shall proceed in the direction of the end laps in the Prefabricated Membrane (i.e., from overlap to underlap); and where more than one width of pavement is to be laid the first width shall be laid starting at the lower edge transversely (i.e. on super elevated bridges the first mat of pavement is to be started at the lower edge of the deck to avoid shifting of the mat on the membrane).
 - (e) the Contractor shall take due precautions to avoid damage to the waterproofing Prefabricated Membrane during the paving operations by the paver and hauling trucks;
 - (f) the paver shall move continuously and at constant speed avoiding any unnecessary stopping or turning;
 - (g) hauling trucks shall maintain a steady supply of asphalt mix to the paver;
 - (h) excessive accumulation of asphalt mix in the paver hopper and auger shall be avoided;
- (i) unloading of the asphalt mix from the hauling trucks onto the Prefabricated Membrane surface will not be permitted;
 - (j) paver screed burners shall be set in such a way as to avoid any damage to the Prefabricated Membrane from excessive heat;
 - (k) the path in front of the screed side end plate must be kept continuously free from spilled asphalt mix. The bottom of the side end plate shall slide freely, otherwise the entrapped asphalt mix could damage the underlying Prefabricated Membrane;
 - (l) screed adjustments during paving shall be made in such a way as to achieve a specified thickness of mat, surface texture, smoothness and designed pavement geometrics;
 - (m) the bridge deck joints, drains and other openings (except weep holes) shall be protected to avoid intrusion of asphalt during the paving operation; and
 - (n) during a "one lane at a time" construction method, as on bridges partially open to traffic, the constructed pavement mat shall terminate not less than 150 mm from the edge of installed Prefabricated Membrane in order to provide a sufficient area for lapping of the next sheet of the Prefabricated Membrane.

419.36.03 Asphalt Pavement Compaction – In addition to standard compacting practice:

- (a) drum vibration mode must not be used during compaction.
- (b) steel drum shall run as close as possible to the outer edge, curb or parapet. Outer edge, curb or parapet areas that are inaccessible by the steel drum shall be compacted by a plate compactor.
- (c) compaction along the end asphalt plug fillet, expansion joints or concrete dams shall be performed transversely with a steel drum or plate compactor if access by the steel drum is not practicable.
- (d) sudden stops or sharp turns by rollers shall be avoided.

419.36.04 Hand Work – Placing of asphalt mix in areas inaccessible to large paving equipment shall be done in accordance with SS 502.

419.37 Protection of the Environment – The Contractor shall comply with SS 165, appropriate environmental requirements of Fisheries and Oceans Canada, the Ministry of Environment, and any related Municipal Regulations.

Special precautions shall be taken during the construction operations to prevent spillage of materials into water bodies.

419.38 Safety and Health Hazards – The Contractor shall obtain all relevant Safety and Material Data information and appropriate handling instructions on Prefabricated

Membrane and accessory materials from the Supplier/Manufacturer to comply with required health and safety regulations.

The Contractor shall comply with the Workers Compensation Act, the Occupational Health and Safety Regulation and manufacturer's requirements for handling and installing Prefabricated Membrane and accessory materials as these materials have flammable, toxic, hot and other potentially harmful properties to workers and the public. The Contractor shall ensure personnel wear and use protective equipment.

MEASUREMENT

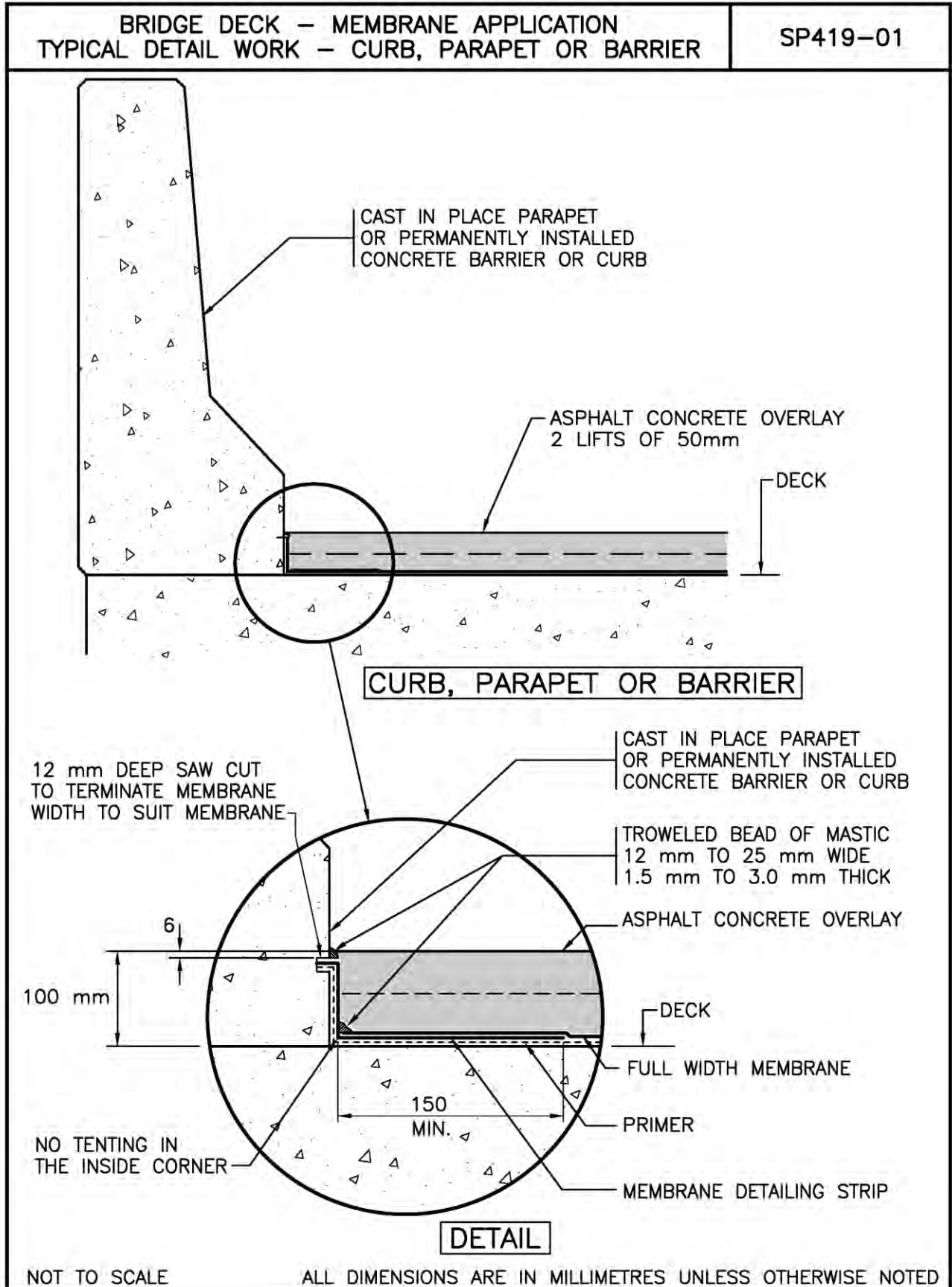
419.80 General – Prefabricated Membrane will be measured in square metres installed. The area measurement for payment will be based on the actual plan area covered by the Prefabricated Membrane waterproofing system, excluding laps and vertical application areas.

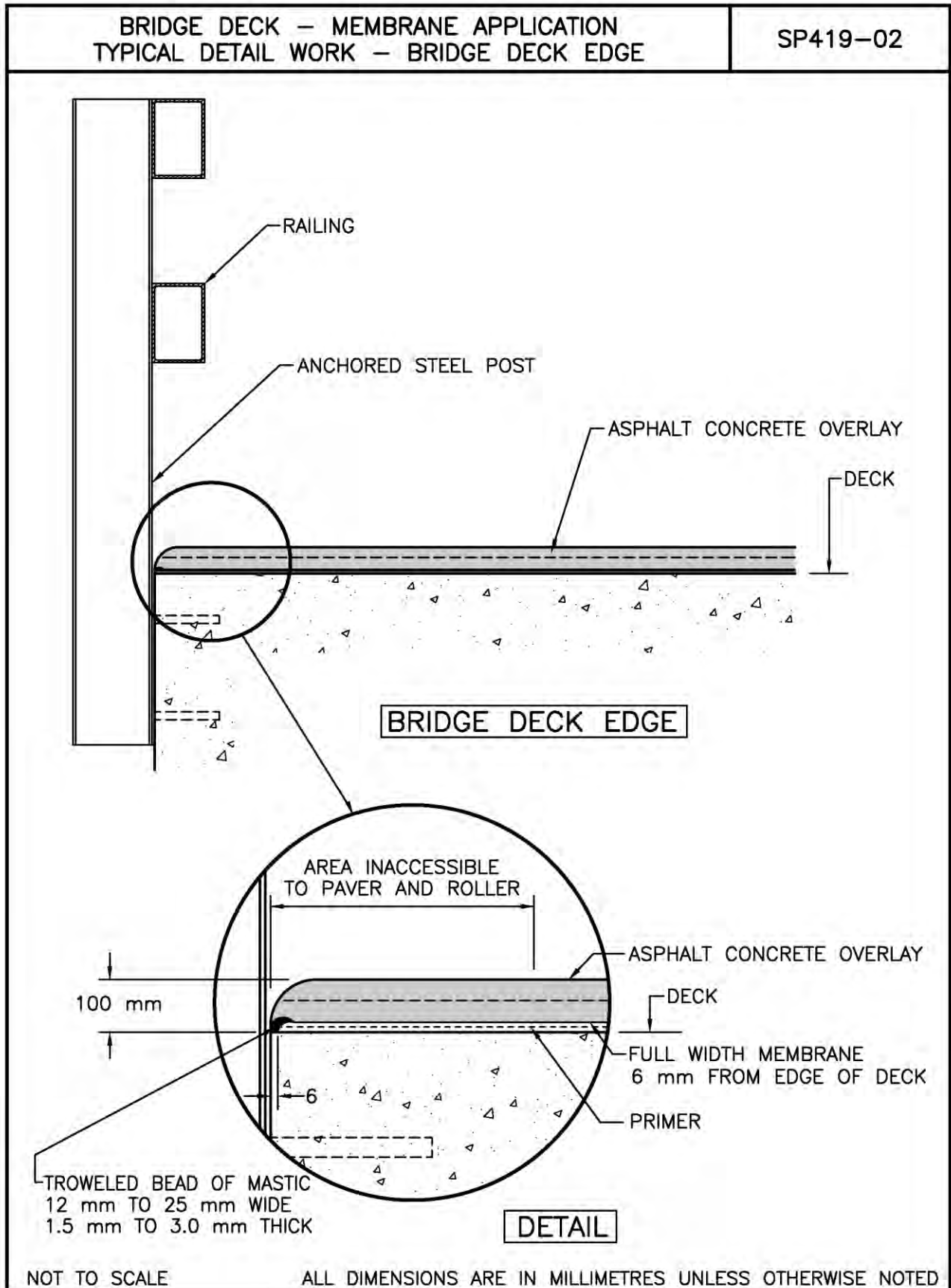
Asphalt pavement will be measured in tonnes in place.

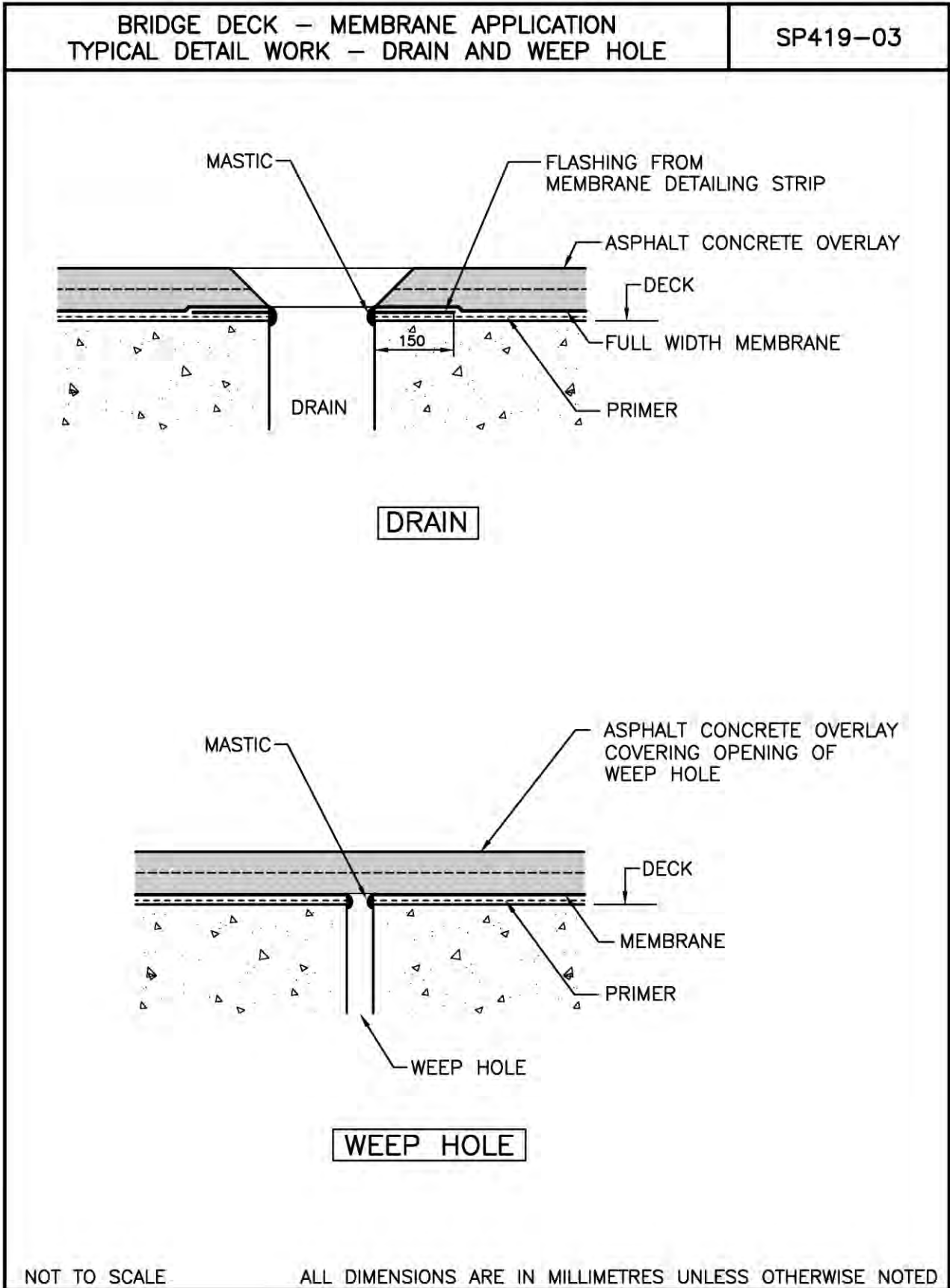
PAYMENT

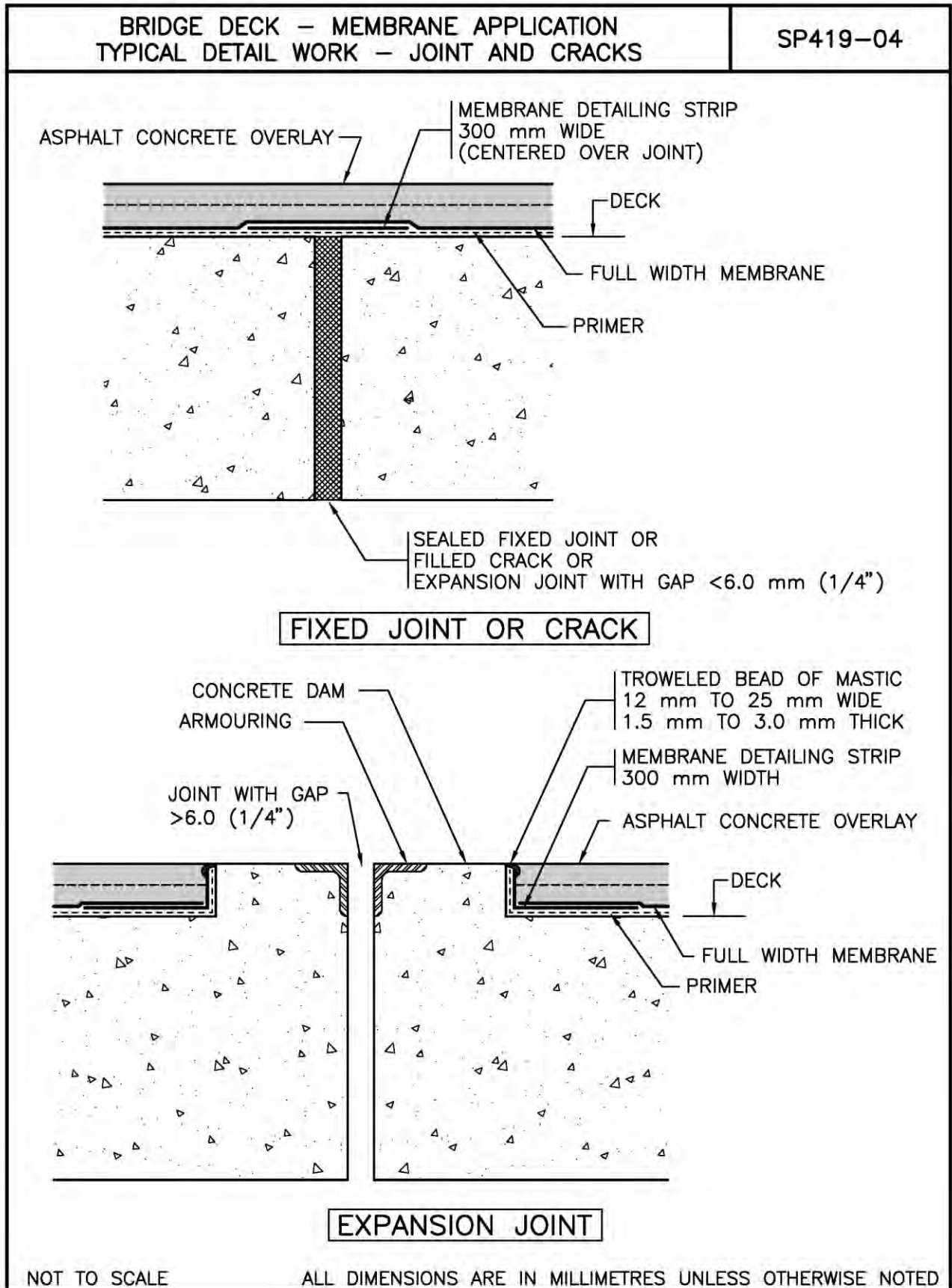
419.90 General – Payment for Prefabricated Membrane will be made at the Unit Price bid per square metre. The Unit Price shall include everything done and furnished for the full and complete supply and installation of the Prefabricated Membrane including bridge deck preparation. Payment will include the preparation of the concrete deck surface and the supply and installation of weep holes, Primer, Membrane Detailing Strips, full width Prefabricated Membrane, mastic, and related work for the supply, application and installation of the Prefabricated Membrane.

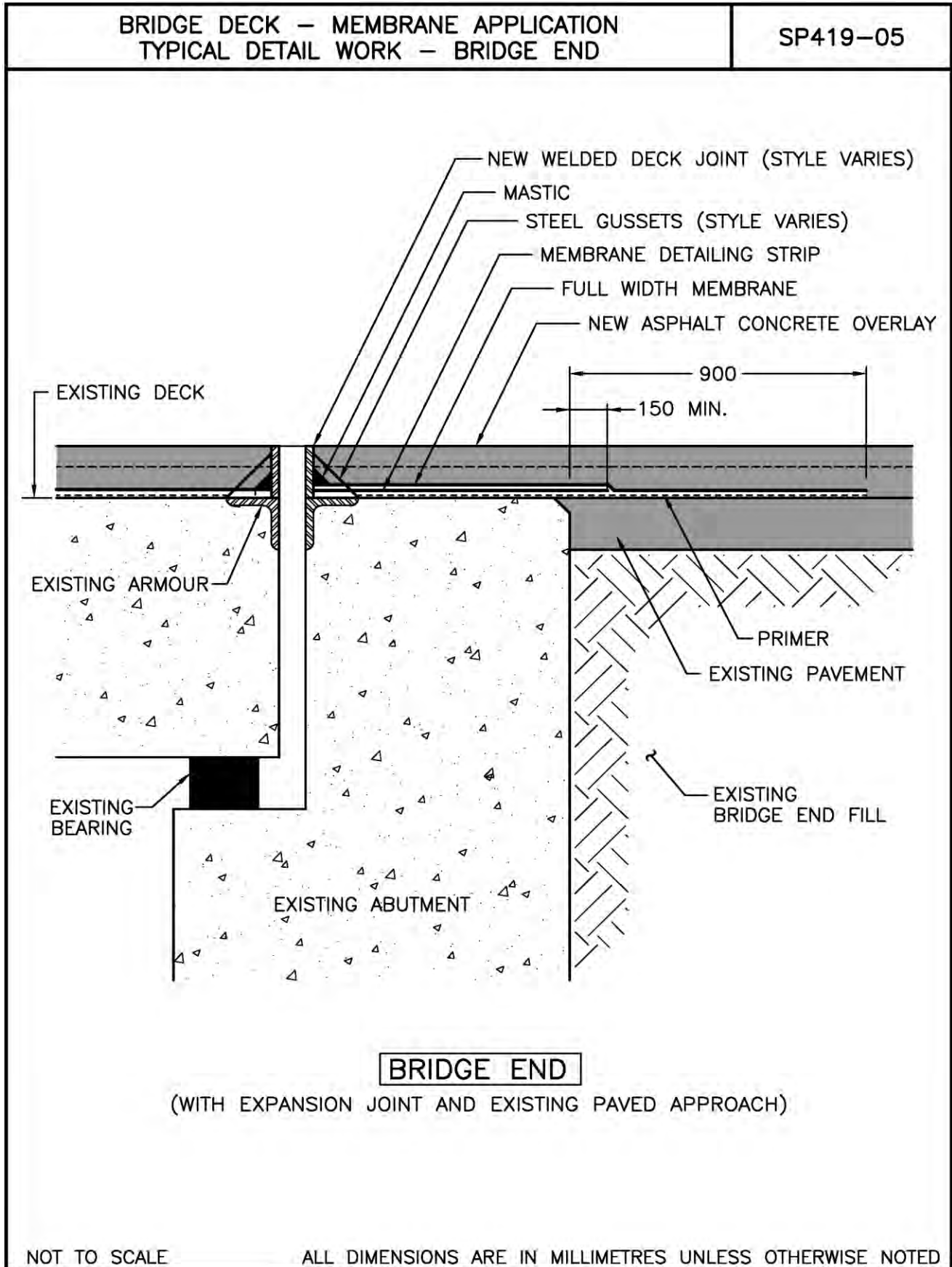
Payment for the Asphalt Pavement and related works to meet the requirements for SS 502 and SS 952 will be made at the Unit Price bid per tonne. Payment shall include pavement mix design, supply of asphalt cement and asphalt mix aggregates, quality control, feeding, blending, mixing, loading, weighing, hauling, dumping, spreading, compacting and finishing of the asphalt pavement overlay and quality control.

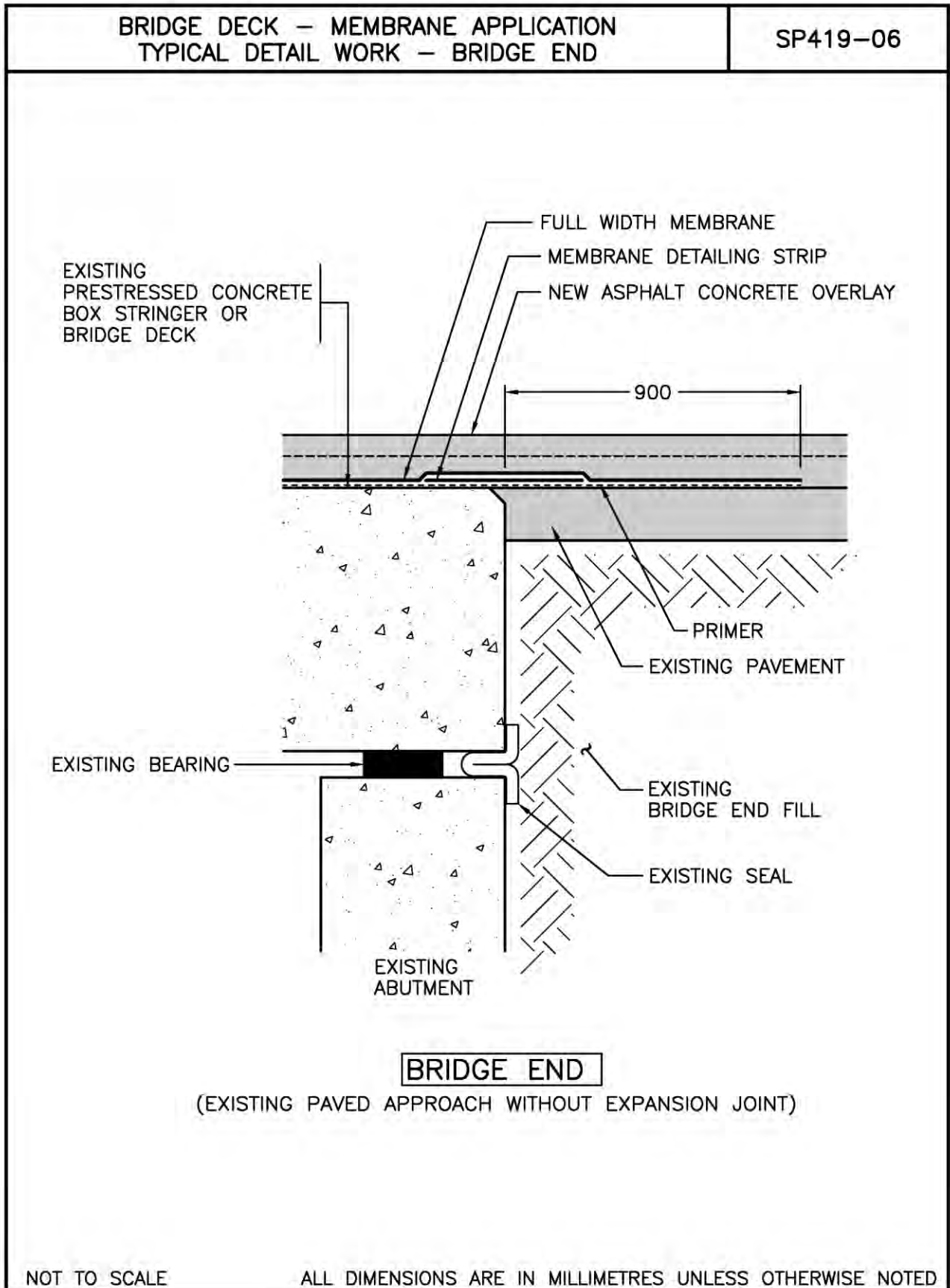


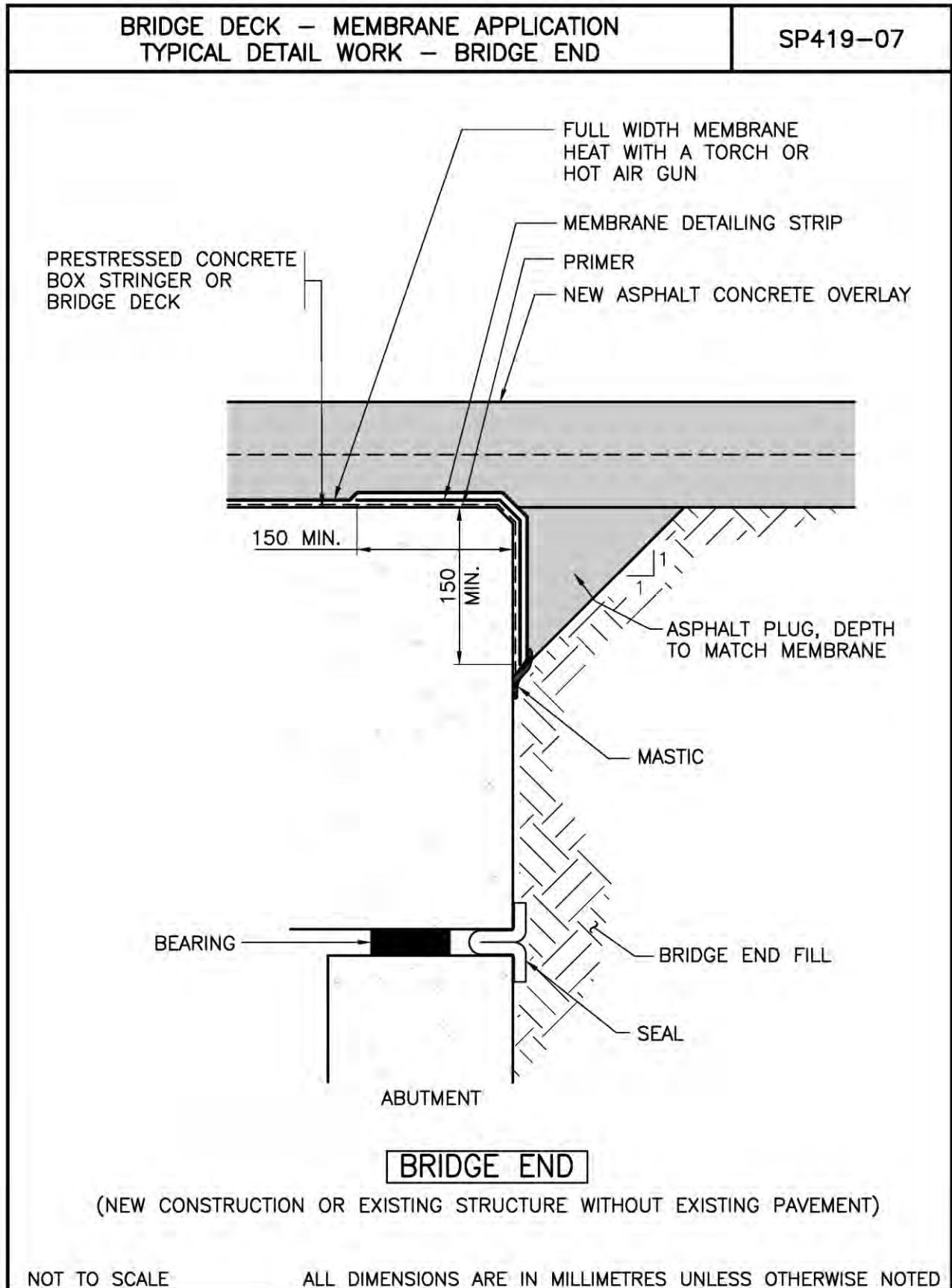


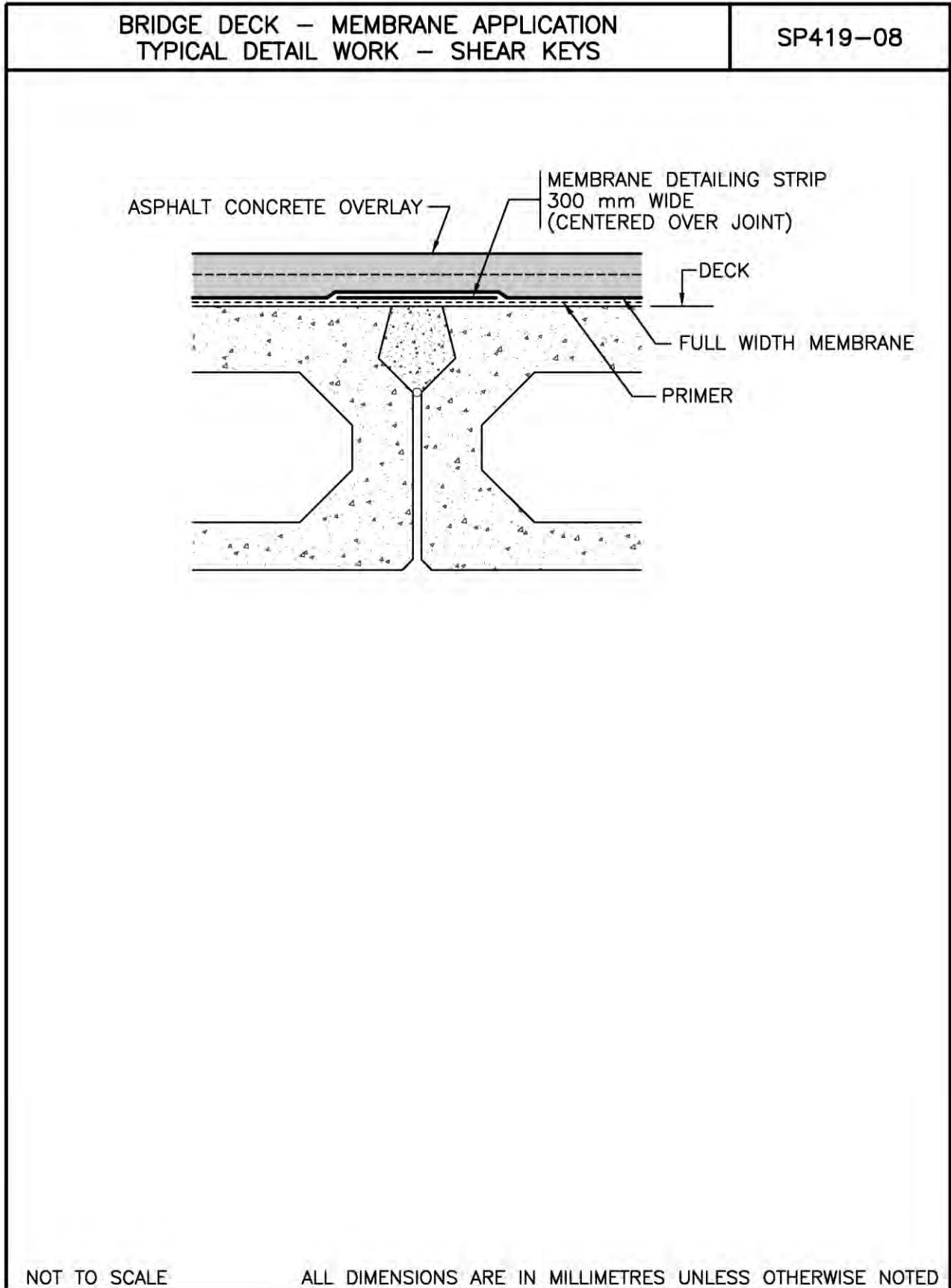












SECTION 420

HOT APPLIED WATERPROOFING MEMBRANE AND ASPHALT OVERLAY

SECTION UNDER DEVELOPMENT

SECTION 421

STRUCTURAL STEELWORK

DESCRIPTION

421.01 Scope – This Section covers the supply, fabrication, erection and quality control program of all major structural steel elements.

In general, all shop connections shall be welded, and all field connections high-strength bolted.

Slip-critical bolted connection locations shall be as identified on the Drawings, Special Provisions or as directed by the Ministry Representative.

Except as otherwise specified herein, steelwork shall be fabricated and erected in accordance with the AASHTO LRFD Bridge Construction Specifications, Division II, Section 11. Welding and associated Work shall be in accordance with the current edition of CSA W59 – Welded Steel Construction. Fracture control requirements, welding requirements for fracture critical members and welding requirements for primary tension members shall be in accordance with CSA S6 and the Ministry Supplement to CSA S6.

421.02 Prefabrication Meeting – The Ministry will at its discretion, convene a prefabrication meeting with the Contractor to confirm the Ministry’s requirements and to review issues such as but not limited to, Schedule, lines of communication, fabricator and sub-fabricator scope of work, location of all work, procedures on quality control, welding procedures, procedures for non-destructive testing, mill certificates and heat numbers, splices, coatings, and any other specific requirements of this specification as it relates to the Work.

421.03 Alternative Details – All details shall, in general, conform to those shown on the Drawings. Any proposed variation shall be submitted in accordance with the provisions of SS 125 – Value Engineering Proposals. The submission of drawings showing alternative details shall be prepared and sealed by a professional engineer registered with the Association of Professional Engineers and Geoscientists of the Province of British Columbia (APEGBC).

Any consideration or implementation of a submitted VEP by the Ministry will also be in accordance with SS 125.

421.04 Working Drawings – Working drawings shall consist of the following:

- Shop drawings
- Transportation details and
- Erection drawings

Transportation details and erection drawings shall be prepared and sealed by a professional engineer registered with the

Association of Professional Engineers and Geoscientists of the Province of British Columbia (APEGBC).

When the Contractor is responsible for the design of items that are detailed on the shop drawings, the shop drawings shall be prepared and sealed by a professional engineer registered with APEGBC.

Working drawings shall be in the same system of units as the design drawings.

Lettering for notes and dimensions shall be at least 2.5 mm and 4 mm for headings. Drawings shall be legible when printed on 11” x 17” sheets.

421.04.01 Shop Drawings – Shop drawings shall show all information and details needed for the fabrication of the members including, but not limited to, such items as member shapes and dimensions, camber diagram, complete geometric information that member dimensions and shapes are based on, connection details, material and product standards, mark numbers and general arrangement of member locations, details of attachments, fastener details, weld details, mass of members, special tolerances, special handling instructions, coating details, lifting details and lifting locations.

A copy of the shop drawings shall be available at all times at the location where the components shown on the drawings are being fabricated. Changes to the steelwork from what is shown on the reviewed shop drawings, or repairs made during fabrication and/or construction, shall be indicated by the Contractor on a marked-up set of shop drawings in digital format and submitted to the Ministry Representative at the completion of the Work.

421.04.02 Transportation Details – Transportation details shall include such items as:

- Description of hauling and handling equipment
- Weight of members
- Length and height of loads
- Location and method of member support
- Details for handling, storing, and loading of members.

421.04.03 Erection Drawings – Erection drawings shall show in detail the method of erection including, but not limited to, the following:

- Erection procedures
- Procedures for off-loading of members upon delivery
- Details for temporary storage and support of members on site prior to erection
- Equipment to be used

SECTION 421

STRUCTURAL STEELWORK

- Layout or general arrangement drawing showing the layout of the members, equipment positioning, and access roads
- Crane make, model, and capacity charts, boom length(s), crane placement, and access for transporting of members to crane(s)
- Radii and loads for crane lifts
- Rigging details
- Mass of members, rigging and special installation equipment
- Details for installation and removal of all falsework, temporary supports, temporary bearings, bracing, guys, dead-men, and lifting devices
- Attachments to the bridge members and bridge structure for temporary support and special launching equipment
- Detailed description of sequence of operations
- Details for special installation equipment such as a launching truss, launching nose, head frames, spreader beams and rollers
- Details for installation of members onto the permanent bearings
- Traffic control plan for roadway and rail traffic
- Details for protection of existing utilities affected by the erection procedures
- Layout and details of fall protection and their sequence of installation.

The Contractor shall be responsible for the lateral stability of members and shall design and provide bracing as necessary until completion of the Work.

The erection drawings shall be complete in detail for all anticipated phases and conditions during erection and during the temporary support of members. The Contractor shall submit calculations, upon request, to the Ministry Representative that demonstrate that specified factored demand/capacity ratios or allowable stresses are not exceeded in members, falsework, temporary bracing and temporary supports and that member capacities and final geometry will be correct. These calculations shall be sealed by the professional engineer who sealed the erection drawings.

Falsework, temporary supports and temporary bracing shall meet the requirements of CSA Standard S269.1, “Falsework and formwork” and shall also meet all the requirements for falsework given in Clauses 20.17 to 20.26 inclusive of the WCB Occupational Health and Safety Regulation.

A professional engineer registered with APEGBC shall be responsible for any field designs and any changes made to the erection procedures. Field designs and changes to the erection procedures must be documented and sealed by the responsible

professional engineer and must be available at the Site prior to the affected erection Work being carried out.

Immediately before placement of loading on falsework, the Contractor must ensure that the falsework is inspected and a sealed engineering certificate is issued by a professional engineer registered with APEGBC which:

- Indicates the specific areas inspected and
- Certifies that the falsework has been erected in accordance with the latest approved erection drawings and supplementary instructions.

421.04.04 Submittals

The Contractor shall submit to the Ministry Representative one set of all working drawings in digital format. Prior to submission to the Ministry Representative, working drawings shall be reviewed and approved by the Contractor. By this review and approval, the Contractor represents that it has determined and verified all field measurements, field construction criteria, materials, and similar data, and that it has checked and coordinated each working drawing with the requirements of the work and the contract documents. The Contractor shall indicate its review and approval by including on each drawing the date and signature of a person designated by the Contractor as being responsible for the Work. Working drawings shall be submitted at least fourteen days prior to the fabrication of the Work and shall be accompanied by a transmittal listing each of the drawings submitted. At the time of submission, the Contractor must notify the Ministry Representative in writing of any deviations in the shop drawings from the requirements of the contract documents. Any Work done or materials ordered prior to the review of the working drawings shall be at the Contractor's risk. The Ministry will review the drawings for general compliance with the contract requirements.

Shop drawings will not be reviewed without the transportation details and erection drawings applicable to the members in question.

Erection will not be allowed to proceed without the Ministry Representative's review of the method proposed.

Review of working drawings shall not relieve the Contractor of responsibility for carrying out the work in accordance with the contract documents.

If so agreed to in advance by the Ministry Representative, working drawings may be submitted in paper format. The reference to working drawing submittal copies shall be increased to four paper copies in this case. At least 14 days before fabrication is to commence, or as otherwise requested by the Ministry Representative, the fabricator shall submit a schedule of fabrication to the Ministry Representative in the form of a Gantt Chart. At the discretion of the Ministry, the schedule shall be updated on no less than a monthly basis. The schedule shall be provided to the Fabricator's Quality Control and to the Ministry Representative for reference and planning of inspections and progress reporting. At the request of the

Ministry Representative, the Contractor shall report any interim variations to the schedule.

421.04.05 Working Drawing Format – The Contractor shall transmit working drawings through attachments to e-mail. Unless otherwise agreed to by the Ministry Representative, electronic attachments to an email must total no more than 7 MB and must be submitted unzipped. Drawing files shall be submitted in Portable Document Format (PDF) to print out on 11” x 17” size pages. PDF sets shall be created by “distilling” CAD sheets rather than by scanning paper plan sets. Unless otherwise agreed to by the Ministry Representative, electronic attachments greater than 7 MB in size shall be sent in two parts by separate emails, denoting “1 of 2” and “2 of 2” in the subject lines after other required subject-line information.

If agreed to by the Ministry Representative, the Contractor may employ a document and data management service such as SharePoint® to transmit working drawings. If this process is used, the limitation on drawing file size is waived. The Contractor shall be responsible for setting up the appropriate folders within the document and data management software and for providing access to these folders to the fabricator, Ministry Representative and the design engineer. The Contractor and the Ministry Representative shall send e-mail notification to each other whenever they post drawings to the document and data management folders.

The resolution of drawings shall be such that the finest detail must be legible at full scale on a 21_inch monitor without zooming in (1_in. width on an 11” x 17” sheet is 1 in. on the monitor).

Drawings shall be black images on a white background.

All PDF sheets shall be assembled within a single file, subject to the maximum file size indicated above, ensuring that all sheets are rotated to a “ready to read” orientation within the PDF file set. Generally, 11x17 plan sheets should be in landscape and 8 1/2x11 note sheets in portrait, so that the majority of text is vertical. PDF sheets shall be ready to print out on appropriately sized paper sheets with no additional formatting required by the viewer.

Drawing sets that are not legible or that do not conform to submission requirements will be returned to the Contractor without being reviewed and the Contractor shall submit four paper copies of the drawing set as substitutes. Drawings of large or complicated pieces, where it is not practical to show all details on an 11” x 17” sheet, may be submitted on full size paper drawings, in which case the Contractor shall submit four copies of the drawing set. When revisions to full size paper drawings are required, the Contractor shall supply four complete sets to the Ministry Representative.

421.05 Quality Control – The Contractor shall implement a quality control program to meet the Contract requirements. The quality control plan shall be made available to the Ministry Representative for review.

421.06 Quality Assurance – The Ministry will implement a quality assurance program by auditing the Contractor's quality control program and by inspection at its discretion.

The Contractor shall notify the Ministry Representative at least 14 days before fabrication has commenced. In addition, the Contractor shall provide a minimum of 48 hours notice to the Ministry Representative that a product will be available for inspection. If the schedule is subsequently changed, the Contractor shall provide the 48 hours notice from the time that the Ministry is notified of this change. If the product is not available or is not sufficiently complete for inspection as notified, at the sole discretion of the Ministry Representative, the Contractor shall be charged stand-by costs for the Ministry's quality assurance inspector. The Contractor shall allow the Ministry representatives safe access to all parts of the Work and shall supply such information and assistance as is required. The Contractor shall provide samples of any materials requested by the Ministry. Inspection by the Ministry shall not relieve the Contractor from obligation to perform the Work in accordance with the Contract.

At its discretion and expense, the Ministry will test any weld by non-destructive testing methods, in accordance with CSA W59. Generally, fillet welds will be tested by the dry powder magnetic particle method (MT) and butt joints by radiography (RT), or by any other method of testing the Ministry deems necessary, such as die penetrant (PT), ultrasonic (UT) and phased array ultrasonic testing methods. Selection of the appropriate testing method will be at the discretion of the Ministry.

The Ministry will attempt to schedule non-destructive testing operations so as not to interfere with the progress of the Work.

The extent of non-destructive testing will usually be in accordance with, for each girder.

The inspection of welds shall be carried out as soon as possible after the completion of welding, with the exception that restrained joints shall be inspected in accordance with the requirements of CSA W59 Clause 12. The Contractor shall provide reasonable notice to the Ministry Representative/Inspector regarding requirement for inspection of welds.

The Contractor shall be prepared to move and support the pieces being inspected. In general, inspection shall be done on the flat and a minimum of 1.25 m of headroom shall be available.

The Contractor shall bear the cost of re-inspection of welds after defects are repaired. Results of re-inspection shall be provided to the Ministry Representative immediately upon completion of the re-inspection work.

The Ministry will inspect bolted connections in accordance with the AASHTO LRFD Bridge Construction Specifications, Section 11.

The Ministry Representative shall be notified of any defects found in the Work. In general, no repair shall be made until

agreed to by the Ministry. In the case of minor corrections, as described by CSA S6-19 Section 10.23.6.4, approval to proceed may be given either verbally by the Ministry's Inspector, or in advance provided that written repair procedures are submitted for Ministry acceptance prior to the Work commencing. In such cases as repair of cracks, or repairs as described by CSA S6-19 Section 10.23.6.5, or a revised design to compensate for deficiencies, the means of correction shall be prepared and sealed by a professional engineer registered with APEGBC. It shall be submitted in writing, with adequate sketches, to the Ministry for review.

The Ministry Representative may reject any items which, in the Ministry Representative's opinion, do not comply with the requirements of the Contract. The Ministry in its sole discretion may back charge all inspection costs for the rejected material to the Contactor.

Each main member or structural component shall be audited by a Ministry quality assurance representative before it is shipped from the shop. This audit shall not relieve the Contractor of responsibility for subsequent damage or for defects which become apparent before the Work is finally accepted by the Ministry. The Contractor shall provide 5 days notice to the Ministry Representative regarding intent to ship a unit or product and the product shall be made available for inspection prior to loading and shipping.

Members shall only be allowed to be shipped out for erection after they have been approved by Quality Control and accepted by the Ministry quality assurance representative. A certificate of compliance may be issued by the Ministry quality assurance representative to acknowledge acceptance.

421.07 Quality and Details of Welds – The quality and details of welds shall be in accordance with CSA W59.

421.08 Design Specifications – CSA S6 and the Ministry Supplement to CSA S6 shall be used in the design of alternative details and calculating the effect of stresses incurred in fabrication and erection.

No increase in allowable stresses due to vertical loads shall be used for erection conditions.

MATERIALS

421.11 Materials

421.11.01 Structural Steel – Steel shall conform to the requirements of CSA G40.20/G40.21 and shall be of the grades called for on the Drawings. Plates provided from coils shall not be used. Prior to fabrication, the Contractor shall supply to the Ministry Representative the manufacturer's mill certificates giving details of all chemical and physical properties of steel to be used in the Work. The boron content of steel shall not exceed 0.0008%.

Chemical composition of steel to be galvanized shall be in accordance with SS 421.42.

Steel shall be supplied free of surface defects and internal discontinuities, with due regard for the end use of the steel in the Work.

Table 421-A: Non-Destructive Testing for Girders

Element	Test Method	Minimum Extent of NDT
Butt Joints		
<ul style="list-style-type: none"> For joints < 65 mm in thickness 	RT	100%
<ul style="list-style-type: none"> For joints ≥ 65 mm in thickness 	UT	100%
Flange/web fillet welds	MT	100%
Flange/stiffener fillet welds	MT	100%
Web/stiffener fillet welds for ½ depth from Tension Flange	MT	100%
Fillet welds for connection plates for ½ of the depth from Tension Flange	MT	100%
Transverse fillet welds on tension flanges	MT	100%
Other fillet welds	MT	25%
Upon removal of runoff tabs at the edge of a flange member	PT	100%

Edges of all plates will be subject to inspection by the Ministry. Any discontinuities will be examined and may be accepted.

The Ministry Representative shall be supplied with a record of all observed discontinuities and repair methods.

Repairs to defective plates shall not proceed until review of the proposed repair method by the Ministry Representative is completed.

421.11.02 Welding Consumables – All electrodes shall match the base metal specified in accordance with CSA W59, Table 12.1. The deposited weld metal shall provide strength, ductility, impact toughness and corrosion resistance equivalent to the base metal.

Welding consumables for all processes shall be certified by the Canadian Welding Bureau (CWB) as complying with the requirements of CSA W48.

421.11.03 High-Strength Bolts – Unless otherwise specified on the Drawings, high-strength bolts, nuts and washers shall

conform to the requirements of ASTM F3125, grade A 325, and as follows:

- (a) for painted steelwork, Type 1 bolts shall be provided and painted after installation in accordance with the field painting specifications.
- (b) for unpainted portions of weathering steel, Type 3 bolts shall be provided.
- (c) for painted portions of weathering steel, Type 3 bolts shall be provided and painted after installation in accordance with the field painting specifications.
- (d) for connections of members that are galvanized, Type 1 bolts hot-dipped galvanized to the requirements of ASTM A153/A153M shall be provided.

421.11.04 Shear Connector Studs – Shear connector studs shall conform to the requirements of ASTM A108, Grades 1015, 1018 or 1020.

421.12 Material Storage and Care

421.12.01 Steel – Structural material, either plain or fabricated, shall be stored above the ground in an upright and shored position upon platforms, skids, or other supports unless otherwise permitted by the Ministry. Should permission be granted to stack the steel in a horizontal position, the Contractor shall provide sufficient support of the steel in order to prevent bending of the plate prior to incorporation into the Work. It shall be kept free from dirt and other foreign matter and shall be protected as far as practical from corrosion. Long members shall be supported on skids placed near enough together to prevent overstress from deflection.

Prior to fabrication, all steel shall be marked for identification by heat number and specification by a marking system approved by the Ministry.

421.12.02 Welding Consumables – Electrodes and fluxes shall be stored and kept in condition as required by CSA W59, Section 5.2.

Gas for welding shall be stored in marked steel bottles and shall not be subjected to temperatures in excess of 50°C or temperatures of less than 0°C.

421.13 Fabrication – Prior to fabrication, the welders and welding operator's qualifications, shop drawings, welding procedures, mill certificates and welding consumable certificates shall be submitted for the Ministry's review.

No fabrication, welding or coating of steelwork shall commence until permission to do so has been received from the Ministry.

421.14 Ministry's Plant Office – The Contractor shall provide in the fabrication plant or nearby a suitable lock-up office for the sole use of the Ministry Representative throughout the period of fabrication.

The office shall be at least 2.5 m x 3 m and shall be weathertight and reasonably soundproof, provided with ample window area and ventilation, furnished with a 0.75 m x 1.2 m desk, a 1 m x 2 m drafting table, a two-drawer filing cabinet, two chairs, a drafting stool and 3 m of shelves, and equipped with a telephone, and high-speed internet access which shall be operational throughout the period of fabrication. All long-distance telephone calls will be paid for by the Ministry.

The Contractor shall clean, heat and light the office throughout the period of fabrication.

EQUIPMENT

421.21 Qualifications and Equipment

421.21.01 Contractor – The Contractor shall produce evidence that the plant is currently fully certified by the CWB to the requirements of CSA W47.1, Division 1 or Division 2 prior to beginning Work.

The Fabricator shall also produce evidence of at least three years of satisfactory experience in the fabrication of bridge structural steelwork.

The Contractor shall employ or retain a registered professional engineer experienced in steel bridge fabrication, to provide guidance throughout the work.

A qualified welding supervisor shall be employed on each shift where welding is done on the work.

Prior to fabrication, the Contractor shall submit to the Ministry the names of the welding engineer, the engineer experienced in steel bridge fabrication, and the welding supervisors who are to be employed on the Work.

421.21.02 Certification – Structural steel elements shall be fabricated in plants that have third party certification under an industry recognized quality certification program specific to steel bridge fabrication.

Acceptable third party certifications include:

- the [Canadian Institute of Steel Construction Steel Bridge Certification – Complex Bridges](#) or
- the [American Institute of Steel Construction Certification Program for Steel Bridge Fabricators](#), in either the intermediate or advanced bridge categories, as applicable to the type of Work.

Certification shall be in effect prior to the beginning of the Work and shall be maintained throughout the period of manufacture.

421.21.03 Welders and Welding Operators – The Contractor shall produce evidence that all welders and welding operators to be employed on the Work are currently qualified by the CWB at the time of fabrication in the processes in which they are to be employed on the Work.

The Contractor shall also produce evidence relative to each welder and welding operator, that they have been executing satisfactory welding in the required processes within the six-month period previous to the award of this Contract.

421.21.04 Welding Equipment – All equipment to be used in the Work shall be in good working order and shall be subject to the inspection of the Ministry.

For any arrangement of automatic welding, the Ministry Representative may require that a preliminary test run of the equipment be made, without welding, over the length of the joint, to prove that the disposition of the equipment and the method and accuracy of travel are satisfactory.

421.22 Welding Procedures – The Fabricator shall submit copies of the proposed welding procedures for review by the Ministry. Gas metal arc welding shall not be used. Where the submerged arc or flux cored arc process is to be used, the Ministry Representative may order that each welder and welding operator make a weld specimen not less than 1 m in length for fillet welds and 150 mm in length for butt joints. Steel of the same specifications and thickness as that to be used in the work shall be used in the specimen welds.

No welding shall be done on the Work until the welders' and welding operators' qualifications are established to the satisfaction of the Ministry Representative.

Welding procedures shall be accompanied by documentary proof that they have been qualified previously by the CWB at the plant where the Work is to be carried out.

The procedures shall include the following information: joint type, welding process, welding position, base metal specification, welding consumable specification and size, preheat requirements, amperage and voltage requirements, speed, polarity, and welding equipment, including a description of travel for automatic welding. Additional information, as described in CSA W47.1, Annex C, shall also be included in the procedures.

421.23 Butt Joints – Except as called for on the Drawings, or as reviewed and approved by the Ministry's design engineer, butt joints will not be permitted. Approval must be given prior to fabrication.

The Fabricator may submit an alternative butt joint design, to that shown on the Drawings, or propose a butt joint design if one is not shown on the Drawings, provided that all such designs have been approved by the CWB. Acceptance of alternative butt joint designs shall be subject to approval by the Ministry. Location and details of all butt joints shall be explicitly shown on the shop drawings.

421.24 Assembly and Welding Sequences – If requested by the Ministry Representative, the Contractor shall supply full details of the proposed assembly and welding sequence of any particular weldment.

CONSTRUCTION

421.31 Preparation of Material

421.31.01 Straightening Material – Prior to being used in fabrication, all structural steel shall be straight and free from kinks or bends. The flatness tolerance of plate in excess of 900 mm wide shall be in accordance with the tolerances of the finished product as specified in CSA W59. If straightening is necessary, it shall be done by methods that will not injure the metal. The steel shall not be heated unless permission is given by the Ministry Representative.

In no case shall the temperature of the steel exceed 620°C. After straightening, the surfaces of the metal shall be carefully inspected for evidence of fracture and if necessary, the material shall be replaced or repaired to the satisfaction of the Ministry Representative. Sharp kinks and bends will be cause for rejection of the steel.

421.31.02 Camber – Girders shall be cambered as shown on the Drawings.

421.31.03 Edge Preparation – Steel may be cut to size by sawing, shearing, plasma cutting or flame cutting. All steel before cutting shall be marked by a method agreed to by the Quality Manager and Ministry Representative so that its specification may be immediately identified.

All cut edges shall be smooth and regular, free from fins, cracks, tears and notches. Freehand cutting shall be done only where approved by the Ministry Representative. Roughness of cut surfaces shall not exceed the American National Standards Institute ASME B46.1 value of 1000. Roughness exceeding this value shall be removed by machining or grinding. Occasional gouges will be tolerated only at the discretion of the Ministry Representative and shall be repaired in accordance with a procedure approved by the Ministry.

Sheared edges of plates more than 16 mm in thickness shall be planed to a depth of 6 mm.

Re-entrant flame cuts shall be cut to a radius of not less than 20 mm.

All exposed edges and corners of members, rolled or cut, which are to be coated, shall be ground to a minimum radius of 2 mm.

All corners of oxygen cut and plasma cut edges of main stress-carrying members including the outside edges of flange splice plates, except unpainted bearing stiffeners and girder webs, shall be ground to a minimum radius of 2 mm.

Special attention shall be given to the cutting of flange plates. All flange plates prepared by flame cutting shall be preheated in accordance with SS 421.34.

The Fabricator will carry out Brinell hardness testing of the edges of flange plates and flange splice plates on fracture critical members. For flange plates, readings shall be taken near both ends and the centre on each side of the plate for a total of 6 readings minimum per plate. For flange splice plates

readings shall be taken near the centre on each side of the plates for a total of two readings minimum per plate. The locations and readings shall be recorded for review by the Ministry Representative. The Brinell hardness of the edges of flange plates and flange splice plates on fracture critical members shall not exceed 220. If greater hardness is measured, the edges shall be ground by the Fabricator to remove the hard layer. The Ministry Representative may conduct Quality Assurance testing of Brinell hardness once the flanges have been prepared to meet the hardness requirements. The Contractor shall bear all costs for re-inspection and retesting.

Surfaces to be welded shall be free from loose scale, slag, rust, grease, moisture or other material that will prevent proper welding. Mill scale that withstands vigorous wire brushing, a light film of drying oil or a thin rust inhibitive coating may remain except that all mill scale shall be removed from the surfaces on which flange-to-web welds are to be made. Surfaces within 100 mm of any weld location shall be free from any paint or other material that would prevent proper welding or produce objectionable fumes while welding.

Edges of material thicker than specified in the following list shall be trimmed if and as required to produce a satisfactory welding edge wherever a weld along the edge is to carry calculated stress:

- (a) Sheared edges of material thicker than 12 mm;
- (b) Rolled edges of plates (other than Universal Mill Plates) thicker than 10 mm;
- (c) Toes of angles or rolled shapes (other than wide flange sections) thicker than 16 mm;
- (d) Universal Mill Plates or edges of flanges of wide flange section thicker than 25 mm.

421.31.04 Direction of Rolling – Steel plates for main members and splice plates for flanges and main tension members shall be cut and fabricated so that the direction of rolling is parallel to the direction of the primary stresses.

421.31.05 Bolt Holes – Standard holes for high tensile bolts shall be either punched, sub-punched and reamed, or drilled, and shall be of a nominal diameter not more than 2 mm in excess of the nominal bolt diameter, except that the following bolt/hole combinations will be permitted:

- (a) either 3/4 inch or M20 bolts in 22 mm holes;
- (b) either 7/8 inch or M22 bolts in 24 mm holes;
- (c) either 1 inch or M24 bolts in 27 mm holes.

Oversize or slotted bolt holes will be allowed only in special circumstances or as shown on the Drawings.

Punched holes shall be clean cut, without torn or ragged edges. The diameter of the die shall not exceed the diameter of the punch by more than 2 mm. If a punched hole must be enlarged to admit a high tensile bolt, it shall be reamed.

Reamed holes shall be cylindrical and perpendicular to the member. Where practicable, reamers shall be directed by mechanical means. Reaming shall be done with twist drills or reamers.

Drilling shall be done with twist drills or core drills. Burrs on the outside surfaces shall be removed.

Poor matching of holes will be cause for rejection.

421.31.06 Faying Surfaces – In the parts of a structure where steelwork is specified to be painted, faying surfaces shall meet the requirements of SS 216.

In the parts of a structure where steelwork is specified to be uncoated, the following shall apply:

- (a) For non-slip critical uncoated connections – faying surfaces shall be free from loose scale, unacceptable burrs, dirt, oil and foreign material.
- (b) For Class A uncoated slip-critical connections – faying surfaces shall have a clean uncoated mill scale surface free from loose scale, paint, lacquer, or any other coating in all areas within the bolt pattern and for a distance beyond the edge of the bolt hole that is the greater of 25 mm or the bolt diameter.
- (c) For Class B uncoated slip-critical connections – faying surfaces shall be blast cleaned to SSPC-SP10 / NACE No. 2.

When assembled, all joint surfaces, including those adjacent to bolt heads, nuts, and washers, shall be free from loose scale, unacceptable burrs, oil, dirt, and foreign material that would prevent the solid seating of the parts.

421.32 Marking – Prior to fabrication, all steel shall be marked for identification by heat number and specification by a marking system approved by the Ministry Representative. Steel which is unidentified shall not be used in the Work.

421.33 Assembly of Weldments – The shop assembly of the various components of the weldments shall be executed in accordance with CSA W59.

Tack welding shall be done by qualified welders, using the smallest size weld required to hold the components of the assembly together. Tack welds shall be incorporated into the final weld.

421.34 Preheat and Interpass Temperatures – No welding shall be done when the temperature of the base metal is lower than -18°C. At temperatures below 0°C, the steel shall be preheated to a temperature of at least 10°C in excess of that stated in CSA W59, Table 5.3.

Preheat shall be applied to all steel to be welded so that the steel within the greater of 75 mm of the weld, or the thickness of the thickest part to be welded, is heated to the temperatures shown in CSA W59, Table 5.3.

Preheat shall be applied in such a manner that moisture from the heating equipment does not penetrate the joint.

Preheat temperatures above the minimum shown in CSA W59, Table 5.3 may be required for highly restrained joints if designated by the Ministry.

Preheat and interpass temperatures for repair of fracture critical members shall be in accordance with CSA S6 Table 10.16.

Preheat temperature shall in no case exceed 200°C.

Preheat requirements for tack welds shall be meet the above requirements except that where single pass tack welds are used and are to be incorporated and consumed in a weld made by the submerged arc process, preheat is unnecessary.

421.35 Welding – Welding shall be done by the shielded metal arc, metal-cored arc, flux-cored arc or submerged arc processes in accordance with the reviewed procedures and CSA W59, Section 5.

Any weld between the web and flange of a bending member shall be made by a mechanized submerged arc process, which provides a continuous weld throughout the length of the member.

All welding shall be done under cover and, in the case of flux-cored arc welding, shall be done in an area free from wind or draft.

Where the submerged arc process is to be used, the Contractor shall:

- (a) Carry out a preliminary test run of the procedure over the length of the joint to prove that the disposition of the equipment, the handling of hoses, and the method and accuracy of travel are satisfactory.
- (b) Have each operator make a weld specimen not less than 1200 mm in length for fillet welds and 150 mm in length for butt welds. Steel of the same specification and thickness as that to be used in the Work shall be used in the specimen welds. No welding shall be done on the Work until such a specimen is satisfactory to the Ministry Representative.

Welds in butt joints shall be extended beyond the edges of the parts to be joined by means of start and run-off tabs providing sufficient thickness to avoid the weld burning through and with a joint preparation similar to that on the main material. Welds that connect run-off tabs to the main member shall be placed inside the joint so that they are incorporated into the final weld. For manual shielded metal arc welding the width of the tabs shall be not less than the thickness of the thicker part being joined or 75 mm, whichever is greater. For submerged arc, welding the width of the tabs shall be not less than 75 mm. Each weld pass shall be carried far enough beyond the edge of the parts being joined to ensure sound welds in the joint. Tabs shall be removed upon completion by flame cutting and grinding. The weld shall be cooled without damage to the parent plate. The end of the weld shall be made smooth and flush with the edges of the abutting parts.

In flux-cored arc welding the equipment shall be capable of sustaining a gas flow rate of from 0.85 to 1.25 m³/h.

Where, in the opinion of the Ministry Representative, excessive repairs are required during the fabrication of components, the fabricator shall submit a plan to the Ministry Representative for review and acceptance, which shows revisions to the fabrication process, including personnel if necessary, to reduce or eliminate future fabrication non-conformances.

421.36 Shear Connector Studs – Shear connector studs shall be welded in the locations shown on the Drawings, to the requirements of CSA W59, Section 6. Shear connectors will be inspected and shall be repaired if necessary in accordance with the same standard.

421.37 High Strength Bolts – Installation of high-strength bolts shall be in accordance with the AASHTO LRFD Bridge Construction Specifications using the “turn of nut” method.

Galvanized bolts shall be lubricated with beeswax or other approved lubricant before installation.

Heads of bolts shall be placed on the outsides of girders and box members, and generally on the more conspicuous side, if any, of any connection.

421.38 Bent Plates – When bending plates, the plates shall be so taken from the stock plates that the bend line will be at right angles to the direction of rolling. Before bending, the corners of the plate shall be rounded to a radius of 2 mm throughout that portion of the plate at which bending is to occur. Bending shall be done by methods that will not crack, tear or otherwise injure the metal.

Cold bending and the minimum radii for cold bending shall be in accordance with CSA S6.

Hot bending of plates shall not be permitted without prior approval of the Ministry. Any proposed method for hot bending of plates shall be submitted to the Ministry for review and approval by the Ministry prior to proceeding with the work.

Bending of flange plates shall not be permitted without prior approval of the Ministry. Any proposed method for bending flange plates shall be case specific and shall be submitted to the Ministry for review and approval by the Ministry prior to proceeding with the work.

421.39 Shop Assembly of Bolted Connections – Holes in girder and truss field splices shall be drilled while assembled in the shop, or sub-punched or sub-drilled and reamed while assembled. Unless otherwise specified, the structure shall be progressively assembled in accordance with AASHTO LRFD Bridge Construction Specifications. The Ministry shall be provided sufficient notice by the Fabricator, to allow inspection of any assembly, including camber, alignment and accuracy of holes before drilling or reaming is commenced.

Connecting parts assembled in the shop for the purpose of reaming or drilling holes shall be match-marked by a method

agreed to by the Ministry to indicate the location and orientation of all pieces.

Alternatively, the Contractor may drill holes full-size using automatic drilling equipment, as described in AASHTO LRFD Bridge Construction Specifications. In this case a check assembly will be required for the first of each major structural type, as described in AASHTO LRFD Bridge Construction Specifications. Reaming of holes shall be as specified in SS 421.47, if the bolt holes do not line up during field assembly.

Progressive assembly in the yard may be allowed, subject to approval by the Ministry Representative, provided the Fabricator can demonstrate that yard assembly will provide as high a quality product as shop assembly.

421.40 Dimensional Tolerances – Except as noted herein, the dimensions of completed members shall comply with the appropriate dimensional tolerances as specified in CSA W59.

Alignment or position of secondary members shall be within ± 6 mm.

The tolerance in flange width shall be $\pm(b/100)$, where b is the flange width, but not less than 5 mm and not greater than 25 mm.

The tolerance on the width of stiffeners and plates for secondary members shall be -3 mm to +10mm.

Misalignment of stiffeners on opposite faces of a web shall be less than one third of the web thickness for bearing stiffeners and half the web thickness for intermediate stiffeners.

The maximum deviation from specified length shall be $\pm L/1000$ but not over 20 mm.

Warpage of box members shall be determined by taking measurements at any two cross-sections in a member, at opposite edges of one face of the member. Warpage is defined as the distance by which any point deviates from a plane defined by the other three points. This warpage shall not exceed 1/200 of the width of the member, or 3 mm, whichever is greater.

421.41 Machined Surfaces – Machine-finished surfaces, as designated on the Drawings, shall be coated with an approved protective compound.

421.42 Galvanizing and Metallizing – All steelwork to be galvanized shall be identified in the Drawings, Special Provisions or elsewhere in the Contract. All steelwork to be galvanized shall be galvanized after complete fabrication to the requirements of ASTM A123M and ASTM A385. The galvanizer shall safeguard against embrittlement as required in ASTM A143. Galvanized members shall be subject, at the discretion of the Ministry Representative, to the tests for embrittlement outlined in ASTM A143.

The chemical composition of steel being galvanized shall be as follows:

- Carbon less than 0.25%

- Phosphorus less than 0.04%
- Manganese less than 1.3%
- Silicon less than 0.04% or between 0.15% and 0.22%

For steel not meeting these chemical composition requirements, special galvanizing techniques shall be developed by the galvanizer to ensure that the specified coating thickness and adherence is achieved. A detailed description of the special techniques shall be submitted to the Ministry Representative for review two (2) weeks prior to galvanizing.

All steelwork to be metallized shall be identified in the Drawings, Special Provisions or elsewhere in the Contract. All steelwork to be metallized shall, after complete fabrication, be treated in accordance with the current SSPC-CS23.00 / AWS C2.23M / NACE No. 12. The zinc coating shall not be less than 0.3 mm in thickness.

421.43 Shop Painting – The Drawings or Special Provisions shall specify whether the structure is to be painted, or what parts of a structure are to be painted. This subsection applies to those parts of the steelwork which are to be painted.

If painting is specified on the Drawings or in the Special Provisions, the structural steel of weathering steel bridges shall be painted for the larger of the following two distances from deck joint locations:

- 3000 mm; or
- 1.5 times the superstructure depth (including girder, haunch and slab thickness).

Coating work shall be in accordance with SS 216 unless noted otherwise in SS 421. For the short lengths of coating required adjacent to deck joints on weathering steel bridges, third party Quality Control inspectors shall be qualified to at least NACE CIP Level 2 or SSPC BCI Level 2.

All coatings shall be applied according to their manufacturer's product data sheet unless specified otherwise in the Contract Special Provisions.

All steelwork which is to be painted, except as noted herein, shall be given three shop coats of paint – primer, stripe coat and midcoat. Paint shall be chosen from SS 308 System SS1. Paint shall be supplied by the Contractor.

The topcoat may be applied in the shop or in the field. The topcoat coat shall be chosen from either SS 308 System SS1 or SF2. If the topcoat is to be applied in the shop, then the Quality Control Program shall specifically address the integrity of the topcoat through to project completion.

Surfaces surrounding bolt holes at connection locations shall receive the prime coat only and shall be masked off so that no stripe coat, midcoat or topcoat paint will be under the bolt heads, washers or nuts. These masked off areas shall have the stripe coat, midcoat and topcoat paint applied in the field after installation of the bolts.

Faying surfaces shall receive the prime coat only, as per SS 216.12.01(b).

Paint shall be applied in a covered area in accordance with the manufacturer's specifications and SS 216.

All edges, corners, crevices, bolts, nuts, protrusions and welds (unless ground flush) shall be stripe painted by brush for a width of 50 mm with the midcoat paint before the midcoat coat is applied. Stripe coating paint shall be applied as per SSPC-PA1 and may be applied by spray, but shall be brushed in. Stripe painting shall be allowed to cure before the midcoat coat is applied.

Unless a different coating is called for on the Drawings, the exposed steel surfaces of bearings, and bearing assemblies, shall be cleaned and painted as structural steelwork.

Surfaces shall be cleaned to "near-white" per SSPC-SP10 / NACE No. 2 and 50 to 75 μm (2 to 3 mils) sharp/angular profile.

The midcoat shall not be applied until the primer and stripe coat are accepted by the Ministry Representative to be sufficiently cured.

Any surfaces inaccessible after erection, except faying surfaces and tops and sides of top flanges, shall be given the topcoat in the shop.

If the topcoat is to be applied in the shop, then the topcoat shall be applied after the midcoat is accepted by the Ministry Representative to be sufficiently cured.

The colour of the topcoat will be selected by the Ministry. For weathering steel, unless noted otherwise, the topcoat colour shall match the expected colour of the oxidized surfaces (Federal Color 30045 unless noted otherwise). The proposed colour shall be subject to the acceptance of the Ministry Representative.

421.44 Marking and Shipping – Each member shall be marked by a method agreed to by the Ministry Representative with an erection mark, corresponding to the mark shown on the erection diagram.

Members shall be loaded in such a manner that they can be transported to and unloaded at their destination without being damaged.

After steelwork has been delivered to Site it shall be inspected by the Contractor's quality control inspector. The Contractor shall clean the steelwork after it has arrived at Site of any dirt, road salts, slush or other contaminants accumulated during transport and shall carry out any other surface preparation work necessary to meet the specified surface preparation requirements.

421.45 Field Assembly – The parts shall be accurately assembled as shown on the drawings and any matchmarks shall be followed. Hammering which will injure or distort the members shall not be done. Bearing surfaces and surfaces to be in permanent contact shall be cleaned before the members

are assembled. Field connections shall have one half of the holes filled with bolts and cylindrical erection pins (half bolts and half pins) before final bolting. Fitting-up bolts shall be the same nominal diameter as the high tensile bolts, and cylindrical erection pins shall be 1 mm larger.

421.46 Straightening Bent Material During and after Fabrication – The straightening of plates and angles or other shapes shall be done by methods that will not produce fracture or other injury. Any proposed straightening plan, whether by heating and or mechanical straightening methods shall be case specific and shall be submitted to the Ministry Representative for review and acceptance prior to commencing with the work. Generic plans may be referred to as part of the straightening plan. The heating shall not exceed the requirements of CSA W59, Clause 5.15. After heating, the metal shall be cooled as slowly as possible, typically at ambient shop temperature.

Following the straightening of a bend or buckle, the surface of the metal shall be carefully inspected for evidence of fracture, and if necessary, replaced or repaired to the satisfaction of the Ministry. The cost of any non-visual inspection deemed necessary by the Ministry Representative as a result of the straightening process shall be borne by the Contractor.

421.47 Misfits – For all primary connections, and secondary connections having eight or more bolts, 85% of the holes shall accept bolts without reaming. The remaining 15% may be reamed to accept the designed diameter bolts. The diameter of the reamer shall be the same as the drilled hole.

Holes in plates showing more than 5 mm of offset shall be cause for rejection of that plate. New plates, if necessary, shall be field drilled using the hole pattern in the senior member as a template.

421.48 Erection Tolerances – Unless otherwise specified, the assembly tolerances and misalignment of members after erection shall be within the dimensional tolerances specified in CSA W59 and AASHTO LRFD Bridge Construction Specifications.

Misalignment shall be measured from vertical lines in the case of columns or towers, and from lines joining the ends of any test length of a member.

Joints, which are required on the drawings to be milled to bear, shall have at least 75% of the entire contact area in full bearing. The separations of any remaining portions shall not exceed 0.25 mm except locally at toes of flanges where a separation of 0.60 mm is permissible.

Where joints are not milled, the opening shall not exceed 13 mm.

421.49 Field Painting – This section applies to those parts of the steelwork which are to be painted. Coating Work shall be in accordance with SS216 unless noted otherwise in SS 421. Build the coating using coatings from the specified SS 308 System. The coating system shall be from the same

manufacturer as the shop coating system on the steel. After the completion of all deck and overhead concrete Work, steelwork shall be thoroughly cleaned of all rust, dirt, dust, oil and other foreign materials. Non-visible salts shall be removed in accordance with SSPC-SP WJ-4 to meet the NVC levels per SS 216.07.02(b). The shop coats of paint shall be touched up as necessary.

421.49.01 Field Touch-up – Bare, rusty or damaged areas shall be cleaned to SSPC-SP11, Power Tool Cleaning to Bare Metal. Feather edges into the existing coating and build the coating as per SS 216.

After installation, Type 3 bolts, nuts and washers in the areas to be painted shall be cleaned to SSPC-SP11. These bolts, nuts and washers shall then receive the prime coat, stripe coat, midcoat and topcoat paint.

The masked off surfaces surrounding the bolt holes at connection locations shall be cleaned and shall receive the stripe coat, midcoat and topcoat paint.

421.49.02 Topcoat – The topcoat coat shall be chosen from either SS 308 System SS1 or SF2.

If the topcoat has not been applied in the shop, then the steelwork shall be given one topcoat coat of paint applied in accordance with the manufacturer's specifications and SS 216.

The colour of the finish coat will be selected by the Ministry. For weathering steel, unless noted otherwise, the topcoat colour shall match the expected colour of the oxidized surfaces (Federal Color 30045 unless noted otherwise). The proposed colour shall be subject to the acceptance of the Ministry Representative.

At the completion of the Contract, all steelwork, painted or unpainted, shall be cleaned of concrete spatter, mud, oil and other foreign materials.

421.50 Unpainted Weathering Steel – In the case of unpainted weathering steel, the outer faces of the girders and stringers, which includes the exposed edge of the top flange, the underside of the top flange, the girder web, and the top, bottom and outer edge of the bottom flange, and any other surfaces mentioned in the Drawings and Special Provisions, shall present a uniform surface free of mill scale and shall be sandblasted to SSPC-SP6 prior to installation. Cleaning shall also include all shop marks located on the exterior faces of the girders, and in all areas of interior girders that are readily visible to the public, as determined by the Ministry Representative. Concrete splatter adhering to the steel surfaces after the construction of the deck shall be removed and the steel surfaces cleaned.

421.51 Touch-up of Galvanizing and Metallizing – All field welds and other damage in galvanized and metallized coatings shall be touched up as follows:

If the Drawings or Special Provisions call for touch-up by metallizing, the damaged areas shall be locally sandblasted to

"near white" per SSPC-SP10 / NACE No. 2. All dry abrasive blast cleaned areas shall be metallized in accordance with SSPC-CS23.00/AWS C2.23M/NACE No. 12, Specification for the Application of Thermal Spray Coatings (Metallizing) of Aluminum, Zinc, and Their Alloys and Composites for the Corrosion Protection of Steel, to provide a zinc coating not less than 0.3 mm in thickness.

Cut metal edges may be harder and consequently have less profile. If that is the case, they shall be ground back to softer metal and reblasted to achieve the specified profile.

Otherwise, the damaged areas shall be thoroughly cleaned and painted with two coats of Ministry-approved organic zinc-rich paint.

QUALITY CONTROL PROGRAM

421.61 Quality Control Program – These Subsections describe the general Quality Control Program required by the Ministry for any Fabricator undertaking the fabrication of permanent steel bridges and steel bridge components. The Quality Control Program shall be part of the overall Quality Management Plan for the project.

The term "Quality Control" defines those activities that the Fabricator performs to conform to the contract.

The term "Quality Assurance" defines those activities that the Ministry performs to audit the Contractor's quality control program and to ensure conformance to the Contract.

These Subsections contain statements of the Quality Objectives and Policies that the Ministry considers essential for successful and economical quality management. They also outline the Procedures and Documentation to implement and confirm that objectives are met.

The provisions of the Quality Control Program set forth in these Subsections shall apply to all steel bridges and bridge components contracted by or for the Ministry.

These Subsections require the establishment of a Quality Organization with the responsibility for the successful and timely implementation of all necessary Quality Control activities. Some positions shown in the Suggested Organization Chart may be held by the same individual. For example, the General Manager may also be the Contract Administrative Manager and the Purchasing Manager. The Plant Superintendent may also be the Receiver and the Welding Supervisor.

421.62 Quality Objectives and Policies

421.62.01 Quality Objectives – The Quality Objectives of the Ministry cover all steel bridges and steel bridge components produced under contract to the Ministry, as follows:

- (a) Completed products shall conform fully to the governing Codes and Specifications stipulated in the Contract.

- (b) The Quality Control Program shall be fully integrated into the ongoing manufacturing activities of the Fabricator.
- (c) The operations of the Quality Control Program shall protect the interests of the Ministry with respect to scheduled delivery date and contracted price.

421.62.02 Quality Policy – The Quality Policy ensures that the product meets the Quality requirements of the Contract, is delivered on time, and is produced in a cost-effective manner.

The Quality Control Program applies to all stages of the design, drafting, procurement, manufacturing and testing of the product.

A Fabricator's Quality Control Manager shall be appointed with defined responsibilities in resolving quality matters and shall report to a senior management level. At each hold point, the Fabricator's Quality Control Manager shall:

- (a) Document the successful completion of each stage as it progresses through fabrication and erection;
- (b) Identify and report nonconforming components;
- (c) Initiate or recommend disposition of nonconforming components;
- (d) Verify corrections.

Any persons assigned to perform quality control inspections shall be other than those performing or directly supervising the Work and they shall not report directly to immediate supervisors responsible for producing the Work.

The Quality Control Program is not subordinate to any design, drafting, procurement, manufacturing or testing activities.

421.63 Scope of the Quality Control Program – The Quality Control Program governs the fabrication of steel bridges and bridge components for the Ministry. The Suggested Organization Chart is shown in Figure_1.

The Fabricator's General Manager shall be responsible for:

- adhering to the Quality Control Program in all respects.
- ensuring that completed bridges or bridge components shall conform fully to the applicable design and to the fabrication and welding codes stipulated in the Contract.
- ensuring that all required documentation is produced according to the Quality Control Program.

421.64 Range of Capability – The Fabricator shall provide the necessary knowledge, skill (in-house or on a contract basis) and equipment to perform the following work on steel bridges and bridge components:

- Design of connections and joints not shown on Drawings (if applicable);
- Preparation of shop fabrication drawings;
- Preparation of bills of material;

- Preparation of material requisitions and purchase orders;
- Receiving, checking and storing materials for bridges;
- Layout, cutting, forming and fitting of parts;
- Assembly, tacking and welding;
- Dimensional checking and verification;
- Resolution of non-conformances;
- Documentation of all stages of work with capability of tracking all major components;
- Cleaning, painting, storing and shipping;
- Erection of bridges and bridge components (if applicable).

421.65 Drawings and Specifications – The Fabricator's Chief Draftsperson shall:

- obtain the latest revision of the design drawings and specifications for the Work;
- submit shop drawings and erection drawings to the Ministry Representative for acceptance before commencement of the Work;
- prepare material requisitions containing a full description of the material sizes, material specifications, and certifications required for conformance to the Contract;
- deliver the material requisitions to the Purchasing Manager in ample time to permit ordering, delivery and documentation without delaying the Work;
- issue requisitions for all sub-contracted drafting work and shall ensure that all conditions of the Contract are part of such sub-contracts.

The Fabricator's Chief Design Engineer shall be responsible for the design of any connections or joints not shown on the design Drawings; and for the design of the transportation details, erection procedures and any special erection equipment needed.

421.66 Material Control – All materials for the Work shall be ordered by the Fabricator's Purchasing Manager in full conformance with the material requisitions provided by the Fabricator's Chief Draftsperson.

The Purchase Orders shall contain all information necessary to ensure that materials purchased will comply fully with the terms of the Contract. Where mill certificates and test reports are required, it shall be so stated on the Purchase Order. Instructions shall state when the certificates and reports are to be delivered to the Fabricator.

If a supplier proposes a substitute for any material, the Fabricator's Purchasing Manager shall refer the proposed substitution to the Fabricator's Chief Design Engineer for review. If the substitute is acceptable to the Ministry Representative, the Fabricator's Chief Draftsperson shall

amend all drawings and requisitions, withdraw old issues, and issue the new versions.

The Fabricator's Receiver shall:

- inspect all materials on arrival for conformance with the Purchase Orders;
- confirm that mill certificates and test reports are provided and that they correctly identify the materials delivered;
- arrange with the Fabricator's Plant Superintendent to store all materials for the Contract in segregated areas. Clear identification with the Contract shall be provided;
- issue a non-conformance report covering overage, shortage or damage to the materials, and copies of the report shall be provided to the Fabricator's Purchasing Manager and the Quality Control Manager.

The Fabricator's Purchasing Manager shall deliver all documentation to the Fabricator's Quality Control Manager for inclusion in the Quality Control file for the Contract.

421.67 In-Progress Inspection and Reporting – The Fabricator's Quality Control Manager shall ensure that only documented materials are used for the Contract.

All materials intended for incorporation into the Work shall be examined after cutting to size, forming and rolling. The Fabricator's Chief Inspector shall ensure conformance with the detailed shop drawings, shall report any non-conformance to the Fabricator's Quality Control Manager and shall order all work affected by the non-conformance to stop, pending approval of remedial action.

Before assembling any plate girders or complex parts, the surfaces of all materials shall be examined for imperfections revealed during previous fabrication operations. The joint edge preparation for all groove welds shall be verified as conforming to the Fabricator's CWB-approved welding standards and shall be within the acceptable tolerances.

After assembly of any plate girders or complex parts, and before commencing the strength welding, the assembly shall be checked for dimensional conformance. The Fabricator's Welding Supervisor shall ensure that the fit-up of all welded joints conforms to the approved welding standards.

The Fabricator's Quality Control Manager shall file a written report of verification with the Fabricator's General Manager and report any non-conformance.

421.68 Correction of Non-conformance – When a non-conformance is encountered, the Fabricator's Quality Control Manager shall determine a recommended disposition and obtain the Ministry's approval as quickly as possible.

If there is non-conformance to the material specification stipulated in the Purchase Order, the Fabricator's Purchasing Manager shall immediately find out the reasons for the delivery of non-conforming material. If the material is of a grade superior to that ordered, the Fabricator's Quality Control Manager and the Chief Engineer shall be notified.

They shall verify that the material is an acceptable alternative in all respects, and this verification shall include consultation with the Ministry Representative. If the material is of a grade inferior to that ordered, it shall be rejected and the correct material or a superior material shall be obtained.

If there is non-conformance of material delivery that will delay production, the Contract Administration Manager and Fabricator's Plant Superintendent shall be notified immediately. They shall be given revised delivery dates for the adjustment of production scheduling. The Fabricator's General Manager shall determine the alternatives available and shall notify the Ministry Representative.

If there are non-conformities in material preparation, assembly, joint edge preparation and fit-up before strength welding, the Fabricator's Quality Control Manager and Welding Supervisor shall immediately review the non-conformance and notify the Fabricator's Chief Design Engineer who may require further investigation prior to submitting corrective action to the Ministry Representative for approval. If the necessary corrective action will result in delay to production, the Fabricator's General Manager shall be notified for adjustment to the production schedule. The Fabricator's Plant Superintendent shall inform the Fabricator's Quality Control Manager when the corrective actions are being done so that conformance can be verified and the non-conformance report cancelled.

421.69 Welding – All welding on structural and mechanical components shall be done by the company certified to CSA W47.1 (Division 1 or 2) and W186-M as applicable, and shall be done in accordance with the Fabricator's CWB-approved welding standards.

The edge preparations for all groove welds shall conform to the dimensions established in the approved welding standards.

If the joint is to be welded from one side only without back-gouging, the root gap and root face shall be checked to ensure conformance with the required geometry so that the root pass can be successfully completed.

The strength level and chemical composition of all filler materials used in structural and mechanical assemblies shall conform to the approved shop drawings.

All welding consumables shall conform to the approved welding standards and shall be received, stored and conditioned according to the applicable welding standards.

Any preheat required before welding shall be according to the approved welding standards.

The welding procedure followed in welding any joint in a structural or mechanical component shall conform to the applicable Data Sheet in the approved welding standards.

All welders and welding operators welding on structural or mechanical contracts shall be qualified under the requirements of the CSA Standard governing certification.

421.70 Heat Treatment – This covers any post-weld heat treatment necessary to conform to the approved welding standards.

The Fabricator's Chief Design Engineer shall:

- decide whether any structural or mechanical components shall receive post-weld heat treatment to conform to the contract conditions or to the applicable Codes and Standards.
- inform the Fabricator's Chief Draftsperson of any such requirements so that they may be incorporated on to the approved shop drawings.

Any heat treatment stipulated on the approved shop drawings or contained in the approved welding standards shall be carried out at the appropriate time and according to the approved documents.

The Fabricator's Plant Superintendent shall ensure that any heat treatment stipulated is done according to the established procedures, shall obtain all documentation and reports, and shall deliver them to the Fabricator's Quality Control Manager.

421.71 Non-destructive Testing – The Ministry will test any weld by non-destructive testing methods, as described in SS 421.06.

The Fabricator's Plant Superintendent shall schedule the manufacturing operation to facilitate non-destructive testing.

If non-destructive testing of any welded joint reveals imperfections that are marginally more than the acceptance standards, the Fabricator's Quality Control Manager shall consult with the Ministry Representative regarding the location and nature of the imperfections. The effects of leaving minor defects in place shall be assessed in relationship to the loads carried by the joint and the possible adverse effects of making an unnecessary repair.

Weld repairs shall conform to the approved welding standards.

421.72 Calibration of Measurement and Test Equipment – All measurement and testing equipment owned by the Fabricator and used in the Quality Control Program shall be calibrated and re-calibrated at the intervals and in the manner stipulated in the Manufacturer's Instruction Manuals. Any adjusting devices shall be sealed or otherwise protected from unauthorized adjustment or tampering.

421.73 Records Retention – The Fabricator's record file for each Contract shall contain the pertinent drawings, purchase orders, bills of material, material mill certificates, test reports, Quality Control documents, NDT reports and certificates of compliance.

The Fabricator's record file shall be made available to the Ministry's Inspectors upon request.

Items in the Fabricator's record file shall be retained as per company policy.

The Fabricator's Quality Control Manager shall ensure that each file is complete in all respects before it is placed in the Company archives.

421.74 Hold Points – In planning the workflow, the Fabricator shall coordinate with the Ministry Inspector to decide "hold" points (as defined in SS 145.12) for inspection or non-destructive testing. A list of hold points shall be drawn up by the Fabricator and the Ministry Representative at a prefabrication meeting. The Ministry Representative shall be informed of progress so that delays are minimized.

The "hold" points will typically include some of, but not be limited to, the following:

- verification of materials
- after plate is prepared for splicing
- after splicing of plates
- after web to flange weld
- after stiffeners are applied
- camber of girders
- shop assembly
- cleaning and coating
- bearing plate attachment
- shipping arrangements
- subassemblies.

Work shall not proceed past a "hold" point until it has been signed off by Quality Control and Quality Assurance. Reports shall be completed promptly.

421.75 Transportation and Installation Procedures – Transportation and installation procedures shall be prepared and submitted for review before any installation takes place on Site. The procedures shall be sealed by a professional engineer experienced in bridge erection and registered with the APEGBC. Consideration shall be given to the following items during the preparation of the procedures:

421.75.01 Girder Transportation

- (a) Brief description of hauling equipment;
- (b) Location of girder support points;
- (c) Engineering backup if supports vary from specifications; and
- (d) Details of coating protection during loading transporting and erection.

421.75.02 Installation Drawings

- (a) Bridge site plan showing piers, abutments and access roads;
- (b) Crane make, crane chart, boom length(s) and crane locations;

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- (c) Mass of girder and access to crane(s); and
- (d) Special installation equipment such as a launching truss, head frames and falsework.

421.75.03 Commentary – Brief point form description of installation sequence.

421.75.04 Bearings – Placement procedure for bearings to be included for multiple span continuous girders.

421.75.05 Traffic Control – Any arrangements that will be made for road and/or rail traffic.

421.75.06 Utilities – Safety and protection.

421.75.07 Fall Protection – Method and date of installation as required in specifications.

PAYMENT

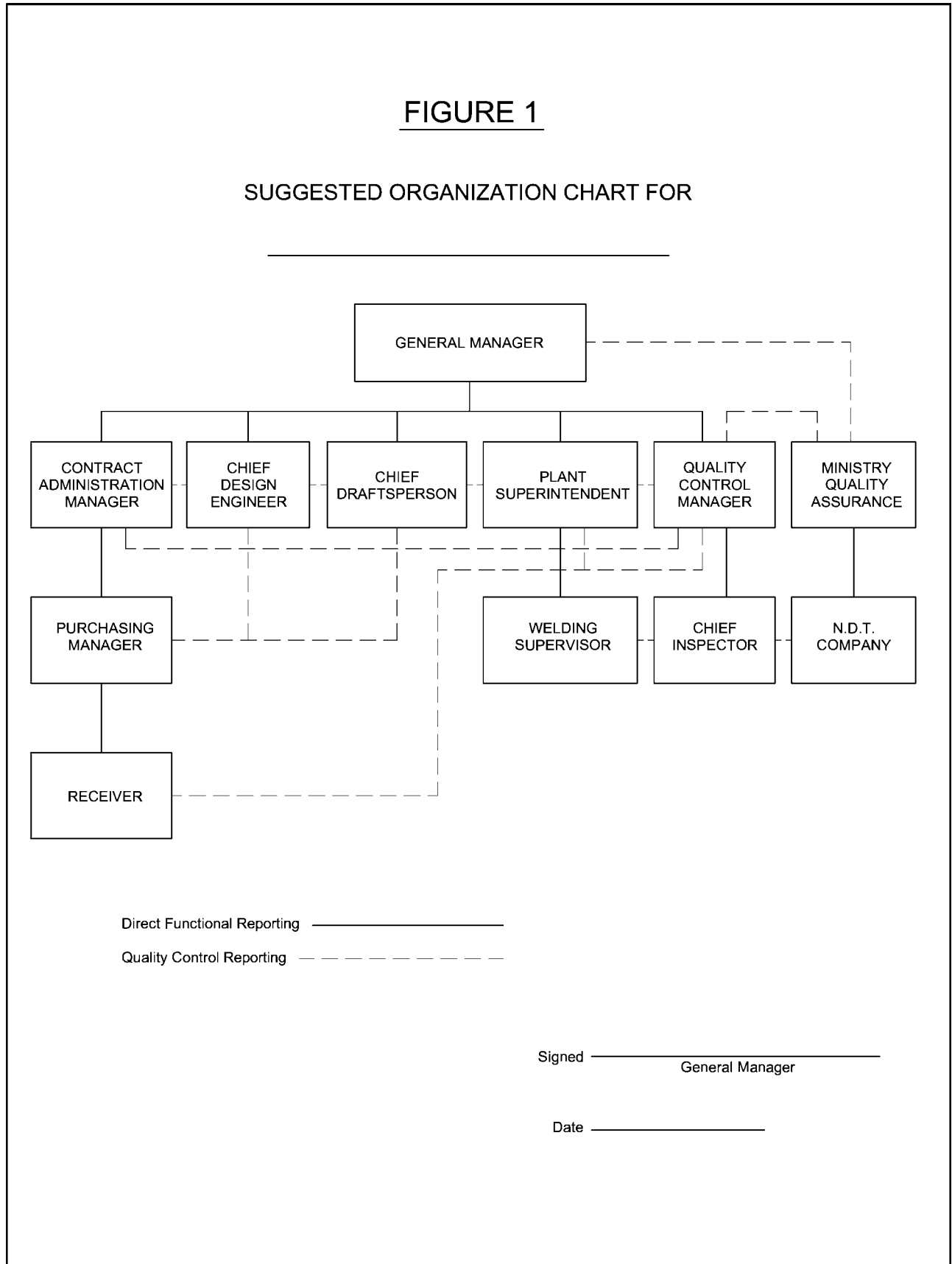
421.91 Supply and Fabrication – Payment for supply and fabrication of structural steelwork will be made at the lump

sum price bid. Payment shall be for quality control, working drawings, the supply, bending, cutting, assembly and welding of steelwork, including any galvanizing or metalizing required. Payment shall also cover trial assembly and storage as necessary.

421.92 Shipping and Erection – Payment for shipping and erection of structural steelwork will be made at the lump sum price bid. Payment shall be for quality control and the loading, shipping and unloading of steelwork. Payment shall also cover falsework if necessary and the erection of steelwork.

421.93 Painting – Payment for painting of structural steelwork will be made at the lump sum price bid. Payment shall be for the surface preparation and the supply, application and touch-up of paint as required.

Figure 1: Suggested Organization Chart



Sample Form 421-1: Receiving Report

<p style="text-align: center;"><u>RECEIVING REPORT</u></p> <p>_____</p> <p>SUPPLIER _____ P.O. No. _____ WORK ORDER No. _____ INVOICE No. _____</p>	<p>DATE: _____</p>	<p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p>	<p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p>	<p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p>
<p><u>QUANTITY</u></p>	<p><u>DESCRIPTION</u></p>	<p><u>MILL CERTS.</u></p>	<p><u>HEAT No.</u></p>	<p><u>VISUAL INSPECTION</u></p>
				<p><u>THICKNESS CHECK</u></p>
<p><u>HOLD MATERIAL:</u> Describe material, problems and procedures.</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p> <p>_____</p>				
<p>_____</p> <p>QUALITY CONTROL APPROVAL:</p>				

Sample Form 421-2: Non-conformance Report

NON-CONFORMANCE REPORT

BRIDGE NAME:

BRIDGE NUMBER:

JOB NUMBERS:

IDENTIFICATION DETAILS:

DESCRIPTION OF NON-CONFORMITY:

20____

Date Signature

CORRECTIVE ACTION:

ACTION AGREED UPON:

Quality Control Supervisor Quality Assurance Inspector

DATE COMPLETED: 20 ____

Signature

HOLD REMOVED BY: 20 ____

Date

QUALITY CONTROL SUPERVISOR: 20 ____

Date

QUALITY ASSURANCE INSPECTOR: 20 ____

Date

Sample Form 421-3: MoTI Steel Fabrication Check List

Ministry of Transportation and Infrastructure
Steel Fabrication Compliance Check List

BRIDGE NAME:

BRIDGE NO.

MARK NUMBER/S:

Q.C. Q.A.

Material: Mill Certificates

(Physicals, Chemicals, Impact Category)

Identification on materials check

Heat number transferred and recorded

Welding: Procedures approved for each process

Consumables checked

Date Completed

Sample Form 421-4: MoTI Steel Fabrication Compliance Check List

Ministry of Transportation and Infrastructure

Steel Fabrication Compliance Check List

BRIDGE NAME: _____ BRIDGE NO. _____

MARK NUMBER/S: _____

Q.C. Q.A.

Welding: Weld preparation and fit up

Welding within parameters

Identification on materials check

Shear studs

Visual Inspection

NDT outside agency results

Q.C. Q.A.

Req'd Actual Verified

Dimensions: Flanges Top Width 1

Width 2

Flatness

Tilt

Bottom Width 1

Width 2

Flatness

Tilt

Web Depth

Section Length

Bearing Ctr. Length

Sole Plate Contract

Camber

Sweep Tolerance

(1/1000)

Splicing Camber

Alignment

Holes

Date Completed

Sample Form 421-5: Coating Compliance Check List

Ministry of Transportation and Infrastructure
Coating Compliance Check List

BRIDGE NAME: _____ BRIDGE NO. _____

MARK NUMBER/S: _____

Q.C. Q.A.

Coating: WHMIS Safety Data Sheets _____

Technical Spec. Sheets _____

Surface Preparation _____

Ambient Conditions – Req'd _____

- Actual _____

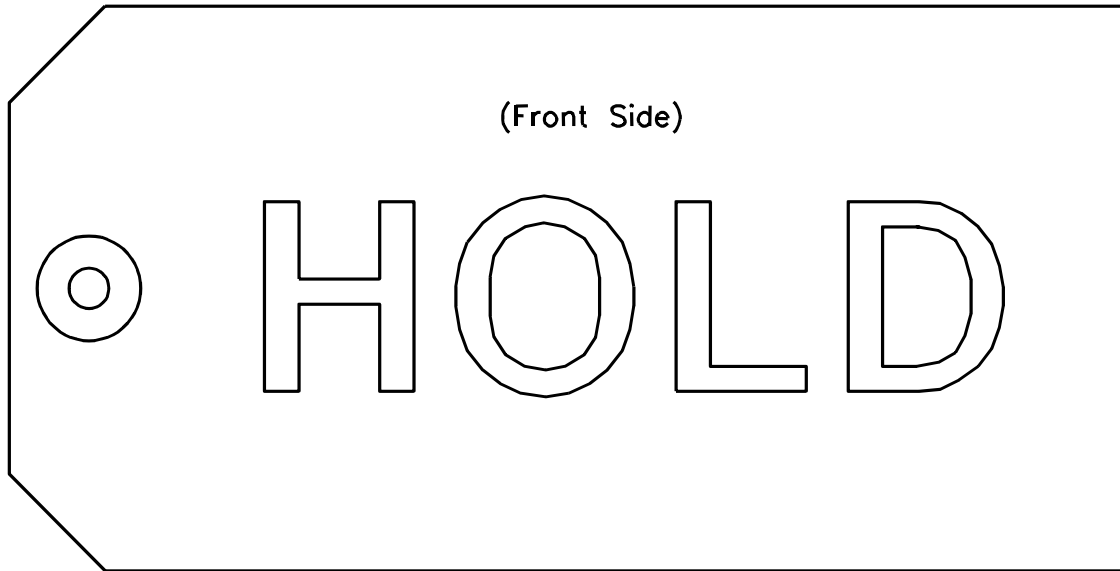
Application _____

Test Results _____

Date Completed _____

Sample Form 421-6: Sample Tag

(Front Side)



HOLD

(Back Side)

Reason for Hold

Q.C. Manager _____

Date _____

SECTION 422

MISCELLANEOUS STEELWORK

DESCRIPTION

422.01 Scope – This Section covers steelwork apart from major structural steel elements (see SS 421).

422.02 Adherence to Drawings – Steelwork items shall be fabricated in strict accordance with the Drawings.

422.03 Working Drawings – The working drawings shall show all information necessary for the fabrication of the steelwork.

Working drawings shall be in the same system of units as the design Drawings.

Lettering for notes and dimensions shall be at least 2.5 mm and 4 mm for headings. Drawings shall be legible when printed on 11" x 17" sheets.

In the case of simple items, where the design Drawings are complete enough to be used for fabrication, the Contractor may, with approval of the Ministry Representative, dispense with working drawings. The Contractor shall notify the Ministry Representative of such intention in writing.

The Contractor shall submit to the Ministry Representative one complete set of all working drawings in digital format, for the Ministry's information only, two weeks prior to the fabrication of the steelwork. Prior to submission to the Ministry Representative, working drawings shall be reviewed and approved by the Contractor. By this review and approval, the Contractor represents that it has determined and verified all field measurements, field construction criteria, materials, and similar data, and that it has checked and coordinated each working drawing with the requirements of the Work and the contract documents. The Contractor shall indicate its review and approval by including on each drawing the date and signature of a person designated by the Contractor as being responsible for the Work.

The Contractor shall transmit working drawings through attachments to e-mail. Unless otherwise agreed to by the Ministry Representative, electronic attachments to an email must total no more than 7 MB and must be submitted unzipped. Drawing files shall be submitted in Portable Document Format (PDF) to print out on 11" x 17" size pages. PDF sets shall be created by "distilling" CAD sheets rather than by scanning paper plan sets. Unless otherwise agreed to by the Ministry Representative, electronic attachments greater than 7 MB in size shall be sent in two parts by separate emails, denoting "1 of 2" and "2 of 2" in the subject lines after other required subject-line information. Drawings shall be black images on a white background.

All PDF sheets shall be assembled within a single file, subject to the maximum file size indicated above, ensuring that all sheets are rotated to a "ready to read" orientation within the PDF file set. Generally, 11"x17" plan sheets should be in landscape and 8½"x11" note sheets in portrait, so that the majority of text is vertical. PDF sheets shall be ready to print out on appropriately sized paper sheets with no additional formatting required by the viewer.

If agreed by the Ministry, the Contractor may submit shop drawings in paper format, in which case four complete sets of working drawings shall be submitted.

If agreed to by the Ministry Representative, the Contractor may employ a document and data management service such as SharePoint® to transmit working drawings. If this process is used, the limitation on drawing file size is waived. The Contractor shall be responsible for setting up the appropriate folders within the document and data management software and for providing access to these folders to the fabricator, Ministry Representative and the design engineer. Email notifications shall be sent to all parties whenever updates are made.

A copy of the shop drawings shall be available at all times at the location where the components shown on the drawings are being fabricated. Changes to the steelwork from what is shown on the shop drawings, or repairs made during fabrication and/or construction, shall be indicated by the Contractor on a marked-up set of shop drawings and submitted to the Designer and Ministry Representative at the completion of the Work.

422.04 Quality Control – The Contractor shall implement a quality control program to meet the Contract requirements. The quality control program shall be made available to the Ministry Representative for review.

422.05 Quality Assurance – The Ministry will implement a quality assurance program by auditing the quality control program and by inspection and testing at its discretion.

The Contractor shall notify the Ministry Representative at least 14 days before fabrication is to commence. In addition, the Contractor shall provide a minimum of 48 hours notice to the Ministry Representative that a product will be available for inspection. If the schedule is subsequently changed, the Contractor shall provide the 48 hours notice from the time that the Ministry is notified of this change. If the product is not available or is not sufficiently complete for inspection as notified, at the sole discretion of the Ministry Representative, the Contractor shall be charged stand-by costs for the Ministry's quality assurance inspector. The Contractor shall allow the Ministry Representative safe access to all parts of the Work and shall supply such information and assistance as is

SECTION 422

required. The Contractor shall provide samples of any materials, when requested by the Ministry. Inspection by the Ministry shall not relieve the Contractor from obligation to perform the work in accordance with the Contract.

Any welding work found to be unacceptable shall be corrected in accordance with CSA W59, Section 5.10.

The Contractor shall provide 5 days notice to the Ministry Representative regarding intent to ship a unit or product and the product shall be made available for inspection prior to loading and shipping.

422.05 Rejections – The Ministry may reject any items which, in their opinion, do not comply with the requirements of the Contract. The Ministry in its sole discretion may back charge all inspection costs for the rejected material to the Contractor.

MATERIALS

422.11 Materials – Structural steel, except where shown otherwise on the Drawings, shall conform to CSA-G.40.21, Grade 300 W or better. The boron content of steel shall not exceed 0.0008%.

Chemical composition of steel to be galvanized shall be in accordance with SS 422.36 – Galvanizing.

The quality and care of electrodes shall conform to the requirements of the latest CSA W59.

422.12 Storage of Materials – Structural material shall be stored above ground. It shall be kept free from dirt and other foreign matter, and shall be protected as far as practicable from corrosion. Long members shall be supported on skids placed near enough together to prevent injury from deflection.

CONSTRUCTION

422.31 Welding – All welding shall be done in accordance with procedures approved by the Canadian Welding Bureau (CWB) and reviewed by the Ministry.

All welded fabrication shall be done to the requirements of the latest CSA W59.

Welding shall be undertaken by a company approved by the CWB to the requirements of CSA W47.1, Division 2 or better.

422.32 Welders and Welding Operators – The Contractor shall produce evidence that all welders and welding operators to be employed on the work are currently qualified by CWB in the processes in which they are to be employed.

422.33 Flame Cutting – Steel may be oxygen or plasma cut, using a mechanical guide, provided a smooth surface is secured. Flame cutting by hand shall be done only where approved by the Ministry and the surface shall be made

MISCELLANEOUS STEELWORK

smooth by planing, chipping or grinding. The cutting flame shall be so adjusted and manipulated as to avoid cutting beyond the prescribed lines. Re-entrant flame cuts shall be filleted to a radius of not less than 20 mm. Edges of tension members shall be ground to a radius of 2 mm.

All exposed corners of members which are to be painted, shall be ground to a radius of 2 mm.

422.34 Bent Plates – When bending plates, the plates shall be so taken from the stock plates such that the bend line will be at right angles to the direction of rolling. Before bending, the corners of the plate shall be rounded to a radius of 2 mm throughout that portion of the plate at which bending is to occur. Bending shall be done by methods that will not crack, tear or otherwise injure the metal.

Cold bending and the minimum radii for cold bending shall be in accordance with CSA S6.

Hot bending of plates shall not be permitted without prior approval of the Ministry. Any proposed method for hot bending of plates shall be submitted to the Ministry for review and approval by the Ministry prior to proceeding with the work.

422.35 Coating of Steelwork – This subsection applies to those parts of the steelwork which are to be coated.

All coatings shall be applied according to their Manufacturer's product data sheet unless specified otherwise in the Contract Special Provisions.

Coating work shall be in accordance with SS 216 unless noted otherwise in SS 422.

All exposed edges and corners of members, rolled or cut, which are to be coated, shall be ground to a minimum radius of 2 mm.

All steelwork which is to be coated, shall be given three shop coats of paint – primer, stripe coat and midcoat. Paint shall be chosen from SS 308 System SS1. Paint shall be supplied by the Contractor.

The topcoat may be applied in the shop or in the field. Field applied topcoat coat shall be chosen from either SS 308 System SS1 or SF2. If the topcoat is to be applied in the shop, then the Quality Control Program shall specifically address the integrity of the topcoat through to project completion.

Faying surfaces shall receive the prime coat only, as per SS 421.31.06. Faying surfaces shall not be coated with the stripe coat, midcoat or topcoat paint. Coating thicknesses and curing shall be those specified in the Manufacturer's Class B certificate.

Paint shall be applied in a covered area in accordance with the manufacturer's specifications and SS 216.

All edges, corners, crevices, bolts, nuts, protrusions and welds (unless ground flush) shall be stripe painted by brush for a width of 50 mm with the midcoat paint before the

SECTION 422

midcoat coat is applied. Stripe coating paint shall be applied as per SSPC-PA 1 and may be applied by spray, but shall be brushed in. Stripe painting shall be allowed to cure before the midcoat coat is applied.

Surfaces shall be cleaned to "near-white" per SSPC-SP10 / NACE No. 2 and 50 μm to 75 μm (2 to 3 mils) sharp/angular profile.

The midcoat shall be applied to all surfaces including tops and sides of the top flanges, except faying surfaces. The midcoat shall not be applied until the primer and stripe coat are accepted by the Ministry Representative to be sufficiently cured.

Any surfaces inaccessible after erection, except faying surfaces, shall be given, in addition to the three shop coats, one coat of the topcoat paint appropriate for the paint system being used.

If the topcoat is to be applied in the shop, then the topcoat shall be applied after the midcoat is accepted by the Ministry Representative to be sufficiently cured.

The colour of the topcoat will be selected by the Ministry. The proposed colour shall be subject to the acceptance of the Ministry Representative.

422.36 Galvanizing (if required) – All steelwork to be galvanized shall be galvanized after complete fabrication to the requirements of ASTM A123 and ASTM A385. The galvanizer shall safeguard against embrittlement as required in ASTM A143. Galvanized members shall be subject, at the discretion of the Ministry, to the tests for embrittlement outlined in ASTM A143.

The chemical composition of steel being galvanized shall be as follows:

- Carbon less than 0.25%
- Phosphorus less than 0.04%
- Manganese less than 1.3%
- Silicon less than 0.04% or between 0.15% and 0.22%

For steel not meeting these chemical composition requirements, special galvanizing techniques shall be developed by the galvanizer to ensure that the specified coating thickness and adherence is achieved. A detailed description of the special techniques shall be submitted to the Ministry Representative for review 2 weeks prior to galvanizing.

422.37 Metallizing (if required) – All surfaces to be metallized shall be treated after complete fabrication in accordance with SSPC-CS23.00 / AWS C2.23M / NACE No. 12, Specification for the Application of Thermal Spray Coatings (Metallizing) of Aluminum, Zinc, and Their Alloys and Composites for the Corrosion Protection of Steel. The zinc coating shall not be less than 0.3 mm in thickness.

MISCELLANEOUS STEELWORK

422.38 Installation – Steelwork shall be installed as shown on the Drawings.

Railings and fences shall be adjusted to produce uniform height and smooth alignment.

Field welding will be permitted only as shown on the Drawings.

422.39 Field Painting (if required) – This section applies to those parts of the steelwork which are to be painted.

Coating work shall be in accordance with SS 216 unless noted otherwise in SS 422.

All steelwork shall be thoroughly cleaned of all rust, dirt, dust, oil and other foreign materials. Non-visible salts shall be removed in accordance with SSPC-SP WJ-4 / NACE WJ-4 to meet non-visible contaminant (NVC) levels per SS 216.07.02(b). The shop coats of paint shall be touched up as necessary.

422.39.01 Field Touch-up – Bare, rusty or damaged areas shall be cleaned to SSPC-SP 11, Power Tool Cleaning to Bare Metal. Feather edges into the existing coating and build the coating as per SS 216. Build the coating using coatings from the specified SS 308 System. Coating system shall be from the same manufacturer as the shop coating system on the steel.

422.39.02 Topcoat – The topcoat coat shall be chosen from either SS 308 System SS1 or SF2. If the topcoat has not been applied in the shop, then the steelwork shall be given one topcoat coat of paint applied in accordance with the manufacturer's specifications and SS 216.

The colour of the finish coat will be selected by the Ministry. The proposed colour shall be subject to the acceptance of the Ministry Representative.

422.40 Touch-Up of Galvanizing and Metallizing – All field welds and other damage in galvanized and metallized coatings shall be touched up as follows:

If the contract documents call for touch-up by metallizing, the damaged areas shall be locally cleaned to the requirements of SSPC-SP 5 or SSPC SP 11, with an angular profile of 50 to 100 μm . All cleaned areas shall be metallized in accordance with SSPC-CS23.00 / AWS C2.23M / NACE No. 12, Specification for the Application of Thermal Spray Coatings (Metallizing) of Aluminum, Zinc, and Their Alloys and Composites for the Corrosion Protection of Steel, to provide a zinc coating not less than 0.3 mm in thickness.

Cut metal edges may be harder and consequently have less profile. If that is the case, they shall be ground back to softer metal and reblasted to achieve the specified profile.

Otherwise, the damaged areas shall be thoroughly cleaned, and painted with two coats of Ministry-approved organic zinc-rich paint.

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422.41 Clean-up – At the completion of the Contract, all miscellaneous steelwork shall be cleaned of concrete spatter, mud, oil, shop markings if visible to the public and other foreign materials.

MISCELLANEOUS STEELWORK

PAYMENT

422.91 Payment – Payment for miscellaneous steelwork will be made at the Lump Sum price(s) bid. Payment shall be for shop drawings and the supply, fabrication, galvanizing and/or painting if required and installation of the steelwork.

SECTION 502

ASPHALT PAVEMENT CONSTRUCTION END PRODUCT SPECIFICATION (EPS)

GENERAL

502.01 Preliminary and General – This Section describes the materials, equipment, professional standards, and end product requirements for the construction of hot mix asphalt pavement and related shouldering operations.

End Product Specifications (EPS) contain the acceptance and payment criteria based on the results of specified sampling and testing. Payment of the Contract Unit Prices for the asphalt pavement product is contingent on the product meeting the Quality Control (QC) Plan, professional standards and quality requirements of this Section and is subject to payment adjustments upward and downward in accordance to the provisions provided in this Section.

When used in this Section and subject to the General Conditions:

- “Acceptance” means agreement with past actions or decisions made, within the scope of the Contract.
- “Authorization” means formal approval for future actions, frequently changing the Contract requirements.

502.02 General Description of the Work – Generally, the work associated with the construction of asphalt pavement and shouldering by EPS consists of the following:

- Preparing a QC Plan for review before commencing the Work and providing at the production site a testing facility to provide the data needed to implement that plan;
- Supplying, screening, crushing, processing and improving aggregate to produce asphalt mix aggregate;
- Supplying and delivering asphalt cement and spray primer meeting the requirements of SS 952;
- Preparing mix designs which, once reviewed and accepted by the Ministry Representative, become the basis for the accepted Job Mix Formula;
- Heating the asphalt mix aggregate and mixing it with asphalt cement to produce asphalt mix that meets the Job Mix Formula;
- Hauling, placing, compacting and finishing the asphalt mix; and
- Supplying, hauling, placing and compacting shoulder gravels.

The Contractor shall provide a paving end product conforming in professional standard, quality and accuracy of detail to the QC Plan and the dimensional and tolerance requirements of the contract. Where no tolerances are specified, the standard of workmanship shall be in accordance with normally accepted good practice and the provisions of this Section. Payment is subject to upward or downward adjustments based on quality acceptance tests performed by the Ministry Representative and calculations performed by the Contractor with respect to application rate.

502.03 Definitions

502.03.01 Actual Asphalt Content – The amount of asphalt cement in the mix as determined by the Ministry’s Quality Assurance Program.

502.03.02 Additives – Solid or liquid materials to enhance the properties of the liquid asphalt cement or mix.

502.03.03 Aggregate – The crushed or screened gravel.

502.03.04 Asphalt Cement (AC) – A bitumen-based liquid binder used in asphalt pavement.

502.03.05 Asphalt Content – The percentage of asphalt cement in the mix expressed as percentage by weight of the total dry aggregate in the mix determined by the Oven Test procedure.

502.03.06 Asphalt Mix (AM) – Hot plant mixture of asphalt cement and aggregate.

502.03.07 Asphalt Mix Aggregate (AMA) – The processed crushed aggregate just prior to the addition of asphalt cement.

502.03.08 Asphalt Mix Design (AMD) – The asphalt mix design that is developed through the initial trials and testing to determine and optimize the Job Mix Formula for the end product of the asphalt mix.

502.03.09 Asphalt Pavement (AP) – Compacted asphalt mix.

502.03.10 Cutback Asphalt – Asphalt cement which has been blended with light petroleum distillates.

502.03.11 Design Asphalt Content – The asphalt content upon which the Job Mix Formula is initially established.

502.03.12 Driving Lane – A single lane in any area of the pavement other than a shoulder or a barrier flare.

502.03.13 Emulsified Asphalt – Asphalt cement that has been blended with water and emulsifying agents to form aqueous emulsions, including anionic type, cationic type and high float type.

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502.03.14 End Product Specification (EPS) – A specification whereby the Contractor is responsible for the workmanship and quality control of the construction processes, and whereby the Ministry reviews the workmanship and may perform the specified quality assurance sampling and testing of the end product for the purpose of determining acceptance/rejection and payment.

502.03.15 Job Mix Formula (JMF) – The asphalt mix “recipe”, proposed by the Contractor in accordance with SS 502.08.04 or an accepted variation in accordance with SS 502.08.10 and accepted by the Ministry, establishing the aggregate proportions, gradation, and the asphalt content to be used for production of asphalt mix.

502.03.16 Levelling Course (LC) – Asphalt mix used to improve crossfall, level, and strengthen existing pavements.

502.03.17 Lift – A layer of asphalt mix laid in a single application then compacted.

- **Top Lift** The uppermost Lift, forming the final running surface.
- **Lower Lift** Any Lift below Top Lift.
- **Bottom Lift** The lowest Lift (excluding Level Course),

502.03.18 Lot and Sub-Lot – A Lot is a portion of the work being considered for acceptance and for the determination of payment adjustments. For density, AC content, gradation and smoothness, each Lot is comprised of a number of Sub-Lots, each of which is sampled, and then aggregated to determine the acceptability of the Lot. Lot and Sub-Lot sizes are defined in the appropriate payment adjustment provisions of this Section.

502.03.19 Reclaimed Asphalt Pavement (RAP) – Asphalt Pavement that has been removed and processed, for the purpose of recycling.

502.03.20 Recycled Asphalt Mix (RAM) – A carefully controlled hot plant mix of asphalt cement, graded high quality aggregate, and reclaimed asphalt pavement.

502.03.21 Reject Mix – Asphalt mix that is deemed unacceptable for use in the project.

502.03.22 Sample Mean – The arithmetic mean of a set of test results constituting the sample.

502.03.23 Smoothness – A measure of the longitudinal profile of the pavement surface, measured as International Roughness Index (IRI).

502.03.24 Surplus Aggregate – Aggregate surplus to the works, in split or un-split stockpiles which singly or combined will meet the asphalt mix aggregate gradation.

502.03.25 Random Sample – A set of test measurements taken, one each from a number of separate areas or Sub-Lots within a Lot, in an unbiased way.

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502.03.26 Voids in Mineral Aggregate (VMA) – The space available to accommodate the effective volume of asphalt (asphalt not absorbed into the aggregate) and the volume of air voids necessary in the mixture.

502.04 Quality Control

502.04.01 General – Quality Control shall be performed by the Contractor in accordance with SS 101 and the requirements of this Section.

502.04.02 Quality Control (QC) Plan – The paving component of the Contractor’s Quality Control Plan shall fulfill or exceed the requirements of Appendix 502-A and function integrally with any other Quality Management provisions of the Contract.

The QC Plan shall be submitted in accordance with the timelines established in SS 101.02.03(c).

The QC Plan must include a detailed description of the means by which the Contractor shall use the quality control test results to ensure that the workmanship, asphalt materials, aggregate, mix production, paving and pavement compaction processes will be controlled to keep the product within the specified limits. The QC Plan must clearly show the flow of information from the quality control laboratory to the individuals who shall make the actual adjustments to the processes and equipment to affect this control. The plan will show time allowance for each step, the names and positions of all the people involved, and a clear description of the responsibilities of each.

502.04.03 Quality Control Testing and Inspection – The Contractor shall provide and maintain equipment and qualified personnel to perform all laboratory testing, field testing and inspection necessary to determine and monitor the characteristics and properties of all the materials produced and incorporated into the work. They shall also monitor the workmanship of the final product in accordance with the QC Plan as most recently submitted and accepted.

The Contractor shall provide a testing facility(s) that meets the requirements necessary to carry out all the test procedures listed within this Section. The facility(s) must have the equipment specified under the appropriate test designation to perform the tests. The Ministry shall have access at all times to the quality control facility(s).

502.04.04 Quality Control Records – Quality Control inspections shall be recorded on check sheets and/or diaries at the time of inspection.

The results from Quality Control testing shall be reported on test logs and plotted on charts immediately after each test is completed. The Contractor shall report all test results on Ministry-supplied forms (available from the Ministry Representative) or Contractor-supplied forms acceptable to the Ministry Representative.

For the purposes of confirming delivery of asphalt mix and shouldering aggregate to the road and the calculation of

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material application rates, the Contractor shall provide to the Ministry Representative a copy of the Road Checker's Summary and the weigh tickets for each load received at the placement operation at the end of each shift. The Road Checker's Summary shall include, but not be limited to, the following information:

- Truck Number;
- Weigh Ticket Number and Net Weight of load;
- Date, time and location by station of delivery;
- Material Application Rate Dimensions and Calculations. Calculation frequency minimally shall be for every 10 loads; and
- Notes pertaining to the paving of any appurtenances (letdowns, intersections, tapers, etc.).

In addition to the equipment calibration requirements of SS 101, for the purpose of obtaining accurate and consistent results between the Contractor's Quality Control testing and the Ministry's Quality Acceptance testing, correlation of the Marshall hammer and ignition ovens to be used on the project shall be performed prior to mix production. Correlation procedures to be followed for both the Marshall hammer and ignition oven are included as Appendices to this SS 502.

502.04.05 Final Quality Control Testing Reports – Prior to the issuance of a Completion Certificate, the Contractor shall provide the Ministry Representative with:

- A summary of all aggregate quality control test results;
- Copies of all quality control test results for asphalt mix properties and compaction; and
- Copies of all quality control charts.

MATERIALS

502.05 Materials Testing

502.05.01 Test Procedures – Where a test is specified to conform to an ASTM procedure, the listed corresponding AASHTO test may be used, or vice versa. See Table 502-A.

502.05.02 Sieve Sizes – All aggregate gradation tests shall use the following sieve sizes: 37.5 mm, 25 mm, 19 mm, 16 mm, 12.5 mm, 9.5 mm, 4.75 mm, 2.36 mm, 1.18 mm, 0.600 mm, 0.300 mm, 0.150 mm, and 0.075 mm.

502.06 Aggregates

502.06.01 Aggregates – The Contractor shall supply all aggregates for the Work from sources acceptable to the Ministry Representative.

502.06.02 Work in Ministry Pits or Quarries – Ministry pits offered in the Special Provisions as available sources

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may be used without charge provided they are developed and worked in accordance with SS 145.26, SS 202 Part C, and the pit development plan. Deviation from the pit development plan will require the authorization of the Ministry Representative. No guarantee is given or implied that aggregate from a Ministry pit(s) will meet the specification requirements or provide requisite quantities.

Without limitation and unless otherwise specified in the Special Provisions, the following items shall not be left in a Ministry pit:

- Surplus milled or rubblized asphalt pavements;
- Reject mix;
- Fuel contaminated materials; or
- Other waste products.

502.06.03 Supply of Aggregates, Aggregate Production and Characteristics – The Contractor shall not produce aggregate until the Contractor has received written notification from the Ministry Representative that their QC Plan is in accordance with SS 502.04.02, and has in place testing facilities for aggregate production that are in accordance with the QC Plan.

For the production of aggregate within Ministry pits, the Contractor shall follow the Ministry's pit development plan, and provide crushing equipment such that all aggregate, which will pass through 375 mm x 450 mm slotted openings, shall be used for the production of crushed aggregate. Rocks, which will not pass through these openings, shall be stockpiled safely at a location in the pit as directed by the Ministry Representative. Crushing and screening equipment shall be provided with adequate facilities and capacity to be able to bleed off reject aggregate and remove any excess fine aggregate, dust or objectionable aggregate coatings, to make it generally acceptable for use. No portion of the products from crushing or screening plants that can be used shall be wasted but shall be stockpiled or used as directed by the Ministry Representative.

Where the Ministry has any available test result information on the properties shown in Table 502-B, for a Ministry pit or other source, the Ministry Representative will upon request, provide that information to the Contractor. Otherwise, sampling and testing to determine and demonstrate the compliance of aggregate with the requirements of this Section shall be the responsibility of the Contractor.

502.06.04 Shouldering Aggregate – Shouldering aggregate shall meet the following unless otherwise specified in the Special Provisions or authorized by the Ministry Representative:

- (a) Aggregate may be AMA, 25 mm WGB, or HFSA; and

Table 502-A: Standard Sampling and Testing Procedures

Test Ref. No.		Standard Test Method for... (ASTM test title, unless otherwise noted)
AASHTO	ASTM	
Aggregates:		
T 11	C117	Materials Finer than 75 µm (No. 200) Sieve in Mineral Aggregates by Washing
T 85	C127	Relative Density (Specific Gravity), and Absorption of Coarse Aggregate
T 84	C128	Relative Density (Specific Gravity), and Absorption of Fine Aggregate
T 27	C136	Sieve Analysis of Fine and Coarse Aggregates
T 112	C142	Clay Lumps and Friable Particles in Aggregates
T 255	C566	Total Evaporable Moisture Content of Aggregate by Drying
T 2	D75	Standard Practice for Sampling Aggregates
T 304	C1252	Standard Test Methods for Uncompacted Void Content of Fine Aggregate (as Influenced by Particle Shape, Surface Texture, and Grading) [Method "A"]
	D421†	Standard Practice for Dry Preparation of Soil Samples for Particle-Size Analysis and Determination of Soil Constants [aka Atterberg Limit]
T 176	D2419	Sand Equivalent Value of Soils and Fine Aggregate
	D4318	Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils
	D5821	Determining the Percentages of Fractured Particles in Coarse Aggregate
T 327	D6928	Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus
Asphalt Cement:		
T 49	D5	Penetration of Bituminous Materials
T 202	D2171	Viscosity of Asphalts by Vacuum Capillary Viscometer
T 350	D7405	T 350 Standard Method of Test for Multiple Stress Creep Recovery (MSCR) Test of Asphalt Binder Using a Dynamic Shear Rheometer (DSR)
Asphalt Mix and Pavement:		
	D979	Standard Practice for Sampling Bituminous Paving Mixtures
T 283	D1075	Effect of Water on Compressive Strength of Compacted Bituminous Mixtures
	D1188	Bulk Specific Gravity and Density of Compacted Bituminous Mixtures Using Paraffin Coated Specimens
	D2041	Theoretical Maximum Specific Gravity and Density of Bituminous Paving Mixtures
	D2726	Bulk Specific Gravity and Density of Non-Absorptive Compacted Bituminous Mixtures
T 269	D3203	Percent Air Voids in Compacted Dense and Open Bituminous Paving Mixtures
	D4469	Calculating Percent Asphalt Absorption by the Aggregate in an Asphalt Pavement Mixture
T 245	D5581	AASHTO Standard Method of Test for Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus
T-308	D6307	Asphalt Content of Hot Mix Asphalt by Ignition Method
	D6927	Marshall Stability and Flow of Asphalt Mixtures
Shouldering:		
	D698	Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft ³ (600 kN-m/m ³)) [aka Standard Proctor]
	D5220	Standard Test Methods for Water Content of Soil and Rock in Place by Neutron Depth Probe Method
	D6938	In-Place Density and Water Content of Soil and Soil- Aggregate by Nuclear Methods (shallow depth)

Note: † – Practice has been “withdrawn” by ASTM, but last published version D421-85(2007) is still applicable to the Work.

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Table 502-B: Requirements for Coarse Aggregate

Test Ref. #		Procedures	Superpave	Class 1	Class 2
AASHTO	ASTM				
T 85	C127	Maximum Water Absorption, % by mass	2	2	2
T 112	C142	Maximum % by mass of clay balls and friable particles	1.0	1.0	1.5
	D5821	2 Fractured Faces, Minimum % by Mass retained on the 4.75 mm sieve	90	85	70
T 327	D6928	Maximum Micro-Deval abrasion loss factor, %	18	18	20

(b) Aggregate gradations and physical properties shall comply with:

- (i) SS 502.06, if AMA is to be used; or
- (ii) SS 202.04 and SS 202.05, for any other aggregates.

502.06.05 Paving Aggregates – Paving aggregates shall meet the following requirements:

(a) Coarse Aggregates

- (i) Shall be all mineral matter retained on the sieve designated in the test procedures for each individual test.
- (ii) Shall consist of crushed stone, crushed gravel, or combination thereof, or materials naturally occurring in a fractured condition, or materials naturally occurring of highly angular nature or rough texture.
- (iii) Shall be free from coating of clay, silt or other deleterious material.
- (iv) Shall meet the requirements listed in Table 502-B.

(b) Fine Aggregate

- (i) Shall be all mineral matter passing the sieve designated in the test procedure(s) for each individual test.
- (ii) Shall be clean, tough, durable, moderately sharp, and free from coatings of clay, silt, or other deleterious material, and shall contain no clay balls or other aggregations of fine material.
- (iii) Shall have a sand equivalent of not less than 40 (not less than 45 for Superpave mixes) when tested in accordance with ASTM D2419.
- (iv) For Class 1 and Superpave mixes, shall have a minimum value of 45 when tested according to the AASHTO T 304, Method “A”, - Uncompacted Void Content of Fine Aggregate when determining Fine Aggregate Angularity.

(c) Mineral Filler and Mineral Dust

- (i) Mineral filler shall consist of all matter passing the 0.600 mm sieve and mineral dust shall consist of all mineral matter passing the 0.075 mm sieve.
- (ii) Shall be free from organic matter.
- (iii) Mineral filler shall be non-plastic when tested in accordance with ASTM D4318.

(d) Superpave Aggregates – Aggregates for Superpave mixes shall have properties and the gradation limits as specified below and in accordance with the latest version of the Asphalt Institute’s *Superpave Series Publication – Superpave Mix Design (current version)*. Changes and/or variations from these limits shall be outlined within the Special Provisions.

- (i) 90% fractured aggregate with a 12.5 mm nominal maximum size, including sufficient manufactured fines to provide fine aggregate angularity.
- (ii) The aggregates must meet all the requirements for angularity, toughness deleterious materials, clay content, and flat and elongated particles.
- (iii) Design ESALs will be 10 – 30 million.

502.07 Supply of Asphalt Cement and Primer – The Contractor shall supply the types and grades of asphalt cement and primers as specified in the Special Provisions and in accordance with SS 952. The supply of these materials includes, but is not limited to, ordering, scheduling delivery of, receiving, handling, storing, sampling, and testing of the materials and other related work.

The Contractor shall supply the Ministry Representative with copies of the asphalt supplier’s weigh-bill and records of all asphalt materials received on a daily basis.

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502.07.01 Asphalt Binder Testing – The Contractor shall provide supplier’s Asphalt Binder testing and grade information upon request.

502.07.02 Testing for Viscoelastic Properties – The Ministry may test an Asphalt cement to determine its viscoelastic properties.

The selected grades of Asphalt Cement may be tested at a temperature of 58°C to determine the average percent recover at 3.2 kPa ($R_{3.2}$) according to the requirements of ASTM D7405.

The minimum $R_{3.2}@58^{\circ}\text{C}$ value for selected asphalt binder grades shall be determined as outlined in Table 502-C.

Table 502-C: MSCR – Elastic Recovery Requirements

PGAC	Minimum $R_{3.2}$ @ 58°C
58-34 64-28	25%
58-37 58-40 64-34 70-28	40%
64-37 76-28	55%

502.08 Asphalt Mix

502.08.01 Responsibility for Asphalt Mix Design – Preparation and submission of the project asphalt mix designs for Ministry review is the responsibility of the Contractor. All costs incurred in mix design formulation are the responsibility of the Contractor.

The Contractor shall utilise a qualified registered member of the Association of Professional Engineers and Geoscientists of British Columbia or a qualified, registered member of the Applied Science Technologists and Technicians of British Columbia who shall sign off the asphalt mix design. The Contractor shall utilise a [CCIL certified testing laboratory](#) for

- meeting the requirements of SS 101;
- assessing the aggregate material proposed for use; and
- preparing Asphalt Mix Designs as required in SS 502.

502.08.02 Requirements for Asphalt Mix Design

- (a) **All Mixes** – Asphalt mix designs shall be performed using the asphalt cement grade specified in the Special Provisions and which is from the same refinery contracted to supply the asphalt cement for the duration of the project.

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Any subsequent change in the asphalt cement supplied by the Contractor will require a new asphalt mix design unless permitted otherwise by the Ministry Representative.

Asphalt mix designs, Job Mix Formulas, and field adjustments made in accordance with SS 502.08.10 must all be based on an aggregate gradation meeting the requirements of Table 502-D and the criteria specified in Table 502-E or Table 502-F, as applicable.

- (b) **Marshall Mixes** – The asphalt mix design for Class 1 and Class 2 pavements shall be carried out under Marshall design criteria using the designated equipment and procedures as contained in the Asphalt Institute’s “*Mix Design Methods for Asphalt Concrete MS-2*” (current version). Marshall hammers shall be correlated in accordance with Appendix 502-F.
- (c) **Superpave Mixes** – The Superpave asphalt mix design shall be carried out in accordance with the latest edition of the Asphalt Institute’s “*Superpave Mix Design, Superpave Series No. 2*” (current version) and this specification.

The JMF aggregate gradation shall fall under the maximum density gradation line for the 0.300 mm to 2.36 mm sieves, inclusive.

- (d) **Open Graded Friction Course (“OGFC”) Mixes** – The OGFC asphalt mix design shall be carried out under the Marshall design criteria using the designated equipment and procedures in accordance with the latest edition of the Asphalt Institute’s “*Superpave Mix Design, Superpave Series No. 2*” (current version).

502.08.03 Asphalt Mix Antistrip Additives – For all mix designs, the Contractor shall determine the Tensile Strength Ratio (TSR) of each asphalt mix in accordance with AASHTO T 283.

An antistrip additive shall be added to the asphalt mix when:

- Special Provisions so direct;
- TSR is less than 80; or
- Ministry Representative specifically requests it.

The Contractor shall select the antistrip additive from the Ministry’s “Recognized Products List” and add it to all asphalt mix used in the Work, at an application rate of 0.3% additive by weight of asphalt cement. The Recognized Products list is available on-line at:

http://www.th.gov.bc.ca/publications/eng_publications/geo_tech/Recognized_Products_Book.pdf

502.08.04 Asphalt Mix Design Submittals – The Contractor shall submit each Asphalt Mix Design to the Ministry Representative for review. The Ministry will review the Asphalt Mix Design to ensure it complies with the requirements of the Contract.

Table 502-D: Asphalt Mix Aggregate Gradation Limits

Sieve Size (mm)	Percentage Passing by Mass						
	Coarse Mix	Medium Mix		Fine Mix	Asphalt Base Course (ABC)	Asphalt Bound Open Graded Base (ABOGB)	Superpave ¹
	37.5 mm	19 mm	16 mm	12.5 mm	25 mm	25 mm	Nominal 12.5 mm
37.5	100						
25.0	80 – 100				100	100	
19.0	60 – 92	100			80 – 94	75 – 100	100
16.0			100				
12.5	50 – 85	84 – 95	90 – 100	100			90 – 100
9.50	40 – 80	73 – 90	73 – 90	90 – 100	50 – 84	30 – 60	
4.75	30 – 65	50 – 75	50 – 75	55 – 80	25 – 55	5 – 30	
2.36	20 – 50	35 – 57	35 – 57	32 – 64	20 – 45	0 – 10	28 – 58
1.18	15 – 35	26 – 45	26 – 45	24 – 51	15 – 35		
0.600	8 – 30	18 – 34	18 – 34	17 – 40			
0.300	6 – 22	10 – 26	10 – 26	13 – 29	5 – 20	0 – 8	
0.150	3 – 15	6 – 17	6 – 17	8 – 18			
0.075	1 – 7	3 – 7	3 – 7	4 – 10	0 – 5	0 – 4	2 – 10

Note (1): from Appendix B in SuperPave SP-2

Table 502-E: Marshall Design and Production Criteria

Property of Laboratory Compacted Paving Mixture	Pavement Class	
	1	2
Number of blows each face of test specimens	75	75
Minimum % Voids in Asphalt Mix Aggregate for 19 mm Medium Asphalt Mix	14	14
Minimum % Voids in Asphalt Mix Aggregate for 16 mm Medium Asphalt Mix	14.5	14
Minimum % Voids in Asphalt Mix Aggregate for 12.5 mm Fine Asphalt Mix	15	15
% air voids in laboratory compacted mixture for 19 mm Medium Asphalt Mix	2.5 to 4.5	2.5 to 4.5
% air voids in laboratory compacted mixture for 16 mm Medium Asphalt Mix	2.5 to 4.5	2.5 to 4.5
% air voids in laboratory compacted mixture for 12.5 mm Fine Asphalt Mix	2.5 to 4.5	2.5 to 4.5
Minimum Marshall Load, N @ 60°C for 80 – 100 Pen. and 120 – 150 Pen.	9000	7000
Minimum Marshall Load, N @ 60°C for 150 – 200 Pen. and 200 – 300 Pen.	7000	6000
Flow index, units of 0.25 mm	8 to 14	8 to 16
Minimum Asphalt Film Thickness, μm (microns) (see Appendix 502-E)	8.0	8.0
Minimum Index of Retained Stability after immersion in water at 60°C for 24 hours	85%	75%

Table 502-F: Superpave Design and Production Criteria

Property of Laboratory Compacted Paving Mixture For Design ESALs = 10 - 30 million, at N _{Design} = 100 gyrations	SuperPave
% voids in the Mineral Aggregate, minimum	14
Required density: % of Theoretical Maximum Specific Gravity in a laboratory compacted mix at N _{max} = 160 gyrations at N _{Design} = 100 gyrations at N _{Initial} = 8 gyrations	98 95 – 97 ² <89
% voids filled with Asphalt Cement	65 – 75
Dust to Binder ratio ³	0.6 – 1.2

² For design Superpave density shall be 96. ³ Consideration shall be given to increasing the dust to binder ratio to 0.8 – 1.6

Paving Work shall not proceed until the Contractor receives the results of the review in writing from the Ministry Representative. The Contractor's submissions shall include the following information:

- Gradation (ASTM C117) of each aggregate to be used in each mixture;
- Percentage by mass of each aggregate to be used in each mixture;
- Design gradation (ASTM C117) of the combined aggregate for each mixture for each of the sieve sizes applicable to the mix, per the first column of Table 502-D;
- Estimated dry sieve gradation (ASTM C136) correlated to the JMF gradation;
- All Asphalt Mix Design data used in arriving at the final mix designs;
- Design Asphalt Content expressed as a percentage of the dry weight of the aggregate;
- Design Mix Temperature, which shall be the temperature at which the kinematic viscosity of the asphalt cement is 0.17 Pa•s (170 centistokes) or as per the Asphalt Cement supplier's recommendations; and
- Recommended compaction temperature.

502.08.05 Ministry's Review of Asphalt Mix Design - The Ministry Representative will require up to five (5) calendar days from the time of receipt of the Asphalt Mix Design for review. The mix design must be reviewed and accepted by the Ministry Representative prior to commencement of pavement construction.

Upon acceptance of the Asphalt Mix Design, the Contractor shall prepare the laboratory equipment calibration samples, submit them to the Ministry Representative, and participate in the calibration and correlation process described in Appendix 502-C Blank Aggregate Sample Preparation and Appendix 502-D Ignition Oven Correlation Procedure.

502.08.06 Ignition Oven Correlation – After receiving acceptance of the Asphalt Mix Design, the Contractor shall prepare blank aggregate samples (aggregate-only samples prepared to match the mix design) for correlation of the Contractor, Ministry and appeal laboratory ignition ovens. Blanks shall be prepared in accordance with Appendix 502-C Blank Aggregate Samples.

The Ministry Representative shall randomly select which of the individual blanks will be used.

Within 3 working days, and prior to any mix production, the Contractor and the Ministry shall prepare and test asphalt mix samples in accordance with Appendix 502-D Ignition Oven Correlation.

502.08.07 Use of Calibration Factors in Reporting Asphalt Cement Content – Results from testing of any asphalt mix shall report the measured AC Content and the corrected AC Content after applying the applicable laboratory calibration factor.

502.08.08 Verification of Job Mix Formula from the Asphalt Mix Design - Verification of the Asphalt Mix Design will be carried out by the Contractor during the course of production of the first 1,000 tonnes of mix using the Asphalt Mix Design.

During the first 1,000 tonnes of plant production, the Contractor may make any adjustments it chooses to the Asphalt Mix Design, testing the mix, and refining the Asphalt Mix Design to a state that fully complies with Table 502-D and Table 502-E (or Table 502-F as applicable) and the Special Provisions.

All mix of the Asphalt Mix Design laid must be tracked by the Contractor, and reported to the Ministry Representative, as to lay-down location and the Asphalt Mix Design values in effect at the time that mix was produced, to ensure appropriate values are used in comparing design to sampled properties.

After production of the first 1,000 tonnes, the Contractor shall declare their Job Mix Formula (JMF) to the Ministry Representative and provide volumetric properties/test data

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on the final mix produced. Any future adjustments to the JMF shall comply with all requirements of SS 502.

Where the JMF varies from the Asphalt Mix Design by a cumulative amount greater than any tolerance specified in Table 502-D, the Contractor shall do a single point confirmatory Asphalt Mix Design and report the results to the Ministry Representative.

502.08.09 AC Content Bump – Upon receipt of a Job Mix Formula meeting all Contract requirements, the Ministry may direct the Contractor to increase the asphalt cement content by a “bump” of up to 0.3% by weight of dry aggregate in the mix.

502.08.10 Field Adjustment of Job Mix Formula - A field adjustment to the Job Mix Formula is defined as a change in the asphalt cement content of the mix, aggregate gradation and/or proportioning of various aggregate sizes, within the specified limits as shown in Table 502-G without review and acceptance of a new Asphalt Mix Design.

Table 502-G: Field Adjustment of Job Mix Formula

Job Mix Formula Property	Maximum Cumulative Field Adjustment
Percentage Passing by Sieve Size:	
• 37.5 mm, 25.0 mm, 19.0 mm, and 16.0 mm	±2.0%
• 12.5 mm and 9.5 mm	±2.0%
• 4.75 mm and 2.36 mm	±1.5%
• 1.18 mm and 0.600 mm	±1.5%
• 0.300 mm and 0.150 mm	±1.5%
• 0.075 mm	±0.5%
Asphalt Cement content	±0.3%

The proposed field adjustment shall be submitted in writing together with supporting documentation to the Ministry Representative. Within four hours of receipt of the proposed field adjustments. The Ministry Representative will review the field adjustment for conformance with the contract requirements and notify the Contractor whether or not it is acceptable.

The Contractor’s field adjustment to the Job Mix Formula must comply with the Asphalt Mix Design requirements of SS 502.08.02 through SS 502.08.04 inclusive. The Contractor shall provide all supporting verification data.

After the Job Mix Formula has been established in accordance with SS 502.08.10, no field adjustment to that Job Mix Formula will be permitted without prior written authorization by the Ministry Representative. The Ministry Representative will limit the number of field adjustments of

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the Job Mix Formula to two from the originally derived Asphalt Mix Design.

No field adjustment will be acceptable if it results in a change from the Asphalt Mix Design, for any property, in excess of the maximum adjustment for that property permitted in **Table 502-G**. All production Mix shall meet either Table 502-E or Table 502-F

EQUIPMENT AND PLANT

502.15 All equipment and plant shall be in good mechanical condition and be capable of performing the Work in accordance with this section.

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502.20 Minimum Acceptable Construction Practices – Professional standards in accordance with the Contractor’s QC Plan and construction industry best practices are a core requirement of the Work. Any construction practice or activity that results in an obvious defect must be corrected by the Contractor. Construction practices shall include but are not limited to the following:

502.21 Prime Coat and Tack Coat - Applications of prime coat and tack coat are required and shall be applied unless otherwise directed by the Ministry Representative.

502.21.01 Surface and Weather Conditions – The liquid asphalt for prime coat or tack coat shall be applied when surface and weather conditions are favourable. The application of prime coat and tack coat shall meet manufacturer’s requirements.

502.21.02 Spray Temperature – The liquid asphalt shall be sprayed within the temperature range specified by the supplier.

502.21.03 Prepared Granular Bases and Old Pavements – All prepared granular bases, old pavements, and portions of all other infrastructure that will be in contact with the new pavement paved shall be prime coated or tack coated at specified rates.

502.21.04 Application – The spray bars shall produce double coverage at one pass, with uniform spray and even pressure with application rates controlled to within ±25% of the specified application rate.

Spray bar nozzles shall be of the same type and size, set to produce uniformly fan-shaped sprays without atomization.

502.21.05 Spraying Faults – Any spraying faults shall be corrected by the Contractor.

502.21.06 Excess Liquid Asphalt – Excess liquid asphalt remaining unabsorbed shall be blinded with sand or fine aggregate at no expense to the Ministry.

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502.21.07 Surface Condition – The surface to be tack coated shall be cleaned of dirt or other foreign material.

502.21.08 Application – Tack coat shall not be applied on sections of roadway longer than will be covered by one day of asphalt plant production.

502.21.09 Traffic – Traffic shall not be permitted on tack coat until it has cured and on prime coat until the primer has been absorbed into the granular surfacing.

Where it is not possible to keep the traffic off the treated surface, the surface shall be blinded with sand or fine aggregate, at no expense to the Ministry, prior to allowing traffic on surface.

502.21.10 Curing – Prime coats shall be allowed to cure for a minimum of 24 hours or to the satisfaction of the Ministry Representative prior to the placing of asphalt mix and tack coats shall be allowed to cure to a state that minimizes tracking prior to the placing of asphalt mix.

502.21.11 Maintenance - The Contractor shall, at the Contractor's expense, maintain the prime coat and/or tack coat.

502.22 Adjacent Mats, Joints, Edges, and Let-downs

502.22.01 Longitudinal Joints – Longitudinal joints in the Top Lift of asphalt pavement will only be permitted where lane dividing lines are to be painted. Longitudinal joints in a Lift shall be offset within 150 to 300 mm from joints in the underlying Lift. Joints shall be pinched as soon as practicable, using best compaction practices.

502.22.02 Longitudinal Edge – Any longitudinal edge that has been damaged by traffic or equipment shall be trimmed to provide a vertical abutting face. Vertical surfaces of roadway appurtenances shall be tack coated to the top of the new pavement only.

502.22.03 Contact Edge – The contact edge of abutting cooled asphalt pavement shall be thoroughly painted with a uniform coat of emulsified asphalt.

502.22.04 Paving Adjoining Mats – When paving an adjoining parallel mat there shall be an overlap onto the previously paved surface of 25 ±15 mm. This overlap shall be properly constructed to form a homogeneous bond between the two mats.

502.22.05 Disposal of Raking Material – Any raked material from the joint shall not be placed on the new mat or placed in or in front of the paver. The Contractor shall dispose of the material in a manner acceptable to the environmental agencies and the Ministry Representative.

502.22.06 Discontinued Paving – When paving is discontinued in any lane on a traffic bearing roadway, the asphalt pavement shall be tapered down at a minimum slope of 25:1. When paving resumes this letdown shall be cut back so as to form a vertical face that matches the required depth being paved. Material removed from the taper shall

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be disposed of in a manner acceptable to the environmental agencies and the Ministry Representative.

502.23 Stockpiling Aggregates

502.23.01 Stockpile Construction – Stockpiles shall be constructed as specified in SS 202.21.

502.23.02 Stockpiles for Different Materials – Stockpiles of different types of material shall be located and constructed in such a manner as to prevent intermingling of the types and to prevent segregation.

502.23.03 Stockpile Requirements Prior to Mix Production – Before plant mixing commences, the Contractor shall have in stockpile a minimum of 20,000 tonnes of asphalt mix aggregate or 50% of the total quantity of asphalt aggregate required for the mix, whichever quantity is greater. These quantities shall be maintained throughout the crushing period. These requirements may be waived by the Ministry Representative in circumstances where such stockpiles cannot be accommodated, such as where materials are being barged in or are being hauled to the plant from a remote site.

502.24 Mix Production

502.24.01 Mixing Temperature – The temperature of the asphalt mix measured at the plant discharge chute shall be maintained at ±15°C of the Design Mixing Temperature designated in the accepted Mix Design, with adjustments within that range made at the Contractor's discretion. Where the Contractor plans to adjust the actual mix temperature to 10°C or more above the Design Mixing Temperature, the Contractor shall notify the Ministry Representative prior to making the adjustment. To optimize mix properties during inclement weather or to address other specific circumstances, the Ministry Representative may agree, in advance, to a higher mixing temperature.

Mix produced at a temperature above the upper tolerance limit may be deemed Reject Mix by the Ministry Representative.

502.24.02 Residual Moisture Content – The residual moisture content of the asphalt mix prior to compaction shall not exceed 1% by mass. Should foaming or bubbling persist, even below the allowed percentage of moisture, the Contractor shall modify operations accordingly.

502.25 Placing the Asphalt Mix

502.25.01 Surface Condition – Asphalt mix shall only be placed on clean dry surfaces free from all foreign materials, and when weather and conditions are suitable. Normally, asphalt mix is only placed when the ambient air temperature is 5°C and rising and for Top Lift when the average surface temperature is also above 5°C.

502.25.02 Asphalt Levelling Course Requirements – Asphalt Levelling Course shall be placed in one or more Lifts, with a maximum thickness of 75 mm per Lift. The amount of Levelling Course placed shall not exceed the

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quantity listed in Schedule 7 unless otherwise authorized in writing in advance by the Ministry Representative.

All asphalt mix shall be paver laid, unless otherwise authorized by the Ministry Representative.

Any mix laid prior to the establishment of the Job Mix Formula (per SS 502.08.10) shall normally be placed in a location where performance of the mix is of lower concern, typically as level course, on a low volume side road, and/or in Bottom Lift. The location proposed by the Contractor shall be subject to the authorization of the Ministry Representative.

502.25.03 Lay-Down Operation – Asphalt mix should be delivered to the paver at a constant rate sufficient to allow continuous placement.

502.25.04 (not used)

502.25.05 Persons in the Vicinity of the Paver – When in the vicinity of an operating paver and in front of the screed, workers and inspectors shall stand in full view of the paver operator, clear of side arms, wings, and screed, and never directly in front of the machine hopper.

502.25.06 Vibrating Tampers and Hand Rollers – Along curbs, manholes, headers and similar structures which are not accessible to rollers, thorough compaction shall be obtained, by use of vibrating tampers and hand rollers or other suitable methods.

502.25.07 Traffic – Traffic will not be permitted on the finished pavement until it has cooled to such a temperature as to ensure that no deformation or flushing of the surface will occur.

502.25.08 Release Agents – Hydrocarbon-based release agents are not permitted. Other release agents may be used in accordance with their manufacturer's recommendations. Application rates of any release agent (including water) shall be limited to the minimum necessary, without any excess, to achieve the desired result.

502.25.09 Ponding – Areas of ponding will be noted as an obvious defect and remedial works will be required.

502.26 Shoulder Build-up – Shoulder build-up is typically required where an existing roadway is being overlaid with wider shoulders. It involves construction of base coarse aggregate (adjacent to a Lift of pavement) which will be overlaid by one or more Lifts of asphalt. Shoulder build-up shall be constructed:

- (a) In accordance with the SS 202.26;
- (b) To the lines and grades indicated in the Contract, generally to a finished compacted level that matches level and grade of the adjacent asphalt level; and
- (c) With cross-fall that matches that of the finished asphalt surface above the shoulder build-up area.

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502.27 Shouldering

502.27.01 Shouldering Aggregate – Shouldering aggregate shall be conform to SS 502.06.04

502.27.03 Lines and Grades – The finished surface of the granular shoulder shall be true to grade and cross section, and be free from ruts, segregation or other irregularities.

502.27.04 Aggregate Segregation – All granular shoulder materials shall be handled in such a manner that segregation does not occur.

502.27.05 Moisture Adjustment – If necessary, for compacting, the moisture content of the shoulder aggregate shall be adjusted through either drying or applying water. Addition of water is incidental.

502.27.06 Compaction – Shoulders shall be thoroughly compacted to a state that will not rut more than 5 mm under a pick-up truck steering axle wheel load.

Where the base under the shoulder has been constructed as part of the Work, the degree of compaction of the granular shoulder measured in accordance with ASTM D6938 shall be greater than 98% of the Standard Proctor maximum dry density determined in accordance with ASTM D698.

502.27.07 Adjacent Pavement Damage – If the adjacent asphalt pavement is damaged during the shouldering operation, the damage shall be repaired to the satisfaction of the Ministry Representative, at no expense to the Ministry.

502.27.08 Interim Shouldering – For safety reasons, when a roadway with gravel shoulders is used by the travelling public, an intermediate layer of shouldering aggregate shall be placed following final rolling of levelling course or multiple Lifts of asphalt pavement thicker than 50 mm, in advance of the main shouldering operation.

Should the Work extend past the Completion Date and be carried over to the next paving season suitable interim drainage control measures shall be put in place, all at no cost to the Ministry.

PAYMENT

502.30 General – Payment at the Unit Prices for the supply of primer/tack coat materials, the application of the spray primer/tack coat, the supply of paving aggregates into stockpile, the construction of asphalt pavement including the supply of asphalt cement, and for shouldering shall be full compensation for completing the supply and installation of asphalt pavement and shouldering on prepared surfaces in accordance with the contract requirements. Applicable payment adjustments (Additions or subtractions as applicable) shall be applied in accordance with the Payment Adjustment section of this Section.

Compensation for Quality Control shall be at the price bid for Quality Control or, in the absence of such a bid Item in

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Schedule 7 – Approximate Quantities and Unit Prices, incidental to the Work.

502.31 Acceptance at Adjusted Payment – Acceptance of any Lot at adjusted payment will occur if it complies with the requirements of the QC Plan and the sections on Equipment and Plant; and Construction of this Specification, contains no obvious defects as per SS 502.32, and if:

- Test results for EPS acceptance parameters (density, gradation, asphalt content, smoothness, segregation, and application rate) are such that the Lot meets the requirements for acceptance at a reduced payment;
- Lot is acceptable in respect of all other requirements; and
- Contractor has not notified the Ministry Representative in writing that it will exercise its option to either repair or remove and replace the work, at its own cost, with work meeting the requirements for acceptance at full or increased payment.

502.32 Rejection for Workmanship Defects – Work may be rejected if it does not comply with the requirements of the QC Plan and the Construction section of this Section.

Additionally, the finished surface of any Asphalt Pavement Lift shall have a uniform texture and be free of visible signs of poor workmanship. Any obvious defects as determined by the Ministry Representative such as, but not limited to the following, may be cause for automatic rejection of asphalt pavement regardless of the values of any other acceptance parameter:

- Individual bumps and dips that exceed 12 mm over 3 m;
- Areas of excess or insufficient asphalt;
- Improper matching of longitudinal and transverse joints;
- Roller marks or roller pick-ups;
- Excess tracking of prime or tack coat;
- Areas contaminated by fuel oil or other deleterious materials;
- Tire marks; or
- Cracking or tearing.

When asphalt pavement is rejected because of obvious defects, the minimum area of rejection will be the actual length of the defect for the full width of the driving lane in which the defect exists.

Rejected work shall be promptly repaired, remedied, overlaid, or removed and replaced in a manner acceptable

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to the Ministry Representative. The Contractor shall be responsible for all costs including materials.

No payment will be made for work in any Lot or Sub-Lot, which has been rejected, until the defects have been remedied.

502.33 Partial Payment for Rejected Work – In the Ministry Representative’s sole discretion and without setting precedence, where any Work is reject but the Ministry Representative determines that it may be left in place, the Ministry Representative may authorize partial payment to the Contractor as full compensation for any residual value the Work may have. Notwithstanding the foregoing, the Ministry is under no obligation to make any payment for such Work.

502.34 Primer and Tack Coat

502.34.01 Supply of Primer and Tack Coat – Payment to Supply Primer and Tack Coat shall be at the Unit Price per litre for the quantity of material actually sprayed onto the roadway measured prior to any additional diluting by the Contractor where applicable. The measured quantity shall not be greater than the total of the bills of lading.

Such payment shall be full compensation for supplying, handling, storing, sampling and testing of the material and all other related work.

502.34.02 Application of Primer and Tack Coat – Payment for the application of Primer and Tack Coat will be at the Unit Price per litre of primer actually sprayed onto the roadway. In the case of an emulsion, the payment quantity shall include any water added by the Contractor with the authorization of the Ministry Representative.

Such payment shall be full compensation for diluting the material when required and spraying it on the surfaces to be paved, and for all incidental costs arising from priming and tack coating.

502.35 Paving Aggregate into Stockpile – Payment for Paving Aggregate into Stockpile shall be at the Unit Price per tonne for the quantity of paving aggregate placed into stockpile in accordance with the following.

As the aggregate is crushed into stockpile, progress payments will be made against the bid item, up to the quantity shown in the “Approximate Quantity” column of the Schedule 7 - Schedule of Approximate Quantities and Unit Prices.

The final payment quantity for paving aggregate quantity will be equal to the corresponding asphalt mix quantity actually laid, without any adjustment for AC or moisture content. As a result, the final quantities and payments may vary from those used in prior progress payments; down if the mix quantity underruns and up if it overruns.

Such payment shall be full compensation for all work including but not limited to the production, supply and stockpiling of all paving aggregates.

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502.36 Asphalt Pavement – Payment for Asphalt Pavement constructed in place shall be at the Unit Price per tonne for the quantity of mix placed in accordance with the Contract requirements.

Subject to the exception noted below, only acceptable asphalt pavement will be included in the payment quantity.

Where overlays are used as a corrective measure, in accordance with SS 502.58:

- The overlay quantity will not be included in the payment quantity, but
- The quantity of asphalt pavement covered by the overlay will be included in the payment quantity, whether or not it was acceptable.

Such payment shall be compensation in full for all work including but not limited to: loading the aggregate into the feeders; drying the aggregate; supplying, metering, and adding the asphalt cement; mixing, loading, weighing, hauling, dumping, spreading, compacting and finishing the asphalt pavement.

502.37 Asphalt Mix Antistrip Additives

502.37.01 Where Specified in the Special Provisions – Where the Special Provisions direct the addition of an antistrip additive, all costs associated with supplying, storing, and blending the additive into the asphalt mixes shall be incidental to the prices bid for the varying classes of asphalt mix.

502.37.02 Where Not Specified in the Special Provisions – Where the Special Provisions do not direct the addition of an antistrip additive but the need was identified in accordance with SS 502.08.03, compensation for supplying, storing and blending the additive into the asphalt mixes shall be made at a negotiated price or on a Force Account Basis and any authorized payments will be made from the Provisional Sum for Site Modifications.

502.38 Asphalt Cement “Bump” – The Contractor will be compensated for additional oil ordered by the Ministry Representative in accordance with SS 502.08.09 at the AC cost FOB the asphalt plant, demonstrated by the AC supplier’s invoice, with mark-up on a Force Account Basis for the material only.

Payment will be calculated in accordance with Equation 502-1, on a Lot-by-Lot basis based on the difference in asphalt cement content determined from Ministry Quality Assurance (QA) samples and that in the Contractor’s production Job Mix Formula, to a maximum of the bump and a minimum of the Contractor’s proposed Job Mix Formula AC content. No compensation will be made for additional AC in any rejected mix.

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Equation 502-1: Payment for Asphalt "Bump"

$$\$_{Lot\ n} = 110\% \times Cost_{AC} \times t_{Lot\ n} \times \left[\left(\frac{AC_{Burn}}{100 + AC_{Burn}} \right) - \left(\frac{AC_{JMF}}{100 + AC_{JMF}} \right) \right]$$

Where:

$\$_{Lot\ n}$ = Compensation due for additional AC in Lot “n”

$Cost_{AC}$ = Supplier’s invoiced cost for AC, dollars per tonne

$t_{Lot\ n}$ = Tonnes of asphalt mix laid and accepted in Lot “n”, including AC

AC_{Burn} = Asphalt cement content (% dry mix) of Lot determined from Ministry QA samples, to a maximum of the bumped AC content and a minimum of AC_{JMF}

AC_{JMF} = Asphalt cement content (% dry mix) of Contractor’s production Job Mix Formula before the bump

Note: 110% includes the percentage markup for materials paid on Force Account

502.39 Payment for Rejected Work Made Acceptable

– When defects have been remedied in Lots or Sub-Lots which had been rejected, payment for the original quantity of material in those Lots or Sub-Lots will be made subject to payment adjustments and penalty assessments and subject to SS 502.58.

No payment adjustment will be made for any material used to replace, repair or overlay rejected work and all corrective work shall be performed entirely at the Contractor’s expense.

502.40 Shouldering – Payment for Shouldering will be at the Unit Price bid per tonne or cubic metre (to the neat lines), whichever is specified in the Schedule 7 - Schedule of Approximate Quantities and Unit Prices. The price bid shall be full compensation for supplying, hauling, placing, moisture adjustment, and compaction of the aggregate to the required grades and crossfall.

502.41 Surplus Aggregate

502.41.01 Surplus Aggregate in Private Pits – At the discretion of the Ministry Representative, the Ministry may or may not purchase surplus aggregate in stockpile in a private pit. Generally, the Ministry will provide payment for the processing costs of surplus aggregate in stockpile in Ministry Pits only, and only to a limited quantity. However, should the Contractor produce surplus aggregate in a private pit, and the Ministry intends to purchase these surplus aggregates, the Contractor shall be required to provide a written agreement with the owner of the property. This document shall indicate that the Ministry will have free access to and use of the surplus aggregate in stockpile for a period of 12 months after the completion of the contract work. If the Contractor undertakes private work from

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within the private pit, measurements for surplus aggregate in stockpile will not be taken until the completion of the private work, ensuring that the Ministry does not pay for aggregate used on private works. All surplus aggregate shall be properly stockpiled.

502.41.02 Ministry Purchase of Surplus Aggregate - Should the Ministry proceed with the purchase of surplus aggregate, upon completion of the contract, the Ministry will purchase surplus paving mix aggregate as indicated herein.

The provisions below apply only to AMA, including AMA used as shouldering aggregate. All other surplus aggregates, including grading aggregates used as shouldering surfacing, base and subbase aggregates will be paid in accordance with SS 202.

If the quantity of Asphalt Pavement actually incorporated into the works is less than the estimated quantity, as stated in the Schedule 7 - Schedule of Approximate Quantities and Unit Prices, the Ministry will purchase aggregate up to 100% of the quantity required by the contract, to be paid under the appropriate Unit Price Item for Aggregate in Stockpile. The surplus aggregate must, when singly or combined, meet the gradation requirements set out in the Job Mix Formula.

The portion of material remaining in a stockpile eligible for compensation as surplus aggregate shall be the lesser of:

- (a) The quantity of mix as stated in the Schedule 7 - Schedule of Approximate Quantities and Unit Prices minus the quantity of aggregate actually incorporated in the works, and
- (b) The volume of the remaining stockpile as determined under SS 502.41.03

No payment shall be made for any surplus shoulder and paving aggregate remaining at contract completion in excess of the contract requirement. Material remaining in a Ministry pit is the property of the Ministry.

502.41.03 Measurement of Surplus Aggregate - The Ministry will determine the volume of aggregate by surveying the stockpile(s) using string-line techniques, and determine volume using prismatic volumes between surfaces. Alternatively, the Ministry Representative may elect to survey using cross-section techniques and/or determine volumes using end-area volumes.

The base of the pile will be determined from a pre-stockpile survey or, where such a survey is not available, from a surface determined by the Ministry Representative as being a reasonable interpolation from the intersection of the pile sides with the adjacent ground level. Where there are two or more stockpiles of aggregate meeting the same gradation classification, the compensation will be based on the cumulative quantity of those stockpiles.

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Where necessary, stockpile volume shall be converted to mass using the conversion factor of 2.0 tonnes per cubic metre.

PAYMENT ADJUSTMENTS

502.50 Payment Adjustment – Payment adjustments resulting from the application of this Section will be effected on each progress payment as follows.

502.50.01 Density, Asphalt Content, and Gradation – For each Lot paid for by the tonne (t), the applicable payment adjustment derived from Table 502-I or Table 502-J (Density), Table 502-K (Asphalt Content), and Table 502-L (Gradation), in dollars per tonne, will be expressed as positive in the case of increases and negative in the case of decreases.

The algebraic sum of these unit adjustments will then be applied to the payment quantity for the Lot. The resulting amount, in dollars, will be the net payment adjustment, positive or negative, for that Lot.

502.50.02 Application Rate – Payment Adjustments from Table 502-M (Application Rate) shall be computed for the Lot.

The algebraic sum of the net payment adjustments for all such Lots for which payment is authorized on the current progress payment, computed in dollars, shall be the total payment adjustment for density, asphalt content, aggregate gradation, and material application rate for the current progress payment.

502.50.03 Segregation and Smoothness – For each Lot with payment adjustments based on kilometre (km), the applicable unit adjustment derived from Table 502-O (Segregation) and Table 502-P (Smoothness), in dollars per Lot (\$/lane km), will be expressed as positive in the case of increases and negative in the case of decreases, and will be the payment adjustment, positive or negative, for that Lot.

The algebraic sum of the payment adjustments for all such Lots for which payment is authorized on the current progress payment, computed in dollars, shall be the total payment adjustment for segregation and smoothness for the current progress payment.

502.50.04 Total Payment Adjustment – The algebraic sum of the total payment adjustments for density, asphalt content, aggregate gradation, material application rate, segregation and smoothness, derived in accordance with SS 502.50.01 through SS 502.50.03 above, shall be the total payment adjustment, positive or negative, in dollars, for all attributes for the current progress payment.

The total payment adjustment will be made by a single entry in computing the current progress payment.

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502.50.05 Progress Payments – The process set out in SS 502.50.01 through SS 502.50.04 above will be used in computing each progress payment to which it is applicable.

502.51 Initial 1,000 tonnes of Mix - For the first 1,000 tonnes of asphalt mix produced under a Contract, the following provisions take precedence over all other payment and payment adjustment provisions of SS 502.52 through SS 502.57 inclusive, but do not take precedence over the rejection criteria.

502.51.01 Payment Adjustments – Unless requested otherwise by the Contractor in writing in advance of mix production, the first 1,000 tonnes of asphalt mix production and placement shall not be subject to the bonus/penalty payment adjustments for AC content, density and gradation. Payment adjustments will apply to smoothness, segregation and application rate if the mix is applied in a Top Lift location.

502.51.02 Acceptance Limits – Contrary to any other provision of SS 502, any mix produced during the initial 1,000 tonnes will only be considered acceptable if:

- (a) Asphalt Mix Design has been accepted by the Ministry Representative in accordance with SS 502.08.05;
- (b) Aggregate gradation per SS 502.08.02 is within the gradation limits specified in Table 502-D or the banana formed by applying the Table 502-L column 3 variation limits to the JMF gradation;
- (c) All other properties fall inside the allowable limits specified, in SS 502 and the Special Provisions, for the mix, including
 - For Marshall mixes, Table 502-E; and
 - For Superpave mixes, Table 502-F.
- (d) AC content of the sample is within $\pm 0.5\%$ of the design value for top lift and is within $\pm 0.55\%$ of the design value for the bottom lift.

All values are measured against the Asphalt Mix Design value at the time the sampled mix was produced.

Any mix with any characteristic outside the above limits is Reject Mix.

Additionally, rejection limits for smoothness, segregation, density and application rate shall apply in accordance with SS 502 and the Special Provisions.

502.52 Density

502.52.01 Lot – A Lot for density shall be one day's scheduled production of at least 7 hours plant production where no changes have occurred to criteria such as but not limited to:

- Accepted Job Mix Formula;
- The specific Lift that is being placed; and
- The required material application rate.

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A change in any above criteria may require a new Lot designation.

Where one day's production is less than 7 hours, the material will be added to the next Lot that has the same criteria, as described above, except that if a test indicates that this production is subject to a payment adjustment or to rejection, or if no further material will be produced with the same criteria, this production will be designated as a separate Lot.

A Lot shall be no more than two days total production even if the above criteria have not changed or been met.

502.52.02 Sub-Lot – For density, a Lot shall be divided into three equal Sub-Lots, defined by lineal metres of production

502.52.03 Coring – The Contractor shall be responsible for providing all core samples for quality assurance and payment adjustment purposes. The randomly selected locations for the cores shall be supplied by the Ministry Representative to the Contractor. The Contractor shall provide cores sized in accordance with Table 502-H. Coring shall occur in the presence of a Ministry Representative, within two days after the Lot has been completed and prior to application of any subsequent lift. The Contractor shall prepare the cores prior to the submission by removing all material not representative of the pavement Lift to be tested. The Contractor shall deliver all cores taken (even those damaged during extraction) to the Ministry Representative, within 24 hours of being provided the locations for the coring, to a designated safe storage location.

Table 502-H: Test Sample Size and Source Locations

Test	Sample Source
Density	150 mm dia. road cores for Superpave, Coarse Mix, and any other product with maximum aggregate size greater than 25 mm. 100 mm dia. road cores for all other mixes
AC Content	Loose Mix Samples
Smoothness	Centre of the Lane
Aggregate Gradation	Loose Mix Samples
Segregation	Roadway Pavement

The Contractor shall fill all core holes before the roadway is re-opened to traffic. Core holes shall be filled by the following method:

- Empty the hole of water and loose material.
- Remove any excess moisture by wiping the inside with a dry towel.

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- Tack coat the inside surfaces and the outside perimeter with an emulsified asphalt.
- Place asphalt mix in loosely, so that the compacted Lifts do not exceed 75 mm.
- With a minimum of 20 blows per Lift, compact the loose material using a minimum 2 kg sledgehammer and tamper.
- For additional Lifts repeat Steps (a) to (e).
- The final Lift shall be a minimum thickness of 25 mm and finished to a level higher but not exceeding 6 mm, than the elevation of the surrounding pavement.

The Contractor may use an alternative method if acceptable to the Ministry Representative.

All costs associated with obtaining the cores, including the filling and compaction of the core holes are considered incidental to the Contract and are the responsibility of the Contractor.

502.52.04 Percent Density – The average in-place density will be determined from core samples of the completed Lift of pavement.

One random core sample will be obtained from each Sub-Lot and the test results for the three Sub-Lots will be averaged to determine the percent density for the Lot.

- (a) **Marshall Mixes** – For the determination of Marshall Briquette Density, the Contractor shall obtain the samples as outlined in Appendix 502-2 for each Sub-Lot. All costs associated with sampling, shall be the Contractor's responsibility.

From each Sub-Lot sample, the Contractor will form three briquettes as per the procedure identified in Table 502-A and tested, and the ASTM D2726 test results will be averaged to obtain the test value for that sample.

The Marshall Briquette Density for a Lot is the average of the test results from the three Sub-Lots. The Marshall Briquette Density value for the Lot shall be provided to the Ministry Representative prior to the provision of the coring locations to the Contractor.

Equation 502-2: Marshall Percent Density

$$\text{Marshall Percent Density} = \left(\frac{\text{In-place Density of Sample}}{\text{Marshall Briquette Density}} \right) \times 100$$

At the discretion of the Ministry Representative, the Ministry may at any time, perform and use the Ministry results in accordance with procedures as outlined above, to obtain the value for Marshall Briquette Density which will replace the values achieved under Quality Control Testing.

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Should the initial test results in any Sub-Lot be lower than 97.0%, one additional core will be taken and tested, and that result shall be averaged with the initial result to determine the new percent density for the Sub-Lot.

In addition, the Contractor shall determine the individual Maximum Theoretical Densities (MTD) per ASTM D2041 for each of the Sub-Lot samples and average the results of them to obtain the Lot MTD. The Contractor shall report the Lot MTD to the Ministry Representative prior to the provision of the coring locations to the Contractor.

- (b) **Superpave Mixes** – The Contractor shall determine the individual Maximum Theoretical Densities (MTD) per ASTM D2041 for each of the Sub-Lot samples and average the results of them to obtain the Lot MTD. The Contractor shall report the Lot MTD to the Ministry Representative prior to the provision of the coring locations to the Contractor.

At the discretion of the Ministry Representative, the Ministry may at any time use the same procedures to obtain the values for Maximum Theoretical Density which will replace the values determined by the Contractor.

The average in-place density will be determined from core samples of the completed Lift of pavement. Cores shall be sized in accordance with Table 502-H.

Equation 502-3: Superpave Percent Density

$$\text{Superpave Percent Density} = \left(\frac{\text{In-place Density of Sample}}{\text{Lot MTD (ASTM D 2041)}} \right) \times 100$$

The Maximum Theoretical Density for a Lot is the value calculated, defined as the average of the test results, from the three Sub-Lots. The Maximum Theoretical Density value for the Lot shall be provided to the Ministry Representative prior to the provision of the coring locations to the Contractor.

Should the initial test results in any Sub-Lot be lower than 92.0%, one additional core will be taken and tested, and that result shall be averaged with the initial result to determine the new percent density for the Sub-Lot.

- 502.52.05 Payment Adjustments** – The payment adjustment for percent density will be the amount shown in Table 502-I or Table 502-J, as applicable, for the Sample Mean of the test results for the Lot.

502.52.06 Rejection Limit – The rejection limit for percent density is the limiting value of the Sample Mean as shown in Table 502-I or Table 502-J, as applicable. If any Sub-Lot has a value within the reject zone, the Sub-Lot is rejected and not paid for.

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Table 502-I: Payment Adjustments for Density – Marshall

Marshall % Density Lot Average	Payment Adjustment (\$ per tonne)
≥ 98.5	+\$2.00
≥ 98.0 to < 98.5	+\$1.50
≥ 97.5 to < 98.0	+\$1.00
≥ 97.0 to < 97.5	+\$0.50
≥ 96.5 to < 97.0	-\$1.00
≥ 96.0 to < 96.5	-\$2.00
< 96.0	REJECT

Table 502-J: Payment Adjustments for Density - Superpave

Superpave % Density Lot Average	Payment Adjustment (\$ per tonne)
≥ 95.0	+\$2.00
≥ 94.0 to < 95.0	+\$1.50
≥ 93.0 to < 94.0	+\$1.00
≥ 92.0 to < 93.0	+\$0.50
≥ 91.6 to < 92.0	-\$1.00
≥ 91.0 to < 91.6	-\$2.00
< 91.0	REJECT

Acceptance or rejection will be determined for each Sub-Lot. One opportunity will be provided to the Contractor, using non-destructive test methods, to isolate the area of low density and conduct additional compaction providing no damage is incurred to the new pavement. The Contractor shall mark out the extents of the area identified using chalk or other temporary marking acceptable to the Ministry Representative. One additional core will be taken randomly in the area of low density and will replace the core with previous low density and a new value will be calculated. The Ministry Representative shall be advised as to the scheduling of the re-compaction effort prior to the work taking place.

Where any Marshall Mix core density is less than 96% (91% for Superpave mixes), the Contractor shall either overlay or remove and replace the previously placed area of reject mix. If the test result for density of a Sub-Lot is outside the acceptance limits, the Sub-Lot is rejected automatically regardless of the values of other acceptance parameters. To minimize the cost of rejection to the Contractor, the Contractor will be provided the opportunity to isolate the area of low density within the Sub-Lot. The limits of the low-density area must be verified and accepted by the Ministry Representative before remedial work proceeds.

502.52.06 Payment Adjustment for Density Rejected Work Made Acceptable – The payment adjustment for density will be based on testing of the replacement or overlay material where applicable. Where replacement or overlay material does not cover the entire Lot or Sub-Lot, prior tests of the uncovered area will be averaged with new tests on the corrective work.

502.53 Asphalt Content

502.53.01 Lot – A Lot for AC content shall be one day’s scheduled production of at least 7 hours plant production where no changes have occurred to criteria such as but not limited to:

- Accepted Job Mix Formula;
- The specific Lift that is being placed; and
- The required material application rate.

A change in any above criteria may require a new Lot designation.

Where one day’s production is less than 7 hours the material will be added to the next Lot that has the same criteria, as described above, except that if a test indicates that this production is subject to a payment adjustment or to rejection, or if no further material will be produced with the same criteria, this production will be designated as a separate Lot.

A Lot shall be no more than two days total production even if the above criteria has not changed or been met.

502.53.02 Sub-Lot – For AC content, a Lot shall be divided into three equal Sub-Lots, defined by lineal metres of production.

502.53.03 Determination of Asphalt Content – The asphalt content of the asphalt mix will be determined from loose mix samples obtained from each Sub-Lot (3 per Lot) and tested in accordance with the ASTM D6307 test procedures listed in Table 502-A.

One random sample will be obtained from each Sub-Lot as per Appendix 502-B. The asphalt content of the asphalt mix will be determined using the average of the results obtained from all of the Sub-Lot samples. The actual Asphalt Content of the Lot will be compared to the Job Mix Formula Asphalt Content and the deviation will be used for payment adjustment purposes.

The ignition oven calibration factor will be applied to the AC Content measured by the ignition oven, and the corrected AC Content used to determine acceptability of the mix and any payment adjustments.

502.53.04 Payment Adjustments – The payment adjustment for asphalt content will be the amount shown in Table 502-F, where deviation in AC Content is determined by the percent difference, based on kg of AC per 100 kg of dry aggregate, between the JMF AC content and the actual AC Content.

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502.53.05 Rejection Limit – Rejection limits for asphalt content are the limiting values of the Sample Mean as shown in

Table 502-K, beyond which the Lot is rejected and not paid for.

If the asphalt content of a Lot is within the reject zone, the Lot is rejected automatically regardless of the values of other acceptance parameters.

For Top Lift deviations of more than 0.50%, the Contractor shall either overlay or remove and replace the Lot.

For lower Lift deviations of more than 0.55%, no payment will be made, and the Ministry Representative will determine whether removal and replacement is necessary.

Table 502-K: Payment Adjustments for Deviation of Asphalt Content (AC)

Differences of Actual AC Content from Designed AC Content Specified in JMF (AC in %)		
Deviation from Asphalt Mix Design JMF	Payment Adjustment \$ per tonne	
	Top Lift	Lower Lifts
-0.56 or less	REJECT	REJECT
-0.55 to -0.51	REJECT	-9.00
-0.50 to -0.46	-8.00	-8.00
-0.45 to -0.41	-7.00	-7.00
-0.40 to -0.36	-5.00	-5.00
-0.35 to -0.31	-3.00	-3.00
-0.30 to -0.21	-1.00	-1.00
-0.20 to -0.06	0.00	0.00
-0.05 to +0.15	+2.00	+2.00
+0.16 to +0.30	+1.50	+1.50
+0.31 to +0.35	0.00	0.00
+0.36 to +0.40	-2.00	-2.00
+0.41 to +0.45	-3.50	-3.50
+0.46 to +0.50	-5.00	-5.00
+0.51 to +0.55	REJECT	-6.50
+0.56 or greater	REJECT	REJECT

502.53.06 Payment Adjustment for Asphalt Cement Content Rejected Work Made Acceptable – The payment adjustment for Asphalt Content will be based on testing of the replacement or overlay material where applicable. Where replacement or overlay material does not cover the entire Lot or Sub-Lot, prior tests of the uncovered area will be averaged with new tests on the corrective work.

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502.54 Aggregate Gradation

502.54.01 Lot – A Lot for aggregate gradation shall be one day's scheduled production of at least 7 hours plant production where no changes have occurred to the criteria such as but not limited to:

- Accepted Job Mix Formula;
- The specific Lift that is being placed; and
- The required material application rate.

A change in any above criteria may require a new Lot designation.

Where one day's production is less than 7 hours the material will be added to the next Lot that has the same criteria, as described above, except that if a test indicates that this production is subject to a payment adjustment or to rejection, or if no further material will be produced with the same criteria, this production will be designated as a separate Lot.

A Lot shall be no more than two days total production even if the above criteria has not changed or been met.

502.54.02 Sub-Lot – For aggregate gradation, a Lot shall be divided into three equal Sub-Lots, defined by lineal metres of production.

502.54.03 Determination of Aggregate Gradation – Table 502-D, Aggregate Gradation Limits, specifies the aggregate gradation limits, which asphalt mix design, Job Mix Formulas, and field adjustments made in accordance with SS 502.08.10 must be based.

L shows the limiting values for acceptance and for the determination of payment adjustments. These limits are expressed as departures, on each specified sieve size, from the percentage of material passing that sieve according to the Job Mix Formula. Aggregate gradation will be determined for each Lot from loose mix samples of the completed Lift of pavement. One random sample will be obtained from each Sub-Lot, and the aggregate gradation for the Lot will be determined by using the average of the 3 Sub-Lot tests to determine the Sample Mean.

Although the JMF design gradation must at all times be maintained within the limits specified in the Aggregate Table 502-D, the gradation of the mix may vary outside those limits, subject to SS 502.54.05.

502.54.04 Payment Adjustments – Where the Sample Mean for every specified sieve size falls within the limiting values prescribed in a particular column of Table 502-L, the Lot will be accepted with a payment adjustment as indicated at the bottom of that column.

502.54.05 Rejection Limit – Where one or more values of the Sample Mean for the specified sieves falls outside the limiting value specified in L, Column 3, the Lot is reject and the Contractor shall either overlay or, remove and replace the previously placed mix. The Lot will be assessed to

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determine whether it may remain in place, with payment made in accordance with SS 502.33.

Table 502-L: Payment Adjustments for Aggregate Gradation

Sieve Size mm	Divergence from JMF Grading Curve Percentage Passing by Mass (ASTM C117 and ASTM C136)		
	Column 1	Column 2	Column 3
12.5	±3.5	±4.5	±7.0
4.75	±3.0	±4.5	±6.0
0.600	±2.0	±3.5	±4.0
0.075	±0.75	±1.0	±1.5
Payment Adjustment \$ per tonne	+\$0.75	\$0.00	-\$1.50

502.55 Material Application Rate

502.55.01 Lot – A Lot for material application rate shall be one day’s scheduled production of at least 7 hours plant production where no changes have occurred to the criteria such as but not limited to:

- The specific Lift that is being placed,
- The required material application rate.

A change in any above criteria may require a new Lot designation.

One day’s production of less than 7 hours will be dealt with as follows:

- The material will be added to the next Lot that has the same criteria, as described above, except that if a test indicates that this production is subject to a payment adjustment or to rejection, or if no further material will be produced with the same criteria, this production will be designated as a separate Lot.

A Lot shall be no more than two days total production even if the above criteria has not changed or been met.

502.55.02 Sub-Lot – Sub-Lots are not used for material application rate.

502.55.03 Determination of Material Application Rate – Asphalt mix shall be applied to the roadway at the rate or rates specified in the Special Provisions, on the Drawings, at a corresponding mat thickness determined by dividing the design mix density by the specified application rate, or as otherwise directed by the Ministry Representative. Material Application Rates will be determined by the tonnage delivered to the paver as recorded by weigh tickets generated by automated scales, divided by the area covered

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by the Lot after allowance has been made for entrance letdowns and/or intersections. The Contractor shall provide the material application rate calculations to the Ministry Representative at the end of each shift.

502.55.04 Payment Adjustments – The payment adjustment for material application rate will be applied as shown in Table 502-M, based on the actual material application rate, expressed as a percentage of the specified material application rate.

502.55.05 Rejection Limits – Where actual application rate for the Lot is within the reject zone as shown in Table 502-M, the Lot is reject.

A Lot rejected for application rate may be corrected by mill-and-fill or rejected with no remedial work required, subject to the authorization of the Ministry Representative.

Table 502-M: Payment Adjustments for Material Application Rate

Actual Application Rate (Percent of specified rate)	Payment Adjustment \$ per tonne of material in the Lot (unless otherwise noted)	
	Bottom Lift or Single Lift	Top Lift of Multiple Lifts
≥ 110	-\$7.00 for all material in the Lot up to 110% and no payment for product in excess of 110.0%	-\$7.00 for all material in the Lot up to 106% and no payment for product in excess of 106.0%
≥ 106.0 to <110.0	-\$5.00	-\$5.00
≥ 105.0 to <106.0		
≥ 104.0 to <105.0	-\$1.00	-\$3.00
≥ 96.0 to <104.0	+\$0.50	+\$0.50
≥ 94.0 to <96.0	-\$2.00	-\$2.00
≥ 92.0 to <94.0	-\$3.00	-\$3.00
≥ 90.0 to <92.0	-\$4.00	-\$4.00
≥ 85.0 to <90.0	-\$7.00	-\$7.00
<85.0	REJECT	REJECT

502.56 Surface Segregation

502.56.01 Lot – A Lot shall be one kilometre length of Top Lift pavement for each driving lane.

502.56.02 Sub-Lot – Sub-Lots are not used for segregation.

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502.56.03 Determination of Surface Segregation – The finished pavement shall be homogeneous, free from segregation and shall be uniform with respect to surface texture. A segregated area is defined as an area within the driving lanes of the pavement wherein the texture differs visually from the texture of the surrounding pavement.

Due to the nature of variation in asphalt mixes and their compactive characteristics, the definition of degrees of segregation will be established on a project by project basis. The Contractor and the Ministry Representative shall establish using photographs or other mutually agreed upon tools, the definition of slight, moderate and severe segregation. The Ministry Representative and the Contractor shall evaluate the first two Lots upon completion of the second Lot, after opening to traffic, to confirm the “agreed to” guidelines. Segregation will then be evaluated only when all paving is complete.

The Ministry Representative, with the Contractor’s Representative, will observe the finished pavement to evaluate the existence, severity and extent of segregation and other surface defects. The evaluation will be completed prior to the issuance of the Completion Certificate

The rating system for categorizing the severity of Top Lift surface segregation, and repair methodology is shown in Table 502-N.

Table 502-N: Segregation – Top Lift Only – Remediation Methodology

Segregation Severity	Visual Appearance	Repair Procedures
None	Uniform surface texture.	N/A
Slight	Matrix of asphalt binder, coarse and fine aggregate exists; visually increased presence of stone sizes.	Sand cement slurry.
Moderate	Significantly more stone than surrounding pavement; matrix of asphalt binder and coated sand particles is reduced.	Seal coat or sand cement slurry patch or neat hot mix patch or mill and fill patch.
Severe	Appears as an area of very stony mix - stone against stone - little or no matrix.	Remove and replace or overlay to limits authorized by Ministry Representative.

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502.56.04 Payment Adjustments – The payment adjustment for Top Lift segregation will be the applicable amount shown in Table 502-O.

Table 502-O: Segregation – Top Lift Only – Payment Adjustments

Payment Adjustment	Number of Segregated Areas, by Category		
	Slight	Moderate	Severe
+\$1,500 per lane km. <i>(Applies only if the criteria in all three adjacent columns are achieved)</i>	0 to 3	0	0
+\$1000 per lane km. <i>(Applies only if the criteria in all three adjacent columns are achieved)</i>	4 to 6	0	0
No payment adjustment	7 to 9	0 to 1	0
-\$500 per lane km. <i>(Applies if the criteria in any one or more of the adjacent columns occurs)</i>	10 to 12	2 to 4	0
-\$1,000 per lane km. <i>(Applies if the criteria in any one or more of the adjacent columns occurs)</i>	13 to 16	5 or 6	0 to 2
-\$3,000 per Lane km <i>(Applies if the criteria in any one or more of the adjacent columns occurs)</i>	17 or more	7 or more	3 or more

502.56.05 Repair – On Top Lift all segregation, including any areas outside the driving lanes assessed for the payment adjustment, shall be repaired according to Table 502-N.

All segregation patch repairs shall be completed to a rectangular shape.

Repair shall be to the neat lines and dimensions of the segregated area using sand cement slurry or other product acceptable to the Ministry Representative.

Acceptable sand cement slurry can be made as follows, with proportions varied as needed for workability:

- 25 litres of SS-1 (or equivalent) emulsion
- 4 – 5 kg (2 shovels) of ≤ 3 mm sand
- 2 – 3 kg (1 shovel) Type GU (general use) or Type GUL (General Use Limestone) Portland cement
- Additional water, if needed for workability.

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- Or other products approved

After repairs, the Lot will be re-evaluated for acceptance, but not for segregations payment adjustments.

The Contractor shall be responsible for all costs associated with the repair of segregated areas.

502.57 Smoothness

502.57.01 Lot – A Lot for smoothness shall be one kilometre length of Top Lift pavement for each driving lane.

502.57.02 Sub-Lot – A Sub-Lot for smoothness shall be a 100 metre section of a Lot, or for the last Sub-Lot within a Lot, a section of up to 100 metres.

502.57.03 Traffic Control for Pavement Smoothness Testing – The Contractor shall provide traffic control for smoothness testing.

The Ministry Representative will provide sufficient notice to the Contractor regarding when the Ministry will perform smoothness testing. As directed by the Ministry Representative, the Contractor will be responsible to schedule and provide traffic control for the Ministry’s smoothness testing, including a Shadow Vehicle and all signage necessary. The Contractor shall also be responsible for sweeping and any other preparation work required for smoothness testing. No extra payment will be made for sweeping or other preparation work; and the provision of traffic control for smoothness testing will also be considered incidental and no extra payment will be made.

The Contractor may also be required to provide an additional Shadow Vehicle or traffic control for smoothness testing as determined by the Ministry Representative. Payment for the provisions of an additional Shadow Vehicle or traffic control, if required, will be made under the Provisional Sum Item for Site Modifications (Schedule 7).

502.57.04 Determination of Pavement Smoothness – The finished pavement surface shall be tested by the Ministry using a Class I precision rolling profile measuring instrument, to determine the longitudinal profile and compute the International Roughness Index (IRI) in each driving lane. Profiles shall be measured and the IRI calculated in the centre of the lane for each Sub-Lot.

For any Sub-Lot between 50 m and 100 m in length, the IRI value shall be considered representative of a complete Sub-Lot. For any Sub-Lot less than 50 m in length, the IRI value will be combined with the preceding Sub-Lot IRI value.

The profile shall be measured over the entire length of the pavement exclusive of structures and shoulder areas. Acceleration, deceleration and turning lanes are considered part of the driving lanes and shall be tested in accordance with this provision. For the measuring process, the Contractor shall provide the Ministry Representative a chalk guideline in the centre of the lane immediately prior to measurement.

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502.57.05 Auxiliary Lanes – For smoothness testing, sections of the driving lanes that do not fall within the continuous through lanes, such as acceleration lanes, deceleration lanes and turning lanes, and lanes which are less than 1 km in length, shall be treated as follows. The ratio of the section length to the standard Lot length of 1 km shall be determined and the payment adjustment shall be pro-rated on this basis as in the following example:

For a segment 565 metres long, the payment adjustment factor is

$$\begin{aligned} &= \text{Length of segment} / \text{Length of standard Lot} \\ &= 565 \text{ m} / 1000 \text{ m} \\ &= 0.565 \end{aligned}$$

Hence the applicable payment adjustment is 0.565 times the payment adjustment for a 1 km Lot as determined from Table 502-P.

502.57.06 Sub-Lot and Lot Acceptance Limits – The reject limit and payment adjustments applicable to a road for smoothness are specified in Table 502-P. The default values in the “Standard” column shall apply except where the Special Provisions specify that “Alternate” values apply.

The International Roughness Index (IRI) value, calculated for each Sub-Lot, will be used to determine if the Sub-Lot or Lot will be accepted, and if so whether it will be subject to any payment adjustment.

The Lot IRI is the average of the individual IRI values for the Sub-Lots within the Lot.

Table 502-P: Payment Adjustments for Smoothness

Lot IRI (m/km)	Payment Adjustment Per Lot	
	Standard	Alternate
≤ 0.80	+\$2,000	+\$2,000
> 0.80 to ≤ 0.90	+\$1,000	+\$2,000
> 0.90 to ≤ 1.00	+\$500	+\$2,000
> 1.00 to ≤ 1.10	+\$200	+\$1,000
> 1.10 to ≤ 1.20	0	+\$500
> 1.20 to ≤ 1.30	-\$100	+\$250
> 1.30 to ≤ 1.40	-\$250	0
> 1.40 to ≤ 1.50	-\$600	-\$300
> 1.50 to ≤ 1.60	-\$1,400	-\$300
> 1.60 to ≤ 1.70	-\$2,000	-\$750
> 1.70 to ≤ 1.80	-\$3,000	-\$750
> 1.80 to ≤ 1.90	REJECT	-\$1,500
> 1.90 to ≤ 2.00	REJECT	-\$2,500
>2.00	REJECT	REJECT

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A Sub-Lot is rejected if:

- It has an IRI within the applicable reject zone shown in Table 502-P, subject to SS 502.57.08;
- For Top Lift only, there are obvious defects per SS 502.32 or it has unrepaired smoothness deficiencies which require remediation in accordance with SS 502.57.09.

A Lot is rejected for Smoothness if any Sub-Lot is rejected.

502.57.07 Payment Adjustments – For Top Lift only, the payment adjustments per the applicable column of Table 502-P shall apply to each Lot.

502.57.08 Remedial Work – If the test results on a Sub-Lot of pavement indicate a payment reduction or rejection because of smoothness, the Contractor may propose remedial work to improve the smoothness. Such proposals are subject to the acceptance of the Ministry Representative, but such acceptance does not imply that the proposed remedy will be successful and does not reduce the Contractor's responsibility for meeting the acceptance requirements. Grinding may be acceptable, but an overlay may be required. Only one attempt may be made to improve smoothness, and this must be completed within ten (10) calendar days from the time the Contractor receives notification from the Ministry Representative of the original smoothness test results for that Sub-Lot.

Following any attempt to improve the smoothness of a Sub-Lot or Sub-Lots, the Ministry Representative will retest the Sub-Lot or Sub-Lots, and the new results will replace the previous data for the purposes of determining acceptance and payment.

No payment will be made for any material, equipment or manpower used to improve, or attempt to improve, smoothness.

502.57.09 Smoothness Deficiencies – Smoothness deficiencies (bumps and dips) less than 12 mm over 3 m will not have remedial work required. Individual deficiencies between 8 mm and 12 mm over 3 m will result in a \$200.00 penalty for each occurrence. Deficiencies exceeding 12 mm over 3 m will require remedial work.

502.58 Overlays as a Corrective Measure – If an overlay is used as a corrective measure on a defective Lot or Sub-Lot, the overlay thickness will be subject to the acceptance of the Ministry Representative, but shall not be less than 40 mm. In all other respects, the whole overlay will be subject to the same specifications as the pavement being overlaid. Where an overlay is used as a corrective measure in any lane, adjacent lanes shall also be overlaid whether acceptable or not.

Whether the overlay is applied as a corrective measure or is placed over otherwise acceptable pavement in order to match an adjacent lane, acceptability and payment will be determined as follows:

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- Acceptability, and eligibility for either positive or negative payment adjustment, will be determined entirely on the results of testing and observations conducted on the overlay, regardless of test results that have been obtained on the underlying, overlaid Lift of pavement; but
- The payment quantity, for application of the Unit Prices for asphalt pavement, and the quantity, to which any payment adjustment is to be applied, will be derived from the tonnage of mix in the underlying, overlaid Lift.

APPEAL TESTING

502.60 Time Limits For Appeal of Test Results – To appeal any test results, the Contractor shall serve written notice of appeal to the Ministry Representative, within the following period after receipt of the applicable test results:

- For Density, Gradation and Asphalt Content, within two (2) working days; and
- For Segregation, Smoothness and Application Rate, within three (3) working days.

502.61 Appeals of Density, Asphalt Content and Gradation – The Contractor may appeal the results of acceptance testing for density, asphalt content and/or aggregate gradation for any Lot only once. Appeals will only be considered if quality control results support an appeal and can be presented.

The Ministry Representative will arrange for an independent testing laboratory to perform the appeal testing. The personnel employed or testing laboratory retained by the Contractor for quality control testing on the project will not be used for appeal testing.

The appeal testing laboratory shall hold current certification from the Canadian Council of Independent Laboratories (CCIL) (<http://www.ccil.com/>) under the Asphalt Laboratory Certification Programs, and at least one technician in the asphalt laboratory shall hold current certification under the Asphalt Technician Certification Program.

502.61.01 Density Appeals – For density appeals, Quality Control test results for density which are provided to the Ministry Representative subsequent to the Contractor's receipt of the quality assurance test results for that Lot will not be considered (when evaluating evidence) for an appeal.

The appeal shall be for the failed sample(s) within the Lot, and there will be no appeal allowed for single tests within a Sub-Lot.

Any attempt to improve density on the appealed Sub-Lot after the Ministry Representative has tested the Sub-Lot for acceptance shall void the appeal and the original test results will apply.

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- The Contractor shall, within two (2) working days of filing the appeal and in the presence of the Ministry Representative, take five (5) core samples from random locations from a Sub-Lot;
- The Contractor shall then deliver new core samples to the Ministry Representative;
- The Ministry Representative shall deliver the core samples and the companion loose mix samples from the appealed Sub-Lots to the appeal testing laboratory;
- The appeal agency shall prepare new briquette densities from the previously taken companion samples as per Appendix 502-2;
- The appeal agency shall determine the Bulk Density (BD) / Maximum Theoretical Density (MTD) from the companion sample and the densities of the cores and report the results to the Ministry Representative and the Contractor;
- The original core test results will be discarded, and a new sample mean will be calculated from the five (5) random cores and shall be used for acceptance and payment adjustments for the Sub-Lot.

502.61.02 Asphalt Content and Gradation Appeals – For asphalt content and gradation appeals:

- The party who has possession of the companion loose mix samples shall deliver them to the appeal testing laboratory.
- The appeal agency shall prepare three (3) new samples from the companion sample and determine the AC contents and gradations, average the results, and report all results to the Ministry Representative and the Contractor.

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- The original test results will be discarded. A new sample mean for the three (3) new test results will be determined using the companion samples and will be used for acceptance and payment adjustments.

502.62 Smoothness Appeals – The Contractor may appeal acceptance test results of a Lot only once.

The Ministry Representative will perform, and the Contractor will be given the opportunity to witness, the appeal testing, and the new results will be binding on the Contractor and the Ministry.

502.63 Segregation Appeals – Appeals of segregation ratings will be handled by a joint review with the Contractor. If consensus cannot be reached then the Ministry Representative shall engage the Joint Committee, comprised of representatives from the Ministry and B.C. Road Builders, to assess the area(s) in question.

502.64 Application Rate Appeals – Appeals of application rate will be handled by a joint review with the Contractor and Ministry Representative. If the issue cannot be resolved, the issue will be resolved through the dispute resolution provisions of the Contract General Conditions.

502.65 Application of Appeal Testing Results – The appeal test values, thus determined, in all cases, will be binding on the Contractor and the Ministry.

502.66 Payment for Appeal Testing – If the new results indicate a change in the payment adjustment in the Contractor's favour, then sampling and testing costs incurred during the appeal procedures for that Lot would be borne by the Ministry.

If the new results verify that, any payment reduction or rejection remains valid for that Lot, then the costs of testing (plus 10% mark-up) incurred during the appeal procedure will be charged to the Contractor.

APPENDIX 502-A

QUALITY CONTROL REQUIREMENTS AND GUIDELINES

1.01 General

The Contract requirements for preparing, submitting and adhering to the paving components of a Quality Control Plan are specified in SS 502.04. This Appendix 502-A titled “Quality Control Requirements and Guidelines” sets out the guidelines for the Contractor’s Quality Control Plan, in addition to those set out in SS 502.04 and the Special Provisions.

1.02 Quality Control Plan

The Contractor shall prepare and submit a Quality Control Plan for review. A qualified Subcontractor or an independent organization/agency may operate the Plan wholly or in part. However, the Quality Control Plan, including compliance with the Plan and its modifications, must remain the responsibility of the Contractor.

The Plan shall address all elements that affect the quality of the hot mix, hot laid asphalt pavement, including but not limited to the following:

- Purchased Materials (asphalt cement characteristics, additives, purchased aggregates, etc.)
- Aggregate production (including source quality, gradation, fracture, crushing procedures, stockpiling, etc.)
- Calibration and Correlation of Testing Equipment (plant sensors, lab equipment, nuclear moisture/density gauges, etc.)
- Mix Design
- Asphalt Plant Mixing procedures (cold feed sampling, AC flow rate, temperature control, records, weigh scale, etc.)
- Product quality (volumetrics, EPS payment elements, etc.)
- Professional standards (joints, placing temperature, rolling procedures, etc.)

The Plan shall also include the following:

- The name of the Quality Control testing agency and its proven capability to provide the specific services required for the project.
- The list of dedicated technical staff, if available, (including names, qualifications and relevant experience) and their proposed roles.
- The list of testing equipment available for project work.

The Quality Control Plan shall include the designation of specific personnel to be responsible for specific quality control duties.

- A Quality Control Manager (QCM) responsible for the development and management of the Quality Control Plan. This person shall be qualified as per the requirements identified in SS 502.04.03. This individual shall also be responsible for signing off Quality Control Testing and Inspection records. This individual shall also be responsible for ensuring the qualifications of quality control staff, implementing and documenting any changes or improvements to the Quality Control Plan.
- There should be a designated Process Control Technician (PCT), with 5 years of related experience, who will ensure that laboratory test results and other quality control practices used to control the quality of aggregates and other mix components, and to adjust and control mix proportions to meet the mix design(s). The PCT is responsible for ensuring that testing equipment, utilized for proportioning and mixing are calibrated and in good working order. The Plan shall describe how the PCT’s duties, including sampling methods and responsibilities are to be accomplished and documented. The Plan should also describe the criteria to be used by the PCT to correct or reject unsatisfactory materials.
- There should also be a Pavement Quality Control Technician (PQT), with five (5) years of related experience, who will ensure that delivered materials meet the requirements of the specifications. In addition, the PQT shall be responsible for periodically inspecting all equipment used in placing, finishing, and compacting to assure its proper operating condition and to assure that placing, application rate, finishing, joint construction, and compaction is in conformance with this specification and the contract requirements.

1.03 Contractor’s Record of Quality Control Testing

Test results should be made on specified forms or charts immediately after completion of each test. These test results are to be made available to the Ministry Representative upon request.

Records of gradation control, both during aggregate production and during the asphalt mixing operation, should be kept on the form H0295 - Mechanical Analysis of Aggregates (available from the Ministry Representative).

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1.04 Material Application Rate

The Contractor shall control the Material Application Rate by monitoring the amount of asphalt mix delivered to the road against the area covered by checking the application rate minimally every ten loads.

The Contractor shall advise the Ministry Representative in writing on an ongoing basis of the application rate.

1.05 Density

The Contractor should take core samples to determine actual pavement density. At the start of paving, the Contractor should take a minimum of two pavement cores from each Sub-Lot. The Contractor may employ a nuclear densitometer (or moisture/density gauge) to ensure intermediate density control. Two nuclear densities may be determined for each Sub-Lot, based on job mix densities obtained from the most recent plant briquettes.

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1.06 Other Quality Control Procedures

The Contractor may initiate other Quality Control procedures as necessary for ensuring production of a quality product and include them in the Quality Control Plan. Procedures may also be introduced after the start of work as necessary as amendments to the Quality Control Plan.

1.07 Quality Control Testing Frequency

Minimum test frequencies guidelines for Quality Control are described in Table 502-Q, on the following page.

These are the minimum frequencies and the Contractor is responsible to assess the need to increase testing frequency, where aggregate source is not uniform or if any other condition exists that may warrant it. QC frequencies may be reduced below this level, subject to the Ministry Representative's authorization, should the Contractor's QC Plan be proven very effective.

Table 502-Q: Appendix 502-A - Guidelines for Minimum Test Frequencies

	ASTM Test	Minimum Frequency
Tests During Aggregate Production	C136, Dry Sieve Analysis of Aggregate	<ul style="list-style-type: none"> Split Stockpiles: 1 for each stockpile for every 2 hours of production. One main stockpile: for every 300 tonnes. Blend Sand: 1 for every 100 tonnes during stockpiling. Natural filler: 1 for every 50 tonnes during stockpiling.
	C117 Sieve Analysis of Aggregates by Washing <i>(Modified for Field Lab with drying done over a hotplate or similar heating element)</i>	
	D5821 Determining the Percentage of Fractured Particles in Coarse Aggregate	Every second coarse aggregate sieve test.
	C117 Sieve Analysis of Aggregates by Washing <i>(Modified for Field Lab)</i>	One (1) per shift on reduced sample obtained from combined samples from the crusher.
Asphalt Products Tests	Tack and Prime	Contractor's option.
Tests During Asphalt Plant Mixing	C136, Dry Sieve Analysis of Aggregate	One (1) of combined aggregate (off the belt) every 300 tonnes.
	C566 & D2216, Moisture Content	<ul style="list-style-type: none"> Aggregate: Two (2) tests per Lot Asphalt mix: One (1) on first Sub-Lot and every second day.
	C117 Sieve Analysis of Aggregates by Washing <i>(Modified for Field Lab)</i>	One (1) per shift on reduced sample obtained from combined samples from the plant cold feed.
	D5581 Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus or D6927 Marshall Stability and Flow of Asphalt Mixtures	One set of three briquettes for 1,200 tonnes or Lot, whichever is less.
	D6307 Asphalt Extraction, Ignition Method	One per Lot.
	D5 Penetration of Bituminous Materials	One per Manufacturer's Batch. Samples should be taken for every 3000 tonnes of mix production.
	D2171 Viscosity	Contractor's Option.
	D2041 Maximum Theoretical Density	One per Sub-Lot.
Test During Asphalt Paving for Density Testing	D5581 Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus or D6927 Marshall Stability and Flow of Asphalt Mixtures	One 15 kg sample for every Sub-Lot or minimum one (1) per day for field testing.
	Core Samples	At start, two cores for each Sub-Lot. After rolling pattern established, only one core for each Sub-Lot. Core sizes shall be in accordance with Table 502-H.

APPENDIX 502-B

OBTAINING AND PREPARING LOOSE MIX SAMPLES

1. Three (3) uncompacted mix samples per Sub-Lot shall be obtained by the Contractor in accordance with ASTM D979 and in the presence of the Ministry Representative or a delegate thereof. Samples shall be taken from the roadway after being laid by the paver, at locations and times chosen by the Ministry Representative. The first sample shall be used by the Contractor for Quality Control, the second sample by the Ministry for Quality Assurance, with the third retained by the Ministry for potential appeal testing.
2. The sizes of the samples taken shall meet the requirements of ASTM D979, Table 1, *Guide for Estimating Minimum Sample Quantity*.
3. The sample size shall be reduced to required laboratory sample size for Marshall and Superpave gyratory briquettes, asphalt content and hot mix gradation determination as outlined below:
 - (a) The sample shall either be reduced using a Riffle splitter or shall be quartered into four approximately equal portions. The two diagonally, opposite quarters shall be combined resulting in two samples. Identify and designate one of the samples as the Quality Companion Sample and set aside. Identify and designate the other resulting sample as the Quality Control Sample. Use the Quality Control Sample for testing.
 - (b) The Quality Control Sample shall be weighed to ensure that the sample so obtained meets the minimum mass required for the ignition test. If the sample does not meet the minimum mass requirements, then the additional materials will be obtained and added to the Quality Control Sample from the Companion Sample. This will be achieved by quartering the Quality Companion sample and adding one quarter of the Quality Companion Sample to the Quality Control Sample.
 - (c) This process is to be repeated for each sample to provide the Quality Acceptance and Appeal Samples.
4. The third sample for appeal purposes shall be set aside and retained in a Contractor-supplied, suitable container labeled with sample location, date sampled, and project information.
5. The Contractor shall deliver the QA and appeal samples to the Ministry Representative for storing.
6. The areas sampled shall be filled with mix immediately after sampling and shall receive the same compactive effort as the rest of the mat.

APPENDIX 502-C

BLANK AGGREGATE SAMPLE PREPARATION

1. Blank aggregate samples replicate the Job Mix Formula design aggregate gradation, without asphalt cement and are used to correlate the Ministry, Contractor, and appeal lab ignition ovens.
2. Twelve (12) blanks are required, each weighing 1900 ± 1 gram. This size approximates the size of the mix samples, less the asphalt cement, that will be used for the project's daily Quality Assurance testing.
3. The blanks shall be prepared from bulk aggregate samples (i.e. coarse aggregates, fine aggregates, blend sand, etc.) that were produced from the same sources and are the same materials as the aggregates that will be used for the project and have been designated in the Mix Design. The amount of each aggregate material required will be 50 kg to 60 kg.
4. The bulk aggregate samples shall first be dried, and then thoroughly blended at the design proportions. The blended material shall then be sieved into each of the individual sieve sizes designated in the Mix Design down to passing 0.075 mm.
5. The aggregate blanks will then be made up from the sieved material, starting with the pan, and progressively adding proportions from each successive individual sieve to conform to the estimated dry aggregate JMF gradation within the tolerances specified below. Prepare two (2) additional dry blank samples within the tolerances specified in Table 502-R. Run washed sieve analysis on both samples and using the average passing on the two samples to arrive at a "Target Adjustment", matching the submitted design JMF for the final blank aggregate preparation. Masses of aggregate added shall be done based on the average washed sieve adjustment as described above.
6. Masses of aggregate added shall be within the tolerances specified in Table 502-R.
7. Samples shall be sealed in cans or bagged in moisture-proof heavy gauge plastic bags, and labeled with the project number, blank number, the preparation date, and a JMF identification number.

Table 502-R: Appendix 502-C - Cumulative Mass Tolerance by Sieve Size

Sieve Size	Cumulative Mass Tolerance (g)
Pan	0.1
0.075	0.1
0.15	0.1
0.30	0.1
0.60	0.1
1.18	0.1
2.36	0.1
4.75	0.1
9.50	0.7
12.5	2.0
16.0	3.0
19.0	6.0
25.0	13.0
37.5	45.0

APPENDIX 502-D

IGNITION OVEN CORRELATION PROCEDURE

1. Asphalt Mix samples are prepared and burned to determine the effect each oven has on the mix.
2. The Contractor and the Ministry shall each prepare three (3) Asphalt Mix calibration samples by adding AC (provided by the Contractor from the supplier and batch that will be used on the project) to randomly selected aggregate blanks prepared by the Contractor in accordance with Appendix 502-C.
3. AC shall be added to the blanks at the proportion specified in the accepted JMF, within a tolerance of ± 0.1 gram.
4. Each party shall burn their mix calibration samples in their ignition oven in accordance with ASTM D6307 to determine their calibration factor.
5. The calibration factors determined above shall be compared and the inter-laboratory correlation determined.
6. Correlation between the Contractor's lab and the Ministry lab shall be completed, reviewed and mutually acceptable to the Contractor and the Ministry Representative a minimum of one (1) working day prior to placement of any mix.
7. In the event of an appeal on AC Content, the appeal laboratory shall also prepare and burn calibration samples to determine their oven calibration factor.
8. Results from testing of any asphalt mix shall report the measured AC Content and the corrected AC Content after applying the laboratory calibration factor.

APPENDIX 502-E

ASPHALT FILM THICKNESS IN BITUMINOUS MIXTURES

1. The following method is to be used to determine the asphalt film thickness on aggregate particles in an asphalt mix. The results of this calculation express the asphalt film thickness in μm (microns).
 - Percentage of Asphalt Absorption
 - Specific gravity of aggregate
 - Gradation of asphalt mix aggregate
 - Specific Gravity of the asphalt cement
2. Data Required:
 - Mix Design percentage of asphalt cement by weight of dry aggregate

Table 502-S: Appendix 502-E - Surface Area (SA) Factors

Total % Passing Sieve No.	Maximum Size	4.75 mm (No. 4)	2.36 mm (No. 8)	1.18 mm (No. 16)	0.600 mm (No. 30)	0.300 mm (No. 50)	0.150 mm (No. 100)	0.075 mm (No. 200)
S.A. (m^2/kg)	0.41	0.41	0.82	1.64	2.87	6.14	12.29	32.77

Surface Area factors shown are applicable only when all the above listed sieves are used in the sieve analysis.

This example tabulation demonstrates the calculation of surface area by this method.

Table 502-T: Appendix 502-E - Example Surface Area Calculation

Sieve Size (mm)	Percent Passing	Surface Area Factor (m^2/kg)	Surface Area (m^2/kg)
19.0	100	0.41	0.41
12.5	95		
9.5	80		
4.75	57	0.41	0.23
2.36	39	0.82	0.32
1.18	31	1.64	0.51
0.600	23	2.87	0.66
0.300	17	6.14	1.04
0.150	12	12.29	1.47
0.075	7.5	32.77	2.46
<i>Surface Area of Sample (sum of the above):</i>			7.1

$$\text{Corrected Surface Area} = \frac{2.650}{\text{Specific Gravity of the Aggregate}} \times \text{Surface Area of Sample}$$

$$\text{Effective Asphalt Cement Content} = \text{Mix Design AC} - \text{Asphalt Absorption}$$

$$\text{Film Thickness} = 10 \times \frac{\text{Effective Asphalt Cement Content}}{\text{Corrected Surface Area} * \text{Specific Gravity of Asphalt Cement}}$$

APPENDIX 502-F

MARSHALL HAMMER CORRELATION PROCEDURE

1. Overview: This procedure is used to correlate the mechanical Marshall hammers of both the Ministry and the Contractor, to produce density results that would be representative of testing on the Ministry's manual Marshall hammer.

For a given mix, this procedure gives the following results:

- D_{Manual} = the average density obtained from the Ministry's manual Marshall hammer at the nominal number of blows.
- N_{Min} and N_{Con} = the respective number of blows to be delivered by the Ministry's and Contractor's mechanical Marshall hammers.
- C_{Min} and C_{Con} = the respective density correlations for the Ministry and Contractor's mechanical Marshall hammers, to be added to subsequent density test results to approximate the densities that would be expected had the Ministry's manual Marshall hammer been used.

2. General:

- (a) Any Marshall hammer must be correlated for each mix used on each project and every time the hammer is relocated.
- (b) The Marshall hammer correlations shall be determined prior to the production of any mix.
- (c) Briquettes shall be prepared and tested in accordance with ASTM D6926.
- (d) All densities shall be determined in accordance with ASTM D2726.
- (e) The Nominal Number of Blows (" N_{Nom} ") to be applied using the Ministry's manual hammer shall be as appropriate to the pavement design requirement (Typically 75 blows for heavy traffic, although either 35 or 50 blows may be used for lighter traffic).

3. Mix Sample Preparation:

- (a) The Contractor shall prepare a minimum of thirty 1200 g (30 x 1200 g) Asphalt Mix Aggregate blanks using the Appendix 502-C procedures, supplying eighteen (18) to the Ministry and retaining twelve (12) for its own use.
- (b) The Contractor shall supply a minimum of 2 kg of the specified AC to the Ministry and retain a similar amount for its own use.

- (c) AC shall be added to the aggregate blanks at the trial JMF rate (± 1 gram); by the Ministry to each of fifteen (15) blanks and by the Contractor to each of twelve (12) blanks.

- (d) Three (3) of the aggregate blanks shall be retained by the Ministry for substitute samples or additional samples, if needed.

4. Ministry Sample Testing:

- (a) Three (3) of the Ministry's mix samples shall be compacted at N_{Nom} blows using the Ministry's manual Marshall Hammer and the average density determined (" D_{manual} ").

- (b) Twelve (12) of the mix samples shall be compacted using the mechanical Marshall hammer. Four (4) sets of three (3) samples each shall respectively receive:
 - (i) N_{Nom} minus 10 blows,
 - (ii) N_{Nom} blows,
 - (iii) N_{Nom} plus 10 blows,
 - (iv) N_{Nom} plus 20 blows.

- (i) N_{Nom} minus 10 blows,

- (ii) N_{Nom} blows,

- (iii) N_{Nom} plus 10 blows,

- (iv) N_{Nom} plus 20 blows.

- (c) The above numbers of blows may be varied at the discretion of the operator to better bracket the anticipated blows required to achieve D_{Manual} .

- (d) The average densities for each set of three (3) samples shall be calculated.

- (e) The average densities calculated above will be plotted against the number of blows received, and the best-fit curve drawn through them. If using computer-assisted calculations, a second-degree polynomial curve fit is to be used.

- (f) Where the curve established above is considered to be a poor fit, a fifth set of three (3) mix samples shall be prepared and compacted at a number of blows appropriate to best supplement the existing data. Typically, the number of blows used would be close to the number estimated to produce a density close to D_{Manual} . The results shall be added to the data set and a revised curve fitted.

- (g) Using the plotted curve or its formula, the number of blows of the mechanical hammer required to achieve the average Ministry manual hammer density shall be determined.

- (h) The above number shall be rounded to the nearest whole number to establish " N_{Min} ", the number of

blows to be delivered by the Ministry's mechanical hammers in all subsequent testing.

5. Contractor Sample Testing:

- (a) The Contractor shall repeat steps 4(b) through 4(h) inclusive on twelve (12) additional mix samples using the Contractor's mechanical hammer and determine " N_{Con} ", the number of blows to be delivered by the Contractor's mechanical hammers in all subsequent testing.
- (b) If considered necessary, the Contractor may prepare and test a fifth set of samples to supplement their data set, regardless of whether the Ministry has used a fifth set.

6. Correlation:

- (a) Using the Ministry hammer's best-fit curve established in step 4 (or the formula for that curve) determine the expected density at N_{Min} blows.
- (b) Subtract the expected density above from the average Ministry manual hammer density determined in 4(a) to determine the Ministry mechanical Marshall hammer correlation (" C_{Min} ").

- (c) Using the Contractor's best fit curve and N_{Con} and the average Ministry manual hammer density determined in 4(a) determine the Contractor's mechanical Marshall hammer correlation (" C_{Con} ").

7. Subsequent Testing and Reporting:

- (a) All subsequent testing by the Ministry and the Contractor shall be conducted on their respective mechanical hammers, delivering the number of blows determined for their respective Marshall Hammer Correlations (N_{Min} or N_{Con}).
- (b) Density results determined by the mechanical hammers shall be adjusted by adding, respectively, C_{Min} or C_{Con} and those correlated results reported and used in all applications related to asphalt mix density.

Note: The Ministry has developed an Excel 2016 spreadsheet to perform the calculations and plot the results of this correlation procedure. The spreadsheet is available from the Ministry Representative.

SECTION 503

BASE PREPARATION AND BASE PROCESSING

503.01 Base Preparation – The work shall consist of shaping and compacting the existing granular base course to the designated grade and cross section.

The existing base course material shall be watered, graded and compacted to provide a non-segregated, tightly-knit surface which conforms to the line, grade and cross sections shown on the Drawings or staked by the Ministry Representative to an accuracy of ± 10 mm.

If necessary, the existing surface shall be scarified and bladed to achieve this. No imported materials are to be incorporated into the work, except where failed areas have to be backfilled.

503.01.01 Failed Areas – Where a failed area is present in the existing base, the Contractor shall excavate and dispose of the material, all as directed by the Ministry Representative. The area shall be backfilled using crushed granular base or surfacing aggregate or road-mix as directed by the Ministry Representative and compacted to the same density as the rest of the base course.

503.01.02 Compaction – If watering is required to attain compaction or for any other reason, it shall be applied uniformly from a distributor of the pressure type, equipped with a spray bar mounting nozzles similar to those used on asphalt distributors. Splash plate type distributors or those equipped with spray bars that eject fine streams of water will not be permitted. The distributor must be provided with a satisfactory means for accurately measuring the quantity of water sprayed. If the Ministry Representative so requires, the measuring equipment shall be calibrated under the Ministry Representative's inspection.

The final surface of the base course shall be compacted to a minimum 100% of the Standard Proctor Density obtained by ASTM D698.

501.01.03 Surplus Aggregate – All surplus loose aggregate arising from the base preparation shall be bladed clear into windrows on the shoulder for later incorporation into the shouldering aggregate or bladed to the side slope and trimmed, as directed by the Ministry Representative.

503.02 Base Processing – Where, in the opinion of the Ministry Representative, the base is so deformed, rutted or unsuitable, that it cannot be adequately prepared for priming under the provisions for base preparation, the Ministry Representative shall order in writing how it shall be processed. This work shall be carried out on an Order for Extra Work basis.

503.03 Payment

503.03.01 Base Preparation – Payment for Base Preparation will be at the Unit Prices bid in the Schedule 7 – Schedule of Approximate Quantities and Unit Prices per square metre for the area actually prepared and shall be full compensation for scarifying done to a maximum depth of 40 mm, watering, blading, shaping, trimming, compacting, finishing and maintaining the granular base to the required line, grade, cross section and density.

503.03.02 Repair of Failed Areas – Payment for supply of material used for backfilling and compaction of failed areas will be at the Unit Price bid in Schedule 7 for the material used. Payment for excavating and disposing of the material from the failed areas will be at the Unit Price bid in Schedule 7 for Type D or, where there is no bid price, in accordance with GC 38.00 Change to Work.

503.03.03 No Payment for Incidental Works – Payment will not be made for superintendence, provision or placement of barriers, lights or other equipment which should normally be provided and available for the general performance of the contract.

SECTION 504

PAVEMENT DRAINAGE

DESCRIPTION

504.01 Scope – This Section covers the provision of pavement drainage with the construction of necessary asphalt curbing and drainage outlets using cast iron catch basins or asphalt spillways together with pipe or paved outfalls. Concrete curbs, concrete and corrugated steel catch basins, cast iron grates and frames where applicable are specified by SS 582.

MATERIALS

504.11 Materials – The Contractor shall supply all necessary materials required to construct the Works, conforming to the specifications indicated in SS 320, SS 502, this SS 504, SS 582, and all other provisions of the Contract.

CONSTRUCTION

504.31 Asphalt Curbs

504.31.01 Integral Asphalt Curb – Where specified in the Contract Documents, asphalt concrete curb integral with the asphalt concrete highway pavement shall be constructed to the dimensions indicated on SS Drawing SP504-01.

504.31.02 Machine-Laid Asphalt Curb – Where specified, machine laid (extruded) asphalt concrete curbs shall be constructed in accordance with SS Drawing SP504-01 and as follows:

- (a) The type of curb to be placed (Type "A" or Type "B") will be as specified or as determined by the Ministry Representative.
- (b) Prior to construction of the curb, the affected pavement area shall be dry and cleaned of loose or deleterious materials. A tack coat of liquid or emulsified asphalt determined by the Ministry Representative or specified in the Special Provisions or Purchase Order shall be uniformly applied at a rate of 0.25 L/m² to 0.5 L/m², and to a width 0.1 m wider than the proposed curb.
- (c) The production and transportation of asphalt mix for curb construction shall be in accordance with SS 502 and SS 952 and shall meet the requirements for Class 1 Fine Mix.
- (d) The machine for constructing the curb shall be self-propelled extrusion-type equipment with a material hopper, distributing screw and adjustable curb forming devices capable of placing and compacting the asphalt mix to the required alignment, grade and cross section in an even homogeneous manner free of surface

defects. The asphalt concrete mix shall be handled to provide a curb meeting the applicable density requirements of SS 502 and the curbing equipment shall be operated in a manner that ensures a smooth, uniform finished curb alignment. Where hand-forming of curb is required, the finished curb shall conform to all dimensions, alignment, density and finish requirements for machine-laid curb.

504.32 Location of Drainage Outlets – On completion of pavement and curb construction and in conjunction with the installation of any precast concrete barriers, the Contractor shall install or construct drainage outlets at the low point of curves in the vertical alignment and at such other locations designated by the Ministry Representative.

The exact low point on the pavement should be determined by allowing water to flow along the surface to the actual lowest point on the pavement adjacent to the curb or barrier. This low point shall be clearly marked on the pavement.

504.33 Excavation – At the locations designated by the Ministry Representative, the asphalt pavement and any curbing shall be accurately cut away and the highway shoulder excavated to the dimensions consistent with the installation of Cast Iron Catch Basins (SS Drawing SP504-03) or the construction of Asphalt Spillways (SS Drawings SP504-02, SP504-04, and SP 504-05). The excavation shall, as nearly as possible, have vertical sides and shall be so done as to cause minimum disturbance of the adjacent compacted base and pavement structures. The outside dimensions of the excavation shall be the smallest compatible with the efficient construction of the drainage facility, as any excess or irregular excavation will be required to be repaired with asphalt mix.

Where applicable, appropriate trench excavation shall be performed to permit the installation and embedment of outlet pipe and associated fittings. Excavation for installation and embedment of outlet pipe shall be considered incidental to the rate bid for pipe installation.

504.34 Drainage Outlets

504.34.01 Cast Iron Catch Basins – Bedding and surrounding mortar for catch basins shall be composed of one part Portland cement to three parts clean well-graded fine aggregate mixed to proper consistency.

Bedding shall be a minimum of 125 mm thick to permit accurate setting of the catch basin in mortar at the correct level true to alignment with curbing, barrier or pavement edge as shown on SS Drawing SP504-03. After securely fixing the adaptor, watertight gasket and connecting pipe, the catch basin shall be surrounded with mortar to the full perimeter of the excavation, and the mortar well tamped and

worked around the connecting pipe for 300 mm from the side of the catch basin.

The mortar shall be allowed to set prior to backfilling and further work.

504.34.02 Asphalt Spillways – Asphalt spillways shall be constructed to the general dimensions shown on SS Drawings SP504-02, SP504-04, and SP 504-05, with the necessary hand-forming in accordance with SS 504.31.02 (d).

504.34.03 Pipe Outfalls General – Except where paved outfalls are specified, corrugated galvanized metal outlet pipes of the required diameter shall be installed, as shown generally on SS Drawing SP504-03. Should the fill material preclude pipe embedment, the pipe shall be secured to the slope by dual angle iron posts placed at 3 m intervals, as shown. The pipe shall be similarly secured at the bend at the base of the slope and at a point approximately 150 mm from its outlet.

The pipe trench shall be backfilled with excavated material as may be directed by the Ministry Representative and the fill thoroughly compacted.

Class 10 riprap, per SS 205, shall be placed at the pipe outlet as shown on SS Drawing SP504-03.

504.35 Dishing, Spillways and Completion – The asphalt pavement adjacent to the drainage outlet shall be constructed to the profile and cross sections indicated for the type of outlet being installed. The asphalt pavement surface shall be so shaped as to channel all collected water to the outlet orifice and no ridges or other obstructions shall be left which may impede the flow of water to the drainage outlet.

The asphalt pavement mix shall be compacted to the density specified by the Ministry Representative to provide an integral and impervious drainage area. Where the asphalt pavement abuts a cast iron catch basin box, the contact face of the box shall be painted with liquid or emulsified asphalt as specified in SS 502.22.03 prior to placing the asphalt concrete.

On completion, the area shall be cleared and cleaned up to the satisfaction of the Ministry Representative.

MEASUREMENT

504.81 Pavement Drainage Outlets – Pavement drainage outlets will be measured per EACH of the type specified.

504.82 Corrugated Metal Pipe – Corrugated metal pipe will be measured by the METRE along its centreline as installed.

504.83 Asphalt Pavement – Unless otherwise specified, asphalt pavement mix used to form drainage aprons, curbs, spillways and outfalls will be measured by the TONNE.

504.84 Asphalt Curb – Asphalt curb will be measured by the METRE.

PAYMENT

504.91 Pavement Drainage Outlets – Payment for the installation of PAVEMENT DRAINAGE OUTLETS will be at the Contract Unit Price for EACH complete outlet of the type specified. The Contract Unit Price shall be full compensation for supply of cast iron basins, boxes and grates; excavation (see SS 504.33); provision of all materials required for setting basins to correct grade including cement mortar; backfill and compaction; spreading, shaping and compaction of asphalt pavement aprons and spillways; provision and setting of riprap at the outfall and all labour, materials, tools, warning signs, barriers and superintendence required to complete the installation of the drainage outlet.

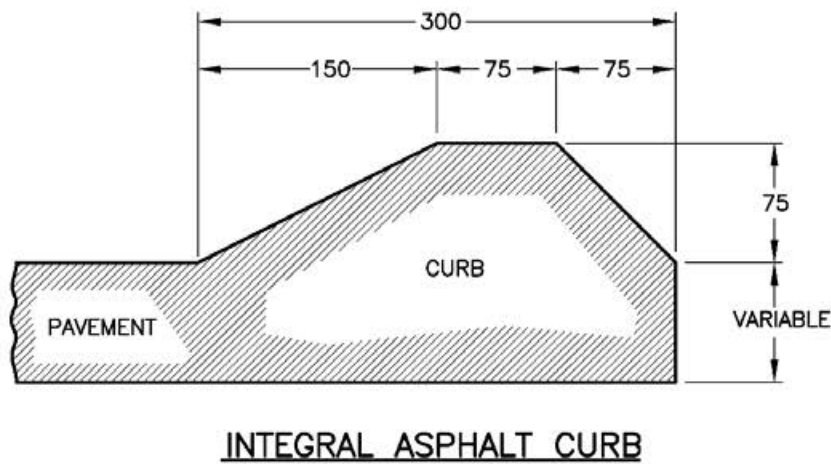
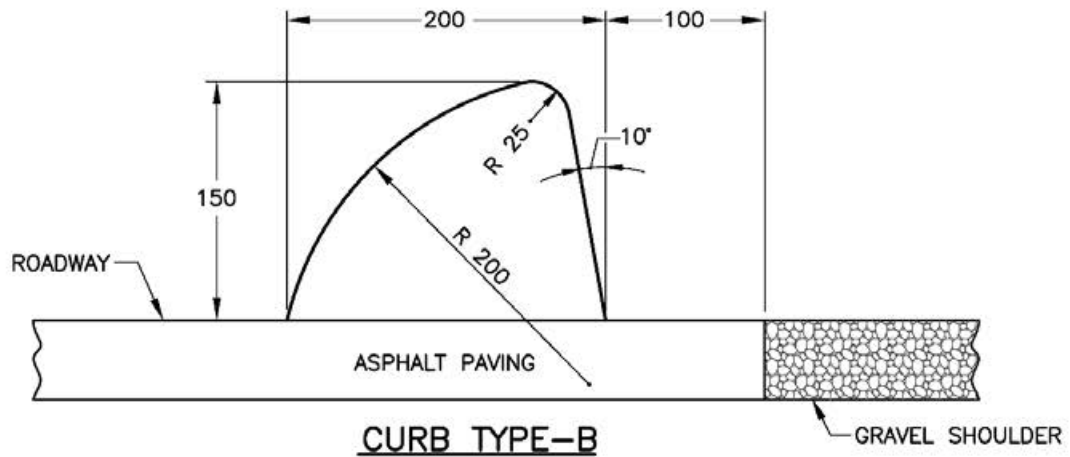
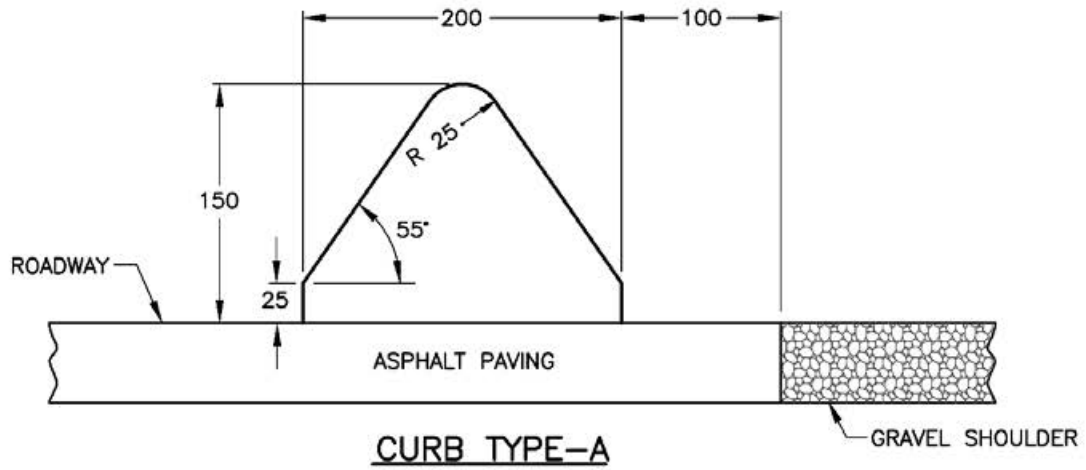
Payment for Asphalt Pavement mix used will in accordance with SS 504.93

504.92 Corrugated Metal Pipe – Payment for CORRUGATED METAL PIPE will be at the Contract Unit Price per metre. The Contract Unit Price shall be accepted as full compensation for supply and installation including all necessary transport of components, excavation and backfilling; provision of posts, brackets, hoops and hardware; labour for connecting and securing pipe fittings, compaction, superintendence, lights, warning signs, barriers and all other things or services necessary for the proper installation of the pipe.

504.93 Asphalt Pavement – Unless otherwise specified, payment for ASPHALT PAVEMENT used to form drainage aprons, curbs, spillways and outfalls will be at the Contract Unit Price per tonne.

504.94 Asphalt Curb – Payment for ASPHALT CURB will be at the Contract Unit Price per metre of the appropriate curb type placed. The Contract Unit Price shall include the formation of offset curb alignment at drainage outlets in accordance with SS 504.31.

DRAINAGE CURBS TYPE-A, TYPE-B & INTEGRAL ASPHALT	SP504-01
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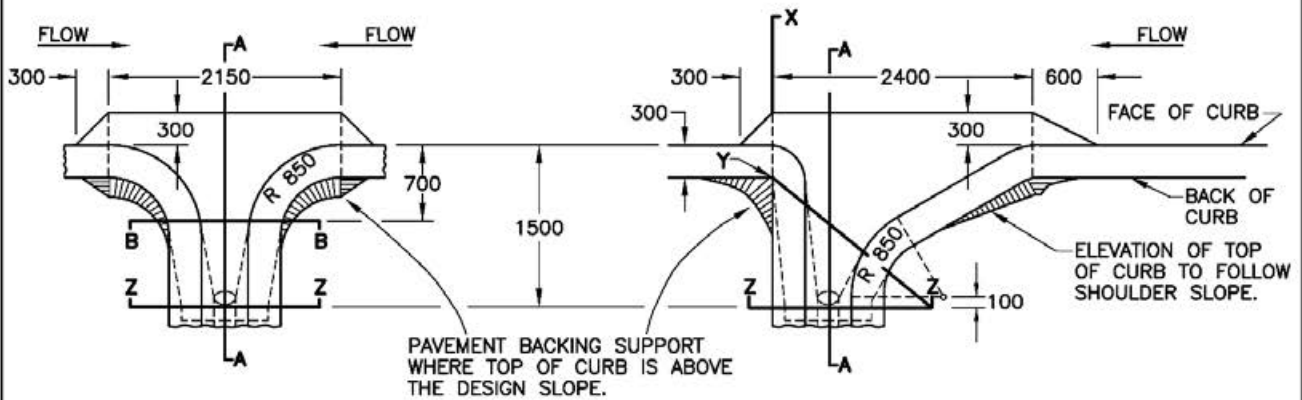
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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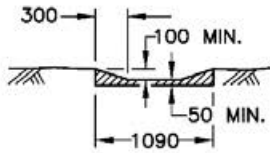
INSTALLATION OF ASPHALT SPILLWAYS

SP504-02



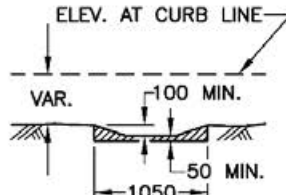
PLAN AT SAG

PLAN ON GRADE

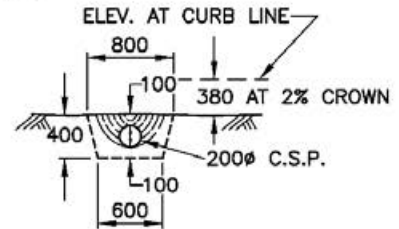


SECTION B-B

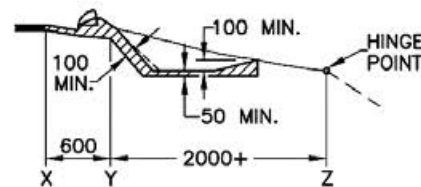
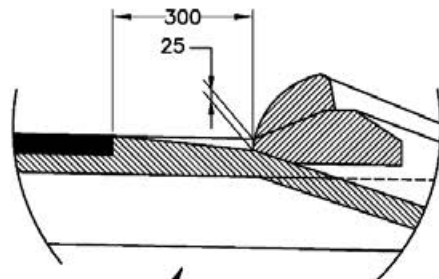
COMMON TO PAVED OR PIPED OUTFALL AT "SAG" SPILLWAY



SECTION Z-Z FOR PAVED OUTFALL

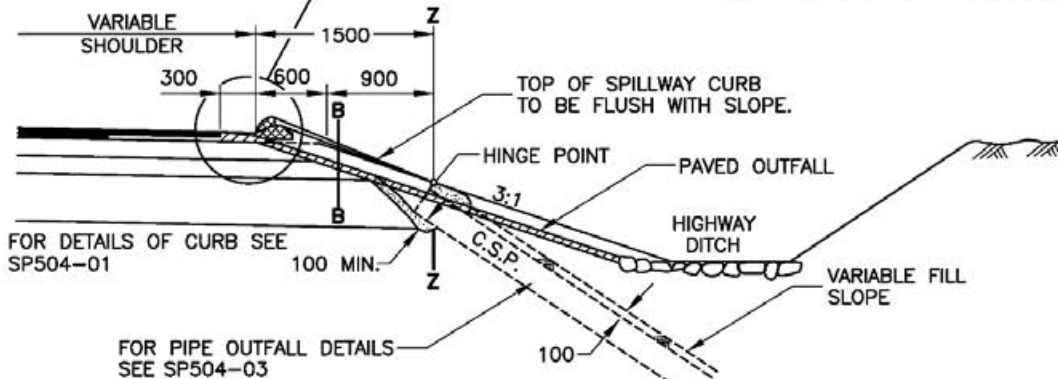


SECTION Z-Z FOR PIPE OUTFALL



SECTION X-Y-Z

COMMON TO PAVED OR PIPED OUTFALL AT "ON GRADE" SPILLWAY



SECTION A-A

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

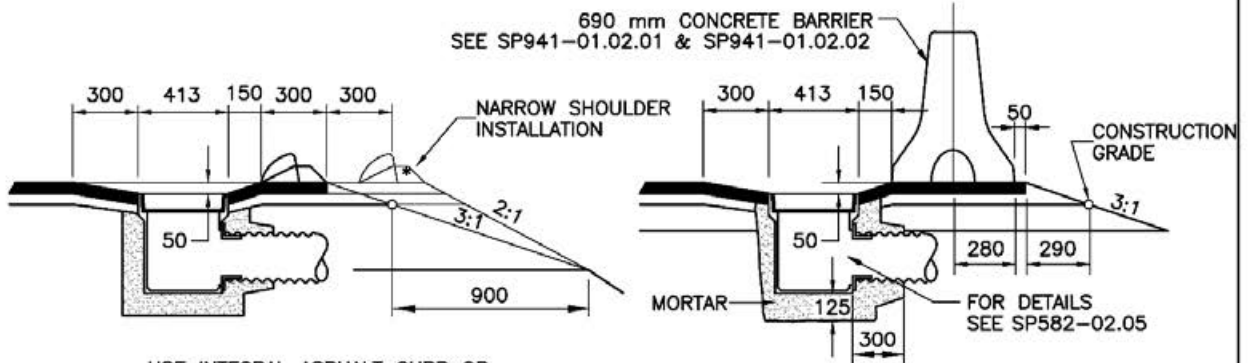


BRITISH COLUMBIA

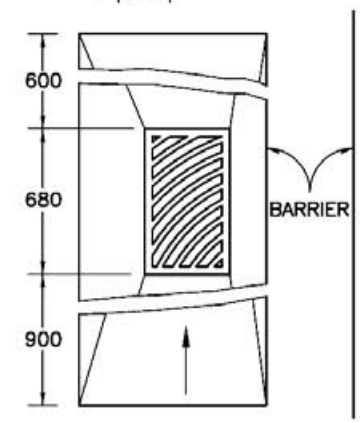
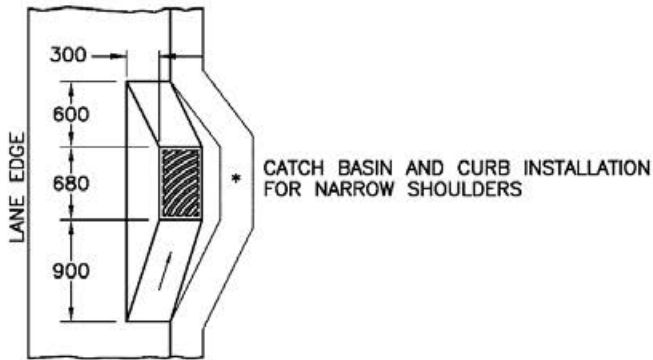
Ministry of Transportation & Infrastructure

INSTALLATION OF CURB, BARRIER AND CAST IRON CATCH BASIN

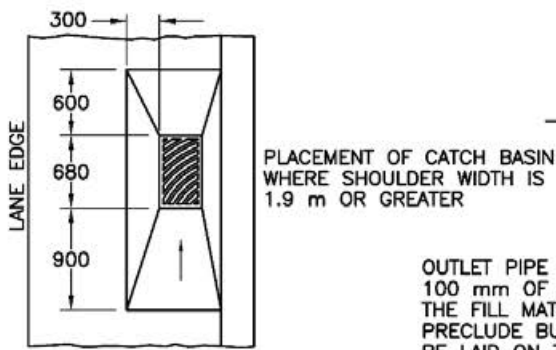
SP504-03



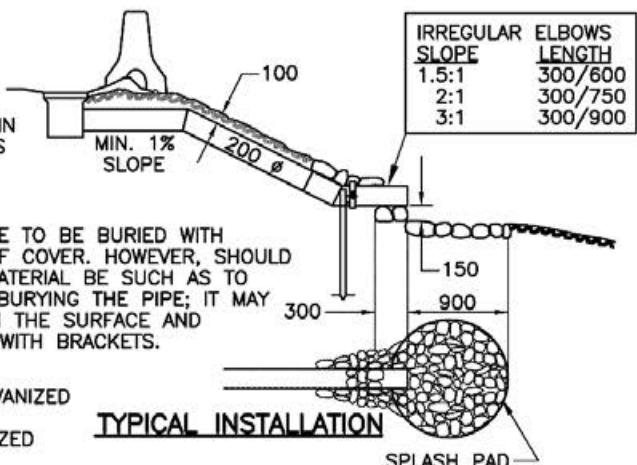
USE INTEGRAL ASPHALT CURB OR TYPE-B CURB (SEE SP504-01)



CATCH BASIN AT BARRIER

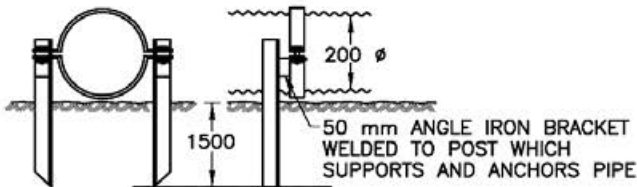


PLACEMENT OF CATCH BASIN WHERE SHOULDER WIDTH IS 1.9 m OR GREATER

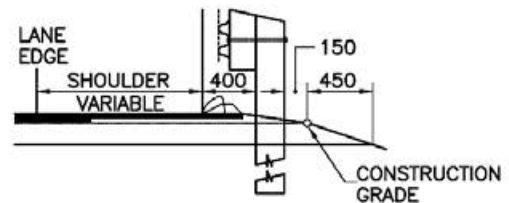


OUTLET PIPE TO BE BURIED WITH 100 mm OF COVER. HOWEVER, SHOULD THE FILL MATERIAL BE SUCH AS TO PRECLUDE BURYING THE PIPE; IT MAY BE LAID ON THE SURFACE AND ANCHORED WITH BRACKETS.

- 50 mm ANGLE IRON BRACKET, GALVANIZED
- 50 mm STRAP IRON, GALVANIZED
- 50 mm ANGLE IRON POST, GALVANIZED



DETAIL OF PIPE, ANCHOR BRACKETS AND POSTS



POST AND BEAM GUIDE RAIL

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

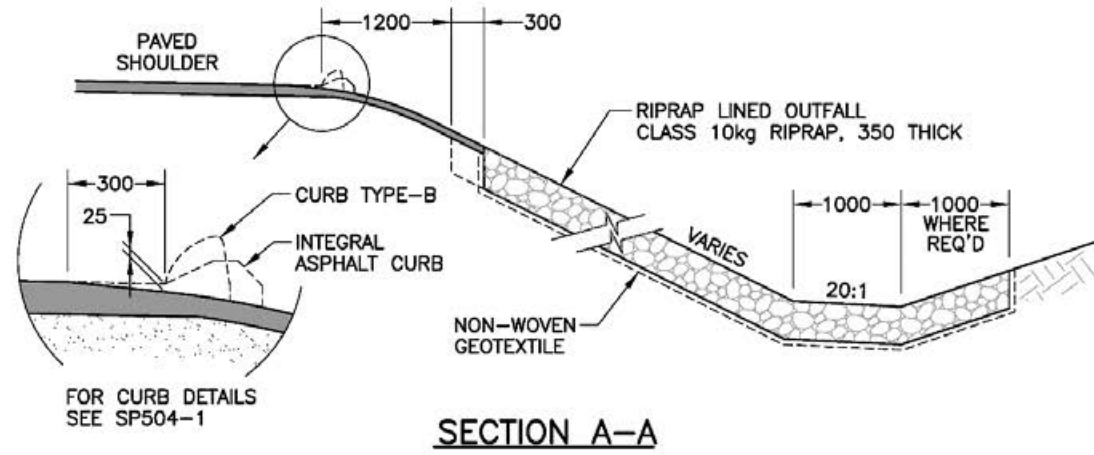
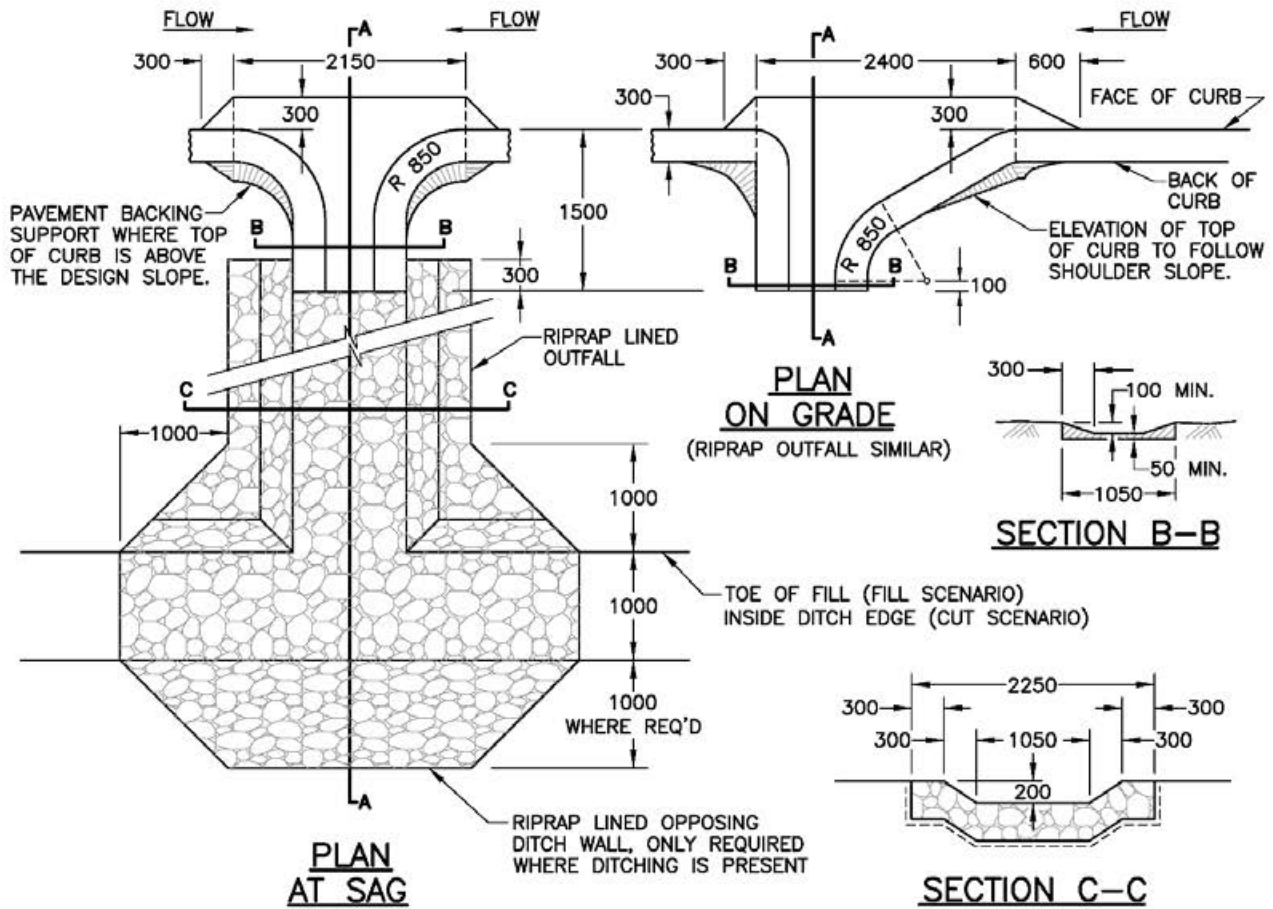


BRITISH COLUMBIA

Ministry of Transportation & Infrastructure

INSTALLATION OF ASPHALT SPILLWAYS
WITH RIPRAP OUTFALL

SP504-04



NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

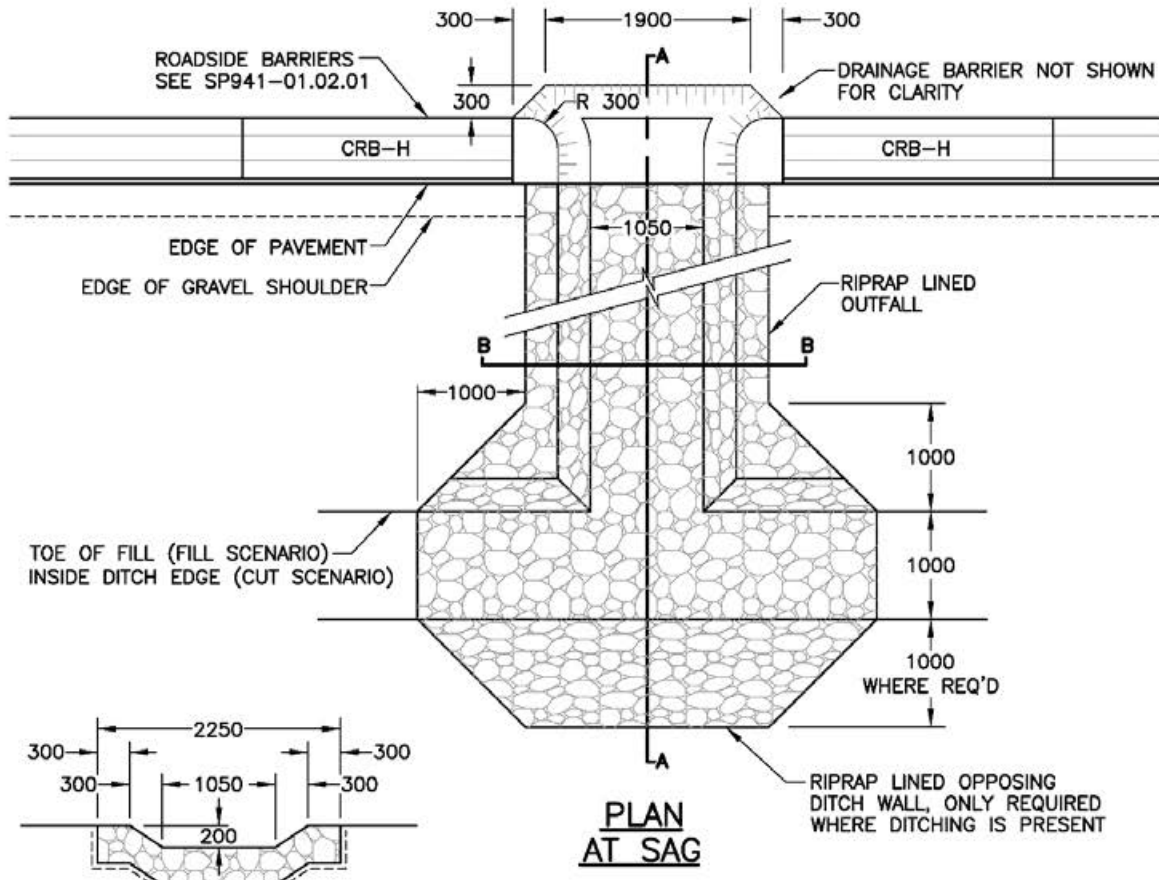


BRITISH COLUMBIA

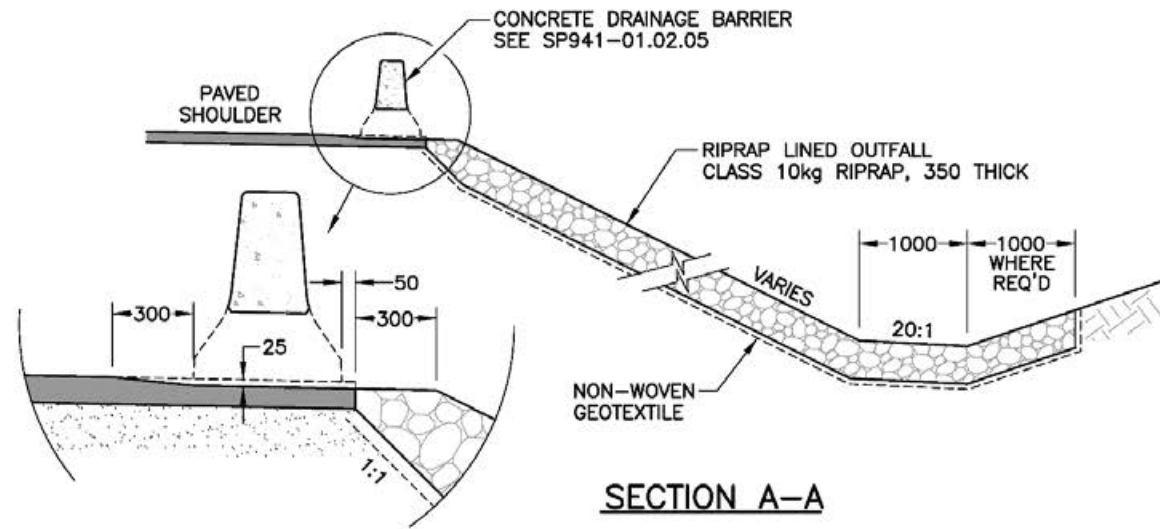
Ministry of Transportation & Infrastructure

INSTALLATION OF DRAINAGE BARRIER SPILLWAYS WITH RIPRAP OUTFALL

SP504-05



SECTION B-B



SECTION A-A

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



SECTION 505

USE OF RECLAIMED ASPHALT PAVEMENT IN ASPHALT PAVEMENT CONSTRUCTION

505.01 Preliminary and General – The work covered by this section covers the use of Reclaimed Asphalt Pavement (RAP) material in hot mix asphalt construction. This specification allows for the use of both Classified and Unclassified RAP.

The Contractor's development and processing of RAP into an end paving product must meet SS 502 Asphalt Pavement Construction (EPS) for Highway Construction.

The maximum allowable percentage of RAP allowed in asphalt pavement will be based on the classification of the roadway and the location of the mix in the pavement structure or as identified in the Special Provisions.

505.02 Roadway Categories – Roadways are divided into the following two categories:

- Category A – All numbered Highways.
- Category B – Other Roads.

The roadway Category shall Category A, unless otherwise identified in the Special Provisions.

505.03 DEFINITIONS

- (a) **Reclaimed Asphalt Pavement (RAP)** – Removed and/or reprocessed pavement materials containing asphalt and aggregates. These materials are generated when asphalt pavements are removed for reconstruction and/or resurfacing. When properly screened and crushed, it consists of high-quality aggregate coated by asphalt cement.
- (b) **Classified RAP** – RAP obtained from Ministry roadways.
- (c) **Unclassified RAP** – RAP obtained from non-Ministry sources or mixed with RAP from other sources.
- (d) **Rheology** – The study of deformation and flow of matter. For asphalt binder rheology, it refers to the flow characteristics of the asphalt binder in the asphalt mix. Deformation and flow of the asphalt binder in asphalt mix is important in determining asphalt pavement performance.

505.04 Allowable RAP Addition Rates – The maximum RAP allowed in the asphalt mix shall be determined by the contribution of the RAP Asphalt Cement (AC) towards the total AC content in the mix by weight as per the percentages specified in Table 505-A.

The Contractor shall notify the Ministry Representative, 14 days in advance of Paving if RAP is proposed to be used in the project, and shall fill all fields as applicable and sign this Appendix 505-B – RAP RECORD SHEET and submit

it to the Ministry Representative 10 days in advance unless the project Special Provisions state differently.

The amount of total AC replaced by AC in the RAP will be calculated as follows:

$$\% \text{ AC Replacement} = \frac{(a \times b)}{c}$$

Where;

a = AC content of RAP

b = RAP percent in mixture by total weight of mix

c = Total Percent AC content in mixture

Inclusion of RAP into the hot mix asphalt will be as per the maximum allowable percentage of AC replacement unless otherwise noted in the Special Provisions.

Table 505-A: Maximum Percent RAP AC Replacement Allowed in Asphalt Mix

Road Classification	Top Lift	Lower Lifts
Category A	15%	30%
Category B	30%	30%

505.05 Materials

505.05.01 RAP – The Contractor shall fulfill or exceed the requirements of Appendix 505-A – RAP Management Best Practices for the management of RAP materials from the time of collection through processing, mix design and quality control practices during the production of asphalt mixtures containing RAP and the Contractor shall provide documentation to the Ministry Representative that best practices have been followed in the handling, classification, and storage of RAP material, prior to being allowed to use the RAP.

505.05.02 Asphalt Cement – When the Percent RAP AC Replacement amount is greater than 15%, the blended AC must meet the penetration and viscosity requirements of the specified asphalt cement specified in the Special Provisions (when penetration graded asphalt cement is specified) or the temperature requirements of the specified Performance Graded Asphalt Cement (PGAC) when a PGAC is specified or substituted for penetration graded asphalt cement.

505.05.03 Use of Rejuvenators – Usage of Softening agents, rejuvenators or recycling agents will not be permitted.

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**USE OF RECLAIMED ASPHALT PAVEMENT
IN ASPHALT PAVEMENT CONSTRUCTION**

505.05.03 Use of Recycled Asphalt Shingles –Usage of Recycled asphalt shingles will not be permitted.

505.06 RAP Sampling – Sampling of the RAP shall be as per Appendix 505-A – RAP Management Best Practices.

505.07 Quality Control – The quality control testing requirements for RAP shall be based on the percent of AC replacement in the asphalt mix and comply with the requirements listed in Table 505-B and Table 505-C, and the corresponding tests specified in SSS 505.07.01.

Table 505-B: Quality Control Testing Requirements

RAP Category	% AC Replacement	Required Tests
Classified RAP	≤ 15%	• Basic Tests
	> 15%	• Basic Tests • Consensus Tests • AC Rheology Testing
Unclassified RAP	≤ 15%	• Basic Tests • Consensus Tests
	> 15%	• Basic Tests • Consensus Tests • AC Rheology Testing

Table 505-C: Minimum Quality Control Test Frequencies

Test	Minimum Frequency
<u>Asphalt Content</u>	<u>One per 750 tonnes</u>
<u>Gradation</u>	<u>One per 750 tonnes</u>
<u>Percent Fracture (%)</u>	<u>One per 750 tonnes</u>
<u>Specific Gravity of coarse fraction of RAP</u>	<u>Minimum of one per 3000 tonnes or three per stockpile</u>
<u>Specific Gravity of fine fraction of RAP</u>	<u>Minimum of one per 3000 tonnes or three per stockpile</u>
<u>Maximum Micro Deval Abrasion loss factor (%)</u>	<u>Minimum of one per 3000 tonnes or three per stockpile</u>
<u>Fine aggregate angularity</u>	<u>Minimum of one per 3000 tonnes or three per stockpile</u>
<u>Flat and elongated particles (For Superpave Only)</u>	<u>Minimum of one per 3000 tonnes or three per stockpile</u>
<u>AC Rheology</u>	<u>Minimum of one per 3000 tonnes of RAP or a minimum of three tests per project.</u>

505.07.01 RAP Aggregate Testing

(a) **Basic Tests** – The following basic properties shall be determined for the proposed RAP:

- (i) Moisture Content;
- (ii) Asphalt Content;
- (iii) Gradation;
- (iv) Percent Fracture; and
- (v) Specific Gravity of coarse and fine fractions (for mix design purposes).

Individual representative RAP samples shall be tested to determine moisture content, gradation, percent fracture and asphalt content.

Tests to determine the specific gravity of the coarse and fine fractions of RAP shall be completed on the combined aggregates (obtained by combining the individual reclaimed aggregates retained after the extraction of the asphalt cement).

(b) **Consensus Tests** – The tests for the consensus properties shall be completed on the combined aggregates (virgin aggregates and reclaimed aggregates) mixed in the proportions proposed for the mix design.

The following aggregate consensus properties shall be determined for the combined aggregate samples:

- (i) Maximum Micro Deval Abrasion loss factor (%);
- (ii) Standard Test Methods for un-compacted void content of fine aggregate (as influenced by particle shape, surface texture and grading) (ASTM C1252); and
- (iii) Percentage of flat and elongated particles (for Superpave mixes only).

505.07.02 AC Rheology Testing – When AC rheology testing and design is required, the blended AC must meet the penetration and viscosity requirements of the specified AC type. For penetration graded asphalt, rheology is measured in terms of penetration at 25°C and viscosity at 60°C. For performance grade asphalt cements (PGAC), rheology is tested as per the procedures outlined in *AASHTO R 29 Standard Practice for Grading or Verifying the Performance Grade (PG) of an Asphalt Binder* and reported in terms of high and low temperature service ratings, i.e. PG 58-34.

Since the rheological properties of asphalt binder vary with temperature, rheological characterization involves two key considerations:

- To compare different asphalt binders, their rheological properties must be measured at some common reference temperature.

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- To fully characterize an asphalt binder, its rheological properties must be examined over the range of temperatures that it may encounter during its life.

For mixes specified to use PGAC, the blended AC must meet the temperature requirements of the specified PGAC. For asphalt mixtures containing RAP and specified to use Performance Graded (PG) asphalts, the RAP rheology and the grade of virgin asphalt to be used shall be determined according to AASHTO R 35 Standard Practice for Superpave Volumetric Design for Asphalt Mixtures.

505.08 Asphalt Mix Design – RAP shall be included at the proposed addition rates during the mix design. The Contractor shall submit a mix design to the Ministry Representative for review at least five (5) business days prior to the start of asphalt mix production.

In addition to the information required in SS 502.08.04 Asphalt Mix Design Submittals, the following information

USE OF RECLAIMED ASPHALT PAVEMENT IN ASPHALT PAVEMENT CONSTRUCTION

shall be provided in the mix design submittal for the asphalt mixes containing RAP:

- All RAP aggregate gradations for each RAP product;
- Asphalt Content for RAP;
- Results for Consensus properties of combined aggregates as specified in Section 505.07.01 (when applicable);
- Design RAP Rheology test results; and
- Blending Charts for the virgin and reclaimed AC. (when applicable)

Should a change in the source or addition rate of RAP be made after the mix design has been approved, a new mix design and/or job mix formula shall be submitted to the Ministry Representative for review.

505.09 Payment Adjustments – SS 502 – PAYMENT ADJUSTMENTS (SS 502.50 through SS 502.58 inclusive) shall apply to all work utilizing reclaimed asphalt pavement in the asphalt mix unless otherwise specified in the Contract.

APPENDIX 505-A

RAP MANAGEMENT BEST PRACTICES

A **best practice** in the context of this Standard Specification is a method or technique that has consistently shown results superior to those achieved with other means, and that is used as a benchmark. A "best" practice may evolve to become better as improvements are discovered. Best practice is considered to describe the process of developing and following a standard way of doing things that are commonly achieved in the industry.

RAP Sources – RAP may be obtained from several sources. The common sources of RAP are:

- **Cold Milling** – This is the most common source of RAP. The milling process should be closely examined to make sure that the milled material is not contaminated with soil, base materials, paving geotextiles or other foreign material (sulphur, asbestos, rubber etc.). The milled material that becomes contaminated should be stockpiled separately from RAP to be used in asphalt mix.

A special milling operation may also be beneficial when it is desirable to mill the surface layer in one pass and the underlying layers in a second pass because the surface course millings may contain aggregates with higher fractured aggregates that could be incorporated in the new surface layers.

- **Full Depth Pavement Removal** – RAP can also be obtained from the removal of the existing pavement using a bulldozer or a backhoe. This process typically results in large chunks of pavement that may be contaminated with underlying soils. This contaminated material should be stored in a separate stockpile and not to be used in the hot mix asphalt.
- **Asphalt Plant Waste** – This includes the waste generated during plant start-up, transition between mixes, plant clean out, mix rejected from projects and excess mix produced that could not be placed.

This material usually has fewer fines than the typical RAP since it wasn't milled or broken up during pavement removal and the AC is less aged than RAP since it hasn't been subjected to environmental aging.

This material should be stockpiled and tested separately from the other RAP.

RAP Contamination – Best practice for RAP stockpiles is to keep them free from contaminants. RAP stockpiles should be treated as a valuable material. Truck drivers bringing the material on the site should ensure that unwanted debris or contaminated material does not end up in the RAP stockpile.

The plant QC personnel and the loader operators should continuously monitor processed and unprocessed RAP stockpiles to make sure that they do not contain deleterious materials. If any contaminants are found, they should be removed immediately so that they are not covered up with other RAP brought on to the yard.

RAP Categories – RAP is categorized as:

- **Classified RAP:** RAP obtained from Ministry sources.
- **Unclassified RAP:** RAP obtained from non-Ministry sources or mixed with RAP from other projects.

RAP Processing – Best practice for RAP processing involves one or more steps to create consistent materials. Screening is used to separate sizes. In some cases, it may be desirable to screen or fractionate RAP to coarse and fine fractions. RAP separation based on sizes increases quality and reduces variability in the RAP properties.

For stockpiles of RAP from multiple sources, particularly stockpiles containing oversize fragments of RAP or pavement slabs, the material should be processed to produce RAP with a maximum size of 37.5 mm for use in hot mix asphalt.

Further processing of RAP may include both screening and crushing to produce a uniform gradation, AC content and other properties. Since crushing RAP will create more aggregate fines, it is best to set up the crushing operation so that the RAP is screened before it enters the crusher.

RAP processing shall occur as early as possible in the construction planning process.

RAP Stockpiling – The best practice to minimize the accumulation of moisture in stockpiles is to cover the stockpile with a shelter, tarp or building to prevent precipitation from getting to the RAP.

RAP stockpiles should be placed on a base with adequate drainage and constructed with minimal segregation. Arc-shaped, conical, uniform stockpiles are preferred for storing processed or unprocessed RAP. No contamination of the RAP from the stockpile base materials should occur.

Use of heavy equipment on top of the RAP stockpile should be minimized to avoid compaction of the RAP. It is also recommended that the RAP stockpiles be limited to 4 m in height to reduce the potential for self-consolidation of the stockpile.

RAP Sampling – Best practice is for representative RAP samples to be collected from the stockpile prepared for the project. At least one sample per 750 tonnes of RAP in the

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stockpile or a minimum of ten samples per stockpile should be taken and retained for testing.

If the asphalt mix from an existing pavement is to be used as RAP, 150 mm diameter cores should be extracted at a frequency of at least one core every 1.5 kilometre in each lane prior to the start of the project. The asphalt mix from the pavement lift to be recycled should be tested to determine the properties of the aggregates and the asphalt cement in the pavement.

Sampling Method – Best practice for sampling aggregates applies to the sampling of RAP as well. RAP stockpiles should be sampled as they are being built at the location where they will be fed into the asphalt plant. Sampling at the time the stockpile is built is the best practice and will be easier and more representative of the stockpile compared to samples taken later after the formation of the crust on the face of the RAP stockpile.

ASTM D75 or AASHTO R 90 sampling procedures, normally used for virgin aggregates, should also be used to sample RAP aggregates.

USE OF RECLAIMED ASPHALT PAVEMENT IN ASPHALT PAVEMENT CONSTRUCTION

Minimum Test Frequencies – The quality control tests on the RAP and the extracted aggregates should be completed at the minimum frequencies specified in Table 505-C.

All RAP aggregates retained after extraction should be combined together into one sample. The combined sample should then be sieved and split into coarse and fine fractions and used to determine the specific gravity of the recycled aggregates.

Stockpile Management – When a stockpile reaches the desired quantity and has been sampled and characterized, no additional RAP material should be added to it. Subsequent RAP material should be stockpiled in a separate stockpile and characterized in the same manner. This process should continue such that characterized stockpiles are not compromised by new RAP materials.

Samples from the RAP stockpile should be taken and the testing completed as per the minimum test frequencies specified in Table 505-C.

**APPENDIX 505-B
RAP RECORD SHEET**

The Contractor shall notify the Ministry Representative fourteen (14) days in advance of paving, if RAP will be used in the project, and shall fill all fields as applicable and sign

this Appendix 505-B – RAP RECORD SHEET and submit it to the Ministry Representative 10 days in advance.

RAP RECORD SHEET

Contractor: _____

Project Name per Signed Contract: _____

Project Number: _____

Numbered Highway Name: (If Applicable): _____

Road Name: (If Applicable): _____

Segment No.	Tonnage	RAP %	Classified RAP	Unclassified RAP	Top Lift %	Bottom Lift %	LKI		
							Left/Right	Start	Finish

Filled by (Name and Date): _____

Address and Contacts: _____

Submitted by: _____

Name: _____

Signature and Date: _____

SECTION 507

PRODUCTION AND STOCKPILING OF ASPHALT MIX

507.01 General – This Section covers the supply of aggregate, mixing it with a specified asphalt material in a central plant and placing the produced mix in a stockpile.

507.02 Aggregate

(a) The Contractor shall supply aggregate with gradation wholly within the limits listed in Table 507-A when tested in accordance with ASTM C136 and ASTM C117 (wash test).

Once a crushed aggregate gradation has been accepted, the maximum permissible variation of the mean of any four consecutive tests from the accepted gradation curve shall be within the limits specified in Table 507-B.

Should there be a substantial change in the type of aggregate exposed as the work proceeds, the Ministry Representative may authorize a change in the mean gradation.

(b) A minimum of 85% by count of all aggregate particles retained on the 4.75 mm sieve shall have at least two fractured faces or shall be naturally angular with sharp edges (ASTM D5821). If this requirement cannot be met for a specified crushed aggregate type then the Contractor shall, at no additional charge, crush aggregate to the next smaller size or such intermediate size as the Ministry Representative may direct, to meet the requirements.

(c) All aggregates shall be free from coatings of clay, silt or other objectionable matter and shall contain no clay lumps or other aggregations of fine-grained materials.

(d) Crushed aggregate shall conform to the following requirements:

(i) The sand equivalent value in accordance with ASTM D2419 shall not be less than 40.

(ii) The degradation factor in accordance with ASTM D6928 shall be not more than 20.

(iii) In the absence of satisfactory performance records for a particular source of aggregate, its soundness shall be tested according to ASTM C88. Aggregate so tested shall be considered unsatisfactory if the loss after five cycles exceeds 20% for coarse aggregate or 25% for fine aggregate.

(iv) Micro Deval maximum abrasion loss shall not be greater than 17% as tested under ASTM D6928.

(e) For testing purposes, fine aggregate shall be all mineral matter passing the 4.75 mm sieve including mineral

fillers. It shall consist of natural and/or manufactured material derived by crushing stone, slag or gravel.

(f) Mineral filler shall:

(i) Consist of all mineral matter passing the 0.425 mm sieve.

(ii) Be dry, free from organic matter, clay particles or lumps.

(iii) Be non-plastic when tested in accordance with ASTM D4318.

(g) Mineral dust shall consist of all mineral matter passing the 0.075 mm sieve.

Table 507-A: Gradation Requirements for Aggregate

Sieve Size (mm)	Percent Passing (By Mass)	
	Patch Mix	Paving Mix
19.0	---	100
16.0	---	100 – 92
13.2	---	100 – 80
9.5	100	90 – 65
4.75	85 – 55	75 – 45
2.36	65 – 40	60 – 30
1.18	50 – 25	45 – 20
0.300	30 – 15	25 – 10
0.150	18 – 8	17 – 5
0.075	12 – 5	9 – 2

Table 507-B: Maximum Permissible Variation of Crushed Aggregate Gradation from the Gradation Curve

Sieve Size	Variation Limit (% Passing)
4.75 mm and larger	± 5.0
1.18 mm to 4.75 mm	± 3.5
0.300 mm to 1.18 mm	± 2.0
0.075 mm to 0.300 mm	± 1.0

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PRODUCTION AND STOCKPILING OF ASPHALT MIX

507.03 Asphalt Material

Asphalts or High Float Emulsified Asphalts used on a Force Account Basis.

- (a) The type of asphalt material to be used will be specified in the Contract's Special Provisions.
- (b) The Contractor shall supply the Liquid Asphalts or High Float Emulsified Asphalts, as specified in the Contract, to meet the requirements of SS 951

507.04 Mix Design

- (a) The Contractor will carry out the Asphalt Mix design, in accordance with specifications provided in the Contract and to for approval by the Ministry.
- (b) At least seven days prior to commencement of mixing, the Contractor shall submit the following to the Ministry Representative:
 - (i) A 50 kg sample of aggregate which is representative of the aggregate to be used on the project.
 - (ii) If available, a 30 kg sample of plant produced asphalt mix whose gradation is deemed to be representative of the gradation of the aggregate to be used on the project.
 - (iii) A 5 L sample of the type of asphalt to be used on the project.
- (c) Once the design grading curve is established, the maximum permissible variation will be as specified under SS 507.02(a).

507.05 Mixing Requirements

- (a) Mixing operations shall be performed in a mixing plant capable of producing the asphalt mix within the required specifications.
- (b) The maximum moisture content of the aggregate immediately prior to mixing with a liquid asphalt (MC or SC) shall not be over 2% by mass of dry aggregate.
- (c) Temperatures of the mix shall be generally in accordance with the limits specified in Table 507-C. Mix temperatures may be lowered to reduce the emission of "blue smoke" from the discharge chute.
- (d) The asphalt content in the mix shall not vary from the design asphalt content by more than 0.3 by mass of total aggregate.
- (e) The asphalt mix shall not contain more than 1% moisture by mass at the discharge from the plant.
- (f) The asphalt mix shall be uniform in appearance with all particles thoroughly coated. The mixing time will be increased if the mix is not completely coated with asphalt.
- (g) The Ministry reserves the right to increase the asphalt content in the mix. The Contractor shall be compensated for the additional quantity of Liquid

Table 507-C: Mix Temperatures

Type of Asphalt	Asphalt Storage Temperature Range (°C)	Mix Temperature Range (°C)
SC – 250	65 – 80	60 – 80
MC – 250	65 – 80	60 – 80
HF – 500M	55 – 65	100 – 120*
HF – 1000M	55 – 65	100 – 120*
* High temperatures are used when the mix is hauled directly to the road and when dictated by high moisture in the aggregate.		

507.06 Hauling – When the haul distance from the mixing site to the stockpile is greater than 1 km, the hauling vehicles shall be equipped with and use a tarpaulin of such size as to completely protect the asphalt mix from the weather and inhibit premature curing of the mix by aeration.

The tarpaulin shall be double quilted canvas or other approved material treated to prevent absorption of the asphalt.

507.07 Stockpiling

- (a) The exact location of the stockpile site will be determined by the Ministry. The Contractor shall place the cold mix within the boundaries of the designated stockpile site.
- (b) The Contractor shall prepare the stockpile site, which shall be shaped to a uniform smooth surface and graded to ensure good drainage.
- (c) Temporary stockpiling of the mix on the ground or in storage bins will be allowed, however, the Contractor will be required to minimize the handling of the mix to prevent premature curing of the mix.
- (d) If conveyor belts are used for temporary stockpiling or final stockpile construction, the free fall distance from the conveyor belt to the stockpile shall not exceed 2 m.
- (e) If the stockpile is constructed using a front end loader or end dumping by truck, the mix shall be deposited in layers of approximately 1 m in depth.
- (f) Plank or protected runways shall be provided for operating equipment on stockpiles, when the Ministry Representative deems them necessary, to prevent

SECTION 507

PRODUCTION AND STOCKPILING OF ASPHALT MIX

contaminants being tracked onto the stockpile and to minimize consolidation of the mix.

- (g) The final stockpile shall be constructed in such a manner that height is not more than 4 m.
- (h) Mixing, hauling or stockpiling will not be permitted during periods of precipitation.

507.08 Measurement – The asphalt mix will be measured in TONNES and the asphalt material(s) in LITRES, corrected to 15°C.

507.09 Payment – Payment for ASPHALT MIX in stockpile will be at the contract Unit Price per tonne.

The unit price will be full compensation for preparing the stockpile site; supplying and heating the aggregate; supplying, heating and storing the asphalt material; mixing, hauling and stockpiling the mix.

SECTION 508

GRADED AGGREGATE SEAL COAT (EPS)

GENERAL

508.01 Preliminary and General – This section describes the materials, equipment, professional standards and the end product requirements for the construction of graded aggregate seal coats. This section shall be read and construed together with all applicable Standard Specifications for Highway Construction. Should this Section conflict with any other Section, this Section shall take precedence unless otherwise stated in the Special Provisions.

508.02 General Description of Work – Surface seals shall consist of the application of a selected asphalt emulsion and embedment of the selected aggregate to provide a new, uniformly textured surface. This surface shall be impervious to moisture, be skid-resistant and have good riding qualities.

508.03 Classes of Graded Aggregate Seals – There are six classes of Graded Aggregate Seal, including but are not limited to graded aggregate seals, a coarse or fine sand seal or any other graded aggregate surface treatment outlined in the Special Provisions.

The Classes and their common applications are:

- **Class A** – Double seals on gravel highways
- **Class B** – Double seals on gravel highways, and single seals low traffic paved roads
- **Class C and Mod C** – Major Highways
- **Class D** – Major Highways
- **Class E** – Residential areas and second seals

508.04 Definitions

- (a) **Coarse Aggregate** – Coarse aggregate is defined as all material retained on or above the 4.75 mm sieve.
- (b) **Fine Aggregate** – Fine aggregate is defined as all materials passing the 4.75 mm sieve.
- (c) **Mineral Filler and Mineral Dust** – Mineral filler shall consist of all mineral matter passing the 0.600 mm sieve and the mineral dust shall consist of all mineral matter passing the 0.075 mm sieve.
- (d) **Blinding Sand** – Fine aggregate used as large aggregate support.

MATERIALS

508.11 Asphalt Materials – The Contractor shall supply the asphalt emulsion as specified in the Special Provisions, which shall meet the requirements of SS 952.

Asphalt(s) to be used on surface seals may be standard rapid curing cutback asphalt; medium curing cutback asphalts; or anionic or cationic emulsions including high float emulsified asphalts. Types and grades of asphalt binder proposed for use on the Project will be specified in the Special Provisions.

The Ministry Representative may require the use of anti-stripping additives or any other materials formulated to improve the coating and adhesive properties of the asphalt binder to the cover aggregate. Instructions concerning the type of additive to be supplied and the method of incorporation with the asphalt binder will be specified in the Special Provisions.

508.12 Aggregates

508.12.01 Mineral Aggregate Supply – The Contractor shall supply mineral cover aggregate meeting the requirements described hereunder unless specifically described otherwise in the Special Provisions.

The Ministry may supply the aggregates. If the Ministry does supply aggregates, then the Ministry will supply quality control information, if available, to the Contractor. The Contractor must verify and be satisfied that the Ministry supplied aggregates will meet the quantity and quality requirements prior to submitting a bid.

508.12.02 Mineral Aggregate: Test Methods – Mineral Aggregate will be sampled and tested in accordance with the standard ASTM procedures and tests listed in Table 508-A.

508.12.03 Test Requirements – Mineral cover aggregate shall consist of clean, sound, hard, durable particles or fragments of sand, gravel or crushed stones or combination thereof, containing a minimum of thin elongated or flaky pieces. It shall be substantially free from loosely bonded aggregations, clay lumps or other objectionable matter and shall not be markedly hydrophilic in character.

The mineral cover aggregate, when tested by the methods listed in Table 508-A, shall meet the following criteria:

- (a) **Clay Lumps** – Coarse aggregate shall not contain more than 0.5% by mass of clay lumps or other aggregation of fine material.
- (b) **Micro-Deval** – No individual test shall have a loss factor of more than 20%.
- (c) **Flat and Elongated Particles** – The percent flat and or elongated particles must be less than 10% using 4:1 ratio.
- (d) **Fracture** – Contrary to ASTM D5821, only those particles retained on or above the 4.75 mm sieve with

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two or more fractured faces shall be measured. The minimum % by mass, for coarse aggregate shall be as follows:

- **Classes A & B** = 75%
- **Classes C, Mod C, & D** = 85%
- **Class E** = not applicable

Table 508-A: Test Methods for Mineral Aggregate

ASTM Test	Name of Test
C117	<u>Standard Test Method for Materials Finer than 75-µm (No. 200) Sieve in Mineral Aggregates by Washing Wash Test of Aggregates.</u>
C127	<u>Bulk Relative Density and Absorption of Coarse Aggregates</u>
C128	<u>Bulk Relative Density and Absorption of Fine Aggregates</u>
C136	<u>Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates Sieve Analysis of Aggregates.</u>
C142	<u>Standard Test Method for Clay Lumps and Friable Particles in Aggregates.</u>
C566	<u>Moisture Content of Aggregate by Drying</u>
D75	<u>Standard Practice for Sampling Aggregates.</u>
D4318	<u>Liquid Plastic Limit Determination</u>
D4791	<u>Standard Test Method for Flat Particles, Elongated Particles, or Flat and Elongated Particles in Coarse Aggregate Flat and Elongated Particles.</u>
D5821	<u>Standard Test Method for Determining the Percentage of Fractured Particles in Coarse Aggregate Fracture Count on Coarse Aggregate.</u>
D6928	<u>Standard Test Method for Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus Micro-Deval.</u>

508.12.04 Graded Aggregate Seal

(a) **Aggregate Gradation** – Gradations shall meet the requirements stated in Table 508-B for each Class of graded aggregate seal.

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(b) **Variation Limits** – For all Classes of seal except Class E, once the grading curve is established within the above limits, the maximum permissible variation of the mean of any four consecutive tests from the grading curve shall be within the tolerances specified in Table 508-C.

These tolerances do not waive the requirement that the running average of four consecutive tests must be maintained at all times inside the limits specified in Table 508-B. Should the Contractor not be able to maintain the grading within the variation limits specified above they shall be required to construct a second stockpile. This does not alleviate the Contractor from consistently maintaining the grading curve within the tolerances indicated in the gradation curves.

Table 508-B: Gradation Limits for Graded Aggregate Seal

Sieve size (mm)	% Passing by Mass by Class					
	A	B	C	Mod C	D	E
19	100	-	-	-	-	-
16	-	100	100	100	-	-
12.5 ¹	60-90	60-90	-	-	100	-
9.5	40-80	35-75	30-70	25 -55	30-80	100
4.75	20-60	15-50	25-45	7 – 30	25-45	0-30
0.600	0-25	0-15	5-20	0 -10	5-20	0-10
0.075	0-7	0-5	0-3	0 – 3	0-3	0-3
Ratio ² (9.5/4.75)	-	-	-	1 – 1.5	-	-

Notes:

¹ The Contractor may use either the 12.5 mm or the 13.2 mm sieve, at the Contractor's option.

² The "Ratio" is the ratio of the mass of material retained on the 9.5 mm sieve, divided by the mass of material retained on the 4.75 mm sieve.

Table 508-C: Variation Limits

Sieve Size (mm)	Maximum Permissible Tolerance % Passing by Mass
4.75 to 19.0	± 5.0
0.600	± 3.5
0.075	± 1.0

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508.12.05 Aggregate Characteristics

- (a) **Coarse Aggregate** – Coarse aggregate shall consist of crushed stone or gravel, or a combination thereof; or materials naturally occurring in a fractured condition; or materials naturally occurring of highly angular nature or rough texture.
- (b) **Fine Aggregate** – Fine aggregate shall be clean, hard, durable, moderately sharp, and free from coatings of clay, silt or other deleterious material and shall contain no clay balls or other aggregations of fine material. All aggregate passing the 4.75 mm sieve shall be non-plastic when tested in accordance with ASTM D4318.
- (c) **Mineral Filler and Mineral Dust** – Mineral filler and mineral dust shall be free from organic matter. Mineral filler shall be non-plastic.

508.12.06 Blinding Sand – The Contractor shall be responsible for the supply of blinding material meeting the requirements of Table 508-D. That portion of blinding sand aggregate passing the 4.75 mm sieve shall be non-plastic when tested in accordance with ASTM D4318.

Table 508-D: Gradation for Blinding Sand

Sieve Size (mm)	Percentage Passing
4.75	100
2.36	50 – 100
1.18	30 – 80
0.300	10 – 55
0.150	3 – 20
0.075	0 – 3

508.13 Graded Aggregate Seal Design – The contractor shall provide a graded aggregate seal coat design, professionally prepared by qualified and experienced personnel. For this purpose, the Contractor shall use a qualified registered member of the Association of Professional Engineers and Geoscientists of British Columbia or a qualified registered member of the Applied Science Technologists and Technicians of British Columbia.

This design shall include application rate of the asphalt emulsion, in litres per square metre, spray temperature and the aggregate spread rate in kilograms per square metre. The design shall be presented to the Ministry Representative for review, at least three (3) days prior to the commencement of work and at any time subsequent changes are made to these rates.

The Contractor shall submit to the Ministry Representative four litres of a representative sample of the emulsion(s) and a twenty-kilogram sample of the aggregate(s) intended for

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use on the project. These samples shall be supplied at least three (3) days prior to the commencement of the work.

EQUIPMENT

508.20 General – The Contractor shall supply plant and equipment necessary for the proper and efficient completion of the work, including clear and effective means of calibration.

508.20.01 Access – The Ministry Representative shall have access at all times to any plant, equipment or machinery to be used on the Project.

508.20.02 Adjustment – If plant or equipment adjustment is necessary, it shall be done by a competent operator or mechanic or instrument repair technician, as required.

508.21 Equipment Characteristics – The following SS 508.21 provisions are the Ministry's preferred equipment characteristics but are not mandatory.

508.21.01 Pressure Distributor – The pressure distributor used for applying asphalt materials is to consist of a fully insulated tank with a minimum capacity of 5000 L, permanently and rigidly mounted on a truck or fifth wheel trailer provided with pneumatic tires and propelled by a power unit capable of maintaining a constant and uniform speed.

The pressure distributor should be provided with the following equipment:

- (a) A quick opening gate in the dome.
- (b) A dipstick calibrated in 20 mm increments and a calibration chart showing the volume for each 20 mm of depth or provided the manufacturer dipstick and chart. Readily visible external depth gauge indicating the quantity of asphalt in the tank to the nearest 50 L.
- (c) A means of heating the asphalt material uniformly to any temperature up to 125°C and maintaining the contents constantly at any selected temperature without any local overheating.
- (d) A heavy duty, industrial-type thermometer and a remote reading dial thermometer.
- (e) Rear-mounted spray bars and nozzles set parallel to the surface to be sprayed and capable of vertical and lateral adjustment. The spray bars to be capable of adjustment to provide a uniformly sprayed surface from a minimum of 0.6 m to at least 4 m width.
- (f) The spray bars to be capable of producing up to triple coverage at one pass. The feed to the spray bars must be arranged so that there is a constant circulation of hot asphalt material to the bars to ensure a uniform viscosity and constant pressure of the liquid asphalt at each nozzle both before and during spraying operations.

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- (g) The feed manifolds to the spray bars are to be provided with positive filtration to prevent clogging of bars and nozzles.
- (h) Rotary adjustable spray bar nozzles to be of the same type, correct size and manufacture; and set to produce uniformly fan-shaped sprays without atomization. Nozzles to be inspected daily and any nozzle having nicked, or damaged edges should be replaced.
- (i) Spray nozzles to be provided with valves capable of instant full opening and positive cut-off.
- (j) A connection to a circulating pressure manifold to which a hose may be attached for a single nozzle hand-operated outlet.
- (k) A pressure pump capable of applying the asphalt material at a minimum rate of 180 L per minute, per metre of spray bar.
- (l) A pressure gauge accurate to within 15 kPa and a metre calibrated in litres per minute should be readily visible.
- (m) A fifth-wheel tachometer, calibrated in metres per minute and readily visible to the operator of the distributor.
- (n) Vehicle spring "tie-downs" to prevent the change in height of the spray bars due to the reduction of the asphalt load during spraying.
- (o) Electronic distance measuring device, with print out connected to the transmission, that is capable of measuring each spray length and the total length sprayed during the day.

508.21.02 Mechanical Spreader – Cover aggregate to be spread by means of an efficient, self-propelled spreader mounted on pneumatic tires and capable of continuously and uniformly spreading closely regulated quantities of crushed aggregates at the application rates selected. The loading hopper should be of such a capacity as to ensure continuous operation between dumps of aggregate. Aggregate to be transferred from the loading hopper to the front spreading device in such a manner as to ensure a uniform flow and prevent segregation of particles.

The spreading mechanism to be so designed as to distribute and spread aggregate across the selected width without segregation. Aggregate to be applied to the freshly sprayed surface by use of a comb screen for Class A and B Graded Aggregate Seals so that the largest particles are first placed on the road with the smaller fractions falling on top. The comb screen is to be mounted beneath the discharge openings of the spreader mechanism in such a manner that the angle of the comb screen relative to the road surface can be easily and quickly changed. Reverse flow aggregate spreaders are exempt from this clause.

Suitable provision to be made to prevent aggregate from rolling on the freshly sprayed surface. The cover aggregate should be applied ahead of the spreader wheels, with

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sharply defined, straight edges and without any lateral movement of the aggregate after initial application.

The spreader is to be designed to apply aggregate at any application width between 2 m and 4.12 m. A 4.26 m hopper is required.

508.21.03 Rollers – Self-propelled pneumatic tired rollers (minimum mass of 11,000 kg) to be used on the work.

All rollers shall be provided with equipment for spraying water continuously on all tires or wheels, while rolling is in progress, and be provided with such means as may be required to keep the tires clean and free of adhering asphaltic binder.

508.21.04 Power Brooms – Brooming work to be carried out using rotary power brooms. The brooms are to have sufficient width, power and brushing capacity to completely clean the surface of the standard lane to be treated or which has been treated within three coverages.

508.21.05 Water Truck – Water trucks to have a minimum capacity of 9000 L. Water to be applied from a distributor of the pressure type, equipped with spray bar mounting nozzles similar to those used on asphalt distributors and capable of applying the water accurately and uniformly.

Splash plate type of distributors or those equipped with spray bars that eject fine streams of water will not be permitted.

The distributor to be provided with a satisfactory means for accurately measuring the quantity of water sprayed. Measuring equipment is to be calibrated. The Contractor shall make all necessary arrangements for obtaining water.

508.21.06 Tanks, Mobile Asphalt Heating, & Storage – Insulated liquid asphalt storage tanks should meet the following minimum criteria:

- (a) A heating system, capable to heat from 10°C to 200°C.
- (b) An automated heating and control system with safety shutdown.

CONSTRUCTION

508.31 Daylight Hours Only – Construction shall be carried out during daylight hours only.

508.32 Traffic Control and Signing – The Contractor shall submit, for the approval of the Ministry Representative, a Traffic Control Plan at least seven days prior to the pre-construction meeting.

Traffic Control on Seal Coat projects shall be in accordance with SS 194 as well as the Ministry's publication "Traffic Control Manual for Work on Roadways".

The Traffic Control shall be maintained at all times when in the opinion of the Ministry Representative, the risk of damage to the mat is likely to become a safety issue or to

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result in unnecessary vehicular damage from loosened rock. Additional traffic control to protect the quality of the product is also the responsibility of the Contractor.

Traffic through the work zone shall be limited to 50 km per hour, or as directed by the District Manager Transportation, until completion of final sweeping.

Extending traffic control beyond the end of the shift, in order to maintain safety and reduce vehicular damage risks, will not preclude the Contractor from providing and applying blinding material to reduce these risks.

The Contractor shall continue to maintain flag-person assisted traffic control over the extent of the day's production, for a minimum of 24 hours for work on numbered routes, 2 hours on other routes, or as specified in the Special Provisions.

508.33 Temperature and Weather – Air temperature shall be a minimum of 6°C for application of graded aggregate seals, and application shall cease when the temperature falls below that level.

No application shall take place when the weather is misty or rainy.

508.34 Grade Preparation – Immediately prior to the application of the graded aggregate seal coat on a previously constructed granular surface, the surface shall be watered, graded and compacted and/or broomed to provide a non-segregated, tightly knit surface free from loose rock. Cross-fall will be at 3% or as directed by the Ministry Representative and super-elevations developed uniformly and carried consistently through each curve.

508.35 Joints – All joints between adjacent passes of seal shall be at centreline or lane lines.

508.36 Communications – The flag people, pilot car or cars, sign truck, spreader and supervisor's vehicle shall have an effective electronic means of communication to relay instructions and information accurately and quickly over the total length of the control area.

508.37 Sweeping – Sweeping shall meet the requirements of SS 145, SS 165 and the following:

- (a) Immediately prior to the commencement of the sealing operations, all loose aggregates, dust, dirt, caked clay or foreign materials shall be removed from the width of the surface to be treated by brushing with power brooms supplemented by hand push brooms, shovels or the use of a power grader, and where necessary by flushing.
- (b) Particular care shall be taken to thoroughly clean to outside edges of the strips to be treated and to ensure that the sweepings are not so deposited on the shoulder as to permit subsequent contamination of the treated surface.
- (c) Following completion of sealing operations, the Contractor shall remove all loose aggregate remaining

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on all surface treated and paved surfaces to the satisfaction of the Ministry Representative. The Contractor will provide a minimum of 72 hours' notice of when the removal of loose aggregate will be complete and ready for inspection. The Ministry Representative will provide written acceptance that the removal of loose aggregate from all surface treated and paved areas has been satisfactorily been completed.

508.38 Repair – Any patching or repair work required, in the opinion of the Ministry Representative, before sealing operations begin, may be done by the Contractor as directed by the Ministry Representative and paid for under Extra Work.

508.39 Double Seal, Second Pass – Where the design calls for a double seal, the second pass of the surface treatment shall commence not less than 24 hours after completion of the first pass, subject to acceptable weather conditions as detailed in SS 508.33, and as the Ministry Representative may direct.

QUALITY CONTROL AND QUALITY ASSURANCE

508.41 Quality Control by the Contractor – The Contractor shall be responsible for providing all resources required to carry out the quality control on all the constituent materials, processes and products, their testing and inspection, within the construction of the seal coat and the quality of the end product (See SS 508 Appendix E – Quality Control Requirements and Guidelines).

508.41.01 Quality Control Plan – The Contractor shall prepare and submit a Quality Control Plan, in accordance with the Contract requirements, at least 7 days prior to the pre-construction meeting.

508.41.02 Sampling, Testing and Observing – The contractor shall provide properly maintained equipment, qualified personnel and shall maintain access to a qualified laboratory so that quality control tests and observations are performed and the results must be reported to the Ministry Representative within 48 hours. For asphalt emulsions, the test results must be received within 21 days of sampling.

The Contractor's Quality Control Plan shall provide details of the equipment provided, its calibration, and application methodology to meet the Contract requirements.

508.42 Quality and Quantity Control Records

508.42.01 Emulsion Quality – For each batch of emulsion supplied to the job Site, the Contractor shall obtain from the supplier certified results for each test specified in Table 508-E (Appendix 508-A), and provide those results to the Ministry Representative prior to incorporating the emulsion into the Work.

When certified batch results are not yet available, the Contractor may elect, at its own risk, to use the uncertified material. If the material is subsequently determined to be

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non-compliant with the specifications or certified batch results are not provided, remediation of any Work using such material will be to the Contractor's account.

508.42.02 Aggregate Quality– The Contractor shall assess the supplied aggregate to determine and ensure the quantity and quality for the intended application. Any modifications deemed necessary to ensure a quality product shall be made at the Contractor's expense.

508.42.03 Emulsion Usage – Daily emulsion usage shall be made available to the Ministry Representative on the Form “Emulsion Usage” (Appendix 508-C). Information shall be made available to the Ministry Representative within 48 hours and prior to demobilization of the storage tanks and pressure distributor from the Site.

Emulsion usage will be reconciled against initial tank volume, delivery records and remaining volume prior to the Contractor demobilizing from the Site.

508.42.04 Aggregate Usage – The Contractor shall monitor the aggregate spread rate on a production day basis and make the results available, in an acceptable format, to the Ministry Representative within 48 hours.

508.42.05 Daily Activity Log – The Contractors' Superintendent or designate shall complete and make available to the Ministry Representative the “Daily Activity Log” (Appendix 508-D) within 48 hours. This form shall be completed and submitted for all calendar days from the first day of mobilization to the last day of demobilization, for any day on which work was performed on-site.

508.42.06 Emulsion Sampling

- (a) The Contractor shall, procure and store a four litre sample of the emulsion from each tanker for use by the Ministry and/or Contractor.
- (b) These samples shall be obtained, handled and stored in accordance with ASTM D140 and also with the manufacturer's recommendations.
- (c) The samples shall be identified and retained by the Contractor until demobilization at which time permission may be granted by the Ministry Representative to discard the samples in an environmentally friendly manner.
- (d) The Ministry Representative may provide the Contractor with a copy of the test results from the Ministry's Quality Management program. The Contractor shall not utilize these tests in place of their own Quality Control Program.
- (e) The absence of Ministry test results shall not relieve the Contractor's obligation to remedy any defect in materials or construction.

508.42.07 Final Quantity Report – The Contractor shall supply to the Ministry Representative, prior to final acceptance of the work, the total amount of emulsion and

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graded aggregate seal aggregate used on the project, and the volume of unused emulsion remaining in the tank(s).

END PRODUCT ASSESSMENT

508.51 General – The Contractor shall provide a product conforming in accuracy and detail as specified by the contract. Payment is based on end product assessment by the Ministry Representative.

508.52 Surface Deficiency Definitions – See Appendix 508-F for examples of surface conditions, photos and details of the conditions listed below.

508.52.01 Blackening – Blackening is the emergence of excess asphalt onto the finished surface whereby the finished surface becomes discoloured, but the coarse surface texture remains. Under this condition, the effective cover stone can still be seen to protrude above the level of the asphalt binder's upward migration

508.52.03 Flushing and Bleeding – Flushing and bleeding is the emergence of excess asphalt and fines onto the finished surface and the significant loss of coarse surface texture, leaving little or no effective cover aggregate visible.

508.52.04 Pock Marks – Pock Marks are indentations in the mat texture, normally less than the thickness of the graded seal layer, evidenced by the absence of effective cover aggregate in a small area (No photograph available).

508.52.05 Pot Holes – Pot Holes are the loss of all cover aggregate and base material(s) into the underlying asphalt or gravel base. Pot Holes may develop because of seal and/or base weakness.

508.52.06 Raveling – Raveling is a general term for loss of cover aggregate, which can include numerous pock marks.

508.52.07 Streaking – Streaking is a continuous visible absence of effective cover aggregate often seen paralleling a ‘ridge’ of excess cover aggregate, characterized by the narrowness of its width and the significant extent of its length. Where streaking displays an absence of effective cover aggregate, the condition is unlike raveling in that there was originally little or no effective aggregate in the area.

508.52.08 Washboard – Washboard is a condition of non-uniform surface texture caused by the buildup of transverse ridges within the graded seal layer itself, and characterized by a very rough, vibratory or chattering ride (No photograph available).

508.53 End Product Evaluation

508.53.01 Lot Definition – A Lot shall be one lane kilometre of highway sealed. Sections of any road/highway remaining which are less than one-half (0.5) kilometres in length, will be included in the previous Lot. Sections remaining which are 0.5 kilometres or greater, but less than

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one kilometre will receive a Lot designation unique for the road/highway.

508.53.02 Rating Time Criteria – The Ministry Representative will rate each Lot, within thirty (30) days of final brooming.

508.53.03 Performance Rating Criteria – Performance rating will be in accordance with Appendix 508-F, Table 508-G, Table 508-H, and the photographs.

508.54 Deficiency Repair and Replacement – Any Lot of sealed surface that is found unacceptable shall be remedied in accordance with Table 508-G (Appendix 508-F), until the whole project has been repaired or replaced as necessary and accepted by the Ministry Representative.

On double seal applications both the lower and upper lifts must be free of deficiencies prior to being accepted.

The sealing Contractor will not be responsible for deficiencies attributed to base, base failures and base work done by others. These may be repaired under a Provisional Sum Item provided for that purpose, or as ordered by the Ministry Representative.

For any defects being repaired through re-seal, the repair must cover the entire width of the highway (or to a centerline barrier), unless the Contractor can demonstrate that a compliant repair can be achieved at a narrower width without any longitudinal ridging which may adversely affect driver control or trap water adjacent to the edge of the repair.

508.55 Attention to Potential Safety Hazards – If, in the opinion of the Ministry Representative, remedial Work is required to correct a potential safety hazard it shall be undertaken immediately.

MEASUREMENT AND PAYMENT

508.61 Surface Seals – Surface Seal will be measured and paid by the square metre.

Payment in a double pass application will be for final top lift surface area only.

Payment will be full compensation for pre-cleaning the surface; loading, hauling, placing the asphalt emulsion and aggregate; compaction; brooming the finished surface; quality control and traffic control.

No extra payment will be made for any additional equipment, tools, labour or material used to replace, repair or overlay Unacceptable Work.

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No extra payment will be made for any additional equipment, tools or labour employed to satisfy special brooming requirements.

508.62 Asphalt Emulsion – Asphalt emulsion will be measured and paid in litres of emulsion actually sprayed on the road, including supply, storage, heating and handling.

508.63 Aggregates – Aggregates will be measured by the cubic metre in stockpile. The volume will be determined by surveying and surface-to-surface volume calculations. Measurement shall be performed by the Contractor at the Contractor's expense using an independent surveying company. Survey results must be verified and signed off by the independent surveying company and survey results must be supplied to the Ministry.

Payment for aggregates will be at the Contract Unit Price per cubic metre in stockpile. Payment for aggregate will be only up to 100% of the Approximate Quantity as stated in Schedule 7.

No payment will be made for the supplying, hauling and placing of blinding sand.

The supply of extra aggregate, required to repair or overlay Unacceptable Work, will be the Contractor's responsibility. Where sufficient aggregate quantities are remaining in stockpile, the Ministry will make these available at no cost to the Contractor.

508.64 Stockpile Bases – The stockpile base must be prepared prior to surveying and stockpiling. No payment will be made for stockpile base preparation work and no payment will be made for materials required for the preparation of stockpile bases.

509.65 Claims for Vehicular Damage – Without in any way limiting the Contractor's obligations or liabilities herein, during construction and up until final acceptance of the whole project by the Ministry Representative, the Contractor shall be fully responsible for all claims for damages caused by the work and shall address and deal with each claim submitted.

508.66 Partial Payment for Rejected Work – In the Ministry Representative's sole discretion and without setting precedence, where any work is reject but the Ministry Representative determines that it may be left in place, the Ministry Representative may authorize partial payment to the Contractor as full compensation for any residual value the work may have. Notwithstanding the foregoing, the Ministry is under no obligation to make any payment for reject work.

APPENDIX 508-A

QUALITY CONTROL TESTS FOR HIGH FLOAT EMULSIONS

Quality Control Tests, to be obtained by the Contractor from the Supplier, for High Float Emulsified Asphalts.

Table 508-E: Quality Control Tests for High Float Emulsified Asphalts

Designation	ASTM Test Title
D5 (CAN2-16.5-M84)	<u>Standard Test Method for Penetration of Bituminous Materials</u>
D88	<u>Standard Test Method for Saybolt Viscosity</u>
D113	<u>Standard Test Method for Ductility of Asphalt Materials [Ductility Test for Polymer Modified Emulsions]</u>
D139 (CAN2-16.5-M84)	<u>Standard Test Method for Float Test for Bituminous Materials</u>
D140	Standard Practice for Sampling <u>Asphalt</u> Materials
D244 (CAN2-16.5-M84)	<u>Standard Test Methods and Practices for Emulsified Asphalts [Residue by Distillation]</u>
D4957 (CAN2-16.5-M84)	<u>Standard Test Method for Apparent Viscosity of Asphalt Emulsion Residues and Non-Newtonian Asphalts by Vacuum Capillary Viscometer [Absolute Viscosity of Asphalts]</u>

**APPENDIX 508-B
HIGH FLOAT EMULSIFIED ASPHALT REPORT (H-539)**

Project No: _____ **Refinery:** _____

Contract: _____ **Asphalt Grade:** _____

Location: _____ **Batch No:** _____

Sampling Date: _____ **Testing Date:** _____

Requirements	Results	Specifications	
		Maximum	Minimum
Residue by Distillation, % by mass			
Oil Distillation, % by volume			
Saybolt Furol Viscosity (SFS), 50°C			
Penetration, 5 s/100 g/25°C			
Viscosity, mK or CM (Pa•s), at 60°C			
Ductility, at 4°C			
Float Test (s), at 60°C			

**APPENDIX 508-C
EMULSION USAGE REPORT**

Project No.: _____ **Product:** _____

Date: _____ **Supplier:** _____

Contractor: _____ **Supplier Lading No.** _____

<u>Emulsion Temp.</u> (°C)	<u>EMULSION QUANTITIES (litres)</u>			<u>SPRAY COVERAGE (m or m²)</u>			<u>APPL. RATE (L/m²)</u> [L/F]	<u>LOCATION†</u> (Sta. to Sta. or LKI, and lane being sealed)
	<u>Start</u> [A]	<u>Finish</u> [B]	<u>Used</u> [L=A-B]	<u>Width</u> [D]	<u>Length</u> [E]	<u>Area</u> [F=DxE]		

TOTALS: _____ = Sums for "Used" and "Area"

AVERAGE: _____ = Total Used / Total Area

The Undersigned certifies that the quantities placed are as indicated in the information above.

Name (Contractor Operator): _____ **Date:** _____

Checked by (Ministry Representative): _____ **Date:** _____

† Example location description: Sta. 3+500 to 5+100, WBSL (where WBSL = Westbound slow lane)

APPENDIX 508-E**QUALITY CONTROL REQUIREMENTS AND GUIDELINES****1.01 GENERAL**

The Contractor's guidelines for preparing, submitting and adhering to a Quality Control Plan are specified in SS 101, SS 508, the Special Provisions and this Appendix 508-E.

The QC Manager shall have at a minimum 5 years related experience in Quality Management.

1.02 QUALITY CONTROL PLAN

The Plan shall address all elements that affect the quality of the materials, end product seal coat, including but not limited to the following:

- Supply of Asphalt Materials
- Identification of source and proof of quality of aggregates to be supplied
- Aggregates production and its gradation control
- Quality of aggregate components during production
- Stockpile management
- Sealcoat Design(s)
- Distributor and spreader calibration
- Process temperature controls
- Material application rates
- Joints and tapers

1.03 CONTRACTOR'S RECORD OF QUALITY CONTROL TESTING

Test results should be made on specified forms or charts immediately after completion of each test. These test results are to be made available to the Ministry Representative upon request.

Records of gradation control during aggregate production should be kept on the form H-295 – Mechanical Analysis of Aggregates (available from the Ministry Representative) or on a similar gradation analysis form acceptable to the Ministry Representative.

1.04 MATERIAL APPLICATION RATE

The Contractor shall control the Material Application Rate by monitoring the amount of emulsion and aggregate delivered to the road against the area covered by checking the application rates of the distributors and spreader.

1.05 OTHER QUALITY CONTROL PROCEDURES

The Contractor may initiate other Quality Control procedures as necessary for ensuring production of a quality product and include them in a Quality Control Plan. Procedures may also be introduced after the start of work as amendments to the Quality Control Plan.

1.06 QUALITY CONTROL TESTING FREQUENCY

Recommended test frequency requirements for Quality Control are described in Table 508-E.

1.07 QUALITY ASSURANCE TESTING FREQUENCY

Quality Assurance Testing shall be approximately 10% of that described for Quality Control and may vary in accordance with the Quality Control Plan and the observed effectiveness of Quality Control.

Table 508-F: Test Frequency Guidelines

Activity	ASTM Test Designation	*Minimum Frequency
Tests Before Aggregate Production	D75, Standard Practice for Sampling Aggregates. Sampling Stone, slag, gravel, sand and stone block for use as Highway Materials	When Required
	C117, Test for Materials Finer than 75 µm (No.200) Sieve in Mineral Aggregates by Washing	One (1) test (Representative Sample)
	C142, Test Method for Clay Lumps and Friable Particles in Aggregates	One (1) test (Representative Sample)
	D6928, Micro Deval	One (1) test (Representative Sample)
	D5821, Fracture Count on Coarse Aggregate	One (1) test (Representative Sample)
	D4791, Test Method for Flat Particles, or Flat and Elongated Particles in Coarse Aggregate	One (1) test (Representative Sample)
Tests During Aggregate Production	C136, Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates	Main Stockpile – One (1) per each 1.5 hours production.
	D5821, Determining the Percentage of Fractured Particles in Coarse Aggregate	Main Stockpile – One (1) per each 1.5 hours production.
	C117, Test for Materials Finer than 75 µm (No 200) Sieve in Mineral Aggregates by Washing	One (1) per shift on reduced sample obtained from combined samples from the crusher.
Tests on Emulsified Asphalt Products	D244, Residue by Distillation	Minimum one (1) per batch
	D244, Saybolt Furol Viscosity	Minimum one (1) per batch
	D5, Penetration of Bituminous Materials	Minimum one (1) per batch
	D139, Float Test	Minimum one (1) per batch
	D4957, Apparent Viscosity on Residue	Minimum one (1) per batch
	D113, Ductility	Minimum one (1) per batch on polymer modified emulsions.
Tests on Application of Sealcoat Materials	Emulsion Pan Test	Three (3) per distributor/day. Test procedure, variation limits and actions should be outlined in Contractor's Quality Control Plan.
	Aggregate Pan Test	Two (2) per spreader/day. Test procedure, variation limits and actions should be outlined in Contractor's Quality Control Plan.

* These are the recommended minimum frequencies and the Contractor is responsible to assess the need to increase testing when required.

APPENDIX 508-F
SURFACE DEFECTS

Table 508-G: Surface Defect and Treatment Repair / Replacement Criteria

Surface Defects	Severity	Rating Parameters	Repair/Replacement Method
Loss of cover aggregate (Raveling)	Very good	No noticeable aggregate loss	None
	Good	A few pock marks, less than 5 per 0.1 m ²	None
	Fair	Frequent pock marks closely spaced, more than 5 per 0.1 m ²	Reseal of affected area***
	Poor	Extensive pock marks or few surface disintegrations	Reseal of affected area***
	Very poor	Disintegration with potholes	Reseal of affected area***
Potholes	Very good	Few minor potholes, only involves graded <u>aggregate seal</u>	All potholes to be filled with asphaltic patching material at contractor's expense
	Good	Few deep potholes, involves granular base	All potholes to be filled with asphaltic patching <u>material</u> . (see SS 508.54)
	Fair	Intermittent potholes	Patch potholes and reseal affected area*** (see SS 508.54)
	Poor	Frequent potholes	Patch potholes and reseal affected area*** (see SS 508.54)
	Very poor	Extensive or potholes throughout	Patch potholes and reseal affected area*** (see SS 508.54)
Flushing / bleeding	Very good	No/very faint colour change in wheel path	None
	Good	Few sections with asphalt on surface	None
	Fair	Intermittent sections with asphalt on surface	Application of sand blotter
	Poor	Frequent sections with asphalt on surface, has wet look or asphalt on surface throughout	Reseal of affected area. Removal of initial surface treatment at Contractor's option***
	Very poor	Wet look with noise like a wet pavement	Reseal of affected area. Removal of initial surface treatment at Contractor's option***
Total failure	Any	Any condition where the asphalt material softens or disintegrates under traffic and aggregate is picked up or "kicked off" by traffic	Remove and dispose of failed surface treatment in its entirety and apply new surface treatment
Streaking (see raveling)	Any	Absence of cover aggregate caused by too little binder – characterized by long, narrow sections of little or no stone cover	Reseal of affected area
Washboard	Any	Any condition of repetitive build-up across the travel lane	Remove and reseal affected area

NOTE: *** Reseal for these defects shall consist of a new graded aggregate surface treatment, single pass application. Area of the reseal repair/replacement shall not be less than one lane width, 10 metres in length. If there is less than 10 metres between two sections in the application pass designated for repair/replacement, the repair/replacement shall be continuous.

Table 508-H: Surface Defects Frequency Definitions

Density of Surface Defects**			
Severity	Raveling (% Length)	Potholes (Number)	Flushing/Bleeding (% Length)
Very Good	<5%	0 – 1	<5%
Good	5% – 20%	2 – 15	5% – 20%
Fair	21% – 50%	16 – 30	21% – 50%
Poor	51% – 80%	31 – 50	51% – 80%
Very Poor	>80%	>50	>80%

Note: ** Based on number of surface defects or percent of surface affected by defect, per Lot as defined in SS 508.53.01.



Blackening





Flushing or Bleeding





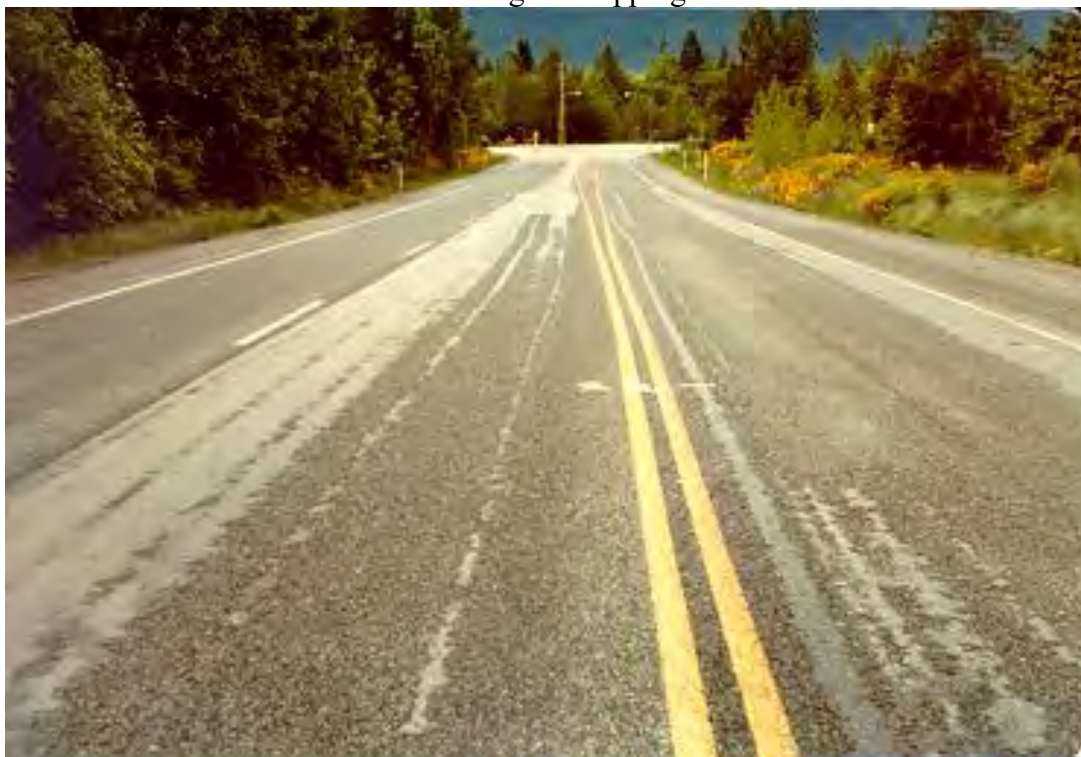
Pot Hole



Streaking



Raveling or Stripping



SECTION 511

COLD MILLING

DESCRIPTION

511.01 Scope – This work consists of preparing a foundation for placement of new surfacing by cold milling the existing pavement surface to the depth and width as indicated on the Drawings.

EQUIPMENT

511.21 Milling Machine – The milling machine shall be self-propelled and shall be equipped with automatic longitudinal and transverse grade control, which shall be used when directed by the Ministry Representative. A profiling ski or boom of a recommended minimum 4 m length, approved by the Ministry Representative, shall be used. No substitute equipment will be permitted. The cutting drum shall be a minimum of 1.8 m in width, totally enclosed and with replaceable cutting teeth. There shall be an effective means of removing the loosened material from the surface and for preventing dust from escaping into the air.

511.22 "Kill" Switch – All pavement milling machines shall be equipped with a "kill" switch installed or approved by the manufacturer of the milling machine. This "kill" switch shall be automatically actuated whenever the operation of the machine is so impeded that a hazardous situation, such as "kick back", would result were that operation to continue. When so actuated the "kill" switch shall instantly shut down the operation of the milling machine.

511.23 Audible Back Up Warning Device – See SS 145.30.04.

CONSTRUCTION

511.31 Running Surface for Lanes Open to Traffic – The Contractor shall ensure that a suitable running surface is maintained on any portion of the highway that is open to traffic during construction.

At all times, during the milling operation, the traveled roadway shall be kept clean of all loose materials.

511.32 Pavement Removal – The existing pavement shall be removed to the depth and width as specified in the Special Provisions, on the drawings or as directed by the Ministry Representative to provide a surface that is free of longitudinal and transverse irregularities. Milling may be specified as a nominal uniform depth, or as variable depth profile milling to achieve design lines and grades.

The use of a heating device to soften the pavement will not be permitted.

At the end of the milling shift or at the end of each section to be milled, prior to reopening the road to traffic, the pavement shall be cleaned and swept so that all loose material is removed. In addition, the transition from the milled surface to the existing paved surface shall be tapered at a maximum slope of 50:1, or as directed by the Ministry Representative. The Ministry Representative shall indicate whether the taper is to be milled or a tar paper letdown shall be constructed using asphalt mix.

At the end of each shift, all travelled lanes shall be milled to the same point to eliminate any uneven edge between lanes. Where median barrier is present, lanes in the same direction of travel shall be milled to the same point; lanes in the opposing direction may be milled to a different point. When the milling operation traverses intersecting roads or on and off ramps, the transition from the milled surface to the existing pavement shall be at a maximum slope of 50:1 or as directed by the Ministry Representative. This transition can be done with the milling machine or by using tar paper letdowns as approved by the Ministry Representative.

If, due to delays between the milling and paving operations, the milled surface starts to pothole or deteriorate, repairs shall be carried out at once using asphalt mix.

At those locations where milling is conducted, the Contractor shall cut drainage channels as required to prevent water from collecting in the milled area.

If washouts occur at any time during the milling operations, they shall be immediately repaired by the Contractor, at the Contractor's own expense, with approved materials and as directed by the Ministry Representative.

511.33 Disposal of Materials – The disposal of materials removed under this section, which are not recycled and used on the project, shall become the property of the Contractor at the point of removal, unless otherwise specified. Materials shall be disposed of outside the limits of the project in a manner satisfactory to the Ministry Representative and shall meet all environmental requirements. The procurement of a disposal site, if required, shall be the Contractor's responsibility.

511.34 Existing Structures – Care must be taken not to disturb or damage any structures or devices such as manholes, catch basins, valves, boxes and other utilities. All metal and concrete faces must be cleaned of old pavement.

The Contractor shall repair, at its own expense, any damage to visible, referenced, or plan-indicated manholes, catch basins, valves, valve covers and concrete/asphalt curb, or any other infrastructure.

MEASUREMENT

511.81 General – Cold Milling will be measured by the SQUARE METRE or the TONNE as specified in the Contract. The area quantities will be determined by measurement of the actual surface area from which the materials have been removed and computed to the nearest square metres. Measurement of material by mass shall be done in accordance with SS 145.18.04 and SS 145.19 before moving to stockpile.

PAYMENT

511.91 General – Payment for COLD MILLING will be at the Unit Price bid per square metre or tonne as specified in the Contract. The unit price shall be for all things furnished and done including servicing all equipment, replacing teeth on the cutting drum, and performing all work to remove the existing pavement by milling, loading, hauling and placing the material in stockpile.

511.92 Asphalt Mix – Asphalt mix required to repair the milled surface as set out in SS 511.32 shall be paid at the Unit Price bid for top lift asphalt, except where, in the opinion of the Ministry Representative, the milled surface damage occurred during a delay that was the Contractor's responsibility, in which case there will be no payment.

SECTION 515

HOT-IN-PLACE RECYCLED ASPHALT PAVEMENT (EPS)

GENERAL

515.01 General – This Section describes the materials, equipment, professional standards, and end product requirements for the construction of Hot-In-Place Recycled Asphalt Pavement (HIP).

End Product Specifications (EPS) contain the acceptance and payment criteria based on the results of specified sampling and testing. Payment of the contract Unit Prices for the Hot-In-Place asphalt pavement product is subject to adjustments upward and downward in accordance to the provisions provided in EPS.

Standards applicable to this Section are listed in Table 515-A.

515.02 General Description of Work – The Contractor will be responsible for the following work associated with the construction of Hot-In-Place asphalt pavement by EPS:

- Preparing a Quality Control Plan for evaluation before commencing the Work and providing at the production site a testing facility to provide the data needed to implement that plan.
- Supplying, screening, crushing, processing and improving aggregate to produce virgin asphalt mix aggregate.
- Producing or supplying, and testing, all materials in accordance with Table 515-B through Table 515-E inclusive, and ensuring compliance with Contract specifications.

- Supplying and delivering asphalt cement meeting the requirement of SS 952.
- Rejuvenating Agent Requirements: The Contractor shall supply a rejuvenating agent from the Ministry's Recognized Product List and meeting the requirements shown in Table 515-A.

The Ministry's Recognized Product List can be found on-line at:

https://www2.gov.bc.ca/assets/gov/driving-and-transportation/transportation-infrastructure/engineering-standards-and-guidelines/recognized-products-list/recognized_products_list.pdf?forcedownload=true

Rejuvenating agents that contain water will not be permitted.

- Heating the virgin asphalt mix aggregate and mixing it with asphalt cement to produce asphalt admix that meets the Job Mix Formula.
- Hauling, and adding the asphalt admix into the recycling process.
- Recycling the existing highway pavement by heating, milling, reprocessing, with or without the addition of rejuvenating agents or admix (virgin mix) as specified, re-mixing, replacing and compacting using a multi-stage, hot milling process and rollers.

Table 515-A: Specifications

ASTM Test	Title of Test	Specification
D2726	Bulk Relative Density of Compacted Bituminous Mixtures Using Saturated Surface Dry Method	
D1188	Bulk Relative Density of Compacted Mixes Using Paraffin Coated Method	
D1559	Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus	
D2170	Kinematic Viscosity @ 60°C	200 – 500 mm ² /s
D70	Specific Gravity	0.98 – 1.02
D92	Flash Point by Cleveland Open Cup	204°C (Min)
D116 (10 mm Hg)	Volatility – IPB <ul style="list-style-type: none"> • 2% v. • 6% v. 	150°C (min) 190°C (min) 210°C (min)
D2007	Saturates, %w	28 (max)
D2006 – 70	Asphaltenes, %w	1.5 (max)
D2006 – 70	Chemical Composition (PC+A ₁)/(S+A ₂)	0.4 – 0.8

Table 515-B: Standard Sampling and Testing Procedures for Virgin Aggregates

Item	Procedure	ASTM Test Reference
1	Sieve Analysis of Fine and Coarse Aggregates	C136
2	Materials Finer Than 0.075 mm (No. 200) Sieve in Mineral Aggregate by Washing	C117
3	Specific Gravity and Absorption of Coarse Aggregate	C127
4	Specific Gravity and Absorption of Fine Aggregate	C128
5	Determining the Percentage of Fractured Particles in Coarse Aggregate	D5821
6	Total Moisture Content of Aggregate by Drying	C566

All virgin aggregate tests shall use the following sieve sizes:

37.5 mm, 25 mm, 19 mm, 16 mm, 12.5 mm, 9.5 mm, 4.75 mm, 2.36 mm, 1.18 mm, 0.600 mm, 0.300 mm, 0.150 mm, and 0.075 mm.

Table 515-C: Standard Sampling and Testing Procedures for Virgin Asphalt Admix and Hot-In-Place Recycled Pavement

Item	Procedure	ASTM Test Reference
1	Bulk Specific Gravity and Density of Non-Absorptive Compacted Bituminous Mixtures	D2726
2	Theoretical Maximum Specific Gravity and Density of Bituminous Paving Mixtures	D2041
3	Sampling Compacted Bituminous Mixtures for Laboratory Testing	D5361
4	Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus	D1559 or AASHTO T 245
5	Determining Asphalt Content by Ignition Methods for Admix	D6307

Table 515-D: Standard Sampling and Testing Procedures for Asphalt Cement

Item	Procedure	ASTM Test Reference
1	Penetration of Bituminous Materials	D5
2	Viscosity of Asphalt by Vacuum Capillary Viscometer	D2171

Table 515-E: Test Sample Source Locations

Test	Sample Source
Density	150 mm diameter road cores
Smoothness	Centre of Lane

- Heating shall be regulated so no overheating of the existing asphalt cement will occur. This will generally require the existing surface to be radiantly heated and no open flame directed at the pavement will be permitted.
- The cutting drum(s) shall mill to the specified cross section and shall be set to produce a fully homogenous recycled mixture free of lumps.
- The activated heated screed shall be complete with augers and strike-off device in accordance with SS 502 and be capable of distributing and placing the reprocessed mix to the full width of the pavement being recycled.

Production shall have a minimum rate of 500 m² per hour except between June 1st and September 8th when the minimum rate shall be 700 m² per hour.

- Hot recycling unit shall be equipped, capable to add and homogeneously blend any specified proportion of virgin Asphalt Mix into the reclaimed material and shall be equipped with a means to introduce a rejuvenating agent homogeneously into the reclaimed mixture.
- Individual pre-heating and reprocessing units shall be equipped in such a way that the operator and other workers are not exposed to any hazardous fumes or gases produced from the equipment operation, the heated pavement or from the addition of the rejuvenating agent, in accordance with all applicable regulations.

The Contractor shall provide an end product conforming in quality and accuracy of detail to the dimensional and tolerance requirements of the contract. Where no tolerances are specified, the standard of workmanship shall be in accordance with normally accepted good practice.

Payment is subject to adjustments based on quality assurance tests performed by the Ministry Representative.

The Ministry shall provide information pertaining to the gradation of the admix, asphalt content, addition rates for rejuvenating agents, and the percentage of admix to be incorporated.

515.03 Definitions Relevant to End Product Specification

515.03.01 End Product Specification (EPS) – A specification whereby the Contractor is entirely responsible for quality control of the construction processes, and whereby the Ministry performs the specified quality assurance sampling and testing of the end product for the purpose of determining acceptance/rejection and payment.

515.03.02 Job Mix Formula – Establishes the virgin aggregate proportions, gradation, the asphalt content and type of asphalt to be used for production of asphalt admix. The Job Mix Formula is based on an asphalt mix design, in accordance with SS 515.05.06, or on a variation in accordance with SS 515.05.07.

515.03.03 Asphalt Admix – Virgin asphalt mix added to the recycled mix. The Asphalt Admix is project specific designed and is blended homogeneously within the recycling process. The Asphalt Admix becomes an integral component of the recycled pavement structure.

515.03.04 Asphalt Content – The quantity of asphalt cement in the virgin admix expressed as a percentage by weight of the total dry aggregate in the mix.

515.03.05 Design Asphalt Content – The asphalt content upon which the Job Mix Formula is initially established.

515.03.06 Approved Asphalt Content – The design asphalt content or subsequent adjustment to it, embodied in a Job Mix Formula or revised Job Mix Formula.

515.03.07 Actual Asphalt Content – The amount of asphalt cement in the admix as determined by the Contractor's Quality Control Testing.

515.03.08 Smoothness – A measure of the longitudinal profile of the pavement surface. The unit for measurement is the International Roughness Index (IRI).

515.03.09 Sample Mean – The arithmetic mean of a set of test results constituting the sample.

515.03.10 Stratified Random Sample – A set of test measurements taken one each from a number of separate (stratified) areas or Sub-Lots within a Lot in an unbiased way.

515.03.11 Lot – A portion of the work being considered for acceptance and for the determination of payment. A Lot is defined as follows for the application of the contract requirements for:

(a) **Density:** A Lot shall be one day's scheduled production of at least 7 hours of pavement recycling where no changes have occurred to criteria such as but not limited to:

- Approved Job Mix Formula;
- Specific thickness to be processed;
- Required materials addition rate;
- Change in the ratio of the components making up the Hot-In-Place Recycled Pavement.

A change in any above criteria may require a new Lot designation.

One day's production of less than 7 hours will be dealt with as follows:

- The material will be added to the next Lot that has the same criteria, as described above, except that
- If a test indicates that this production is subject to a payment adjustment or to rejection, or if no further material will be produced with the same criteria, this production will be designated as a separate Lot.

A Lot shall be no more than two days total production even if the above criteria has not changed or been met.

(b) **Smoothness:** A Lot shall be one kilometre length of recycled pavement for each driving lane.

515.03.12 Sub-Lots for Density and Smoothness – For the application of the contract requirements of:

- (a) **Density:** Each Lot shall be divided into three equal Sub-Lots.
- (b) **Smoothness:** Each Lot shall be divided into 100 metre Sub-Lots.

515.03.13 Driving Lane – A single lane in any area of the pavement other than a shoulder or a barrier flare.

515.03.14 Surplus Aggregate – Aggregate surplus to the Works, in split or un-split stockpiles which singly or combined will meet the virgin asphalt mix aggregate mix gradation. Surplus aggregate will be paid for as indicated in SS_515.32.01. There will be no payment for reject aggregate.

515.03.15 Hot-In-Place Recycled Pavement – Shall be the results of the process where recycling, the addition of admix and/or rejuvenating agents, the spreading, compacting and finishing of the asphalt pavement have taken place to form a quality finished product. The Hot-In-Place Recycled Pavement may be comprised of the following components: existing asphalt pavement, admix, and/or rejuvenating agents.

515.04 Quality Control

515.04.01 General – Quality Control, by the Contractor is the sum total of activities by the Contractor to ensure that a product meets contract specification requirements. Quality Control includes material handling and construction procedures, calibration and maintenance of equipment, production process control and any sampling, testing and inspection that is done for these purposes.

The Contractor is responsible for all Quality Control under this Section. The Ministry Representative will audit and monitor the Contractor's operations and the implementation of the Contractor's Quality Control Plan.

The Ministry will not take samples for quality control testing and will in no manner assist in any degree or in any aspect of the Contractor's operation in the production of asphalt pavement, beginning from the production of aggregate through the compaction of asphalt mix. The only exception shall be in accordance with the provisions of SS 515.05.07, where the Ministry shall have the ability to adjust the ratio of the components of the Hot-In-Place Recycled Pavement based upon the Contractor's Quality Control results and visual inspections.

The Ministry Representative may issue a Stop Work Order to the Contractor if the Contractor fails to adhere to the Quality Control Plan. The Ministry Representative will not issue a Resume Work Order until the Contractor has given the Ministry Representative satisfactory assurance that it has in place adequate capacity to fulfill the requirements of the Quality Control Plan.

515.04.02 Quality Control Plan – The Contractor shall prepare a detailed, written Quality Control Plan, based on the guidelines as shown in Appendix 515-A and functions integrally with any other Quality Management provisions of the Contract. The Contractor's Quality Control Plan shall be submitted to the Ministry Representative not less than five working days before starting the production of paving aggregate. The Ministry Representative will evaluate the Contractor's Quality Control Plan and will respond, in writing, within five days. Any subsequent changes to the Contractor's Quality Control Plan must also be submitted to the Ministry Representative for evaluation.

The Contractor's Quality Control Plan shall include a description and schedule of the intended sampling, testing and reporting. The Quality Control Plan must also include a detailed description of the means by which the Contractor shall use the quality control test results to ensure that the asphalt materials, aggregate, mix production, recycling and pavement compaction processes will be controlled to keep the end product within the specified limits. The Quality Control Plan must clearly show the flow of information from the quality control laboratory to the individuals who shall make the actual adjustments to the processes and equipment to this control. The plan will show time allowance for each step, the names and positions of all the

people involved, and a clear description of the responsibilities of each.

515.04.03 Quality Control Testing and Inspection – The Contractor shall provide and maintain equipment and qualified personnel to perform all laboratory testing, field testing and inspection necessary to determine and monitor the characteristics and properties of all the materials produced and incorporated into the Work. They shall also monitor the workmanship of the final product in accordance with the Quality Control Plan as most recently submitted and accepted.

The Contractor shall use a qualified registered member of the Association of Professional Engineers and Geoscientists of British Columbia or a qualified registered member of the Applied Technologists and Technicians of British Columbia. This person shall be designated as the Quality Control Manager for the purposes of these specifications. The Quality Control Manager shall be responsible for preparation and sign off of the Quality Control Plan, shall be responsible for all Quality Control testing and inspections and shall be responsible for the signing of all Quality Control testing and inspection records and submissions to the Ministry.

The Contractor shall provide a testing facility(s) that meets the requirements necessary to carry out all the test procedures listed within this Section. The facility(s) must have the equipment specified under the appropriate test designation to perform the tests.

All equipment and the laboratory shall be well maintained and in good working condition. All testing equipment shall be calibrated, and evidence of the calibration shall be provided when requested by the Ministry Representative.

515.04.04 Quality Control Records – The results from quality control testing shall be reported on test logs and plotted on charts immediately after each test is completed. These test results, reports and charts shall be available within 24 hours of the end of each shift, to the Ministry Representative and at all times during the progress of the Work.

For the purposes of confirming delivery of asphalt mix to the road and the calculation of material application rates, the Contractor shall prepare and provide to the Ministry Representative, a copy of the Road Checker's Summary and the weigh tickets for each load received at the recycling operation at the end of each shift. The Road Checker's Summary shall include, but not be limited to, the following information:

- Truck Number
- Weigh Ticket Number and Net Weight of Load
- Time and location by station of delivery
- Admix and Rejuvenator Material Application Rate Calculations

- Processed Depth, Lengths, and Widths
- Temperatures (behind screed, windrow, ambient and admix)
- Notes pertaining to the paving of the appurtenances (intersections, tapers, etc.)

515.04.05 Final Quality Control Testing Reports – Prior to issuing a Completion Certificate, the Contractor shall provide the Ministry Representative with:

- (a) A summary of all virgin aggregate quality control test results;
- (b) Copies of all quality control test results for asphalt admix properties, Hot-In-Place Recycled Pavement and compaction; and
- (c) Copies of all quality control charts.

515.04.06 Minimum Acceptable Construction Practices – Good construction practices shall be considered as standard construction procedures to be followed and shall include but be limited to the procedures described below.

- (a) **Recycling** – The existing highway pavement shall be heated, milled, reprocessed, with or without the addition of a rejuvenating agent or virgin mix as specified.
- (b) **Finished Cross Fall Drainage** – Contractor shall ensure that the appropriate cross slopes for safety and positive drainage are maintained or improved after the recycling process.
- (c) **Longitudinal Edges** – Contractor shall ensure that the longitudinal edges of the recycled pavement are blended to conform in elevation with the adjacent pavement unless this surface is scheduled to be recycled.
- (d) **Excess Materials** – Excess materials shall be removed and deposited in a location such that they can be reincorporated into the recycled mixture. At no time shall excess material be cast across the newly processed surface or incorporated into the granular shoulders.
- (e) **Compaction and Finishing** – The Contractor shall supply roller compaction behind the reprocessing operation to achieve densities and smoothness. The recycled mat shall have a uniform appearance and uniform surface. Any deficiencies shall be corrected at the Contractor's expense, to the satisfaction of the Ministry Representative.
- (f) **Stockpiles** – Stockpiles of different types of material shall be located and constructed in such a manner as to prevent intermingling of the types and segregation of material.
- (g) **Haul Vehicles** – Lubrication of the truck boxes with diesel fuel will not be permitted. All vehicles shall have adequately insulated truck boxes and shall be equipped

with an insulating tarpaulin of such size as to completely protect the asphalt mix.

- (h) **Existing Paved Surfaces** – Existing paved surfaces shall be clean to avoid contamination of the recycled pavement. The existing surfaces shall be swept back at least 20 cm wider than the area to be processed.
- (i) **Existing Utility Structures** – The Contractor shall ensure all catch basins and manholes, which are within the operating area of the heating units, are clear of any flammable liquids or otherwise hazardous fumes/gases.
- (j) **Heating of Recycled Material** – The recycled material shall not be heated over an average of 150°C in order to avoid excessive oxidation and hardening of the recycled Asphalt Cement.
- (k) **Minimum Admix Temperatures** – Based on the most recent Temperature Viscosity Curve supplied by the Asphalt Cement Supplier. This information will be used to set minimum and maximum mixing temperatures.
- (l) **Longitudinal Joints** – Longitudinal joints in the top lift of asphalt pavement will generally only be permitted where lane dividing lines are to be painted.
- (m) **Transition Joints** – At the locations where recycling operations begin and end, the Contractor shall ensure transition joints between the processed and unprocessed surface are smooth and without irregularities. If any bumps or irregularities occur resulting from the reprocessing operation, the Contractor shall repair these areas with fresh asphalt mix and/or additional reprocessing, at the Contractor's own expense, as directed by the Ministry Representative.
- (n) **PG Asphalt Cement** – Shall meet requirements set out in Special Provisions and SS 952.
- (o) **Minimum Temperature** – Minimum Temperature behind the paver screed of the recycled material shall not be less than 110°C, unless the Ministry Representative accepts that the circumstances limit the reasonably achievable temperature to a lower value.
- (p) **Rejuvenating Agent Application** – To accurately proportion the rejuvenating agent into the milled mixture the equipment shall comply with the following requirements:
 - (i) Positive feed and shut off of the rejuvenating agent, governed by the movement of the reprocessing machine;
 - (ii) Mechanically or electronically controlled application of the rejuvenating agent relative to and variable with the processing rate of the equipment;
 - (iii) A mechanical or electronic metering device capable of supplying an accurate indication of the

quantity of rejuvenating agent being applied per square metre; and

- (iv) A mechanical, electronic or manual method of measuring quantity of rejuvenating agent in the tank.
- (q) **Metering of Rejuvenating Agents** – To accurately proportion the rejuvenating agent into the hot milled material, the equipment shall control the quantity of the rejuvenating agent to $\pm 0.05 \text{ L/m}^2$ of surfaced reprocessed with an agent application range of 0.0 L/m^2 to 1.0 L/m^2 .

The measurement of the rejuvenating agent applied by means of a metering device shall be capable of recording accumulated litres to the accuracy of $\pm 2\%$ and be fully visible to both the operator and the Ministry Representative.

Heating of Rejuvenating Agent shall be kept to within $\pm 5^\circ\text{C}$ of the application temperature established by the manufacturer and/or the Ministry Representative.

- (r) **Outer Edges** – The outer edge of each lane shall generally be tapered to a 12:1 slope.
- (s) **Recycling Adjoining Mats** – When recycling an adjoining parallel mat there shall be at least a 50 mm overlap onto the previously recycled surface. This overlap shall be properly “raked” to form a homogeneous bond between the two mats.
- (t) **Raking** – Any material that is raked from the joint shall not be placed on the new mat or placed on or in front of the paver. This material may be introduced at a point of the operation where the material shall be reheated and remixed with the Hot-In-Place Recycled Pavement.
- (u) **Discontinued Paving** – At locations where reprocessing operations begin and end, the Contractor shall ensure that the transition between the processed and unprocessed surface is not smooth and without irregularities. If any irregularities occur resulting from the reprocessing operation, the Contractor shall repair these areas with fresh asphalt mix and/or additional reprocessing, at the Contractor’s own expense.
- (v) **Pavement Drainage** – The Contractor shall at all times ensure that the appropriate cross slopes for safety and positive pavement drainage are maintained.
- (w) **Rollers** – Rollers shall normally operate with the drive wheel nearest the paver and at a speed not in excess of 8 km/h. They shall not be allowed to park on the mat prior to complete cooling.

515.05 Supply of Virgin Aggregates, Asphalt Materials, Asphalt Admix Design and Job Mix Formula

515.05.01 Work in Ministry Pits or Quarries – When operating in a Ministry pit or quarry, the Contractor shall comply with all provisions of SS 145.26.

515.05.02 Supply of Virgin Aggregates, Aggregate Production and Characteristics – The Contractor shall not produce paving aggregate until the Contractor has received written notification that the Quality Control Plan is in accordance with SS 515.04.02, and has in place testing facilities for aggregate production that are in accordance with the Quality Control Plan.

For the production of virgin Asphalt Mix Aggregate, within Ministry pits, the Contractor shall provide crushing equipment that will permit all aggregate, that passes through 375 mm x 450 mm slotted openings, to be used for the production of crushed aggregate. Rocks, which will not pass through these openings, shall be stockpiled or disposed of as directed by the Ministry Representative. Crushing and screening equipment shall be provided with adequate facilities and capacity to be able to bleed off reject aggregate and remove any excess fine aggregate, dust or objectionable aggregate coatings, to make it acceptable for use. All products from crushing or screening plants, that can be used shall not be wasted, and shall be stockpiled or used as directed by the Ministry Representative.

Where the Ministry has available any test results information on the properties shown in , for a Ministry pit or other source, the Ministry Representative will upon request, provide that information to the Contractor. Otherwise, sampling and testing to determine and demonstrate the compliance of paving aggregate with the requirements of this Section shall be the responsibility of the Contractor.

Paving aggregate shall meet the requirements of Table 502-D for medium mix asphalt aggregates, be within the variations limits specified in Appendix 515-B, Table 515-J, as well as the following requirements:

- (a) **Virgin Coarse Aggregate:**
 - (i) Shall be all mineral matter retained on the sieve designated in the test procedures for each individual test.
 - (ii) Shall consist of crushed stone, crushed gravel, or combination thereof, or materials naturally occurring in a fractured condition, or materials naturally occurring of highly angular nature or rough texture.
 - (iii) Shall be free from coating of clay, silt or other deleterious material, and shall meet the requirements listed in Table 515-B and Table 515-F.

Table 515-F: Requirements for Virgin Coarse Aggregates

<u>ASTM Test Reference</u>	<u>Requirements</u>	
<u>C127</u>	<u>Maximum Water Absorption; % by Mass</u>	<u>2</u>
<u>C88</u>	<u>Soundness of Aggregate Maximum Loss After 5 Cycles</u>	<u>20</u>
<u>C142</u>	<u>Maximum % by Mass of Clay Lumps and Friable Particles</u>	<u>1.5</u>
<u>D6928</u>	<u>Minimum Degradation Factor</u>	<u>35</u>
<u>D5821</u>	<u>Two (2) Fractured Faces: Minimum % by Mass Retained on the 4.75 mm sieve</u>	<u>85</u>

(b) Virgin Fine Aggregate:

- (i) Shall be clean, tough, durable, moderately sharp, and free from coatings of clay, silt, or other deleterious material, and shall contain no clay balls or other aggregations of fine material.
- (ii) Shall have a maximum mass loss after five cycles of not more than 23% when tested in accordance with ASTM C88.
- (iii) Shall have a sand equivalent of not less than 40 when tested in accordance with ASTM D2419.
- (iv) Shall have a minimum value of 45 when tested according to AASHTO T 304, Method "A", – Uncompacted Void Content of Fine Aggregate when determining Fine Aggregate Angularity.

(c) Virgin Mineral Filler and Mineral Dust:

- (i) Mineral filler shall consist of all matter passing the 0.600 mm sieve and mineral dust shall consist of all mineral matter passing the 0.075 mm sieve.
- (ii) Mineral filler and mineral dust shall be free from organic matter.
- (iii) Mineral filler shall be non-plastic when tested in accordance with ASTM D4318.

515.05.03 Supply of Asphalt Cement and Rejuvenating Agents – The Contractor shall supply the types and grades of asphalt cement and rejuvenating agents as specified in the Special Provisions. The supply of these materials includes, but is not limited to, ordering, scheduling delivery of, supply of Temperature Viscosity Curve information, receiving, handling, storing, heating, blending, sampling, and testing of the materials and other related work.

The Contractor shall supply the Ministry Representative with copies of the supplier's weigh-bill and records of all asphalt and rejuvenating materials received on a daily basis. The amount of asphalt cement used will be calculated based on the Contractor's Quality Control testing results.

The supply of asphalt cement and rejuvenating agents shall be at the Unit Price bid for asphalt cement and rejuvenating materials.

515.05.04 Responsibility for Asphalt Admix Design – The Ministry will provide to the Contractor information related to the Asphalt Admix and rejuvenating ratios in the Special Provisions, but all admix design and formulation costs are the responsibility of the Contractor.

515.05.05 Evaluation of Asphalt Admix Materials – The Ministry Representative will require up to five (5) calendar days upon notification from the Contractor to evaluate the Contractor's Asphalt Admix materials.

515.05.06 Samples Required for Asphalt Admix Evaluation – At the discretion of the Ministry Representative, the Ministry may request samples of the Admix materials and Quality Control results and documentation for evaluation. Shipping costs for samples delivered to the Ministry Representative are the responsibility of the Contractor.

515.05.07 Field Adjustment of the Job Mix Formula and/or Recycled Pavement – During construction, the Ministry may require field adjustments to the job mix formula and/or recycled pavement. A field adjustment to the Job Mix Formula is defined as a change in the asphalt cement content of admix, aggregate gradation, rejuvenating agent and/or proportioning of various aggregate sizes.

PAYMENT ADJUSTMENTS**515.11 Density**

515.11.01 Coring – The Contractor shall be responsible for providing all core samples for quality assurance and payment adjustment purposes. The location of the cores shall be randomly selected and provided to the Contractor by the Ministry Representative. The cores shall be taken within a 300 mm radius of that location unless otherwise agreed to by the Ministry Representative. The Contractor shall provide 150 mm diameter cores for these purposes. The minimum core thickness shall be 37 mm. The Contractor shall prepare the cores prior to submission by removing all material not representative of the pavement lift to be tested. The Contractor shall deliver these cores to the Ministry Representative on-site, within 24 hours of being provided the locations for the coring.

In the case of a core sample location falling in an obvious non representative area, such as a previously patched area, at the sole discretion of the Ministry Representative, a new random location will be chosen for that sample.

All costs associated with obtaining the cores, including the filling and compaction of the core holes are considered incidental to the contract and are the responsibility of the Contractor.

515.11.02 Percent Density – One random core sample will be obtained from each Sub-Lot and tested. The test results for three Sub-Lots will be averaged to determine the percent density for the Lot.

$$\text{\% Density} = \frac{\text{Hot-In-Place In Place Density of Sample}}{\text{Maximum Theoretical Density}} \times 100$$

(ASTM D 2041)

The Maximum Theoretical Density for the Lot will be determined by combining the Sub-Lot cores. The cores shall be prepared in accordance with the procedures outlined in Appendix 515-3 to enable the testing to determine the maximum theoretical density.

515.11.03 Payment Adjustments – The payment adjustment for percent density will be the amount shown in Table 515-G for the Sample Mean of the test results for the Lot.

515.11.04 Rejection Limit – The rejection limit for percent density is the limiting value of the Sample Mean as shown in Table 515-G.

If the test result for the density of a Sub-Lot is outside the acceptance limits, the Sub-Lot is rejected automatically regardless of the values of other acceptance parameters. To minimize the cost of rejection to the Contractor, the Contractor shall isolate the area of low density within the Sub-Lot and perform the necessary corrective measures to ensure specifications are met. The limits of the low density area must be verified and approved by the Ministry Representative before remedial work proceeds.

515.11.05 Payment for Rejected Work Made Acceptable – The payment adjustment for density will be based on testing of the reprocessed, replaced or overlaid material where applicable. Where replacement or overlay material does not cover the entire Lot or Sub-Lot, prior tests of the uncovered area will be averaged with new tests on the corrective work.

Table 515-G: Payment Adjustments for Density

% Maximum Theoretical Density (Lot Average)	Payment Adjustment (\$ per m ²) per Lot
≥ 95.6	+ \$0.50
95.0 – 95.5	+ \$0.30
94.0 – 94.9	+ \$0.20
93.0 – 93.9	\$0.00
92.6 – 92.9	- \$0.20
92.0 – 92.5	- \$0.50
≤ 91.9	REJECT

515.12 Smoothness

515.12.01 Determination of Pavement Smoothness – The finished pavement surface shall be tested by the Ministry using either a Class I precision rolling profile measuring

instrument or a non-contact Inertial Profiling measuring instrument, to determine the longitudinal profile and compute the International Roughness Index (IRI) in each driving lane. Profiles shall be measured and the IRI calculated in the centre of the lane for each Sub-Lot. IRI values will be recorded to a precision of 0.01 m/km for each Sub-Lot. The Lot IRI value is the average of the IRI values calculated for the Sub-Lots within the Lot.

For any Sub-Lot between 50 m and 100 m in length, the IRI value shall be considered representative of a complete Sub-Lot. For any Sub-Lot less than 50 m in length, the IRI value will be combined with the proceeding Sub-Lot IRI value.

The profile shall be measured over the entire length of the pavement exclusive of structures and shoulder areas. Acceleration, deceleration and turning lanes are considered part of the driving lanes and shall be testing in accordance with this provision. For the measuring process, the Contractor shall provide the Ministry Representative a chalk guide line in the centre of the lane immediately prior to measurement with a Class I precision rolling profile measuring instrument. Payment for providing a chalk line for a Class I precision rolling profiler will be paid upon acceptance by the Ministry Representative under the Provisional Sum Item.

515.12.02 Auxiliary Lanes – For smoothness testing, sections of the driving lanes that do not fall within the continuous through lanes, such as acceleration lanes, deceleration lanes and turning lanes, and lanes which are less than 1 km in length, shall be treated as follows. The ratio of the section length to the standard Lot length of 1 km shall be determined and the payment adjustment shall be pro-rated on this basis as in the following example:

$$\text{Length of segment} = 0.565 \times \frac{\text{Standard Lot Length of 1000 m}}{\text{Length of segment}}$$

i.e. 565 m

Hence the applicable payment adjustment is 0.565 times the payment adjustment for a 1 km Lot as determined from Table 515-H.

Table 515-H: Lot Assessment and Payment Adjustments for Smoothness

Lot IRI (m/km)	Payment Adjustment
≤1.00	+\$2,500
>1.00 and ≤1.10	+\$1,200
>1.10 and ≤1.20	+\$700
>1.20 and ≤1.30	+\$400
>1.30 and ≤1.40	0
>1.40 and ≤1.60	-\$500
>1.60 and ≤1.80	-\$900
>1.80 and ≤1.90	-\$2,000
>1.90 and ≤2.00	-\$2,500
>2.00	REJECT

515.12.03 Acceptance Limits – The acceptance limit for smoothness is the limiting value as shown in Table 515-H, beyond which corrective work is required. Payment adjustments are shown in Table 515-H. The International Roughness Index (IRI) value, calculated for each Sub-Lot, will be used to determine if the Lot will be accepted, and if so whether it will be subject to any payment adjustments.

515.12.04 Payment Adjustments for Full or Increased Payment – Acceptance of any Lot at full or increased payment will occur if it contains no obvious defects as per SS_515.22 and in the case of top lift pavement only, the pavement smoothness, as reported by the International Roughness Index (IRI) meets the following requirements:

- All Sub-Lots shall have an IRI value ≤ 2.0

515.12.05 Rejection Limit – If a Sub-Lot has an IRI >2.0 , the Sub-Lot is rejected.

515.12.06 Remedial Work – If the test results on a Sub-Lot of pavement indicate a payment reduction or rejection because of smoothness, the Contractor may propose remedial work to improve the smoothness. Such proposals are subject to the approval of the Ministry Representative, but such approval does not imply that the proposed remedy will be successful and does not reduce the Contractor's responsibility for meeting the acceptance requirements. Reprocessing may be acceptable, but cold milling and repaving may be required. Only one attempt to remediate the reject area (remediation method chosen by the Contractor) may be made to improve smoothness, and this must be completed within ten (10) calendar days from the time the Contractor receives notification from the Ministry Representative of the original smoothness test results for the Sub-Lot. Should a product still be in reject the Ministry Representative will determine the final remediation method at the Contractor's expense.

No payment will be made for any material, equipment or manpower used to improve, or attempt to improve, smoothness.

515.12.07 Smoothness Deficiency Report – Smoothness deficiencies (bumps and dips) less than 8 mm over 3 m will not have a fix or remedial work requirement. Individual smoothness deficiencies between 8 mm and 12 mm over 3 m will result in a \$400.00 penalty per deficiency and smoothness deficiencies over 12 mm over 3 m will require remedial work (see SS 515.22).

ACCEPTANCE AND REJECTION

515.21 Acceptance at Reduced or Adjusted Payment – Acceptance of any Lot at reduced payment will occur if it contains no obvious defects as per SS 515.22, and if:

- The test results for acceptance parameters are such that the Lot or Sub-Lot meets the requirements for acceptance at a reduced payment;
- The Lot and Sub-Lot is approved in respect of all other requirements; and
- The Contractor has not notified the Ministry Representative in writing that it will exercise its option to either repair or remove and replace the work, at its own cost, with work meeting the requirements for acceptance at full or increased payment.

515.22 Rejection for Workmanship Defects – The finished surface of any lift shall have a uniform “closed” texture and be free of visible signs of poor workmanship. Any obvious defects as determined by the Ministry Representative such as, but not limited to the following, will cause for automatic rejection of asphalt pavement regardless of the values of any other acceptance parameter:

- Individual bumps and dips that exceed 12 mm over 3 m;
- Areas of excess or insufficient asphalt;
- Improper matching of longitudinal and transverse joints;
- Roller marks;
- Tire marks;
- Segregation; or
- Cracking or tearing

When asphalt pavement is rejected by reason of obvious defects, the minimum area of rejection will be the actual length of the defect for the full width of the driving lane in which the defect exists.

Rejected work shall be promptly repaired, remedied, or removed and replaced in a manner acceptable to the Ministry Representative. The Contractor shall be responsible for all costs including materials.

No payment will be made for work in any Lot or Sub-Lot, which has been rejected, until the defects have been remedied.

515.23 Appeal Testing

515.23.01 Density – The Contractor may appeal the results of acceptance testing for density for any Lot only once. Appeals will only be considered if quality control results can be presented to support the appeal.

Quality control test results for density which are provided to the Ministry Representative subsequent to the Contractor's receipt of the quality assurance test results for that Lot will not be considered (when evaluating evidence) for an appeal. The appeal shall be for all tests within the Lots, and there will be no appeal allowed for single test within the Lot.

The following procedures will apply for the appeal:

- (a) The Ministry Representative will arrange for an independent testing laboratory to perform the appeal testing. The personnel employed or testing laboratory retained by the Contractor for quality control testing on the project will not be used for appeal testing.
- (b) The original quality assurance results will not be considered in the appeal. The Contractor will sample the pavement at locations randomly re-selected by the Ministry within five (5) days following receipt of the appeal such that a total of five (5) new core specimens are extracted throughout the Lot. The Ministry Representative will observe the sampling process. The procedures used to determine the test results shall be consistent with the original acceptance testing methods.

515.23.02 Smoothness – The Contractor may appeal smoothness acceptance test results of any rejected or penalized Lot, once. The appeal shall be in writing and submitted within the next five (5) days following receipt of the test results.

Any attempt to improve smoothness on the appealed Sub-Lot after the Ministry Representative has tested the Lot for acceptance shall void the appeal and the original test results will apply.

The Ministry Representative will perform, and the Contractor will be given the opportunity to witness, the appeal testing and the new results will be binding on the Contractor and the Ministry.

515.23.03 Application of Appeal Testing Results – For density appeals, the original test will be discarded. A new Sample Mean for the five (5) new test results will be determined and used for acceptance and payment adjustment.

The new values, thus determined, in all cases, will be binding on the Contractor and the Ministry.

515.23.04 Payment for Appeal Testing – If the new results indicate a change in the payment adjustment in the Contractor's favour, then sampling and testing costs incurred during appeal procedures for that Lot will be borne by the Ministry.

If the new results verify that any payment reduction or rejection remains valid for that Lot, then the costs of sampling and testing (plus 10% mark-up) incurred during the appeal procedure will be charged to the Contractor.

515.23.05 Time Limits for Appeals of Test Results – All appeals shall be in writing and submitted within the next three (3) site occupancy days of receipt of the test results.

515.24 Cold Milling and Re-Paving as a Corrective Measure – If cold milling and re-paving is used as a corrective measure on a defective Lot or Sub-Lot, the thickness will be subject to the approval of the Ministry Representative, but shall not be less than 40 mm. In all other

respects, the re-paving will be subject to the same specifications as the pavement being replaced.

Whether the cold milling and re-paving is applied as a corrective measure, acceptability and payment will be determined as follows:

- (a) Acceptability, and eligibility for either positive or negative payment adjustment, will be determined entirely on the results of testing and observations conducted on the re-paving, regardless of test results that have been obtained on the Hot-In-Place recycled pavement;
- (b) The payment quantity, for application of the Unit Prices for asphalt pavement, and the quantity, to which any payment adjustment is to be applied, will be derived from the square metres of recycled pavement affected by the re-paving.

PAYMENT

515.31 General – Payment for Hot-In-Place Recycle of asphalt pavement will be at the Contract Unit Price per Square Metre. The Contract Unit Price shall be for all things furnished and done including sweeping, processing, compacting, patching and any hand work as necessary to complete the work. Payment at Unit Prices for supply of virgin aggregates, the supply of asphalt cement, the supply of rejuvenating agents, the supply of virgin asphalt mix and the Hot-In-Place recycling construction of asphalt pavement, shall be full compensation for completing the supply and installation of Hot-In-Place asphalt pavement on prepared surfaces in accordance with the contract requirements. Applicable payment adjustments (additions or subtractions as applicable) shall be applied in accordance with the PAYMENT ADJUSTMENT Section.

The first 4,000 m² on Hot-In-Place asphalt pavement recycling production will not be subject to bonus/penalty payment adjustments but will be required to meet minimum specification requirements for quality and workmanship.

515.31.01 Rejuvenating Agent and Asphalt Cement

- (a) **Rejuvenating Agent** – Payment to Supply Rejuvenating Agent shall be at the Unit Price per litre for the quantity of material actually metered into the recycling process. The Contract Unit Price shall be to supply, store, heat, apply and proportioning of the agent with the asphalt pavement mixture reclaimed from the roadway.
- (b) **Asphalt Cement** – Payment for Supply of Asphalt Cement for admix shall be at the Unit Price per tonne for the payment quantity determined from

$$\text{Payment Quantity of Asphalt Cement} = \frac{\text{Av. Actual Asphalt Content X Payment Quantity of Asphalt Admix}}{\text{Average Actual Asphalt Content} + 100}$$

Where:

Payment Quantity of Asphalt Admix = quantity determined in accordance with SS 515.31.03,

Actual Asphalt Content = defined in SS 515.03.04,

The actual asphalt content shall be determined on a Lot by Lot basis by averaging the value obtained from the Quality Control tests from each Lot. This average will be used to determine the tonnage of Asphalt Cement used for payment purposes for the Asphalt Cement for the Lot.

The quantity of asphalt cement so determined shall be used for payment purposes regardless of any discrepancy that may be noted between it and any quantity calculated using delivery quantities, tank dips, or other data. No payment will be made for asphalt cement used in reject mix.

Such payment shall be full compensation for supplying the material to the project, handling, storing, heating, sampling, and testing of the material, and other related work.

515.31.02 Paving Aggregate into Stockpile (EPS) – Payment for Paving Aggregate into stockpile (EPS) shall be at the Unit Price per tonne for the quantity of paving aggregate placed into stockpile in accordance with the following:

- (a) The paving aggregate quantity will be identical to 100% of the asphalt admixture quantity.
- (b) As the aggregate is crushed into stockpile, progress payments will be made against the bid item, up to the quantity shown in the “Approximate Quantity” column of the Schedule 7.
- (c) At contract completion, the final payment quantity for asphalt admix aggregate will be in accordance with the following calculation:

$$\text{Tonnes of Asphalt Admix Aggregate} = \frac{\text{tonnes of mix X 100}}{100 + \text{actual calculated asphalt content as per SS 515.03.04}}$$

Such payment shall be full compensation for all work include, but not limited to, the production, supply and stockpiling of all paving aggregates, and shall include all costs of quality control.

515.31.03 Asphalt Admix (EPS) – Payment for Asphalt Admix (EPS) constructed in place shall be at the Unit Price per tonne for the quantity of admix placed in accordance with the Contract requirements.

Subject to the exception noted below, only acceptable asphalt admix will be included in the payment quantity.

Where cold milling and re-paving is used as a corrective measure, in accordance with SS 514.24;

- (a) The overlay quantity will not be included in the admix payment quantity; but

- (b) The admix used in quantity of recycled pavement removed by the cold milling and re-paving will be included in the admix payment quantity, whether or not it was acceptable.

Such payment shall be compensation in full for all work including but not limited to: loading the aggregate into the feeders, drying the aggregate, metering and adding the asphalt cement, mixing, loading, weighing, hauling, dumping, spreading, compacting and finishing the asphalt pavement, and shall also include all costs of quality control.

515.31.04 Hot-In-Place Recycled Asphalt Pavement (EPS) – Payment for Hot-In-Place Recycled Asphalt Pavement (EPS) constructed in place shall be at the Unit Price per square meter for the quantity of recycled pavement in accordance with the Contract requirements.

Subject to the exception noted below, only acceptable recycled asphalt pavement will be included in the payment quantity.

Where cold milling and re-paving is used as a corrective measure, in accordance with SS 515.24;

- (a) The overlay quantity will not be included in the admix payment quantity; but
- (b) The admix used in quantity of recycled pavement removed by the cold milling and re-paving will be included in the admix payment quantity, whether or not it was acceptable.

Such payment shall be compensation in full for all work including, but not limited to: recycling, metering, adding the rejuvenating agent, adding, spreading, compacting and finishing the asphalt pavement and shall also include all costs of quality control.

515.31.05 Payment for Acceptable Work – The following end product properties of recycled asphalt pavement will be measured for acceptance:

- Density
- Smoothness

515.31.06 Payment for Rejected Work Made Acceptable – When defects have been remedied in Lots or Sub-Lots which had been rejected, payment for the original quantity of material in those Lots or Sub-Lots will be made subject to payment adjustments and penalty assessments and subject to SS 515.24.

No payment will be made for any material used to replace, repair or overlay rejected work and all corrective work shall be performed entirely at the Contractor’s expense.

515.31.07 Payment Adjustment – Payment adjustments resulting from the application of this Section will be affected on each progress payment as follows:

- (a) For each Lot paid for by the square metre (m²), the applicable payment adjustment derived from

Table 515-G (Density), in dollars per square metre will be expressed as positive in the case of increases and negative in the case of decreases. The algebraic sum of these unit adjustments will then be applied to the payment quantity for the Lot. The resulting amount, in dollars, will be the net payment adjustment, positive or negative, for that Lot.

The algebraic sum of the net payment adjustments for all such Lots for which payment is authorized on the current progress payment, computed in dollars, shall be the total payment adjustment for density for the current process payment.

- (b) For each Lot, the applicable payment adjustment derived from Table 515-H (Smoothness), in dollars per Lot, will be expressed as positive in the case of increases and negative in the case of decreases, and will be the payment adjustment, positive or negative, for that Lot.

The algebraic sum of the payment adjustment for all such Lots for which payment is authorized on the current progress payment, computed in dollars, shall be the total payment adjustment for smoothness for the current progress payments.

- (c) The algebraic sum of the total payment of the total payment adjustments for density and smoothness, derived in accordance with a) and b) above, shall be the total payment adjustment, positive or negative, in dollars, for all attributes for the current progress payment. This amount shall be added, if positive, or subtracted, if negative, by a single entry in computing the current progress payment.
- (d) The process set out in a) to c) above will be used in computing each progress payment to which it is applicable.

515.32 Ministry Purchase of Surplus Aggregate in Stockpile

515.32.01 Surplus in Private Pits – At the discretion of the Ministry Representative, the Ministry may or may not purchase surplus aggregate in stockpile in a private pit.

Generally, the Ministry will provide payment for the processing costs of surplus aggregate stockpile in Ministry pits only, and only to a limited quantity.

However, should the Contractor produce surplus aggregate in a private pit, and the Ministry intends to purchase these surplus aggregates, the Contractor shall be required to provide a written agreement with the owner of the property. This document shall indicate that the Ministry will have free access to and use of the surplus aggregate in stockpile for a period of 12 months after the completion of the contract work.

If the Contractor undertakes private work from within the private pit, measurements for surplus aggregate in stockpile will not be taken until the completion of the private work,

ensuring that the Ministry does not pay for aggregate used on private works.

All surplus aggregate shall be properly stockpiled.

515.32.02 Ministry Purchase of Surplus Aggregate – Should the Ministry proceed with the purchase of surplus aggregate, upon completion of the contract, the Ministry will purchase surplus paving mix aggregate as indicated herein.

If the quantity of Asphalt pavement actually incorporated into the works is less than the estimated quantity, as stated in the Schedule 7, the Ministry will purchase aggregate up to 100% of the quantity required by the contract, to be paid under the appropriate Unit Price Item for Aggregate in Stockpile.

The surplus aggregate must, when singly or combined, meet the gradation requirements set out in the Job Mix Formula.

The portion of material remaining in a stockpile eligible for compensation as surplus aggregate shall be the lesser of:

- (a) The quantity of mix as stated in the Schedule 7 minus the quantity of aggregate actually incorporated in the works; and
- (b) The volume of the remaining stockpile as determined under SS 502.41.03.

No payment shall be made for any surplus shoulder and paving aggregate remaining at contract completion in excess of the contract requirement. Material remaining in a Ministry pit is the property of the Ministry.

515.32.03 Measurement of Surplus Aggregate – The Ministry will determine the volume of aggregate by surveying the stockpile(s) using string-line techniques, and determine volume using prismatic volumes between surfaces. Alternatively, the Ministry Representative may elect to survey using cross section techniques and/or determine volumes using end-area volumes.

The base of the pile will be determined from a pre-stockpile survey or, where such a survey is not available, from a surface determined by the Ministry Representative as being a reasonable interpolation from the intersection of the pile sides with the adjacent ground level. Where there are two or more stockpiles of aggregate meeting the same gradation classification, the compensation will be based on the cumulative quantity of those stockpiles.

Where necessary, stockpile volume shall be converted to mass using the conversion factor of 2.0 tonnes per cubic metre.

No payment shall be made for any surplus aggregate remaining at contract completion in excess of the contract requirement. Material remaining in the pit is the property of the Ministry.

Surplus shouldering, surfacing, crushed base and subbase aggregates will be paid in accordance with SS 202.

APPENDIX 515-A
QUALITY CONTROL REQUIREMENTS AND GUIDELINES

1.01 GENERAL

The Contractor's responsibility for preparing submitting and adhering to a Quality Control Plan are specified in SS 514.04. This Appendix provides requirements and guidelines for the Contractor's Quality Control Plan, in addition to those set out in SS 515.04.

1.02 QUALITY CONTROL PLAN

As a requirement, the Contractor shall prepare and submit a Quality Control Plan for evaluation. The Plan may be operated wholly, or in part by a qualified Subcontractor or an independent organization/agency. However, the Quality Control Plan, including compliance with the Plan and its modifications, is the responsibility of the Contractor.

The Plan shall address all elements that affect the quality of the Hot-In-Place recycled asphalt pavement including, but not limited to, the following:

- (a) Supply of Asphalt Materials;
- (b) Supply of rejuvenating agents;
- (c) Identification of source and proof of quality of virgin aggregates to be supplied;
- (d) Virgin aggregate production and its gradation control;
- (e) Quality of virgin aggregate components;
- (f) Stockpile management;
- (g) Asphalt plant calibration and proportioning of aggregate components;
- (h) Mixing including asphalt cement content control;
- (i) Process temperature controls;
- (j) Admix Material and Rejuvenating Agent application rates and processed dimensions, placing and finishing;
- (k) Joints;
- (l) Density;
- (m) Smoothness; and
- (n) Segregation

The Plan shall also include the following:

- The name of the Quality Control testing agency and its proven capability to provide the specific services required for the project.
- The list of dedicated technical staff, if available, (including names, qualifications and relevant experience) and their proposed roles.

- The list of testing equipment available for project work.

The Quality Control Plan shall include the designation of specific personnel responsible for specific quality control duties.

- There shall be a designated Quality Control Manager, as per SS 515.04.03. The Quality Control Manager shall be qualified as per SS 515.04.03. The Quality Control Manager shall be responsible for the preparation and sign off of the Quality Control Plan, responsible for all Quality Control testing, and inspections, responsible for the sign off of all quality control testing and inspection records and responsible for all quality control submissions to the Ministry. The individual shall be identified and named in the Quality Control Plan submission. If the Contractor requests a change in the Quality Control Manager, then a formal request to the Ministry Representative shall be made and the Ministry will review the request for approval.
- There may be a designated Process Control Technician (PCT) who would be responsible to ensure the laboratory test results and other quality control practices are used to control the quality of aggregates and other mix components and to adjust and control mix proportions to meet the mix design(s). The PCT is responsible for ensuring that testing equipment, utilized for proportioning and mixing are calibrated and in good working order. The Plan may describe how the PCT's duties, including sampling methods and responsibilities are to be accomplished and documented. The Plan should also describe the criteria to be used by the PCT to correct or reject unsatisfactory materials.
- There may also be a Pavement Quality Control Technician (PQT) who would be responsible to ensure that delivered materials meet the requirements of the specifications. In addition, the PQT shall be responsible for periodically inspecting all equipment used in recycling, placing, finishing, and compacting to assure its proper operating condition and to assure that recycling, placing, application rate, finishing, joint construction, and compaction is in conformance with this specification and the contract requirements.

1.03 CONTRACTOR'S RECORD OF QUALITY CONTROL TESTING

Test results shall be made on specified forms or charts immediately after completion of each test. These test results are to be made available to the Ministry Representative upon request.

Records of gradation control, both during aggregate production and also during the asphalt mixing operation, should be kept on the form H0295.

1.04 MATERIALS APPLICATION RATE

The Contractor shall control the Material Application Rate by monitoring the amount of asphalt admix delivered to the road against the area covered by checking the application rate every five loads.

The Contractor shall monitor the addition of rejuvenating agents by calculating the application rate, through comparing the metered amount with the total area processes every one-half hour.

The Contractor is to advise the Ministry Representative on an ongoing basis of the application rate.

1.05 DENSITY

The Contractor shall take core samples to determine actual pavement density. At the start of paving, the Contractor may take a minimum of two pavement cores from each Sub-Lot. The Contractor may employ a nuclear moisture-density gauge to ensure intermediate density control.

1.06 OTHER QUALITY CONTROL PROCEDURES

The Contractor may initiate other Quality Control procedures as necessary for ensuring production of a quality product and include them in the Quality Control Plan. Procedures may also be introduced after the start of work as necessary as amendments to the Quality Control Plan.

1.07 QUALITY CONTROL TESTING FREQUENCY

Test frequency guidelines for Quality Control are described in Appendix 515-A, Table 515-A-I.

Table 515-A-I: Test Frequency Guidelines

Activity	Test	Minimum Frequency
Test During Crushing (Virgin Aggregate)	ASTM C136: Dry Sieve Analysis of Aggregate	<ul style="list-style-type: none"> Split Stockpiles – One (1) for each stockpile for every Two (2) hours of production. One main stockpile – Every 300 tonnes. Blend Sand – One (1) for every 100 tonnes during stockpiling. Natural filler – One (1) for every 50 tonnes during stockpiling.
	ASTM D5821: Determining the Percentage of Fractured Particles	<ul style="list-style-type: none"> Every second coarse aggregate sieve test.
	ASTM C117: Sieve Analysis of Aggregates by Washing (Field Lab)	<ul style="list-style-type: none"> One (1) per day on reduced sample obtained from combined samples from the crusher.
Tests During Asphalt Plant Mixing (Admix)	ASTM C136: Dry Sieve Analysis of Aggregate	<ul style="list-style-type: none"> One of combined aggregate every 100 tonnes.
	ASTM C566 & ASTM D2216: Moisture Content	<ul style="list-style-type: none"> Aggregate – Two (2) tests per Lot. Asphalt mix – One (1) on first Sub-Lot and every second day.
	ASTM C117: Sieve Analysis of Aggregates by Washing (Field Lab)	<ul style="list-style-type: none"> One (1) per shift on reduced sample obtained from combined samples from the plant cold feed.
	ASTM D6307: Asphalt Extraction Test – Ignition Method	<ul style="list-style-type: none"> Two (2) per Lot.
	ASTM D5: Penetration of Bituminous Materials	<ul style="list-style-type: none"> One (1) per Manufacturer’s Batch.
Rejuvenating Agents Tests	Rejuvenating Agent	<ul style="list-style-type: none"> Contractor’s Option.
Test During Asphalt Paving for Density Testing (Hot-In-Place Recycled Pavement)	ASTM D1559 or AASHTO T245: Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus	<ul style="list-style-type: none"> Contractor’s Option.
	ASTM D2726: Bulk Specific Gravity and Density of Non-Absorptive Compacted Bituminous Mixtures (Briquette or Cores)	<ul style="list-style-type: none"> One (1) per Sub-Lot.
	ASTM D2041: Theoretical Maximum Specific Gravity of Bituminous Paving Mixtures	<ul style="list-style-type: none"> One (1) – see Appendix 515-C for Combining Cores.
	Core Samples	<ul style="list-style-type: none"> One (1) core for each Sub-Lot. All cores to be 150 mm diameter.

APPENDIX 515-B

REQUIREMENT FOR PRODUCTION OF ADMIX AGGREGATES

2.01 GRADATION MAXIMUM PERMISSIBLE VARIATION LIMIT

The variation limits specified in Table 515-J, when applied to the Ministry specified design Admix gradation, shall not result in a gradation that is outside of the limits as specified by the Ministry.

Table 515-J: Maximum Permissible Variation Limits

Sieve Size (mm)	Maximum Variation
19.0 or 16.0	0%
12.5	± 3.5%
9.5	± 3.5%
4.75	± 3.0%
2.36	± 3.0%
1.18	± 2.0%
0.600	± 2.0%
0.300	± 1.5%
0.150	± 1.0%
0.075	± 0.75%

APPENDIX 515-C**METHOD TO COMBINE ROAD CORES INTO A SINGLE COMBINED SAMPLE
FOR THE DETERMINATION OF MAXIMUM THEORETICAL DENSITY****3.01 INTRODUCTION**

After the individual Density for each core has been determined, the Sub-Lot road cores shall be combined into one single sample for the determination of maximum theoretical density.

3.02 SAMPLE PREPARATION

The upper recycled portion of the cores shall be separated from other pavement layers by sawing or other effective methods.

The minimum combined sample size required shall be 2,000 grams and 2,500 grams for 19.0 mm and 25.0 mm maximum aggregate size, respectively.

Select a core trimmer to ensure that the minimum sample size is obtained depending on the thickness of the samples; 100 mm, 114 mm, or 127 mm core trimmers may be used.

Place the core samples in a pan and heat to $130^{\circ}\text{C} \pm 5^{\circ}\text{C}$ for about 20 minutes to allow the specimens to be easily trimmed, do not overheat. The cores can be stacked on top of each other and trimmed together or individually.

Select a core trimmer to attain the minimum combined sample size. Heat the base of the trimmer by placing on a burner for a few minutes.

Centre the heated trimmer on the heated cores. Allow equal clearance on all sides of the core.

Hold the trimmer vertically by its handle, then press it down through the core until it reaches the bottom of the pan. Apply a slight twist if the heated core(s) are difficult to penetrate.

Remove the outside cut rock mix by means of a spatula spoon and discard.

Thoroughly mix the resultant combined sample prior to testing to ensure uniform aggregate coating.

The complete combined sample must be tested. If the resultant sample size exceeds the available capacity of the container used for vacuum saturation, it may be tested a portion at a time.

SECTION 521

ASPHALT STABILIZED BASE COURSE MIXED IN PLACE

DESCRIPTION

521.01 Scope – This Section describes the materials, plant, equipment and work practice required for the in-place stabilization of granular base course material mixed with selected bituminous binder.

EQUIPMENT

521.21 General Condition of Plant and Equipment

521.21.01 The Contractor shall, prior to commencing construction, submit a list of equipment available and ready for use on the Contract, and shall further certify that the equipment listed meets the requirement of this Section in every respect.

521.21.02 Before any work is permitted to commence, the Contractor's equipment and plant will be carefully inspected; should any of it fail to meet the required standards, work will not be permitted to commence until such deficiency is corrected.

521.21.03 All equipment and plant shall be in good mechanical condition and adjustment and be capable of efficiently performing the work required of it.

521.21.04 Once the Contractor's equipment list has been accepted, provision and maintenance of all equipment listed shall become an essential part of the contract and no equipment or plant may be released from the Contract work for use elsewhere until completion, except by the express written permission of the Ministry Representative.

521.21.05 The Ministry Representative shall have access at all times to any plant, equipment or machinery to be used on this contract in order to check calibration, control or operating adjustments. Where adjustment appears necessary, a competent technician or instrument repair laboratory shall do it.

521.21.06 Plant, machinery and equipment specified hereafter shall represent a minimum requirement for the type of machine described and does not represent a comprehensive list of equipment to be supplied by the Contractor, who shall supply whatever plant and equipment may be necessary to the proper and efficient completion of the work.

521.21.07 All plant and equipment shall be under the control of experienced and skilled operators.

521.22 Pressure Distributor – The pressure distributor used for applying bituminous materials shall consist of a fully insulated tank with a minimum capacity of 5000 L permanently and rigidly mounted on a truck or trailer

provided with pneumatic tires and propelled by a power unit capable of maintaining a constant and uniform speed.

521.22.01 Minimum Equipment – In order to be acceptable, the distributor shall be provided with the following minimum equipment:

- (a) Quick opening gate in the dome.
- (b) A measuring dipstick calibrated in 100 L increments or in 20 mm gradations and provided with a calibration chart indicating the number of litres for each 20 mm in depth of contained bituminous material for accurate checking of the distributor contents. In addition, an external and readily visible depth gauge or levelometer indicating the asphalt content of the tank to the nearest 100 L and having an accuracy of ± 50 L.
- (c) An efficient and positive means of heating the bituminous contents uniformly to any temperature up to 175°C and maintaining the contents constantly at any selected temperature without any local overheating.
- (d) An approved industrial type thermometer so situated that its face shall always be readily visible to the spray bar operator. The sensing element of the thermometer shall be placed either directly in contact with the hot bituminous material or enclosed in a thin walled well of approved manufacture, so placed as to accurately measure the temperature of the material in the tank. The thermometer shall have a minimum range of 10°C to 175°C subdivided into 1°C gradations and be accurate to $\pm 0.5^\circ\text{C}$.
- (e) Rear mounted spray bars and nozzles set parallel to the surface to be sprayed and capable of vertical and lateral adjustment. The spray bars shall be capable of adjustment to provide a minimum spraying width of 1.8 m and a maximum spraying width of 7.5 m.

The feed to the spray bars must be so arranged that there shall be a constant circulation of hot bituminous material to the bars such as will ensure a uniform viscosity and constant pressure of the bituminous material at each nozzle, both before and during spraying operations. Feed manifolds to spray bars shall be provided with strainers to prevent clogging of bars and nozzles.
- (f) Rotary adjustable, spray bar nozzles which shall be so designed and set as to ensure a uniformly fan shaped spray without atomization, overlapping on the surface to be sprayed, such that there shall be produced a uniformly sprayed surface to clearly defined edges of the sprayed lane without any reduction of the rate of application adjacent to the edges of the lane. Spray

nozzles shall be provided with valves capable of instant full opening or positive cut-off.

- (g) A connection to a circulating pressure manifold to which a hose may be attached for a single nozzle, hand operated outlet.
- (h) A pressure gauge accurate to 15 kPa and calibrated pressure pump, equipped with a tachometer registering litres per minute and driven by an independent mechanically governed power unit, so designed as to be capable of applying through the spray nozzles fitted, and accurately measuring quantities of bituminous material at a minimum rate of 900 L per minute.
- (i) Both pressure gauge and tachometer shall be mounted to be readily visible to the operator.
- (j) A fifth wheel tachometer, registering metres per minute and so calibrated and mounted as to be readily visible to the driver of the distributor, to enable the operator to maintain the constant speed required for application of bituminous material to the road at the specified rate.

521.23 Blade Graders – Self-propelled blade graders mounted on pneumatic tires shall be of approved modern type, either four wheel drive or tandem type having sufficient capacity to efficiently perform all work that may be required on the contract. Unless otherwise approved by the Ministry Representative, all graders shall meet the criteria listed in Table 521-A.

Table 521-A: Criteria for Blade Graders

Minimum Weight	7.25 t
Minimum Blade Length	3.6 m
Minimum <u>Wheelbase</u>	5.5 m
Minimum Cubic Displacement of Pistons: 4 Cycle Motors	7.0 L
Minimum Cubic Displacement of Pistons: 2 Cycle Motors	4.0 L

521.24 Pulverizing and Rotary Mixing Machines – Pulverizing and rotary mixing machines to be used on the work shall be standard machines, factory produced by a reputable manufacturer, so designed and constructed as to satisfactorily break up all base aggregations into constituent particles and capable of thoroughly and uniformly mixing the granular base material and applied asphaltic binder so as to produce a uniformly coated mix free from lumps, balls or segregation. The Contractor may use any machine of the Contractor's choice, subject to the approval of the Ministry Representative.

521.25 Compaction Equipment – Compaction equipment shall consist of approved steel wheel rollers, pneumatic tired rollers, vibrating rollers or tampers.

521.25.01 Self-Propelled Steel Rollers – Self-propelled steel rollers may be three-wheel rollers or tandem rollers. These rollers shall be equipped with power units of not less than four cylinders, unless otherwise approved by the Ministry Representative, and under working conditions shall be capable of developing a compression in the rear wheels of not less than 4.0 kg/mm² over a minimum roller width of 500 mm. At least one roller on each job shall be capable of developing a roll pressure of 4.5 kg/mm² of width. Rollers shall be in good working condition and free from backlash, faulty steering mechanism or worn parts. Rollers shall be equipped with adjustable scrapers to keep the rolls clean and with an efficient means of keeping the wheels wet to prevent mixes from sticking to the rolls. Rolls shall be free of flat areas, dents, openings or projections, which will mar the surface of the pavement.

521.25.02 Pneumatic Tire Rollers – Pneumatic tire rollers shall be either self-propelled or towed type, single or double axle, having a minimum effective rolling width of 1.2 m. The rollers shall be equipped with smooth tread pneumatic tires of equal size and diameter. The wheels of the roller shall be so spaced that two passes of a single axle roller or one pass of a two-axle roller will accomplish one complete coverage equal to the rolling width of the machine. There shall be a minimum 6 mm overlap of the tracking wheels of a double axle roller. The wheels of towed rollers to be used on granular base preparation may wobble. The roller shall be so constructed that the contact pressure shall be uniform for all wheels and the tire pressure of the several tires shall not vary more than 35 kPa. Pneumatic tire rollers shall be constructed with ample ballast space to provide an operating weight of 4.5 to 8 kg/mm² of tire track width. The total operating weight of the roller may be varied by the Ministry Representative. The towing vehicle for towed type roller shall be equipped with smooth tread pneumatic tires.

521.25.03 Vibrating Tampers – Vibrating tampers to be used for the compaction of bituminous base in places inaccessible to rollers, shall be of a type suitable for the work to be performed and shall be subject to the approval of the Ministry Representative.

521.25.04 Vibrating Compaction Equipment – The Contractor may use vibrating compaction equipment to compact the stabilized base, provided that such equipment produces compaction and surface finish equal to that obtainable by using a steel wheel finish roller having a minimum compression roll pressure of 4.5 kg/mm².

CONSTRUCTION

521.31 Methods – Based on whether the granular base has been completed by others or by the Contractor, the applicable construction method specified below shall be followed.

521.31.01 Method I – Where the granular base has been previously completed by others, Method I shall apply as follows

- (a) The granular base shall be regraded to true line, grade and cross section; the top 65 mm of the base shall be loosened by means of scarifying. The lumps of bonded material are to be broken to constituent gravel by harrowing, blading or rotary tilling. The loose material is to be bladed into a windrow, approximately 10 cm in height on both sides of the road. This windrow will act as a dam in case of spillage or the occurrence of rain after the spraying has taken place.
- (b) The prepared gravel surface shall be primed on the side not obstructed by the windrow, to the outside width required at a rate of 1 L/m² to 1.5 L/m² or as may be ordered, employing the bituminous binder selected by the Ministry Representative within a spraying temperature range which corresponds to a kinematic viscosity of 150 mm²/s to 200 mm²/s in the binder.

Priming shall be applied only when the surface is dry or slightly damp, and unless otherwise permitted by the Ministry Representative, when the air temperature in the shade is not less than 10°C rising or 12°C falling. When the prime has been completely absorbed by the prepared surface, blade the windrow of loose surface course onto the primed half of the road and prime the untreated half of the surface in a like manner.

Generally, the applicable provisions of SS 502.21 shall govern this work.

- (c) Immediately prior to the application of bituminous binder, the windrow of loose gravel shall be bladed across the road to a full width uniform cross section. If wet or damp, it shall be first bladed back and forth until dry unless an emulsion is to be used. Cutback bituminous binder shall not be applied until the moisture content has been reduced to 1.5%. In the case of emulsions, mixing may commence when the moisture content of the gravel has been reduced to 4%.

No asphaltic binder shall be sprayed on the granular aggregate unless the average temperature of the aggregate on the road is 13°C or higher and such temperature can be maintained in the aggregate until the full mixing cycle has been completed.

Upon the layer of graded aggregate, the bituminous binder shall be applied uniformly at a rate of 1.35 L/m², at an application temperature required to produce a

kinematic viscosity of 150 mm²/s to 200 mm²/s in the binder.

A disc harrow or rotary type mixer shall immediately follow the distributor after the application of the bituminous binder and shall continue to operate on the treated strip until all free bituminous material has been mixed into the mineral aggregate. The treated mixture shall then be further mixed with a blade grader until a uniform mixture results.

The mixture shall then be spread uniformly across the road once more, a second uniform application of bituminous binder at a rate of 0.9 L/m² to 1.8 L/m² as may be directed by the Ministry Representative shall be sprayed at an application temperature to give a final asphalt content of 4% to 6% of the mix. The second application of bituminous binder shall be mixed with the mineral aggregate in a similar manner to the first application, taking care that during the blade mixing none of the untreated base below the prime or shoulder material is brought into the mix. Blade mixing shall be continued until the bituminous binder and mineral aggregates are thoroughly mixed to uniform colour, free from fat spots, balls and uncoated particles. If the mixture should become wet before the mixing process is complete, the mixing operations shall be continued until it has dried out. After final mixing, the mixture shall be brought to a single windrow.

- (d) Before the finished mixture is finally spread for compaction, a triangular cut shall be made with a blade at each edge of the base course to provide for a thickened edge of bituminous mixture. The cut shall be approximately 50 mm deep at the outer edge and slope to zero, 0.6 m in toward the centre. In making the cut, the excavated material shall be thrown to the shoulder in a small windrow against which the mixture shall be spread. The bituminous mixture shall be spread for compaction from the large windrow. It shall be bladed from this windrow in a succession of thin layers to correct thickness and a uniform cross section with a camber of 1 in 45 (except at curves, where the correct super-elevation originally set for base shall be followed).

After the mixture has been spread as specified, it shall be compacted by means of rollers, the operation shall begin at the edges of the course and progress toward the centre parallel to the centerline of the roadway, overlapping in successive passes by at least one-half the width of the roller. Initial breakdown compaction may be obtained by use of steel or rubber tired rollers until no appreciable surface movement or roller marks are visible and the surface has been satisfactorily sealed. Final rolling to eliminate rubber tire or other marks and irregularities shall be carried out using an approved steel wheel finish roller or steel wheel vibrating rollers or other satisfactory equipment.

SECTION 521

ASPHALT STABILIZED BASE COURSE MIXED IN PLACE

Compaction shall be carried out to the satisfaction of the Ministry Representative.

Should unsatisfactory areas develop during compaction, they shall be corrected as directed by the Ministry Representative.

521.31.02 Method II – When the granular base and base stabilization are included in the Contract, Method II shall apply as described hereunder:

- (a) The first lift of granular base, 90 mm, shall be hauled to the roadway, spread, graded to line, grade and cross section, and compacted to 100% of the Standard Proctor Maximum Dry Density as obtained in the laboratory following ASTM D698.
- (b) On completion of the above roadways, as described in the foregoing, priming may be commenced.
- (c) On completion of priming and curing the second lift, 60 mm of granular base may be hauled to the primed roadway and spread in a uniform windrow and mixing commenced as described in SS 521.31(c).
- (d) On completion of base stabilization, the remaining granular base may be applied, i.e., from the edge of the stabilized base to the shoulder.

MEASUREMENT

521.81 Spray Asphalt – Spray Asphalt will be measured by the LITRE sprayed on the road.

521.82 Process Base Gravel – Process Base Gravel will be measured by the SQUARE METRE of completed surface, constructed to dimensions ordered.

PAYMENT

521.91 Payment – Payment for asphalt stabilizing granular base course in place will be made as follows:

- (a) **Spray Asphalt** – Payment for Spray Asphalt will be at the Contract Unit Price per litre for the actual number of litres sprayed on the road converted to its volume at 15.6°C. The rate for spraying asphalt shall cover acceptance of asphalt at points of delivery, storing, heating, transport, spraying, traffic control, labour superintendence, equipment and machinery required to carry out all asphalt spraying work including priming described in this Section.
- (b) **Process Base Gravel** – Payment for Process Base Gravel will be at the Contract Unit Price per square metre of measured completed surface, constructed to dimensions ordered. Payment will not be made for extra width of surface mixed or compacted beyond the limits specified.

The rate for processing base gravel shall cover provision of all tools, labour, equipment and services required to cut, shape, dry, pulverize, mix and lay out the base; all compaction; warning signs, barriers, lights, traffic control and superintendence required to thoroughly mix and compact the asphalt and aggregate in place on the road.

Payment of measured quantities at the bid rates for the above two items shall be full compensation for all work and services performed and materials and equipment supplied to complete asphalt stabilization of granular base course material according to the description and intent of this Section.

SECTION 531

ASPHALT SURFACE TREATMENTS

DESCRIPTION

531.01 Scope – This Section specifies the materials, plant, equipment and quality of work required for the construction of asphalt surface treatments with cover aggregates.

531.02 Description of Work – Surface treatment shall consist of the application of a selected asphalt binder material to a previously constructed bituminous pavement, or a well compacted crushed granular surface followed by the application and embedment of a course(s) of selected aggregate to provide a new surface which shall be impervious to moisture, have a non-skid texture, high luminosity and good riding qualities. This normally would be a graded aggregate seal or a coarse sand seal or any other surface treatment outlined in the Special Provisions for the Project work of the Contract.

531.03 Types of Graded Aggregate Seals – There are six types of graded aggregate seals as characterized by the maximum and minimum gradations of the cover aggregate as defined in SS 531.17.

The Classes and their common applications are:

- (a) Class A – Double seals on gravel highways
- (b) Class B – Double seals on gravel highways, and single seals low traffic paved roads
- (c) Class C and Mod C – Major Highways
- (d) Class D – Major Highways
- (e) Class E – Residential areas and second seals

MATERIALS

531.11 Asphalt Materials – Asphalt(s) to be used on surface treatment may be standard rapid curing cutback asphalt; medium curing cutback asphalts; or anionic or cationic emulsions including high float emulsified asphalts meeting the requirements of SS 952. Types and grades of asphalt binder proposed for use on the Project will be specified in the Special Provisions.

531.12 Coating and Anti-Stripping Additives – For general use or for experimental or trial purposes, the Ministry Representative may direct the use of surface active "coating and anti-stripping additives" or any other materials formulated to improve the coating and adhesive properties of the asphalt binder to the selected cover aggregate. In such cases, instructions concerning the type of additive to be supplied and the method of incorporation with the asphalt binder will be specified in the Special Provisions or, if the work has commenced, will be the subject of supplementary written instructions to the Contractor.

531.13 Mineral Aggregate Supply – Mineral cover aggregate meeting the requirements described hereunder shall be supplied by the Contractor unless specifically described otherwise in the Special Provisions.

531.14 Mineral Aggregate – Method of Test – Mineral Aggregate will be tested in accordance with the ASTM tests listed in Table 531-A.

Table 531-A: Test Methods for Mineral Aggregates

<u>ASTM Test</u>	<u>Name of Test</u>
<u>C88</u>	<u>Standard Test Method for Soundness of Aggregates by use of Sodium Sulfate or Magnesium Sulphate</u>
<u>C117</u>	<u>Standard Test Method for Materials Finer than 75-µm (No. 200) Sieve in Mineral Aggregates by Washing Wash Test of Aggregates.</u>
<u>C127</u>	<u>Standard Test Method for Relative Density (Specific Gravity) and Absorption of Coarse Aggregates</u>
<u>C128</u>	<u>Standard Test Method for Relative Density (Specific Gravity) and Absorption of Fine Aggregates</u>
<u>C136</u>	<u>Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates Sieve Analysis of Aggregates.</u>
<u>C142</u>	<u>Standard Test Method for Clay Lumps and Friable Particles in Aggregates.</u>
<u>C566</u>	<u>Standard Test Method for Total Evaporable Moisture Content of Aggregate by Drying</u>
<u>D75</u>	<u>Standard Practice for Sampling Aggregates.</u>
<u>D4318</u>	<u>Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils</u>
<u>D4791</u>	<u>Standard Test Method for Flat Particles, Elongated Particles, or Flat and Elongated Particles in Coarse Aggregate</u>
<u>D5821</u>	<u>Standard Test Method for Determining the Percentage of Fractured Particles in Coarse Aggregate</u>
<u>D6928</u>	<u>Standard Test Method for Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus.</u>

531.15 Mineral Aggregate – Physical Requirements

531.15.01 Composition – Mineral cover aggregate shall consist of clean, sound, hard, durable particles or fragments of sand, gravel or crushed stones or combination thereof; containing a minimum of thin elongated or flaky pieces such as may cause the height constant of the particles to be less than 0.6. It shall be substantially free from loosely bonded aggregations, clay lumps or other objectionable matter and shall not, in the Ministry Representative's opinion, be markedly hydrophilic in character.

531.15.02 Test Requirements – For the purpose of testing, the mineral cover aggregate shall be considered to be divided into coarse and fine (sand) fractions. Fine aggregates shall consist of all material passing through a 4.75 mm standard sieve, unless some other boundary shall be specifically described under the respective Method of Test.

The mineral cover aggregate, when tested by the methods listed in SS 531.14, shall meet the following criteria:

- (a) **Clay Lumps** – Coarse aggregate shall not contain more than 0.5% by mass of clay lumps or other aggregation of fine material.
- (b) **Absorption** – maximum 1.0% by mass for coarse aggregate; maximum 1.5% by mass for fine aggregate.
- (c) **Soundness** – When tested for soundness by ASTM C 88, the aggregate shall not show a weighted loss for five cycles in excess of 12% when Magnesium Sulphate is used (or have a satisfactory record of service).
- (d) **Degradation** – No individual test shall have more than 20 when tested in accordance with ASTM D 6928.

531.17 Graded Aggregate Seal – Requirements

531.17.01 Gradation – Gradations shall meet the limit stated in Table 531-B for each Class of graded aggregate seal.

- (a) **Fracture** – When tested according to SS 202, Appendix 1 fractured faces, the minimum % by mass on coarse aggregate shall be as follows:
 - Classes A & B: 70%
 - Classes C, D, & E: 85%
- (b) **Variation Limits** – Once the grading curve is established within the above limits, the maximum permissible variation of the mean of any five consecutive tests from the grading curve shall be within the tolerance specified in Table 531-C.

These tolerances do not waive the requirement that the running average of five consecutive tests must be maintained at all times inside the limits specified in SS 531.17.01. Should the Contractor not be able to maintain the grading within the variation limits specified above they shall construct a second stockpile.

This does not alleviate the Contractor from consistently maintaining the grading curve within the tolerances indicated in the Gradation Limits below.

Table 531-B: Gradation Limits for Graded Aggregate Seal

Sieve size (mm)	% Passing by Mass by Class					
	A	B	C	Mod C	D	E
19	100	-	-	=	-	-
16	-	100	100	100	-	-
12.5 ¹	60-90	60-90	-	=	100	-
9.5	40-80	35-75	30-70	25-55	30-80	100
4.75	20-60	15-50	25-45	7-30	25-45	0-30
0.600	0-25	0-15	5-20	0-10	5-20	0-10
0.075	0-7	0-5	0-3	0-3	0-3	0-3
Ratio ² (9.5/4.75)	=	=	=	1-1.5	=	=

Notes:

- ¹ The Contractor may use either the 12.5 mm or the 13.2 mm sieve, at the Contractor's option.
- ² The "Ratio" is the ratio of the mass of material retained on the 9.5 mm sieve, divided by the mass of material retained on the 4.75 mm sieve.

Table 531-C: Variation Limits

Sieve Size (mm)	Maximum Permissible Tolerance % Passing by Mass (ASTM C117 & ASTM C136)
4.75 to 19.0	± 4.5
0.600	± 3.5
0.075	± 1.0

531.17.02 Coarse Aggregate

- (a) Defined as all material retained on the 4.75 mm sieve.
- (b) Consist of crushed stone or gravel, or a combination thereof; or materials naturally occurring in a fractured condition; or materials naturally occurring of highly angular nature or rough texture with a maximum result of 20 when tested in accordance with ASTM D6928.
- (c) Free from coating of clay, silt or other deleterious material and shall meet the applicable test requirements of SS 531.14 and SS 531.15.

531.17.03 Fine Aggregate

- (a) Defined as all materials passing the 4.75 mm sieve and shall meet the applicable test requirements of SS 531.14 and SS 531.15.
- (b) Clean, hard, durable, moderately sharp, and free from coatings of clay, silt or other deleterious material and shall contain no clay balls or other aggregations of fine material.

531.17.04 Mineral Filler and Mineral Dust

- (a) Mineral filler shall consist of all mineral matter passing the 0.600 mm sieve and the mineral dust shall consist of all mineral matter passing the 0.075 mm sieve.
- (b) Mineral filler and mineral dust shall be free from organic matter.
- (c) Mineral filler shall be non-plastic when tested in accordance with ASTM D4318.

531.18 Presealing Sand – Sand required for use as a fine aggregate in connection with presealing operations conducted under SS 531.36 shall meet the physical requirements of SS 531.15, but unless otherwise specified by the Ministry Representative, shall have a grading as specified in Table 531 D.

Table 531 D: Physical Requirements for Sand in a Fine Aggregate

SIEVE SIZE (mm)	PERCENTAGE PASSING
4.75	100
2.36	80 – 100
1.18	60 – 90
0.300	25 – 70
0.150	10 – 40
0.075	2 – 12

Note: All aggregate passing the 0.425 mm sieve shall be non-plastic.

EQUIPMENT

531.21 General

531.21.01 A minimum of fourteen (14) days before commencing work under the Contract, the Contractor shall submit a list of equipment available and ready for use on the Project.

531.21.02 Before any work is permitted to commence, the Contractor's equipment and plant will be carefully inspected and should any of it fail to meet the required standards, work will not be permitted to commence until such deficiency is corrected.

531.21.03 Provision of all equipment listed shall become an essential part of the contract and no equipment or plant may be released from the Project work for use elsewhere until

completion, except by the express written permission of the Ministry Representative.

531.21.04 The Ministry inspectors shall have access at all times to any plant, equipment or machinery to be used on the Project in order to check calibration, control or operating adjustments. Where adjustment appears necessary, it shall be done by a competent operator or mechanic or instrument repair technician, as required.

531.21.05 Plant, machinery and equipment specified hereafter shall represent a minimum requirement for the type of machine described and does not represent a comprehensive list of equipment to be supplied by the Contractor who shall supply whatever plant and equipment may be necessary to the proper and efficient completion of the work

531.21.06 All plant and equipment shall be under the control of experienced and skilled operators.

531.22 Pressure Distributor – The pressure distributor used for applying asphalt materials shall consist of a fully insulated tank with a minimum capacity of 5000 L, permanently and rigidly mounted on a truck or fifth wheel trailer provided with pneumatic tires and propelled by a power unit capable of maintaining a constant and uniform speed.

The pressure distributor, to be acceptable, shall be provided with at least the following equipment:

- (a) A quick opening gate in the dome.
- (b) A dipstick calibrated in 20 mm increments and a calibration chart showing the volume for each 20 mm of depth, or provide the manufacturer dipstick and chart. Readily visible external depth gauge indicating the quantity of asphalt in the tank to the nearest 50 L.
- (c) A means of heating the asphalt material uniformly to any temperature up to 175°C and maintaining the contents constantly at any selected temperature without any local overheating.
- (d) A heavy duty, industrial-type thermometer and a remote reading dial thermometer.
- (e) Rear-mounted spray bars and nozzles set parallel to the surface to be sprayed and capable of vertical and lateral adjustment. The spray bars shall be capable of adjustment to provide a uniformly sprayed surface from a minimum of 0.6 m to at least 4 m width.

The spray bars shall be capable of producing up to triple coverage at one pass. The feed to the spray bars must be arranged so that there is a constant circulation of hot asphalt material to the bars to ensure a uniform viscosity and constant pressure of the liquid asphalt at each nozzle both before and during spraying operations.

- (f) The feed manifolds to the spray bars shall be provided with positive filtration to prevent clogging of bars and nozzles.
- (g) Rotary adjustable spray bar nozzles shall be of the same type, correct size and manufacture; and set to produce uniformly fan-shaped sprays without atomization. Nozzles shall be inspected daily and any nozzle having nicked or damaged edges shall be replaced.
- (h) Spray nozzles shall be provided with valves capable of instant full opening and positive cut-off.
- (i) A connection to a circulating pressure manifold to which a hose may be attached for a single nozzle hand-operated outlet.
- (j) A pressure pump capable of applying the asphalt material at a minimum rate of 180 L per minute, per metre of spray bar.
- (k) A pressure gauge accurate to within 15 kPa and a metre calibrated in litres per minute, shall be readily visible.
- (l) A fifth-wheel tachometer, calibrated in metres per minute and readily visible to the operator of the distributor.
- (m) Vehicle spring "tie-downs" to prevent the change in height of the spray bars due to the reduction of the asphalt load during spraying.
- (n) Electronic distance measuring device, with print out connected to the transmission, that is capable of measuring each spray length and the total length sprayed during the day.

531.23 Mechanical Spreader – Cover aggregate shall be spread by means of an efficient, self-propelled spreader mounted on pneumatic tires and capable of continuously and uniformly spreading closely regulated quantities of crushed aggregates at the application rates selected. The loading hopper shall be of such a capacity as to ensure continuous operation between dumps of aggregate. Aggregate shall be transferred from the loading hopper to the front spreading device in such a manner as to ensure a uniform flow and prevent segregation of particles.

The spreading mechanism shall be so designed as to distribute and spread aggregate across the selected width without segregation. Aggregate shall be applied to the freshly sprayed surface by use of a screen so that the largest particles are first placed on the road with the smaller fractions falling on top. The approved screen is to be mounted beneath the discharge openings of the spreader mechanism in such a manner that the angle of the screen relative to the road surface can be easily and quickly changed.

Suitable provision shall be made to prevent aggregate from rolling on the freshly sprayed surface. The cover aggregate shall be applied ahead of the spreader wheels, with sharply

defined, straight edges and without any lateral movement of the aggregate after initial application.

A walkway with protective railings, conforming to the applicable WorksafeBC Regulations, shall be provided at the front of the spreader so that the hopper operator has safe access to the individual gate control levers.

The spreader shall be designed to apply aggregate at any application width between 2 m and 4.12 m. *Note: A 4.26 m hopper is required.*

531.24 Rollers – Self-propelled pneumatic tired rollers (mass range from 13 600 kg to 19 000 kg) shall be used on the work. The rollers shall have a minimum effective rolling width of 1.83 m. The rollers shall be equipped with smooth, wide-tread compaction tires of equal size and diameter. The wheels of the roller shall be spaced so that one pass of the roller shall accomplish one complete coverage equal to the rolling width of the machine. There shall be a minimum overlap of 6 mm of the tracking wheels of the roller. The wheels shall be so mounted as to oscillate in pairs. The roller shall be provided with ample ballast space and be so constructed that the specified wheel load can be uniformly applied to all the wheels, to provide similar contact pressure under all wheels when rolling on a level pavement.

The tires shall have a minimum of 16 ply and their inflation pressure shall not normally exceed the specified pressure by more than 35 kPa and shall at no time be less than specified. Inflation pressure of all of the tires shall be controlled from a central location while the machine is in motion and capable of varying the tire pressure from 206 kPa to 827 kPa. Rollers shall be ballasted and operated to the manufacturers recommended specifications.

The Ministry Representative may approve the use of compactors equal to but differing from the above requirements provided the Ministry Representative is satisfied that equally effective compaction will be obtained by use of the alternative equipment.

The Ministry Representative may approve the use of rubber-covered, vibrating drum rollers (minimum width of 1.52 m and mass of 3000 kg) or tandem steel wheel rollers in the static mode provided the drum or drums can contact the road surface over the entire width of the drums (no bridging). On shoulder seals, the use of pulled wobbly wheel rollers may be used at the discretion of the Ministry Representative. They also may be used on graded aggregate seal on gravel surfaces, where the road is wide enough to permit units to turn without damage to the road surface, sealed or otherwise.

All rollers shall be provided with equipment for spraying water continuously on all tires or wheels, while rolling is in progress, and also be provided with such means as may be required to keep the tires clean and free of adhering asphaltic binder.

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531.25 Power Brooms – Brooming work shall be carried out using rotary power brooms mounted on self-propelled, pneumatic tired tractor units. The brooms shall be capable of vertical and horizontal angular adjustment and must be provided with equipment for spraying water, as required, on the surface to be broomed while the equipment is in operation. The brooms shall have sufficient power and brushing capacity to completely clean the surface of the standard lane to be treated or which has been treated within three coverages.

To be read in conjunction with SS 145.17 and SS 165.

531.26 Blade Graders – Self-propelled blade graders mounted on pneumatic tires shall be of approved modern type, either four wheel drive or tandem type, having sufficient capacity to efficiently perform all work that may be required on the contract.

531.27 Water Truck – Water trucks shall have a minimum capacity of 9000 L. Water shall be applied from a distributor of the pressure type, equipped with spray bar mounting nozzles similar to those used on asphalt distributors and capable of applying the water accurately and uniformly. Splash plate type of distributors or those equipped with spray bars that eject fine streams of water will not be permitted.

The distributor must be provided with a satisfactory means for accurately measuring the quantity of water sprayed. If the Ministry Representative so requires, measuring equipment shall be calibrated under the Ministry Representative's inspection. The Contractor shall make all necessary arrangements for obtaining water at the Contractor's own expense. In addition, the water truck shall be equipped so that it may be used as a flush truck with front-mounted, adjustable spray nozzles that produce a fan-shaped spray in the horizontal mode and can be adjusted in vertical or horizontal direction (ball-mounted). The delivery of water should be variable to single or multiple nozzles so that the desired flushing can be achieved. The water truck shall be equipped with a loading pump capable of pumping 1360 L or more of water per minute.

The spraying and flushing controls must be in the cab and accessible to the driver while the unit is in motion. The minimum spray bar width will be 2.40 m. Gravity single jet or drip bar equipment unit will not be permitted.

531.28 Sign Truck – Truck(s) of sufficient capacity to store and transport all necessary signs, stands, barriers, flashing lights, batteries, flood lights, generators, high-level warning devices, project signs, etc., is required.

531.29 Tanks, Mobile Asphalt Heating, & Storage – Insulated liquid asphalt storage tanks should meet the following minimum criteria:

- (a) A positive high-low heating system, 10°C to 200°C, with a hot oil/steam circulation system.

- (b) Automated heating and control system with safety shutdown.
- (c) Intake/Outlet, two, each with screens installed.
- (d) Positive locking valve on discharge line.
- (e) Sampling valve or spigot.
- (f) Pyrometer; Multi point 20°C – 200°C.
- (g) Direct reading levelometer.
- (h) Manholes, at each end, with ladders.
- (i) Tank(s) with sufficient capacity to hold one day's production requirements.

CONSTRUCTION

531.31 Temperature and Weather – Asphalt binder and aggregate shall be applied only during the hours of daylight when the shade air temperature is at least 10°C for graded aggregate seals and is rising; and when the surface of the road is dry and at a minimum temperature of 20°C and rising. Operations shall cease when the road temperature falls below 20°C for chip seals and 15°C for graded aggregate seals. No application shall take place when, in the opinion of the Ministry Representative, the weather or roadbed conditions are unfavourable, i.e., weather is misty or rainy, precipitation is predicted for the construction area within 12 hours or the atmospheric temperature at the construction area of less than 12°C is predicted within 24 hours. No chip seal shall be applied before June 1 or after August 15.

531.32 Traffic Control – Traffic control shall meet the requirements of SS 194, the Special Provisions and the following subsections.

531.32.01 Signs and Barriers – The Contractor shall provide all warning and instructional signs, barriers, lights, electronic communicating devices and flagpersons required to properly and safely control all traffic around or through the work in accordance with SS 194, and as may be specially required below.

531.32.02 Detours – Where conditions indicate that traffic may be detoured, the Contractor shall arrange to close off the section of road to be sealed and erect detour and route signs. After the completion of the sealing operations traffic shall not be allowed onto the closed sections, for the periods specified in SS 531.32.04, SS 531.36.06 and SS 531.37.04, unless otherwise directed by the Ministry Representative.

531.32.03 Shoulder Detours – Where shoulders are wide and firm enough to accommodate traffic that cannot be detoured along an alternative route, the Ministry Representative may direct that traffic be detoured onto the shoulder. If required, and approved by the Ministry Representative, the shoulder shall be made up, rolled and graded on a force account basis. A designated liquid asphalt shall be sprayed from the edge of the road to the edge of the

shoulder, as required, and allowed to cure before traffic is detoured off the road to be sealed and onto the shoulders.

The Contractor shall provide sufficient pilot cars and barriers to maintain traffic movement at a maximum speed of 40 km/h solely on the shoulders, during the course of work. Once a section has been sealed, traffic will be regulated as indicated in SS 531.32.04.

531.32.04 Traffic Through Work – Where it is not possible to detour traffic, sealing work shall normally be carried out in lane widths and traffic shall be regulated by pilot cars on the opposite traffic lane to that on which construction is in progress. Traffic travelling on the old road surface may be permitted to travel up to a maximum speed of 50 km/h consistent with safety requirements. Traffic shall only be allowed to travel on the newly-sealed lane under control of pilot car(s) which shall enforce a 20 km/h speed limit up to 4 hours and a 30 km/h speed limit for a period of up to 24 hours, as determined by the Ministry Representative, after completion of the sealing operations; except in the case of pre-seal construction when the period of control shall be up to 4 hours after completion of the pre-sealing operation.

In the event of prolonged inclement weather, traffic control will be maintained as long as the Ministry Representative deems necessary to protect newly laid surface seals.

531.32.05 Pilot Cars – Sufficient pilot cars shall be provided so that the delay to traffic shall be no greater than 20 minutes. They shall be equipped in accordance with SS 194.

531.32.06 Communications – The flag people, pilot car(s), sign truck, spreader and supervisor's vehicle shall have an effective electronic means of communication so that instructions and information can be quickly and accurately relayed over the total length of the control area.

531.32.07 Flood Lights – During periods when headlights must be used, including during daytime periods of poor visibility and high dust conditions, two self-contained floodlights shall be used to illuminate each of the flagging stations. These floodlights shall be of at least 1000 watts and be complete with reflector, lens and adjustable stand at least 2.15 m high. The light, reflector and lens must be mounted so that they are capable of both vertical and horizontal adjustment. The stands are to be sufficiently sturdy so that they can withstand a 30 km/h wind without the use of ballast. The generators shall have sufficient capacity to enable them to run one shift under full load.

531.32.08 Flag Personnel – Flag personnel shall be equipped in accordance with SS 194 and provided as required to safely control and conduct traffic through the job.

531.33 Preparation of Roadway – The patching and levelling of the pavement to be sealed may be part of a sealcoating contract or may be effected by others.

531.34 Sweeping – Sweeping shall meet the requirements of SS 145, SS 165, and SS 531.27 regarding equipment for watering and the following:

- (a) Immediately prior to the commencement of the sealing operations, all loose aggregates, dust, dirt, caked clay or foreign materials shall be removed from the width of the surface to be treated by brushing with power brooms supplemented by hand push brooms, shovels or the use of a power grader, and where necessary by flushing.
- (b) Particular care shall be taken to thoroughly clean to the outside edges of the strips to be treated and to ensure that the sweepings are not so deposited on the shoulder as to permit subsequent contamination of the treated surface.

531.35 Patching – In order to avoid delays to the Contractor, any patching not done, and in the opinion of the Ministry Representative required before sealing operations begin, may be done by the Contractor as directed by the Ministry Representative with materials supplied by the Ministry and paid for under an Order for Extra Work.

531.36 Pre-Seal Treatment

531.36.01 Description – Wherever the pavement is in a worn, open, dry or porous condition, or on gravel surfaces which, in the opinion of the Ministry Representative, would result in high absorption of the asphalt binder, the surface shall be pre-sealed. All pre-sealing work on a Contract shall, unless otherwise ordered by the Ministry Representative, be fully completed prior to the commencement of seal coat work.

531.36.02 Application of Asphalt Pre-Seal – After the surface has been prepared in accordance with SS 531.33 and SS 531.34, a selected asphalt binder shall be sprayed, at such application rates as directed, so that just sufficient asphalt material shall be applied to the most absorptive sections of the mat, avoiding an excess elsewhere. The asphalt binder shall be applied at the temperature at which the Saybolt-Furol Viscosity of the material is in the range of 50 to 75 seconds.

531.36.03 Application of Sand Cover

Application of sand in sufficient quantity to completely blot up all excess unabsorbed asphalt material shall follow within five minutes or as may be directed by the Ministry Representative.

At the time of spreading, the sand shall not contain more than 5% moisture by mass.

531.36.04 Rolling – Rolling shall commence immediately after the application of cover sand and shall be completed within 15 minutes. Sufficient pneumatic tired rollers shall be used to ensure that the entire surface of the treated road receives complete coverage within the specified time, at a rolling speed not in excess of 8 km/h.

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531.36.05 Bleeding – Bleeding spots or areas shall receive a further application of sand blotter and be re-rolled. This process shall be repeated until no further bleeding occurs.

531.36.06 Traffic – No traffic shall be permitted on pre-sealed sections for up to four (4) hours or as the Ministry Representative may direct.

531.36.07 Curing Time – No seal coating work shall be carried out on the pre-sealed surface until 24 hours after completion of the pre-seal or as the Ministry Representative may direct.

531.36.08 Sweeping – Prior to commencement of the main seal-coat treatment all loose, unbonded sand shall be swept from the pre-sealed surface in accordance with SS 531.34.

531.37 Single Pass Surface Treatment

531.37.01 Application of Asphalt Material – The selected asphalt material shall be uniformly sprayed on the properly prepared surface at a rate of 0.5 to 2.75 L/m², or as may be directed, and at such temperature as shall cause the Saybolt Furol Viscosity of the material to be within 50 to 75 seconds.

Wherever it is possible to detour traffic, the surface may be sprayed full width at one pass of the distributor. Two spreaders must be used.

Normally, where it is not possible to detour traffic only one half of the roadway shall be sprayed at one time, permitting one-way controlled traffic on the other half. In no case shall a longitudinal joint be so placed that it is in a wheel path, the exception being when a double graded seal is being applied.

In order to ensure a uniform distribution of asphalt material and avoid the formation of a lap or ridge at the transverse junction of two successive applications, building paper shall be spread over the treated surface for a sufficient length back so that the spray jets are operating properly (fully open) when the uncovered surface is reached. The building paper shall then be removed and disposed of prior to the application of aggregate (Refer to SS 165). There shall be a maximum 100 mm overlap of sprayed material at the longitudinal joint between sprayed lanes.

The spray shall be cut off before the distributor tank has been completely emptied to ensure that a full application of asphalt material is made up to the point of cut off. Pressure hand sprays shall be used to properly treat small patches or inaccessible places that have been missed by the distributor. Care shall be taken and building paper shall be used, if necessary, to ensure against staining any curbs, guardrails or faces of any improvements near the work.

The progress of the application of the asphalt material shall be governed by the weather, the curing rate of the asphalt and the rate at which the cover aggregate is applied and rolled. No traffic shall be permitted on the uncovered asphalt material. The Contractor shall organize work so that

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seal-coating can normally be completed to the full width of the road surface each day.

531.37.02 Application of Cover Aggregate

(a) Application – The uniform application of the specified cover aggregate, at the spread rate set by the Ministry Representative, shall immediately follow the application of the asphalt material, before the emulsion breaks or within 2 minutes if a cutback asphalt is sprayed. An approved screen shall be used to place the larger chips or particles of aggregate onto the sprayed asphalt first so that maximum embedment of the larger material is obtained. Any segregation shall be corrected immediately.

(b) Moisture Content Graded Seal Aggregates – At the time of spreading, the moisture content of the aggregates shall be such that the material will flow freely in the hopper without segregation and/or a build-up of material on the screen and shall be less than the maximum allowable in Table 531-E.

(c) Correction of Non-Uniformity – Prior to and during the rolling operation, any non-uniformity in the distribution of the cover aggregate shall be corrected by spreading additional aggregate and/or by light raking or hand brooming over such sections of the road as the Ministry Representative directs.

Table 531-E: Maximum Mass Percentage for Moisture Content of Cover Aggregate

Aggregate Types	Maximum Allowable Moisture Content for Liquid Cut Back and Asphalt Emulsions
Fine Cover Sand (<u>≤ 4.5 mm</u>)	5%
Coarse Cover Sand (<u>>4.5 mm</u>)	4%
Small Cover Aggregate (<u>≤ 9 mm</u>)	3%
Medium Cover Aggregate (<u>9 mm – 12 mm</u>)	2%
Large Cover Aggregate (<u>> 12 mm</u>)	1%

531.37.03 Rolling – Immediately after the aggregate has been uniformly spread, the road surface shall be rolled sufficiently to embed the aggregate into the asphalt material to the satisfaction of the Ministry Representative.

Sufficient pneumatic tired rollers shall be used to ensure that the entire surface of the aggregate receives at least five complete coverages within 15 minutes of the application of

asphalt material; or when asphalt emulsion is used, five complete coverages shall be completed before the emulsion starts to break. The speed of the rollers shall be such that the suction of the tires shall not displace the aggregate from the road surface or a maximum of 8 km/h, whichever is less.

The completed surface shall present a uniform appearance. Any segregation or non-uniformity in the distribution of the aggregate shall be corrected by spreading additional material over the area and re-rolling with five coverages of the rollers.

531.37.04 Protection of the Surface

- (a) No traffic shall be permitted on the sealed surface until after the rolling has been completed and when the Ministry Representative is satisfied that the applied aggregate will not be picked up or be displaced under traffic. Generally, traffic shall not be permitted on the sealed roadway until at least 2 hours after the rolling is completed. If traffic must be passed over a sealed surface before 2 hours have passed then they will be kept to a speed of 20 km/h or less. After 2 hours, then the speed may be increased to 30 km/h up to 4 hours and not more than 50 km/h for 24 hours or until the surface has been swept. The curing period shall be increased if so directed by the Ministry Representative.
- (b) From the completion of rubber tired rolling on any section of sealed surface and for a further continuous period of 3 days or more including weekends and public holidays, the Contractor shall have a vehicle(s) and maintenance crew (a minimum of three employees) immediately available to arrest any tendency of the asphalt binder to bleed through the surface of the cover aggregate and to prevent pickup and blackening of the surface by the action of traffic. All bleeding spots shall be blotted with a sufficient quantity of blinding sand necessary to correct the defect and in such a manner as the Ministry Representative shall direct. No payment will be made for the supplying and placing of the sand.
- (c) Blinding Sand – In the event of rain, traffic will be kept off a newly sealed surface until sand, provided by the Contractor, has been applied in sufficient quantity to protect the surface of the road from damage. Sanding will continue as long as necessary to protect the freshly laid seal. Payment for the supply and application of this sand will be by Order for Extra Work.

531.37.05 Removal of Loose Aggregate – To be read in conjunction with SS 145 and SS 165.

- (a) The Contractor shall, when directed by the Ministry Representative, sweep off all surplus aggregate and spread it neatly along the gravel shoulders. Sweeping of the road surface will normally be completed within 48 hours for graded aggregate seals. This operation shall be done while the road surface is cool and care should be exercised not to disturb the aggregate that has been retained in the asphalt. In the event that cover

aggregate is being disturbed, brooming will cease until curing is complete and the cover aggregate is firmly held in place. Water shall be used in sufficient quantity to reduce dust hazard and to meet pollution control requirements.

- (b) Alongside barriers and curbs and where catch basins or drainage outlets are encountered, the Contractor shall pick up and dispose of the surplus aggregate, including any pertinent cleaning of catch basins, behind the barriers or curbs or within the right-of-way, in any manner the Contractor elects, subject to the approval of the Ministry Representative.

531.38 Double Pass Surface Treatment

531.38.01 Preparation of Existing Surface – Existing surface preparation shall be performed in accordance with SS 531.33 through SS 531.36 as applicable and as detailed in the Special Provisions.

531.38.02 Surface Treatment – First Pass – The first pass of the surface treatment shall be performed in accordance with SS 531.37.01 and SS 531.37.02, using the graded seal cover aggregate specified in the Special Provisions. Rolling, Surface Protection and Removal of Loose Aggregate shall conform to SS 531.37.03 through SS 531.37.05.

531.38.03 Surface Treatment – Second Pass – The second pass of the surface treatment shall commence not less than 24 hours after completion of the first pass, subject to acceptable weather conditions as detailed in SS 531.31, and as the Ministry Representative may direct. Asphalt material shall be applied in accordance with SS 531.37.01, followed by an application of graded seal aggregate as specified in the Special Provisions and detailed in SS 531.37.02, and rolling, surface protection and removal of loose aggregate per SS 531.37.03 through SS 531.37.05.

MEASUREMENT

531.81 Surface Treatment – Surface treatment will be measured by the SQUARE METRE.

531.82 Asphalt Binder – Asphalt binder will be measured by the LITRE.

531.83 Aggregates – The aggregate will be measured by the CUBIC METRE in stockpile. The volume will be determined in accordance with SS 145.21.01.

531.84 Stockpile Bases – No measurement will be made for work and materials required for the preparation of stockpile bases, as directed by the Ministry Representative.

PAYMENT

531.91 Surface Treatment – Payment for SURFACE TREATMENT will be at the Contract Unit Price per square metre. The Unit Price shall cover everything done and

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supplied including ordering, receiving, storing and heating of asphalt materials, traffic control equipment, and all equipment and labour required to complete the work.

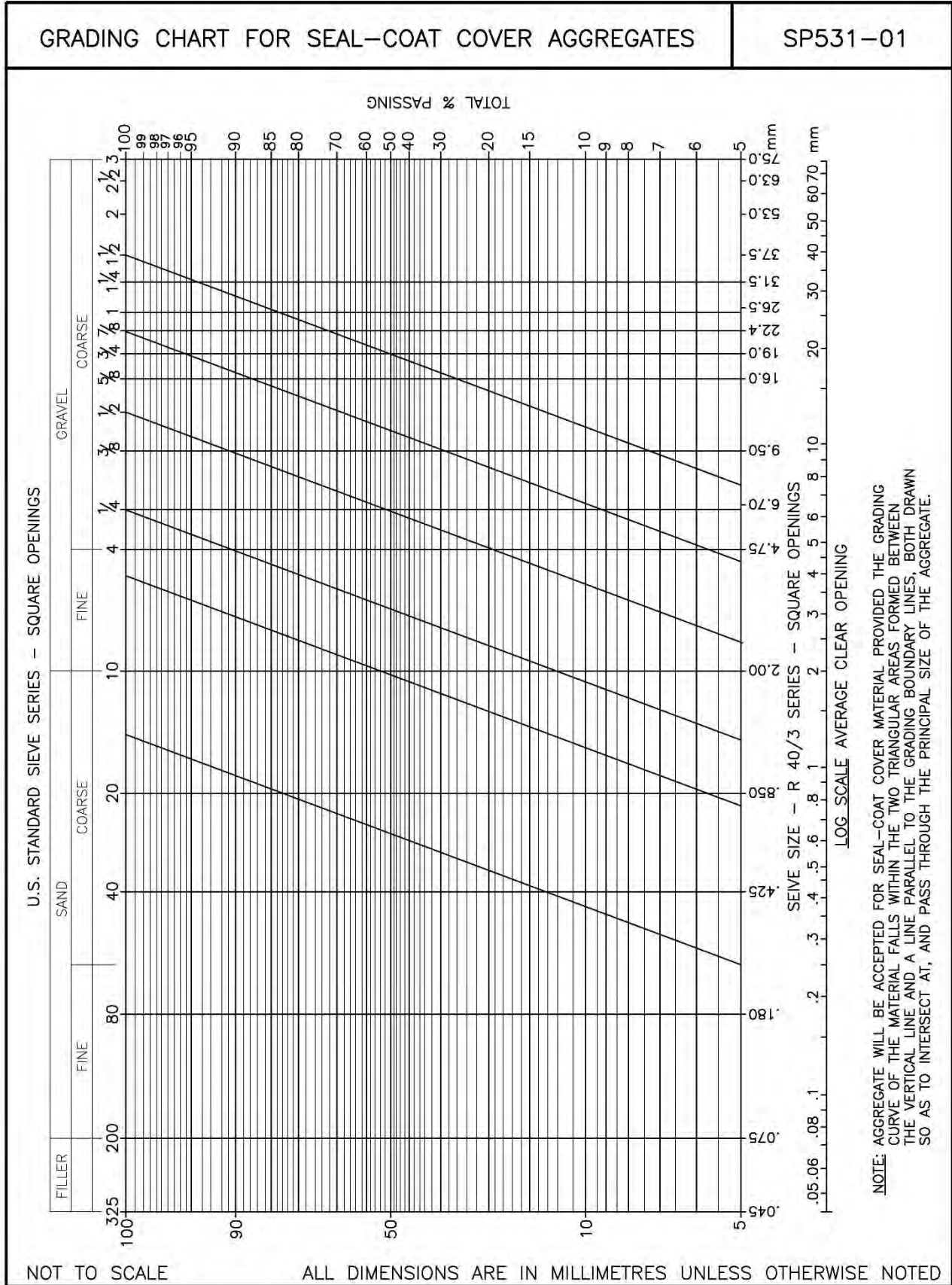
531.92 Asphalt Binder – Payment for ASPHALT BINDER will be at the Contract Unit Price per litre actually sprayed.

531.93 Aggregates – Payment for AGGREGATE will be at the Contract Unit Price per cubic metre in stockpile. No

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additional payment will be made for washing when needed in order to meet gradation requirements.

531.94 Stockpile Bases – All work and materials required for the PREPARATION OF STOCKPILE BASES, as directed by the Ministry Representative, will be paid for, on an Order for Extra Work basis from the Provisional Sum included in the Schedule 7.



SECTION 536

PAVEMENT CRACK SEALING

DESCRIPTION

536.01 Scope – This work consists of restoring the asphalt concrete pavement surface to a less permeable condition. The work includes routing when requested, cleaning and filling the crack with sealant, and dusting or sanding.

536.02 Purpose – The purpose of crack sealing shall be to prevent moisture from penetrating into the road base through surface cracks and to extend pavement life. This shall be carried out by effectively sealing the cracked surface with a highly rubberized and elasticized or high float asphalt sealant product.

MATERIALS

536.11 Asphaltic – The rubberized asphaltic and/or elasticized asphalt sealant products shall meet the requirements listed in Table 536-A.

It shall be noted that ASTM D6690 requirements are not mandatory in the Lower Mainland, Vancouver Island or the Sunshine Coast.

As specified in the Special Provisions the Contractor shall use a Rubberized or High Float Emulsified Asphalt HF 150S or, with the approval of the Ministry Representative, either HF 150P (hot pour) or HF 250S (cold pour).

Table 536-A: Sealant Requirement

ASTM Test	Description
D6690	Specification for joint and crack sealants, hot applied, for concrete and asphalt pavements

536.12 Sand – On pavements that require the use of High Float Emulsified Asphalt, the Contractor shall supply "blinding sand." The sand, when tested according to ASTM C117, shall meet the gradation requirements shown in Table 536-B and shall be approved by the Ministry Representative.

536.13 Dust Cover – Pavements that are sealed with rubberized or elasticized sealant shall receive a dust coating with a material such as Portland Cement, talc, lime or the equivalent material approved by the Ministry Representative.

Table 536-B: Blinding Sand Gradation Requirements

Sieve Size (mm)	% Passing
6.3	100
4.75	95 - 100
2.36	50 - 100
0.600	20 - 70
0.300	5 - 25
0.075	0 - 3

EQUIPMENT

536.21 Router – Where the cracks require routing, the Contractor shall provide a vertical router capable of routing asphaltic pavements to a depth of 25 mm and a width of 16 mm. It shall be capable of a minimum production of 200 metres per hour, even when following meandering cracks without unnecessary pavement cutting.

536.22 Cleaner – The cleaner unit shall be a hot compressed air fed propane fired device capable of cleaning, heating and drying routed cracks with not less than 1.7 m³/min of compressed air at 690 kPa. It shall have valving/hoses and fittings for the mixture of liquid propane gas and compressed air. The hot air exhaust shall not exceed 315°C.

536.23 Melter – The melting kettle shall be of the double boiler type supplying indirect heating to slowly heat the asphaltic material with heat transfer oil. It shall have built in calibrated thermometers for both transfer oil and the sealing compound. The heating of the sealant shall be efficient and thermostatically controlled so as not to exceed the manufacturer's maximum safe heating temperature and shall be such as to maintain a constant temperature once the sealant is heated. The melter shall be capable of constantly agitating the asphaltic material as it is being heated and shall have a pump circulating the sealant from the bottom to the top of the kettle.

The melter must comply with the Gas Safety Act Regulations and Codes and any other applicable acts and regulations.

536.24 Filler Tools – Crack filler devices and strike off tools must be such that successful forming of the bead of sealant over the prepared crack is as specified in this Section.

CONSTRUCTION

536.31 General – Crack sealing shall only be performed when the pavement surfaces are dry, and the crack and road base are dry or nearly dry (no visible moisture), and the surface temperature shall be 10°C or higher.

The Ministry Representative shall select a random sample of virgin sealant from each Lot for testing purposes.

Where required by the Contract, cracks up to 16 mm in width shall be widened by using a router to form a sealant reservoir 16 mm in width and from 19 mm to 25 mm in depth. All routing shall be performed keeping the crack centreline within \pm 8 mm of the centre of the rout and shall be cleaned with a hot compressed air lance. Material removed from the cracks shall be disposed of in an environmentally compliant and sound manner as approved by the Ministry Representative.

The routing speed shall be such that the pavement is carefully cut, not broken or torn out, and the sides of the rout are smooth and uniform. The surface of the pavement and routed crack shall be cleaned of all dust and routing debris.

Routing should not be carried out on pavements that are of such an age that pavement fractures or spalls occur along the edge of the freshly routed crack. Normally pavement fractures or spalls should not occur unless pavements are in excess of ten (10) years old. The decision to forgo routing shall be made by the Ministry Representative.

The crack shall be filled with sealant from the bottom to the surface level in such a manner that the sealant does not bridge entrapped air pockets. Material shall be placed to overfill the crack. It will then be struck off to leave a uniform amount of sealant directly over the crack, with the edges of the spread evenly feathered to overlap on the pavement surface from a minimum of 25 mm to a maximum of 40 mm on each side of the crack. The sealant overband shall not be so thick that it can be removed during snow plowing or produce a noticeable bump when traversed by traffic.

The Contractor shall repair any damage done to the sealant by stones or any other deleterious material being embedded in the sealing compound.

The Contractor shall ensure that traffic is kept off the sealed cracks until the sealant has properly set up and will not be damaged or pulled out by the passage of traffic.

536.32 Rubberized and Elasticized Asphalt Sealants – These shall be used on pavements that are less than seven (7) years old and where the majority of cracks are less than 25 mm in width.

The sealant shall be applied to cracks that have been routed to a uniform depth and width.

Cracks having a width greater than 16 mm need not be routed but shall be cleaned to a minimum depth of 25 mm.

Within two minutes of the completion of the cleaning operation, the crack shall be filled with sealant from a melter using a connecting wand or manual applicator (pouring cone) which ensures minimum pour application temperatures for the product are maintained.

Upon completion of the pouring, the sealant shall be dusted to prevent the asphalt from tracking. Excess dusting material shall be removed.

536.33 High Float Emulsified Asphalt – High Float Emulsified Asphalt shall be used on pavement where there are depressions or lipping at the cracks, or majority of cracks are 5 mm to 35 mm in width.

The cracks shall be cleaned as close to the actual depth as possible and the removed material shall be disposed of in an environmentally compliant and sound manner as approved by the Ministry Representative. Immediately after cleaning, they shall be filled with sealant from a distributor truck or melter. The distributor truck or melter shall have an efficient means of heating the sealant to any temperature up to 100°C and maintaining it constantly at the manufacturer's prescribed temperature without overheating.

Upon completion of the sealing of the crack, the sealant shall be sanded to prevent the asphalt from tracking. Excess sand shall be removed and disposed of, by the Contractor, in an environmentally compliant and sound manner as approved by the Ministry Representative.

536.34 Warranty – The Contractor shall rectify any defects identified during the Warranty Period within twenty-one (21) days from the time first detected by or reported to the Contractor to the satisfaction of the Ministry Representative.

MEASUREMENT

536.81 General – Pavement Crack Sealing will be measured by the LITRE using weight slips.

PAYMENT

536.91 General – Payment for PAVEMENT CRACK SEALING will be at the Contract Unit Price bid per litre. The Unit Price shall be full compensation for all labour, equipment and materials supplied as required to rout or clean with compressed air and seal the pavement cracks, as specified.

SECTION 537

SPRAY PATCHING

DESCRIPTION

537.01 Scope – This specification describes the materials and the professional standards required for supplying and applying tack coat and a mixture of asphalt emulsion and aggregate or a pre-mixed ‘spray patch’ material for repairing pavement cracks and other defective locations on the pavement surface as applicable.

537.02 Purpose – The purpose of spray patching is to repair potholes, depressed pavements and cracks wider than 35 mm.

MATERIALS

537.10 Asphalt Materials – The asphalt emulsion products shall meet the requirements listed in Table 537-A. the Contractor shall ensure that the asphalt emulsion is compatible with the aggregates used for spray patching.

Table 537-A: Emulsion Requirement for Spray Patching

ASHTO Test	Emulsion Property Limits	
	RS-2	WSPE ¹
T 49: Penetration, 100 g, 5 s	100 - 200	90 - 150
T 50: Float test, 60°C, seconds	-	≥ 1200
T 59: Viscosity, Saybolt Furol, 50°C, seconds	75-400	≥ 50
Demulsibility, 0.02 N CaCl ₂ , %	60 min	≤ 65 max
Sieve Test, %.	≤ 0.1	≤ 0.1
Residue by Distillation, %	≥ 65 min	≥ 68
Oil Distillate, Volume %	-	≤ 7

Note 1: Winter Grade Spray Patch Emulsion

537.11 Aggregate – The Contractor shall supply the aggregate necessary to produce a dense, stable and durable patch. The aggregates shall meet the requirements specified herein.

537.11.01 Test Methods – The aggregate will be sampled and tested in accordance with the standard ASTM procedures and tests listed in Table 537-B.

Table 537-B: Test Methods for Mineral Aggregate

ASTM Designation	Name of Test
D75	Standard Practice for Sampling Aggregates
C117	Standard Test Method for Materials Finer than 75-µm (No. 200) Sieve in Mineral Aggregates by Washing
C136	Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates
C142	Standard Test Method for Clay Lumps and Friable Particles in Aggregates
D4318	Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils
D6928	Standard Test Method for Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus

537.11.02 Test Requirements – The aggregate shall be clean, hard, durable, moderately sharp, and free from coatings of clay, silt or other deleterious material and shall contain no clay balls or other aggregations of fine material. The aggregates shall not be markedly hydrophilic in character.

The aggregate, when tested by the methods listed in Table 537-B, shall meet the following criteria:

- (a) **Clay Lumps** – The aggregate shall not contain more than 0.5% by mass of clay lumps or other aggregation of fine material.
- (b) **Micro-Deval** – No individual test shall have a loss factor of more than 20%.
- (c) **Plasticity** – All aggregate shall be non-plastic when tested in accordance with ASTM D4318.

537.12 Aggregate Gradation – The aggregates shall meet the gradation requirements of Table 537-C.

537.13 Spray Patching Mix – The spray patching mix used shall exhibit uniform coating, good cohesion, stability and workability.

Table 537-C: Gradation Requirements for Spray Patching

Sieve Size (mm)	% Passing
9.5	100
4.75	75-100
2.36	0-30
1.18	0-10
0.075	0-5

EQUIPMENT

537.21 Patching Unit - The Contractor shall supply all equipment necessary to complete the Work including a patching unit of a design currently accepted by the Ministry and in use for highway maintenance.

The machine shall:

- (a) provide a minimum of 5.0 m³/min of compressed air to clean cracks and the surface to be patched; and
- (b) be capable of:
 - (i) spraying the asphalt emulsion to provide a tack coat;
 - (ii) blending aggregate and emulsion;
 - (iii) applying the resulting mix to provide a high-density patch; and
 - (iv) covering the repaired area with dry aggregate

The Contractor shall also supply appropriate compaction equipment, suitable for the mix, that is acceptable to the Ministry Representative.

CONSTRUCTION

537.31 General – Spray patching shall only be performed when the pavement surfaces are dry, and the crack and road base are dry or nearly dry (no visible moisture), and the pavement surface temperature shall not be lower than 10°C.

The cracks and surface on which the spray patching material to be placed shall be free of dirt, sand, foreign matter, and loose material, and be lightly tack coated with emulsion.

All cracks, potholes and depressions shall be filled to the level of surrounding surface, producing a level, rideable patch, free of excess asphalt and, when swept, free of loosening rock.

The repaired area shall be compacted to ensure adequate embedment of the asphalt aggregate mixture into and over the crack or depression.

Where the in-place mix exhibits ravelling or excessive tracking, the patch shall be covered with dry aggregate.

QUALITY CONTROL AND QUALITY ASSURANCE

537.41 Quality Control by the Contractor – The Contractor shall be responsible for providing all resources required to carry out the quality control on all the constituent materials, processes and products, their testing and inspection, within the construction of the spray patch and the quality of the end product.

537.42 Sampling and Testing – The Ministry may request the Contractor to provide test results showing that the materials used for spray patching meet the requirements of this specification as determined by the applicable tests.

The Contractor shall be responsible to provide representative samples for the aggregate, emulsion and patching mixture as required when requested by the Ministry. These samples shall be taken in presence of the Ministry Representative.

The above samples may be tested by the Ministry to ensure that they meet the requirements of this specification.

537.43 Acceptance – Spray patches shall be accepted based on following.

- (a) visual observation of the patching material,
- (b) surface appearance of the patch at the time of patching,
- (c) continuity with adjacent surfaces, and
- (d) conformance of the patching materials to the requirements of this specification.

At the request of the Contractor, the Ministry Representative will attend on-Site to observe the initial spray patching and advise on its acceptability, before the Contractor proceeds with the balance of the Work.

Prior to issuance of the Completion Certificate, all finished patches shall be free of defective areas, as determined by the Ministry Representative, including but not limited to, wheel track marking, ravelling, uneven surface, and fat spots.

Areas showing defects shall be deemed Unacceptable Work and shall be repaired by the Contractor at no cost to the Ministry.

MEASUREMENT

537.81 General – Pavement Spray Patching will be measured by the volume of emulsion actually sprayed in LITRES.

PAYMENT

537.91 General – Payment for spray patching shall be made at the Contract Unit Price per LITRE.

The supply and application of aggregate is incidental to the Work and will not be paid separately.

SECTION 537

PAVEMENT SPRAY PATCHING

Payment at the Contract price for the above items shall be full compensation for all labour, equipment, and material to do the Work.

NEW SECTION

SECTION 541

STONE PAVING

DESCRIPTION

541.01 Scope – This Section covers the construction of stone paving for grouted traffic island and median capping, dry-laid and grouted slope protection, culvert and spillway aprons, ditches and other like uses.

A minimum thickness of 100 to 225 mm will be specified for paving rock depending upon location, use and setting requirements. Requirements for rock exceeding 200 mm thickness are specified by SS 205 for hand-laid and grouted riprap.

MATERIALS

541.11 Materials

541.11.01 Rock – Rock for stone paving shall generally be obtained from the roadway excavation, stockpiled where designated and selected as necessary for use at the locations indicated on the Drawings, unless the supply of off-site rock is specified.

Rock will be required as either:

- (a) Angular rock as found or blasted; or
- (b) Rounded rock.

All rock shall be sound, durable and clean, relatively flat on the face to be exposed, reasonably consistent in colour tone, of acceptable size (200 mm to 450 mm across) and of the required thickness range.

541.11.02 Gravel Base – Gravel base shall be 25 mm well-graded crushed granular aggregate.

541.11.03 Bedding Mortar – Bedding mortar shall consist of one part Portland cement to three parts clean, well-graded fine aggregate or coarse sand mixed with water to give a 150 mm slump.

541.11.04 Setting and Jointing Mortar – Setting and jointing mortar shall consist of one part Portland cement to three parts clean, well-graded fine aggregate mixed with water to give a 75 mm slump.

CONSTRUCTION

541.31 Preparation – Graded surfaces to receive paving shall be trimmed and well tamped to the lines and grades indicated

on the Drawings and to the approval of the Ministry Representative.

Edges of paving shall be constructed of thicker stones to form a stable foundation and protection against any undercutting in waterways.

All paved areas shall receive a minimum 150 mm layer of gravel base, well consolidated.

Gravel level inside curbs shall permit the surface of the jointing mortar to be flush with the top of the curb.

541.32 Construction – Construction shall be carried out with all material, labour, tools, equipment and incidentals supplied by the Contractor as necessary to complete all stone paving work in accordance with good work practice.

A gravel base of grouted stone paving shall be well saturated with bedding mortar slurry.

All rock shall be firmly embedded into the base material so that the voids between stones are of minimal width with the rock surfaces relatively even as indicated on SS Drawing SP541-01.

Voids of grouted paving rock shall be filled with setting and jointing mortar, 75 mm thick and, after consolidation and any necessary chinking of the larger voids, the mortar surface shall be smooth finished and sloped to drain freely.

Finished stone paving shall show a clean even appearance, free of disfiguring spalls and, where grouted, cleaned of mortar staining.

MEASUREMENT

541.81 General – Stone Paving work will be measured by the SQUARE METRE of stone in place.

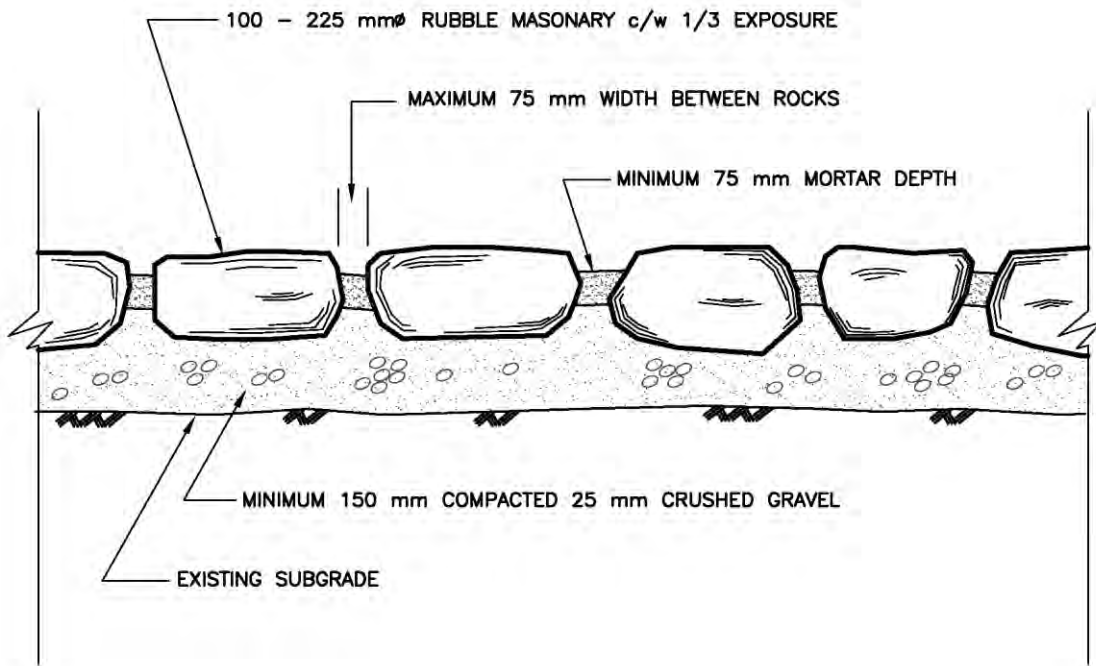
PAYMENT

541.91 General – Payment for STONE PAVING work will be at the Contract Unit Price per square metre of stone in place.

The Contract Unit Price shall be accepted as full compensation for the supply of the gravel base, stone and mortar, for complete installation, including trimming and incidental excavation, compaction, finishing, and for all subsidiary work not required to be separately paid for.

TYPICAL SECTION THRU ROCK PAVING

SP541-01



NOT TO SCALE

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

SECTION 582

CONCRETE CURB AND GUTTER AND STORM DRAINAGE

DESCRIPTION

582.01 Scope – This Section describes the requirements for construction of roadside curb and gutter and associated underground drainage systems. Such work shall consist of constructing and/or installing appurtenances shown or indicated on the drawings and Schedule of Approximate Quantities and Unit Prices.

582.02 Referenced Specifications – Unless otherwise specified, the most recent published metric edition applies.

AASHTO:

- M 306 Standard Specification for Drainage, Sewer, Utility, and Related Casting
- LRFD Bridge Design Specifications
- Standard Specifications for Highway Bridges

ASTM:

- A48 Standard Specification for Gray Iron Castings
- A536 Standard Specification for Ductile Iron Castings
- C14 Standard Specification for Nonreinforced Concrete Sewer, Storm Drain, and Culvert Pipe
- C76 Standard Specification for Reinforced Concrete Culvert, Storm Drain, and Sewer Pipe
- C309 Standard Specification for Liquid Membrane Forming Compounds for Curing Concrete
- C443 Standard Specification for Joints for Concrete Pipe and Manholes, Using Rubber Gaskets
- C478 Standard Specification for Circular Precast Reinforced Concrete Manhole Sections
- C506 Standard Specification for Reinforced Concrete Arch Culvert, Storm Drain and Sewer Pipe
- C507 Standard Specification for Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe
- C1218 Standard Test Method for Water-Soluble Chloride in Mortar and Concrete
- C1433 Standard Specification for Precast Reinforced Concrete Monolithic Box

Sections for Culverts, Storm Drains, and Sewers

- C1677 Standard Specification for Joints for Concrete Box, Using Rubber Gaskets

CSA:

- A23.2 Test Methods and standard practices for concrete
- A23.4 Precast Concrete – Materials and Construction
- A257 Standards for concrete pipe and manhole sections
- A3000 Cementitious Materials Compendium
- S6 Canadian Highway Bridge Design Code (CHBDC)

MATERIALS

582.10 Design of Underground Drainage System

582.10.01 Products – Underground drainage system products shall be designed in accordance with the applicable design codes, standards and referenced specifications, and the following:

- (a) **Highway Live Load** – Underground drainage system products shall be designed for highway design live load as follows unless otherwise specified herein:
- The design live load in accordance with the Ministry Supplement to the CHBDC S6, or
 - AASHTO LRFD Bridge Design Specifications HL-93 live load, or
 - AASHTO Standard Specifications for Highway Bridges H-20 or HS-20 live load
- (b) **Direct Contact Loading** – The highway design live load for components that may be subjected to direct contact with traffic wheels shall also include dynamic impact effects. The dynamic impact allowances shall be in accordance with the applicable design codes and standards.

582.11 Drain Pipe – All drain pipe shall be of first quality, sound, true in form and free from defects of all kinds.

582.12 Manufacture of Precast Concrete Storm Drainage Products – For all contracts awarded after December 31, 2021, all concrete pipe, box culverts, manholes, catch basins, precast concrete endwalls and associated precast concrete products shall be supplied from a manufacturer that is certified by an independent third party

certification agency to produce these products according to applicable CSA and ASTM manufacturing standards.

582.12.01 Certified Manufacturer – For all contracts awarded after December 31, 2021, the manufacturer shall be certified under at least one for the following quality assurance programs:

- (a) The Canadian Precast Concrete Quality Assurance ([CPCQA](#)) Certification Program.
- (b) [The CSA Group Testing & Certification Inc. \(CSA\) Certification Program](#)
- (c) [QCAST](#) Certification Program by the American Concrete Pipe Association for Precast Products.
- (d) National Precast Concrete Association ([NPCA](#)) Plant Certification.
- (e) Other certification organizations acceptable to the Ministry Representative that are accredited by the Standards Council of Canada for the certification of manufacturers for these products.

582.12.01 Product Markings – All products shall be marked with the certification body's logo confirming that the production of the product is in accordance with the quality and requirements of the specified standards for the products.

582.13 Concrete Pipe and Boxes

582.13.01 Reinforced Concrete Pipe – Reinforced concrete pipe shall be designed for flexible rubber gaskets and shall meet the requirements of CSA A257, ASTM C76, ASTM C506, ASTM C507, or ASTM C655.

582.13.02 Plain Concrete Pipe – Plain concrete pipe shall be designed for flexible rubber gaskets and shall meet the requirements of CSA A257 or ASTM C14.

582.13.03 Concrete Box Culverts – Concrete box culverts shall conform to the requirements of ASTM C1433 or ASTM C1577.

582.13.04 Gaskets – Gaskets shall be supplied with all concrete pipe and boxes and shall meet the requirements of CSA A257, ASTM C443, or ASTM C1677, all as appropriate to the application.

582.13.05 Requirements – Pipe and boxes shall be legibly marked with the date of manufacture, the name or trademark of the manufacturer and by marks denoting plain concrete pipe, reinforced concrete pipe or extra strength reinforced concrete pipe, as the case may be. Pipe containing elliptical reinforcement or otherwise requiring special placement shall be marked on the inside of the pipe with the words "Top" or "Bottom" at the correct place to indicate the proper position when installed.

Upon request, the Contractor shall furnish at the Contractor's expense such test and other information as may be required regarding the concrete pipe proposed to be used.

582.14 Corrugated Steel Pipe – Corrugated steel pipe shall conform to the requirements of SS 320.

582.15 Catch Basins and Manholes

582.15.01 Precast Reinforced Concrete – Precast reinforced concrete catch basin and manhole products, including but not limited to, barrels (risers), grade rings, flat slab tops (lids), conical sections, base sections, steps and ladders shall meet the requirements of ASTM C478 or CSA A257, and shall be gasketed or grouted to prevent infiltration and exfiltration.

582.15.02 Corrugated Metal – Corrugated metal barrels shall be in accordance with SS 320.

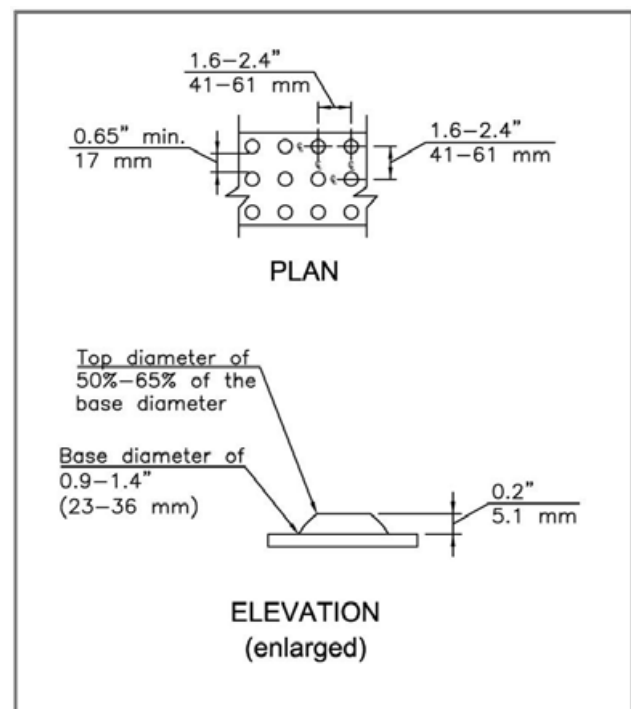
582.15.03 Covers and Frames – Catch basin and manhole covers and frames shall be cast iron and meet the requirements of ASTM A48. Catch basin and manhole covers and frames shall be designed for highway loading in accordance with either of the following:

- (a) AASHTO M 306 using the H-20 or HS-20 proof load testing with the proof load of 178 kN; or
- (b) The design live load in accordance with the Ministry Supplement to the CHBDC S6, including the dynamic impact allowance, with the wheel load application distributed over an area of 250 mm x 250 mm.

582.16 Detectable Warning Mats

582.16.01 General – Detectable warnings shall consist of a surface of truncated domes, sized and spaced as specified below and as shown in Figure 582-1.

Figure 582-1: Detectable Warning Mats – Dome Size and Spacing



582.16.02 Dome Size – Truncated domes in a detectable warning mat surface shall have a base diameter of 2 mm minimum and 36 mm maximum, a top diameter of 50 percent (50%) of the base diameter minimum to 65 percent (65%) of the base diameter maximum, and a height of 5.1 mm.

582.16.03 Dome Spacing – Truncated domes in a detectable warning mat surface shall have a center-to-center spacing of 41 mm minimum and 61 mm maximum, and a base-to-base spacing of 17 mm minimum, measured between the most adjacent domes on a square grid.

582.16.04 Contrast – Detectable warning mat surfaces shall contrast visually with adjacent walking surfaces either light-on-dark, or dark-on-light.

582.17 Cast-in-Place Concrete – All cast-in-place [CIP] concrete shall be in accordance with SS 211 except as modified herein.

582.17.01 CIP Concrete Materials – Concrete material properties shall be as shown on the SS 582 SP Drawings.

(a) **Cement Type** – Types GU or GUL, except where precluded as a result of Sulphate Exposure Class per SS 582.17.03.

(b) **Aggregate Testing** – Contrary to SS 211.03.04 Aggregate testing is required for Coarse and Fine aggregate only. Tests CSA A23.2-14A and CSA A23.2-25A are not required.

(c) **Slag** – Further to SS 211.04.02.03, slag shall conform to the physical and chemical requirements of CSA A3000 for ground granulated blast furnace Type S.

582.17.02 Concrete Mixes

(a) **CIP Concrete Mix Specifications** – All cast-in-place concrete used under this SS 582 shall have the properties specified in Table 582-A:

Table 582-A: Cast-in-Place Concrete Mix Properties

Criteria	Placement Method	
	Hand	Extruded
Minimum 28-Day Compressive Strength	32 MPa	
Minimum Cement Content	335 kg/m ³	
Nominal Maximum Size of Coarse Aggregate	20 mm	10 mm
Entrained Air	5 to 8 %	6 to 9%
Maximum Slump	80 mm	25 mm

(b) **Mix Design** – The Contractor shall submit the mix design documentation in accordance with SS 218, Concrete for Minor Works.

(c) **Trial Mixes** – Contrary to SS 211.03.05, trial mixes are not required.

582.17.03 Sulphate Exposure – The Ministry shall notify the Contractor if S-1 and/or S-2 exposure conditions exist for the soils present on-Site. If no information is provided by the Ministry, S-3 shall be assumed.

For imported bedding material, the aggregate supplier shall perform the sulphate and chloride exposure assessment of its aggregate in accordance with testing requirements provided in Table 582-B.

The test results will be used to determine the site-specific exposure level as defined in Table 3 of CSA A23.1, and the concrete mix component materials modified accordingly. Supplementary cementitious materials (SCM) shall comply with the restrictions as per Table 582-C.

Table 582-B: Testing Requirements for Sulphate Exposure

Bedding Material	Test Parameter	Applicable Test Methods
Recycled Concrete Aggregate	Sulphate content	CSA A23.2-8B
Other Aggregates	Sulphate content	CSA A23.2-3B
	Chloride Content	ASTM C1218*

* Note: Typically used for mortar or concrete but can be adapted for soil.

Table 582-C: SCM Restrictions by Cement Type

Sulphate Exposure Class	GU Cement	GUL Cement
S-1: Type F Fly ash and Type S Slag	With Ministry approval only	GUL Cement shall not be used if concrete is in contact with S1 soils or bedding
S-2: Type F Fly ash and Type S Slag	Min. 0% Max. 40%	Min. 25% Max. 40%
S-3: Type F Fly ash and Type S Slag	Min. 0% Max. 40%	Min. 15% Max. 40%

582.18 Self-Consolidating Concrete – Self-Consolidating Concrete (SCC) may be used in the manufacture of precast concrete pipe (reinforced or plain), box culverts, manhole and catch basin barrels, grade rings, flat slab tops, risers, conical sections, and base sections only under the following conditions:

582.18.01 SCC Limitations – SCC may only be used for products limited to the following sizes and specifications

- (a) pipes and barrels smaller than 3.0 m in any internal cross-sectional dimension manufactured in accordance with CSA A257, ASTM C14, C76, C478, C506, C507 or C655,
- (b) boxes with both internal spans and rise less than 3.0 m manufactured in accordance with ASTM C1433 or C1577,
- (c) with written approval from the Ministry Representative, culvert endwalls manufactured in accordance with CSA A23.4
 - (i) less than 2.0 m in exposed height; or
 - (ii) where Ministry unique structure numbers are precast in, up to 3.5 m of exposed height.

582.18.02 SCC Documentation – The Contractor shall provide the following for review upon request by the Ministry Representative:

- (a) a full copy of the applicable concrete mix design utilized in the manufacture of any product(s) intended to be incorporated into Ministry works; and
- (b) Documentation in accordance with either of the following:
 - (i) Details in accordance with CSA A23.2-24C documenting the proven history of successful performance for the required strength, durability, and other performance requirements of the proposed mix design for the manufacture of any product(s) intended to be incorporated into Ministry works, or
 - (ii) A full-scale test shall be used to verify the self-consolidating characteristics for placement and for the hardened concrete properties of the mix design for the manufacture of any product(s) intended to be incorporated into Ministry works. Documentation in accordance with CSA A23.2-24C shall be submitted demonstrating that the proposed mix design will achieve the required strength, durability, and performance requirements
- (c) The self-consolidating concrete mix shall conform to the requirements of CSA A23.1, including Clause 8.6 and Table 22, and the appropriate CSA and ASTM standards listed in SS 582.18.01 above.

CONSTRUCTION

582.31 Concrete Construction – All concrete work performed under this SS 582 shall meet the requirements of SS 211 except as modified herein and be constructed to the line and grade shown on the Drawings or as otherwise directed by the Ministry Representative. Combined concrete curb and gutter will include returns at the intersections if so directed.

The combined curb and gutter shall be constructed to the standard cross-section shown on SS Drawing SP582-01.01 and to such layout plans as are attached, except where the face of the curb varies and at lanes and driveways where drop curbs are ordered.

Expansion joints 13 mm in thickness cut to the cross-section of the concrete curb and sidewalk, composed of premoulded joint filler shall be installed at approximately 15 m intervals for sidewalks and 9 m intervals for concrete curb. They will also be installed when the sidewalk abuts a rigid structure or changes direction as well as at curb returns, either side of driveways, and other locations as directed by the Ministry Representative. Expansion and isolation joints shall be constructed to the full depth of the slab. Contraction joints shall be placed to a maximum of 3 m intervals, across both sidewalk and concrete curb and be tooled or saw-cut into the concrete to a depth of 1/3 the thickness of the concrete.

Concrete Curb horizontal and vertical alignment shall be in accordance with the Drawings within ± 10 mm, and such that there is no gap greater than 6 mm under a 3 m straight edge.

Ambient temperature forecasts used to trigger hot weather or cold weather concreting procedures shall be as provided by Environment Canada or another source acceptable to the Ministry Representative.

582.31.01 Delivery – Contrary to SS 211.08.03, the temperature of all concrete, from time of batching to complete discharge shall remain between 10°C and 32°C.

582.31.02 Sidewalk Finishing – All cast-in-place concrete shall be finished with a magnesium or wood float, to a closed, uniform surface. Sidewalks shall be textured transversely with a medium broom finish.

582.31.03 Curing – When ambient temperatures are, or are forecast to be, greater than 35°C at any time during the seven (7) days after placement, curing shall be done in accordance with SS 211.15.02.

At temperatures below 35°C, the Contractor shall provide concrete curing using:

- (a) Moist curing per SS 211.15.02; or
- (b) Poly-Alpha methyl-styrene (PAMS), Type 2, Class B, or Acrylic Cure and Seal, ASTM C309 based curing compound to all exposed concrete surfaces at rate

recommended by manufacturer in two (2) applications within 12 hours of placing concrete.

Resin- and wax-based curing compounds shall not be used.

582.31.04 Hot Weather Concreting – Hot Weather Concreting procedures shall be done in accordance with SS 211.18 (except as modified herein) when ambient air temperature is, or is forecast to be, 35°C or higher during the placing period.

Concrete shall not have a temperature greater than 25°C at any time between batching and placement. Concrete shall not be placed against any material/surface which is at a temperature above 35°C.

Curing shall be in accordance with SS 582.31.03 and commence as soon as possible.

582.31.05 Cold Weather Concreting – Cold Weather Concreting procedures shall be done in accordance with SS 211.19 (except as modified herein) when ambient air temperature is, or is forecast to be, below 5°C during placement, and/or is forecast to fall below 5°C during the first seven (7) days after placing.

Concrete shall not be placed against any material/surfaces which is at a temperature below 0°C.

Contrary to SS 211.19(a), the temperature of the concrete shall be between 15°C and 30°C, from the time of batching until placement.

Contrary to SS 211.19(e), the Contractor shall provide protection and heating if required, to:

- (a) maintain a uniform curing temperature, at a minimum 10°C, for at least three (3) days; and
- (b) protect the concrete from freezing for another four (4) days.

582.31.06 Repairs – Contrary to SS 211, superficial defects, including minor cracking and scaling, shall be repaired:

- (a) using materials from the Recognized Product List;
- (b) in accordance with the repair product manufacturer's recommendations; and
- (c) to the satisfaction of the Ministry Representative.

582.32 Catch Basins – Catch basins shall be constructed, where shown and as required in the Drawings or as the Ministry Representative may direct, in accordance with the intent of SS Drawings SP582-02.01 through SP582-02.05.

582.33 Manholes – Manholes shall be constructed at the locations and to invert levels shown in Drawings or as the Ministry Representative may direct in accordance with the intent of SS Drawings SP582-03.01 through SP582-03.08.

582.34 Storm Drains – Storm drains shall be constructed where shown on Drawings, or as directed by the Ministry Representative, and in accordance with the Drawings and specifications covering the various types and as attached herewith. The trench and other preparatory work shall be approved by the Ministry Representative before actual placing starts.

582.35 Placing

582.35.01 Spigot and Bell Pipe – Spigot and bell pipe shall be laid beginning at the lower end with the bell end pointing up-grade. Suitable excavation or bedding must be provided to accommodate the bell so that the pipe is supported along its full length and not at the ends alone.

582.35.02 Concrete Pipe – Concrete pipe shall be laid beginning at the lower end with the groove end pointing up-grade. Pipe with elliptical reinforcement shall be laid with the minor axis of the reinforcement as marked by the manufacturer in a vertical position.

Concrete pipe joints shall be sealed using a rubber gasket, installed in accordance with the pipe manufacturer's recommendations.

582.35.03 Corrugated Steel Pipe – Corrugated steel pipe shall be laid beginning at the lower end and, where applicable, with the outside laps pointing up-grade and the longitudinal joint on the side. The separate sections shall be firmly jointed together and any metal in joints which is not thoroughly protected by galvanizing shall be coated with two heavy coats of zinc rich paint selected from the Ministry's Recognized Products List under the category of "Additional Paint Coatings – Zinc-Rich Touch-up Paints and Primers".

582.35.04 General – Pipes shall be accurately set and laid to even gradients, concentric and in straight lines between manholes.

582.36 Bedding and Backfilling – The bedding of all concrete pipes shall consist of 19 mm or 25 mm crushed gravel to a minimum depth of 150 mm below the pipe. The bedding below the middle 1/3 of the pipe shall be left uncompacted for a depth 75 mm or 1/24th of the pipe diameter, whichever is greater.

The bedding for corrugated steel pipe shall be in accordance with SS 303.

The backfill shall consist of 19 mm or 25 mm crushed gravel with a minimum cover of 150 mm and in the remainder of the trench shall consist of the natural excavated material unless such material is considered unsuitable by the Ministry Representative. Such natural material shall have a maximum size of 75 mm. All materials shall be compacted in horizontal layers not more than 200 mm.

582.37 Concrete End Walls – See SS 303.23.

582.38 Detectable Warning Mats – Detectable warning mats shall be constructed where shown on Drawings, or as directed by the Ministry Representative, and in strict accordance with the recommendations of the manufacturer.

MEASUREMENT

582.81 Portland Cement Concrete Curb and Gutter – Portland Cement Concrete Curb and Gutter will be measured by the METRE.

582.82 Catch Basins – Catch Basins will be measured per EACH.

582.83 Manholes – Manholes will be measured per EACH.

The Contractor shall allow in the bid price for the construction of manholes having an overall depth of concrete section of 2 m (see dimensions marked "variable"). In addition, the Contractor shall bid a price per 0.3 m of depth of manhole, which shall be added to or subtracted from the bid price for the manhole depending on the actual depth of the manhole to be constructed.

582.84 Storm Drains – Storm drains, including box culverts, will be measured by the LINEAR METRE in place.

582.85 Bedding and Backfill – Bedding and backfill will be measured by the CUBIC METRE in place.

582.86 Concrete End Walls – See SS 303.81.

582.87 Detectable Warning Mats – Detectable warning mats will be measured by the SQUARE METRE in place.

PAYMENT

582.91 Portland Cement Concrete Curb and Gutter – Payment for combined Portland cement CONCRETE CURB AND GUTTER will be at the Contract Unit Price per metre. The Contract Unit Price shall include everything

furnished and done in connection with form setting, supply and placing of air entrained concrete; supply and installation of expansion joints; finishing, curing, protection and superintendence.

582.92 Catch Basins – Payment for CATCH BASINS will be at the Contract Unit Price per each. The Contract Unit Price shall include everything furnished and done; excavation, backfilling and tamping around catch basins, formwork, concreting, setting in of pipe drains, setting and securing of cast iron catch basin frames and gratings, steel baffles, reinstatement of surface, cleaning out, protection and superintendence and, all other things necessary.

582.93 Manholes – Payment for MANHOLES will be at the Contract Unit Price per standard 2 m manhole with appropriate depth correction as described in SS 582.83. The Contract Unit Price shall include everything furnished and done; formwork, concreting, setting in of storm drains, construction of smooth invert sections, cement mortar settings, steel ladder rungs, brick construction, setting of manhole frames and covers, cleaning out, protection and superintendence, and all other things necessary.

582.94 Storm Drains – Payment for STORM DRAINS will be at the Contract Unit Price per linear metre in place. The Contract Unit Price shall be accepted as full compensation for everything furnished and everything done in connection herewith, but shall not include payment for excavation, riprap, paving, concrete cradles, end walls, bedding and backfill. These shall be paid for at their respective prices in the Schedule of Approximate Quantities and Unit Prices.

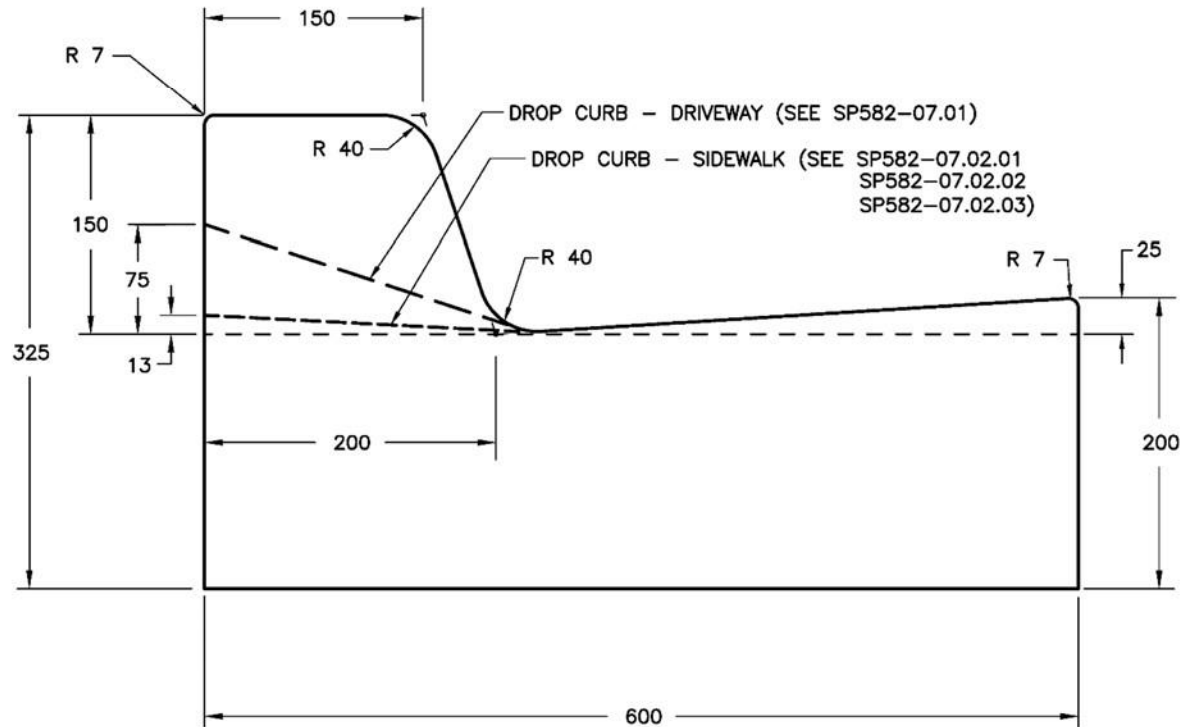
582.95 Bedding and Backfill – Payment for BEDDING and BACKFILL will be at the Contract Unit Price per cubic metre in place.

582.96 Concrete End Walls – See SS 303.91.

582.97 Detectable Warning Mats – Payment for DETECTABLE WARNING MATS will be at the Contract Unit Price per square metre in place.

COMBINED CURB AND GUTTER

SP582-01.01

**NOTES:**

- THE CONCRETE INCORPORATED IN THE CURB SHALL HAVE:
 - MINIMUM COMPRESSIVE STRENGTH OF 32 MPa AT 28 DAYS;
 - COARSE AGGREGATE OF MAXIMUM PARTICLE SIZE;
 - HAND PLACED: 20 mm
 - EXTRUDED: 10 mm
 - MINIMUM CEMENT CONTENT OF 335 kg/m³;
 - ENTRAINED AIR: MAX 10 mm AGGREGATE - 6% TO 9%
MAX 20 mm AGGREGATE - 5% TO 8%
 - SLUMP: HAND PLACED: 65 mm TO 80 mm
EXTRUDED: 0 TO 25mm.
- SULFATE EXPOSURE CLASS TO DETERMINE CEMENT TYPE AND CONTENT OF FLY ASH AND SLAG AS PER SS 582.17.
- CONTRACTION JOINTS SHALL BE PLACED AT MAXIMUM OF 3 m INTERVALS ACROSS THE CURB, TOOLED INTO THE CONCRETE TO A DEPTH OF 1/3 THE THICKNESS OF THE CONCRETE.
- EXPANSION JOINTS 9 mm IN THICKNESS, CUT TO THE CROSS-SECTION OF THE COMBINED CURB AND GUTTER, COMPOSED OF PREMOULDED JOINT FILLER, SHALL BE INSTALLED AT APPROXIMATELY 15 m INTERVALS AS WELL AS AT CURB RETURNS AND EITHER SIDE OF DRIVEWAYS. EXPANSION JOINTS SHALL ALSO BE CONSTRUCTED IN THE CURB AND GUTTER WHERE AN ADJACENT SIDEWALK ABUTS A RIGID STRUCTURE OR CHANGES DIRECTION. EXPANSION AND ISOLATION JOINTS SHALL BE CONSTRUCTED TO THE FULL DEPTH OF THE SLAB.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

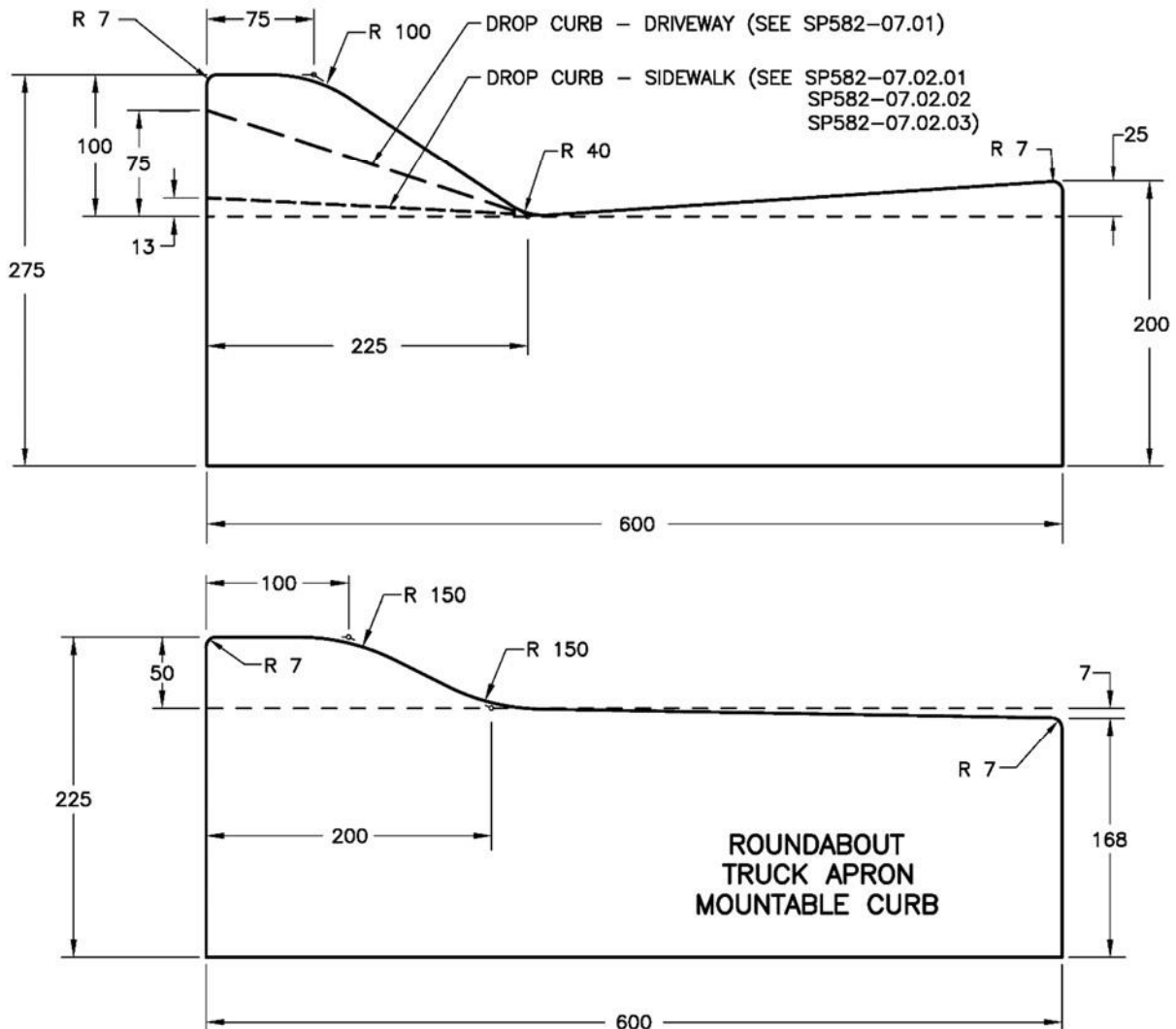


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MOUNTABLE CURB AND GUTTER

SP582-01.02



NOTES:

1. THE CONCRETE INCORPORATED IN THE CURB SHALL HAVE:
 - MINIMUM COMPRESSIVE STRENGTH OF 32 MPa AT 28 DAYS;
 - COARSE AGGREGATE OF MAXIMUM PARTICLE SIZE;
 - HAND PLACED: 20 mm
 - EXTRUDED: 10 mm
 - MINIMUM CEMENT CONTENT OF 335 kg/m³;
 - ENTRAINED AIR: MAX 10 mm AGGREGATE - 6% TO 9%
MAX 20 mm AGGREGATE - 5% TO 8%
 - SLUMP: HAND PLACED: 65 mm TO 80 mm
EXTRUDED: 0 TO 25mm.
2. SULFATE EXPOSURE CLASS TO DETERMINE CEMENT TYPE AND CONTENT OF FLY ASH AND SLAG AS PER SS 582.17.
3. CONTRACTION JOINTS SHALL BE PLACED AT MAXIMUM OF 3 m INTERVALS ACROSS THE CURB, TOOLED INTO THE CONCRETE TO A DEPTH OF 1/3 THE THICKNESS OF THE CONCRETE.
4. EXPANSION JOINTS 13 mm IN THICKNESS, CUT TO THE CROSS-SECTION OF THE COMBINED CURB AND GUTTER, COMPOSED OF PREMOULDED JOINT FILLER, SHALL BE INSTALLED AT APPROXIMATELY 9 m INTERVALS AS WELL AS AT CURB RETURNS AND EITHER SIDE OF DRIVEWAYS. EXPANSION JOINTS SHALL ALSO BE CONSTRUCTED IN THE CURB AND GUTTER WHERE AN ADJACENT SIDEWALK ABUTS A RIGID STRUCTURE OR CHANGES DIRECTION. EXPANSION AND ISOLATION JOINTS SHALL BE CONSTRUCTED TO THE FULL DEPTH OF THE SLAB.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

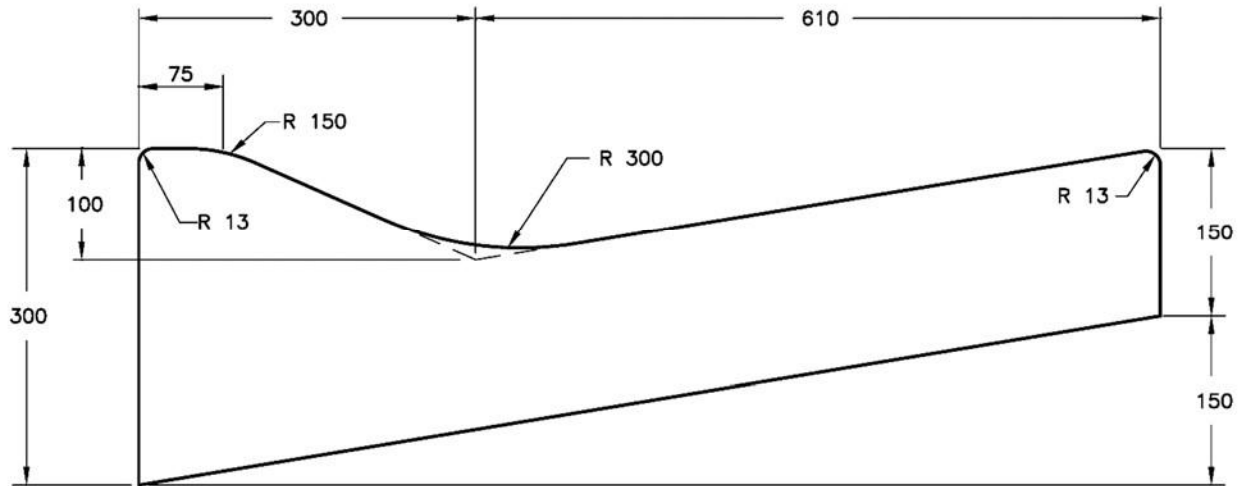


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VALLEY CURB AND GUTTER

SP582-01.03



NOTES:

1. THE CONCRETE INCORPORATED IN THE CURB SHALL HAVE:
 - MINIMUM COMPRESSIVE STRENGTH OF 32 MPa AT 28 DAYS;
 - COARSE AGGREGATE OF MAXIMUM PARTICLE SIZE;
 - HAND PLACED: 20 mm
 - EXTRUDED: 10 mm
 - MINIMUM CEMENT CONTENT OF 335 kg/m³;
 - ENTRAINED AIR: MAX 10 mm AGGREGATE - 6% TO 9%
MAX 20 mm AGGREGATE - 5% TO 8%
 - SLUMP: HAND PLACED: 65 mm TO 80 mm
EXTRUDED: 0 TO 25mm.
2. SULFATE EXPOSURE CLASS TO DETERMINE CEMENT TYPE AND CONTENT OF FLY ASH AND SLAG AS PER SS 582.17.
3. CONTRACTION JOINTS SHALL BE PLACED AT MAXIMUM OF 3 m INTERVALS ACROSS THE CURB, TOOLED INTO THE CONCRETE TO A DEPTH OF 1/3 THE THICKNESS OF THE CONCRETE.
4. EXPANSION JOINTS 13 mm IN THICKNESS, CUT TO THE CROSS-SECTION OF THE COMBINED CURB AND GUTTER, COMPOSED OF PREMOULDED JOINT FILLER, SHALL BE INSTALLED AT APPROXIMATELY 9 m INTERVALS AS WELL AS AT CURB RETURNS AND EITHER SIDE OF DRIVEWAYS. EXPANSION JOINTS SHALL ALSO BE CONSTRUCTED IN THE CURB AND GUTTER WHERE AN ADJACENT SIDEWALK ABUTS A RIGID STRUCTURE OR CHANGES DIRECTION. EXPANSION AND ISOLATION JOINTS SHALL BE CONSTRUCTED TO THE FULL DEPTH OF THE SLAB.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

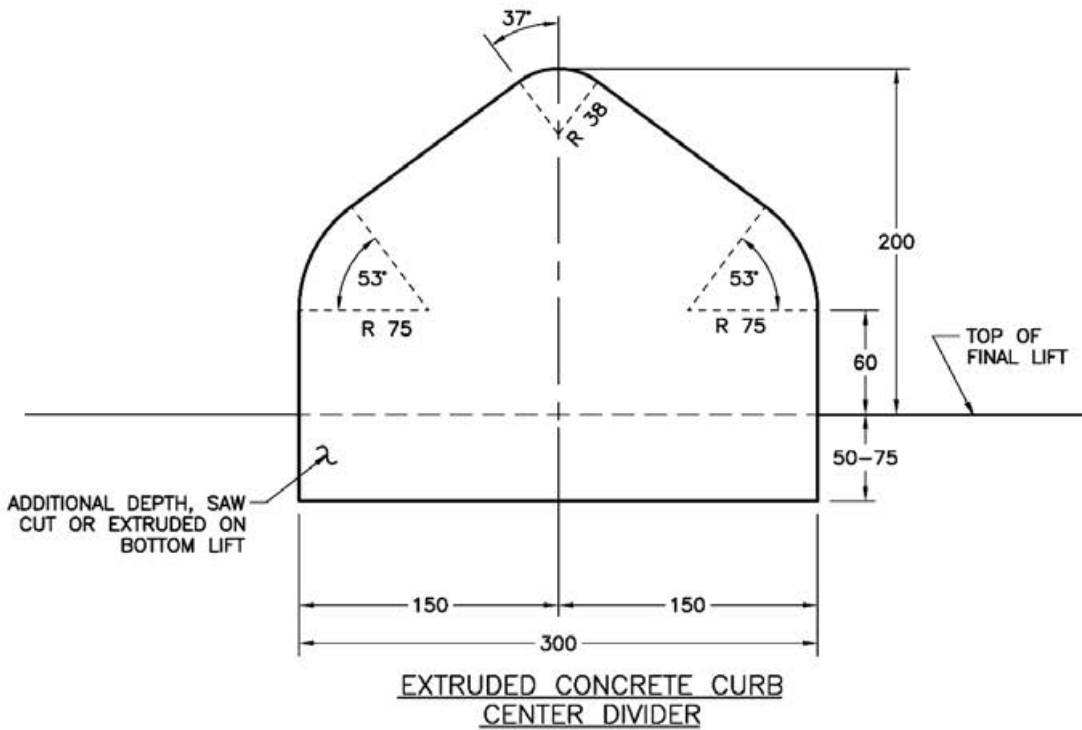
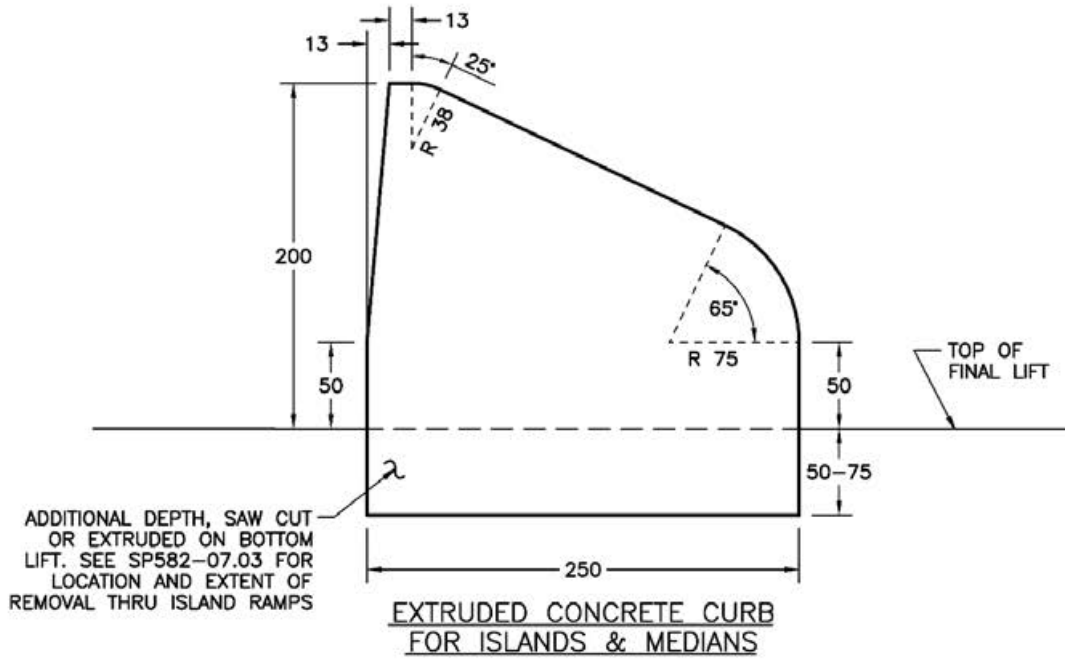


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ISLAND OR MEDIAN CURB

SP582-01.04



TYPICAL SECTIONS

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES

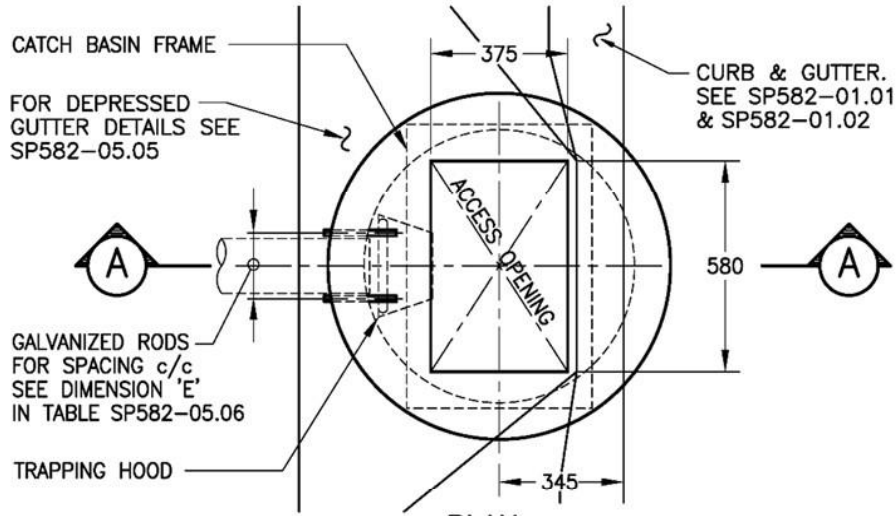


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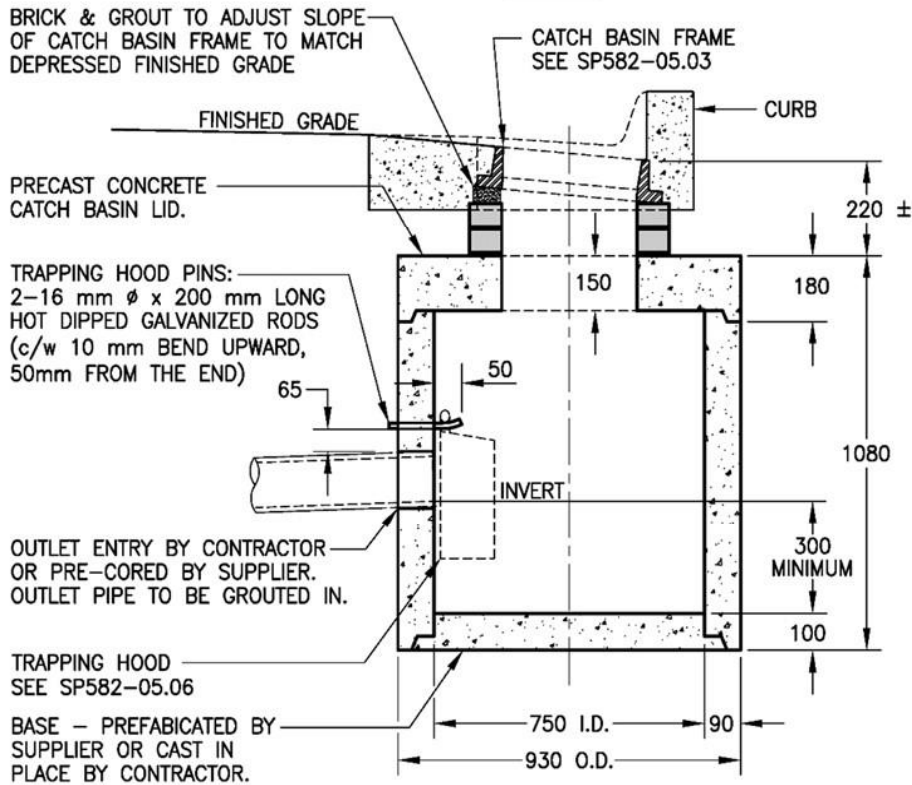
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PRECAST REINFORCED CONCRETE CATCH BASIN

SP582-02.01



PLAN



SECTION A

NOTES:

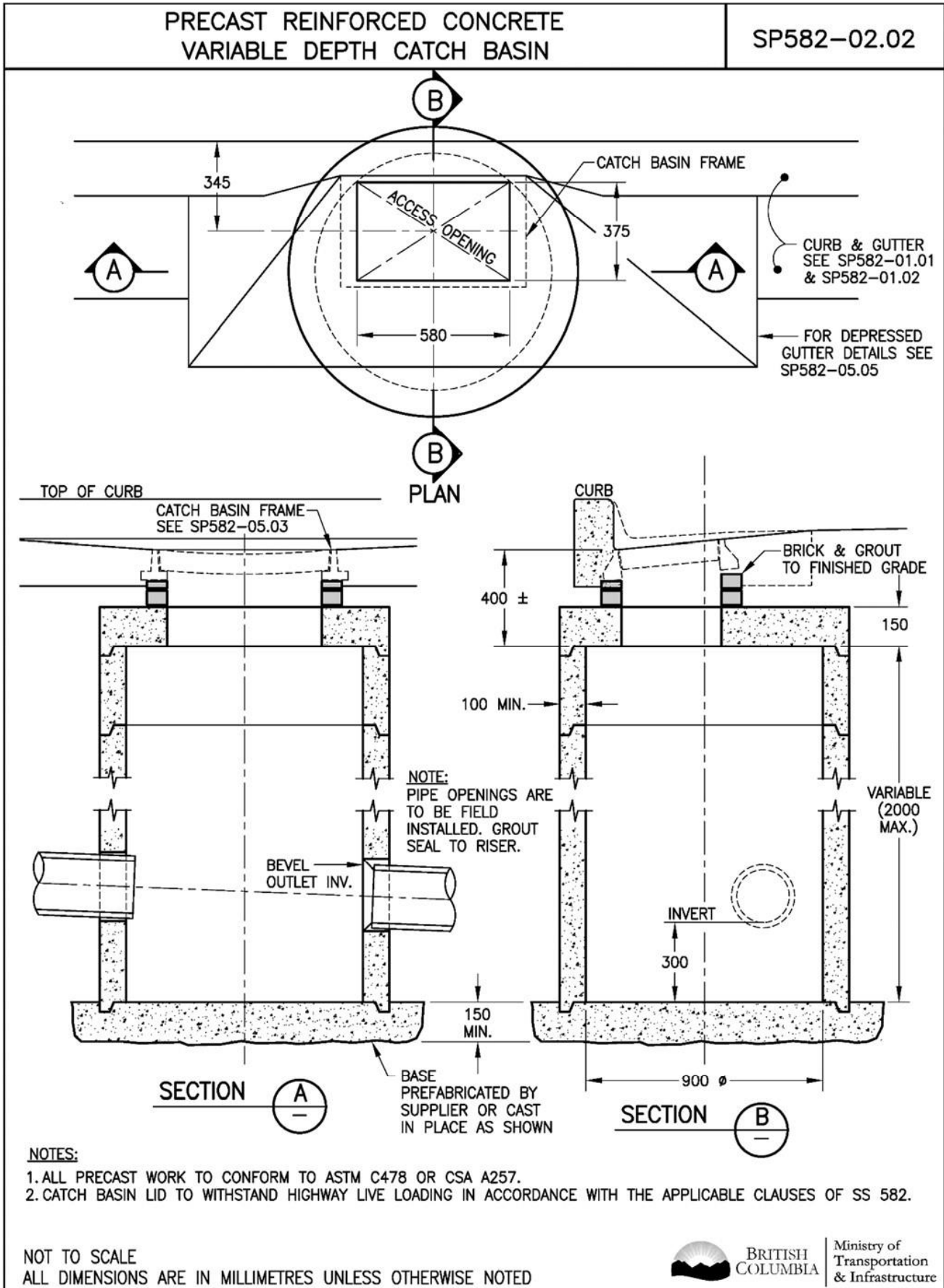
1. ALL PRECAST WORK TO CONFORM TO ASTM C478 OR CSA A257.
2. CATCH BASIN LID TO WITHSTAND HIGHWAY LIVE LOADING IN ACCORDANCE WITH THE APPLICABLE CLAUSES OF SS 582.
3. TRAPPING HOOD PINS SHALL BE SECURELY FASTENED TO THE PRECAST CONCRETE CATCH BASIN BARREL

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



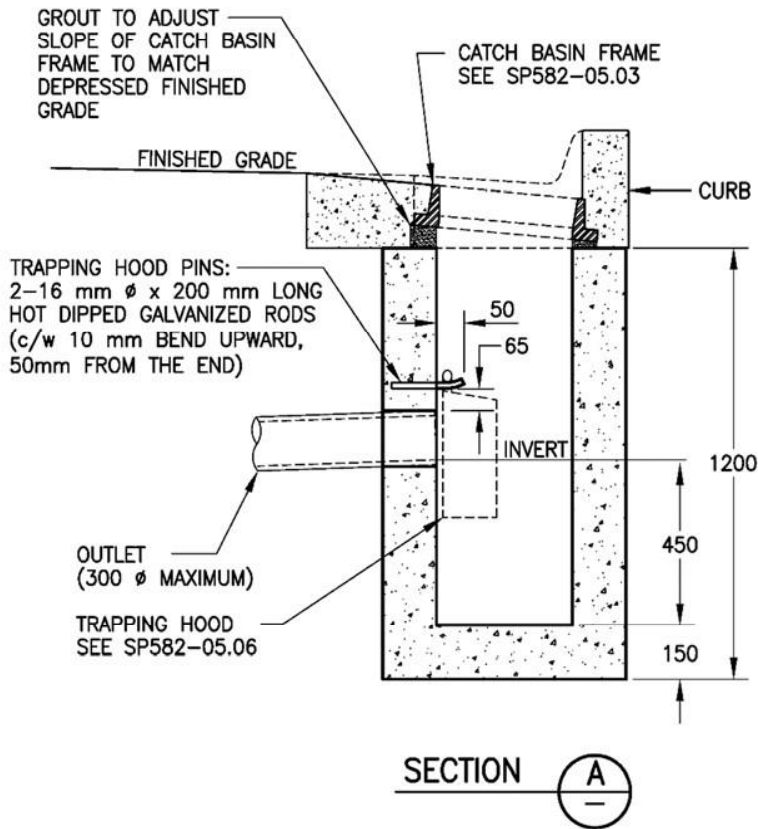
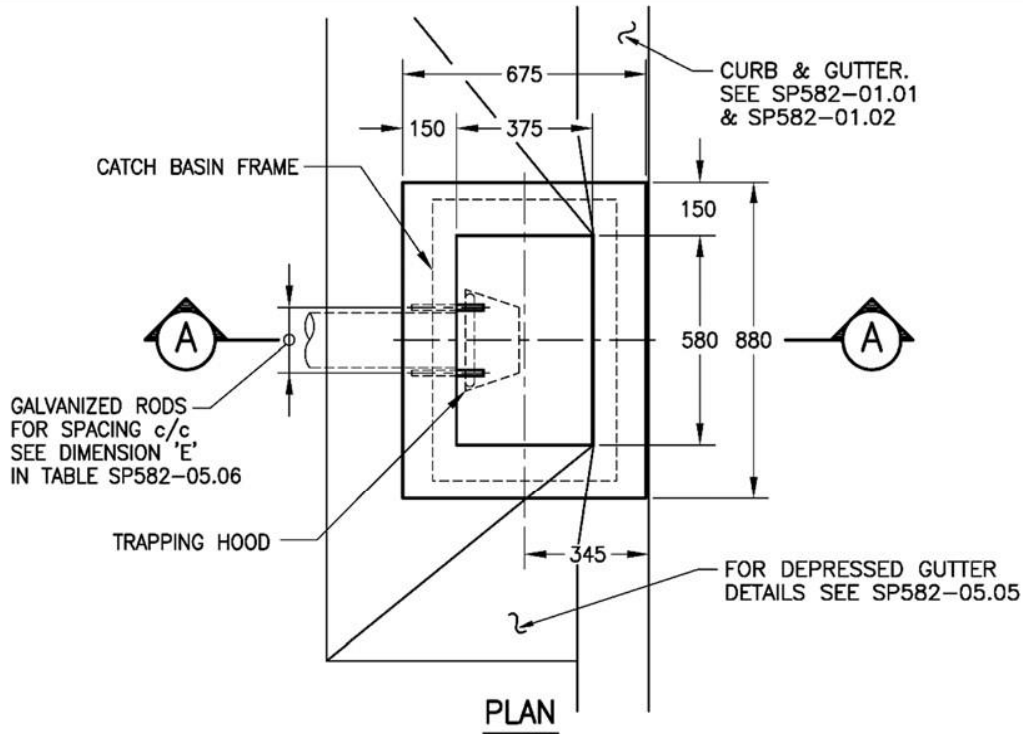
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CAST IN PLACE CONCRETE CATCH BASIN

SP582-02.03



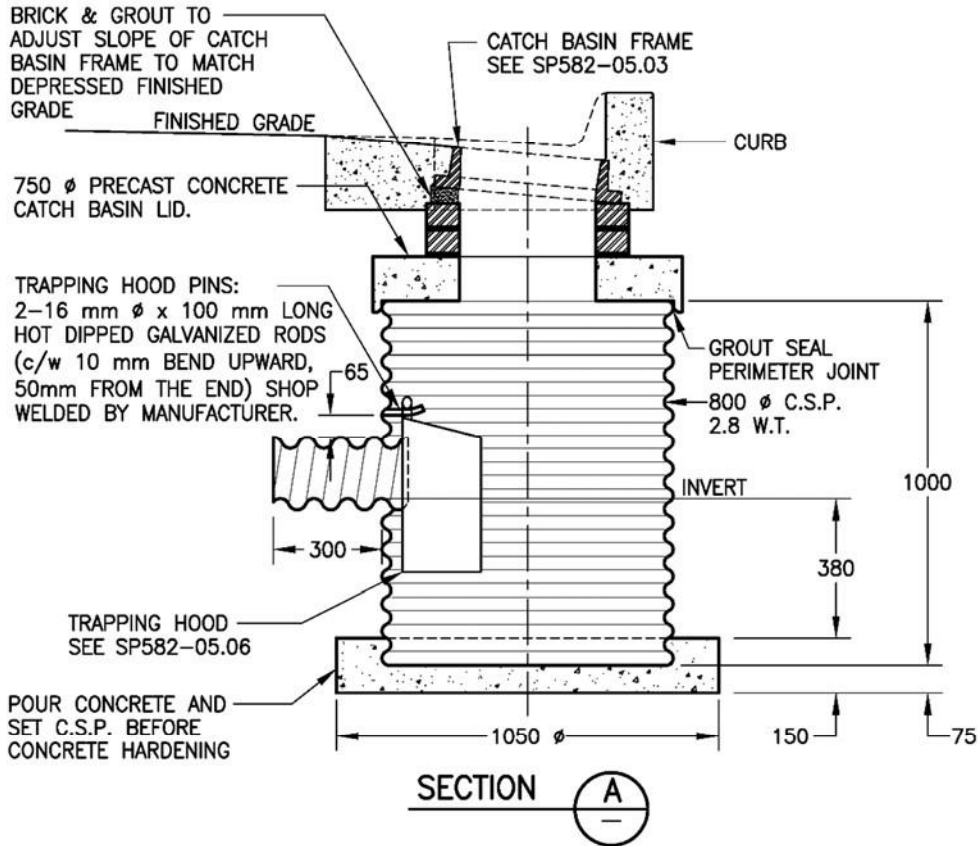
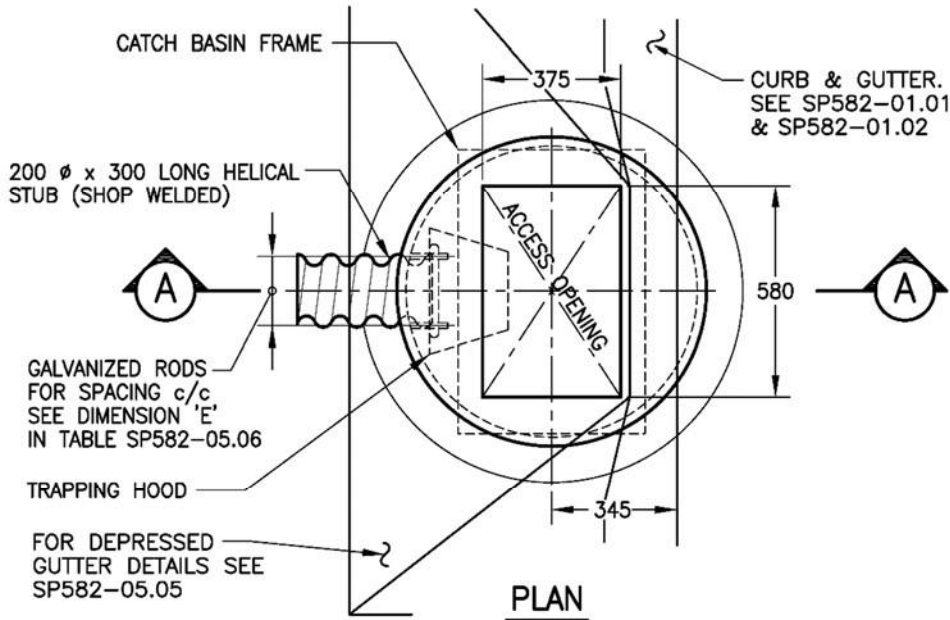
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



Ministry of
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CORRUGATED STEEL CATCH BASIN

SP582-02.04



NOTES:

1. ALL PRECAST WORK TO CONFORM TO ASTM C478 OR CSA A257.
2. CATCH BASIN LID TO WITHSTAND HIGHWAY LIVE LOADING IN ACCORDANCE WITH THE APPLICABLE CLAUSES OF SS 582.

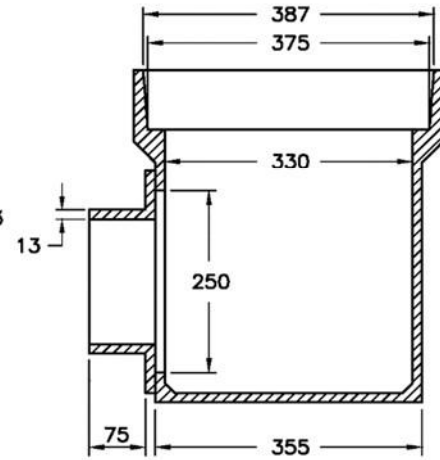
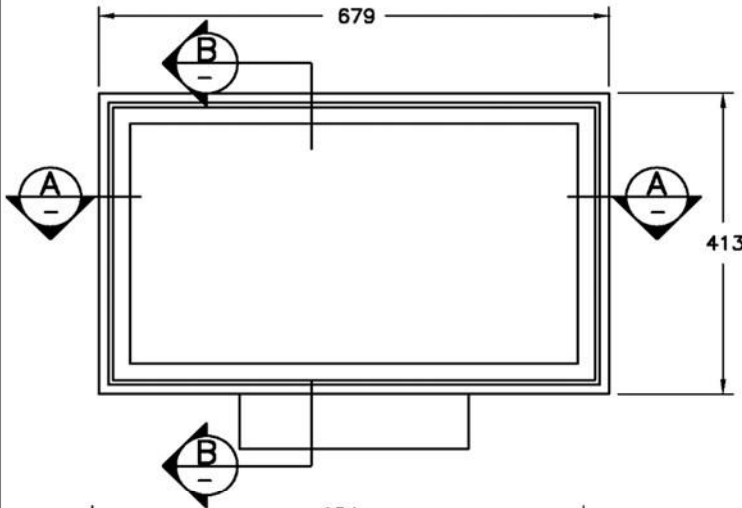
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



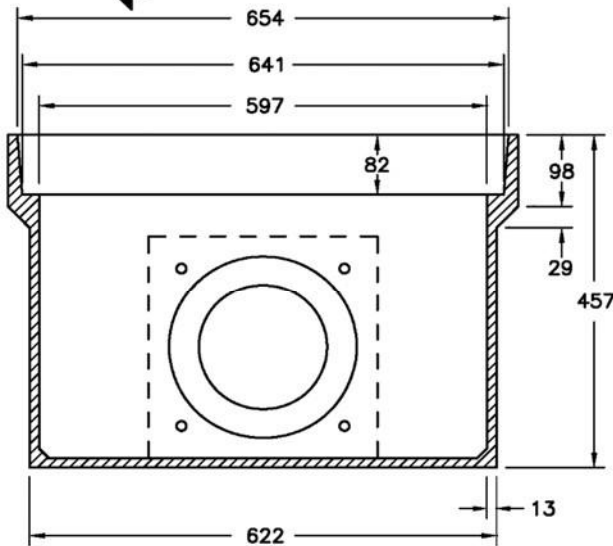
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CAST IRON CATCH BASIN BOX AND ADAPTOR PLATE

SP582-02.05

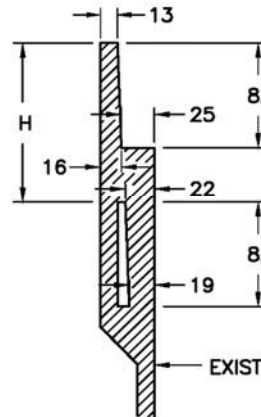


SECTION B



SECTION A

NOTE:
 THIS CATCH BASIN BOX IS TO BE USED IN CONJUNCTION WITH A STANDARD CATCH BASIN GRATE (SEE SP582-05.01 OR SP582-05.02). MATERIAL TO BE CAST STEEL OR CAST IRON AS ORDERED. APPROXIMATE WEIGHT 136 kg

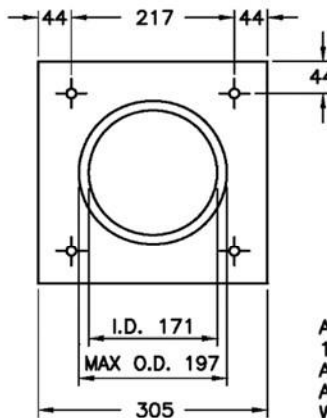


HEIGHT H TYPE A....100
 TYPE B....125
 TYPE C....150

OUTSIDE DIMENSIONS OF CAST IRON EXTENSION ARE 679 x 413

EXISTING CAST IRON CATCH BASIN BOX

EXTENSION FOR CAST IRON CATCH BASIN BOX



ADAPTOR PLATE

ADAPTOR PLATE TO BE DRILLED TO CLEAR 13 mm STUDS. THE FACE OF THE BOX AND PLATE TO BE GROUND SMOOTH, AND A RUBBER GASKET USED TO ENSURE A WATER TIGHT JOINT.

NOT TO SCALE
 ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

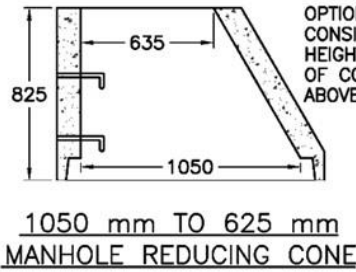


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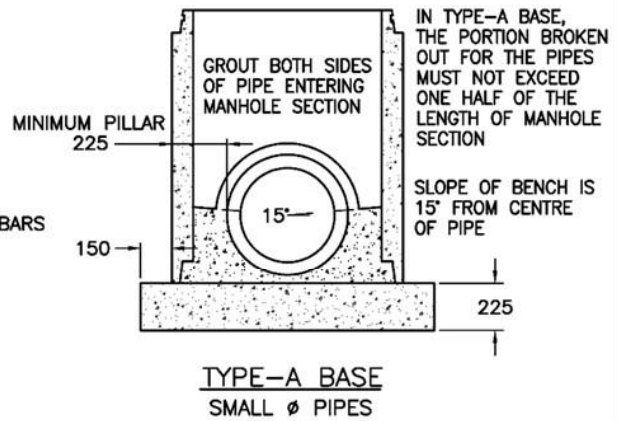
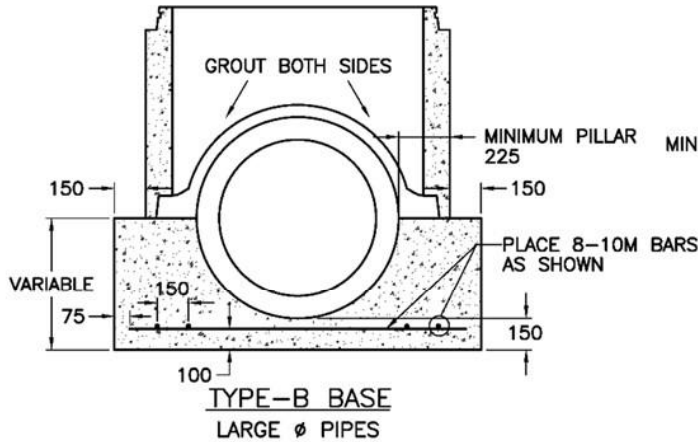
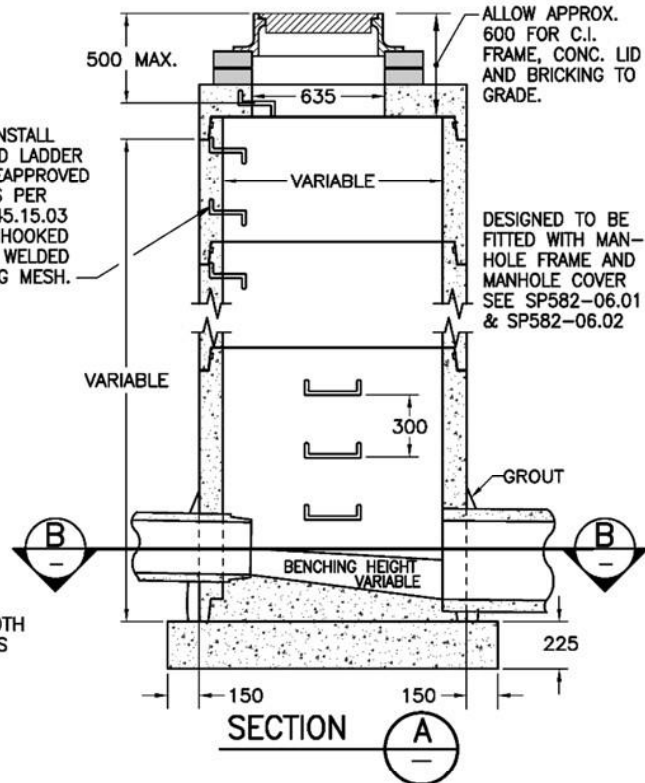
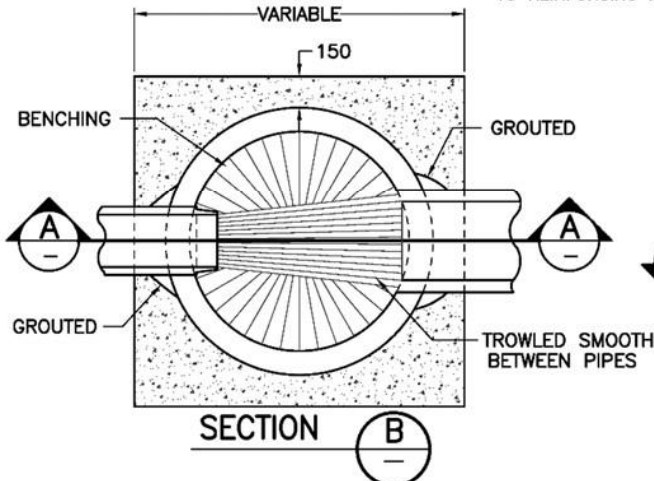
PRECAST REINFORCED CONCRETE MANHOLE

SP582-03.01



OPTIONAL BY DESIGNER. CONSIDERED ONLY WHEN HEIGHT ALLOWS BOTTOM OF CONE TO BE 2 m ABOVE INVERT.

SUPPLIER TO INSTALL 19Ø GALVANIZED LADDER RUNGS OR PREAPPROVED ALTERNATIVE AS PER SUBSECTION 145.15.03 RUNGS TO BE HOOKED AS SHOWN OR WELDED TO REINFORCING MESH.



CONSTRUCTION SEQUENCE

1. POUR A SQUARE CONCRETE BASE 225 mm THICK AND 300 mm WIDER THAN THE OUTSIDE OF THE MANHOLE SECTION.
2. BREAK PIPE ENTRANCES INTO MANHOLE SECTION, SET MANHOLE SECTION OVER THE PIPES ON THE CONCRETE BASE.
3. POUR A CONCRETE BENCH TO THE CENTRE POINT OF THE PIPE AND SLOPE OF 15°.
4. THOROUGHLY GROUT AROUND PIPES ENTERING MANHOLES.

NOTES:

1. ALL PRECAST WORK TO CONFORM TO ASTM C478 OR CSA A257.
2. MANHOLE LID TO WITHSTAND HIGHWAY LIVE LOADING IN ACCORDANCE WITH THE APPLICABLE CLAUSES OF SS 582.

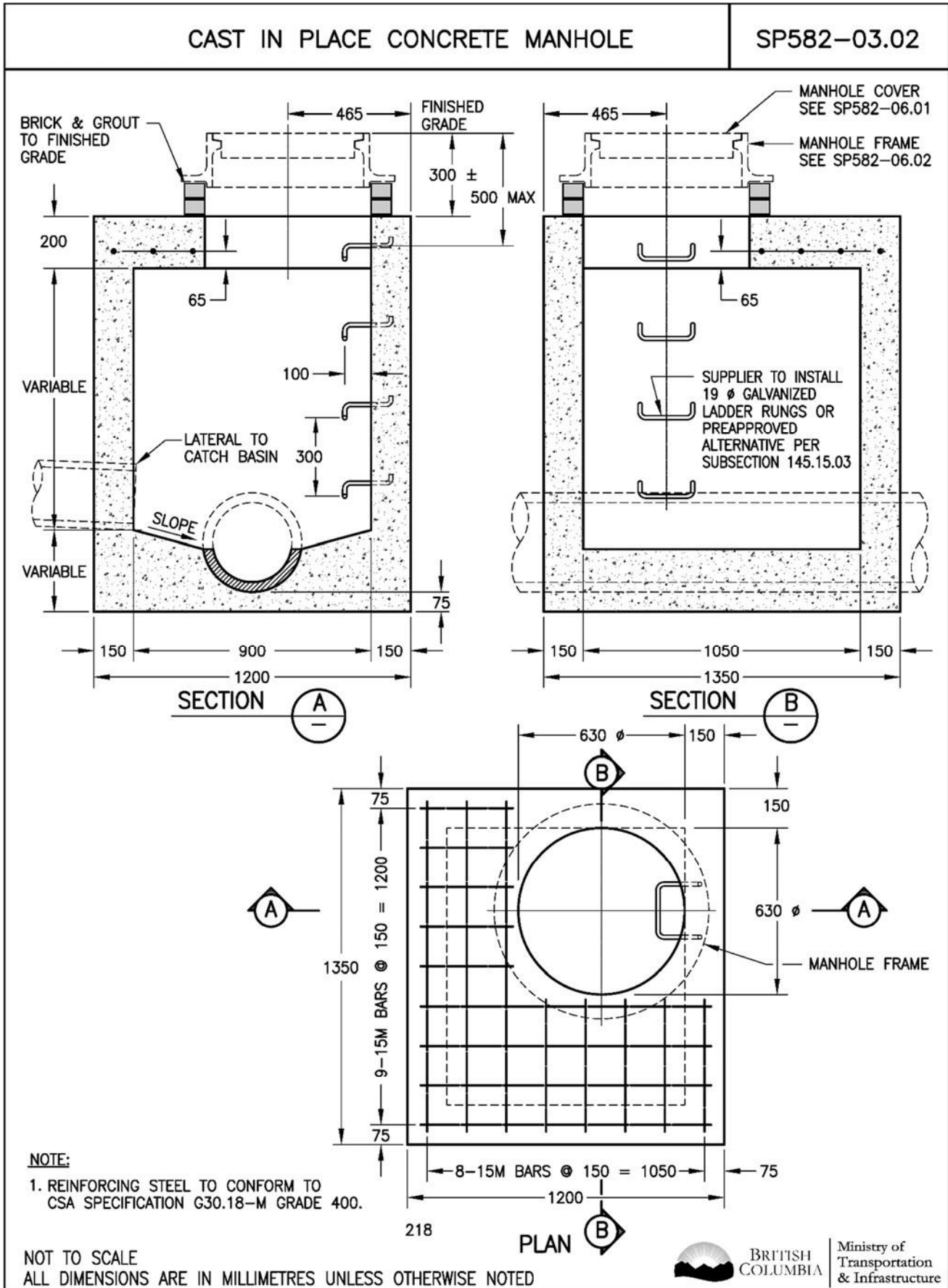
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

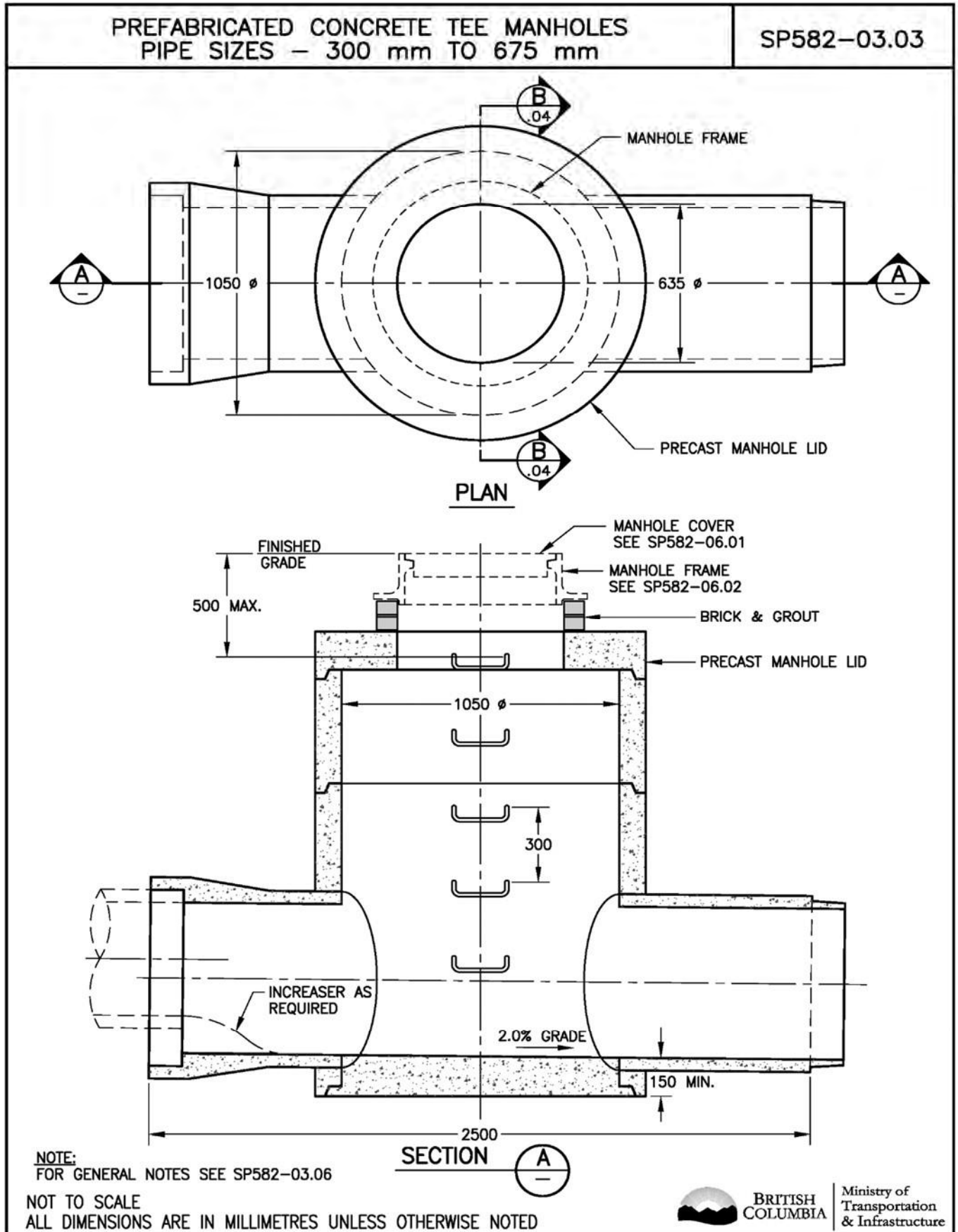
MANHOLE INSIDE DIA.	MAX. PIPE	
	BASE "A"	BASE "B"
750	375	375
900	530	530
1050	610	685
1200		760
1350		915
1500		1065
1800		1370



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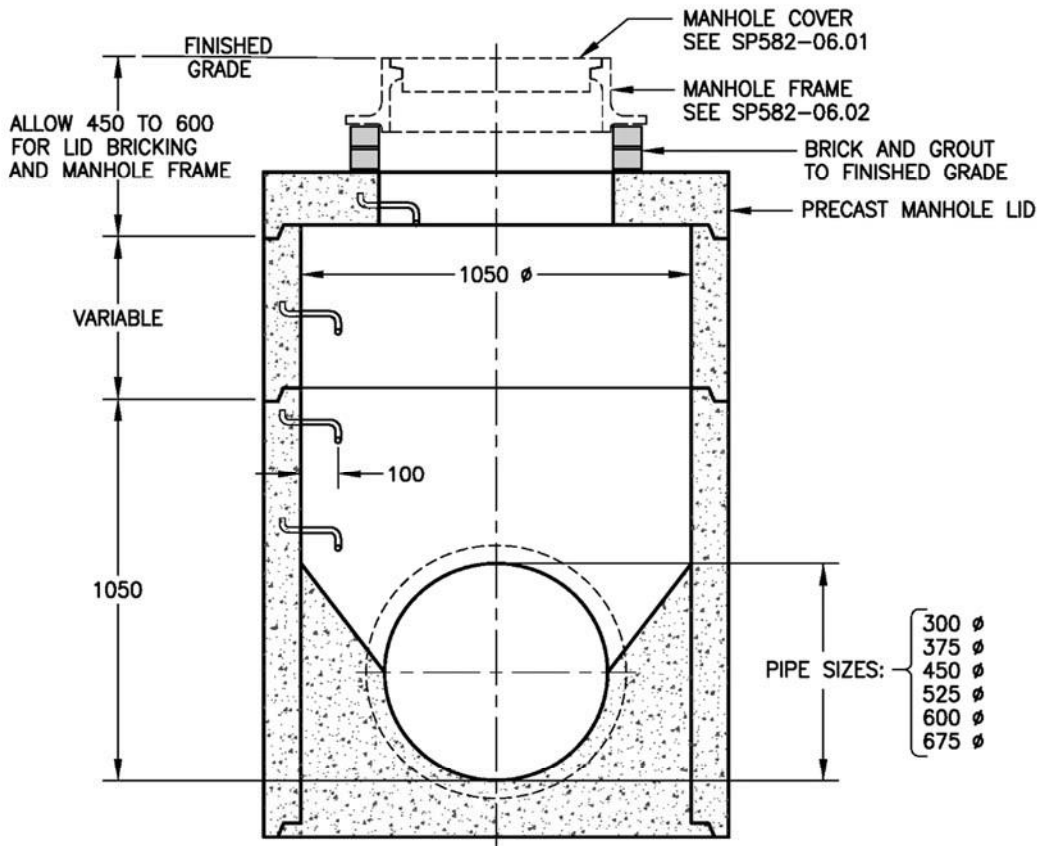
Ministry of Transportation & Infrastructure





PREFABRICATED CONCRETE TEE MANHOLES
PIPE SIZES – 300 mm TO 675 mm

SP582-03.04



SECTION **B**
.03

NOTE: FOR LOCATION OF SECTION
SEE SP582-03.03

NOTE:
FOR GENERAL NOTES SEE SP582-03.06

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

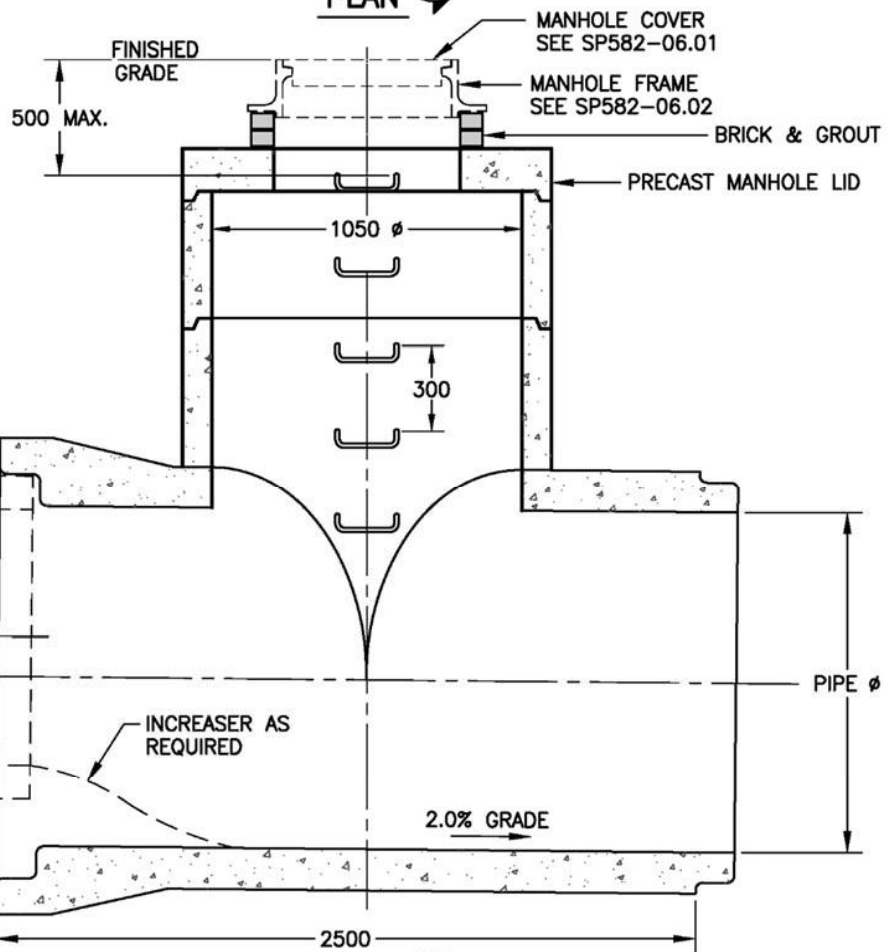
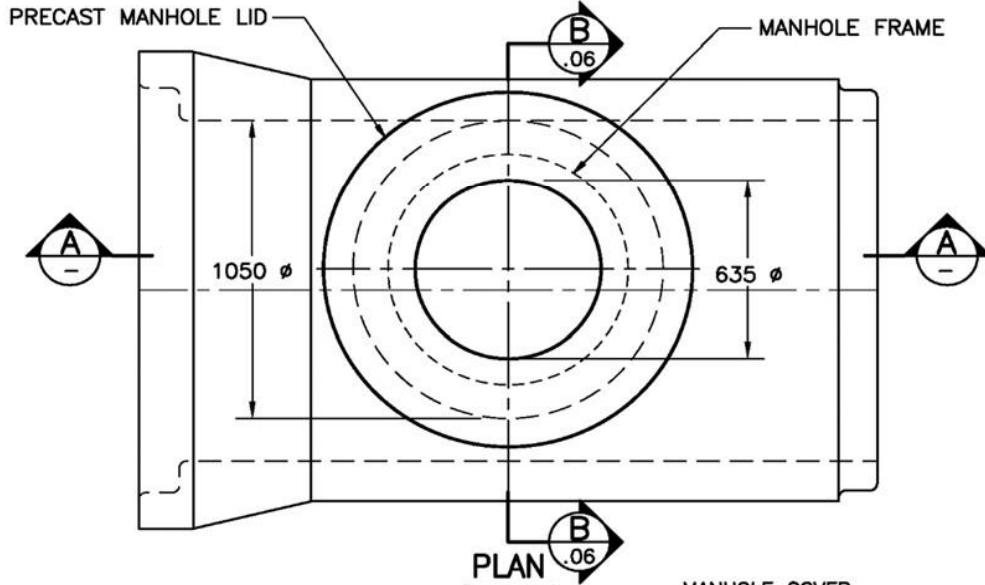


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PREFABRICATED CONCRETE TEE MANHOLES
PIPE SIZES – 750 mm TO 3050 mm

SP582-03.05



NOTE:
FOR GENERAL NOTES SEE SP582-03.06

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

SECTION A

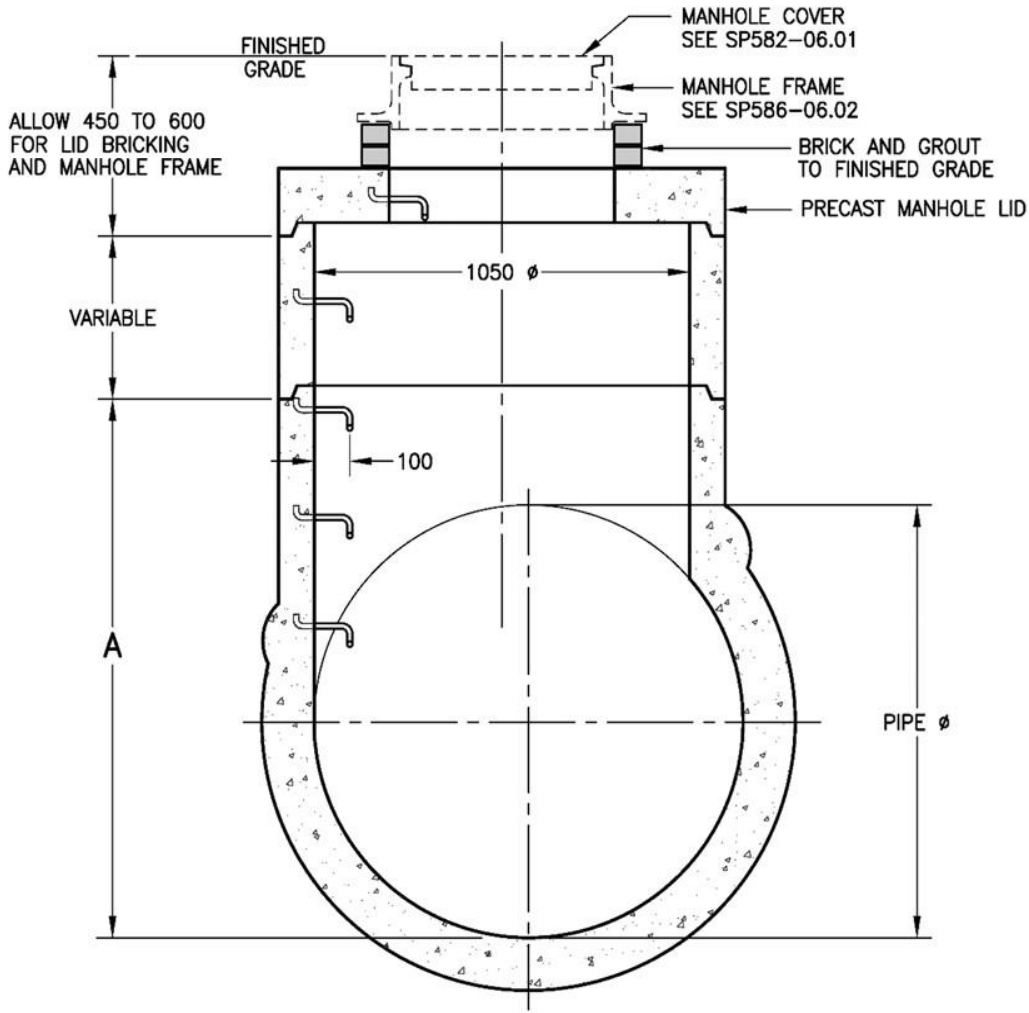


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Ministry of Transportation & Infrastructure

PREFABRICATED CONCRETE TEE MANHOLES
PIPE SIZES – 750 mm TO 3050

SP582-03.06



SECTION **B**
.05

NOTE: FOR LOCATION OF SECTION
SEE SP582-03.05

NOTES:

1. SUPPLIER TO INSTALL 19 Ø GALVANIZED LADDER RUNGS OR PREAPPROVED ALTERNATIVE AS PER SUBSECTION 145.15.03
2. CONCRETE PIPE TO CONFORM TO ASTM C 76.
3. ALL PRECAST WORK TO CONFORM TO ASTM C478 OR CSA A257.
4. MANHOLE LID TO WITHSTAND HIGHWAY LIVE LOADING IN ACCORDANCE WITH THE APPLICABLE CLAUSES OF SS 582.
5. MANHOLE TEE SECTION SHALL BE BEDDED AS PER SUBSECTION 582.37.
6. MANHOLE TEE SECTIONS MAY BE ORDERED AS INCREASERS ON THE UPSTREAM END TO ALLOW FOR PIPE SIZE CHANGES.

PIPE Ø	A
750	1270
900	1400
1050	1550
1200	1720
1350	1880
1500	1970
1800	2210

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

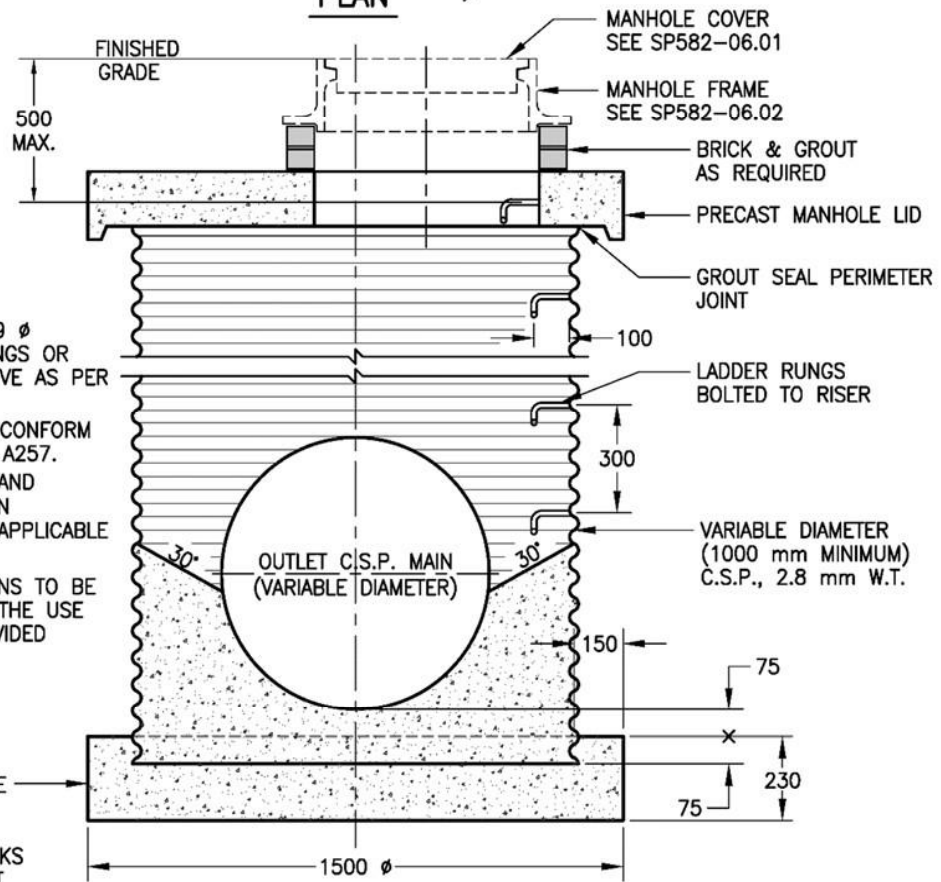
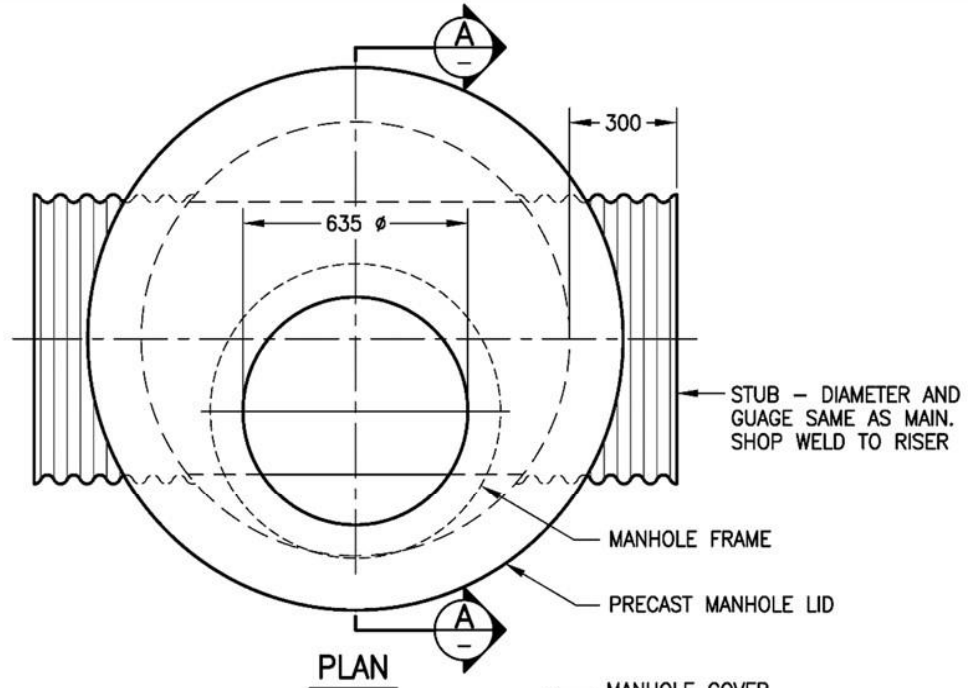


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CORRUGATED STEEL MANHOLE MAIN SMALLER THAN RISER

SP582-03.07



NOTES:

1. SUPPLIER TO INSTALL 19 ϕ GALVANIZED LADDER RUNGS OR PREAPPROVED ALTERNATIVE AS PER SUBSECTION 145.15.03
2. ALL PRECAST WORK TO CONFORM TO ASTM C478 OR CSA A257.
3. MANHOLE LID TO WITHSTAND HIGHWAY LIVE LOADING IN ACCORDANCE WITH THE APPLICABLE CLAUSES OF SS 582.
4. ALL LATERAL CONNECTIONS TO BE MADE IN THE FIELD BY THE USE OF APPROPRIATELY PROVIDED SADDLES.

REINFORCED CONC. BASE TO BE POURED DURING THE INSTALLATION. USE BRICKS OR CONC. BLOCKS AS TEMPORARY SUPPORT FOR VERTICAL BARREL.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



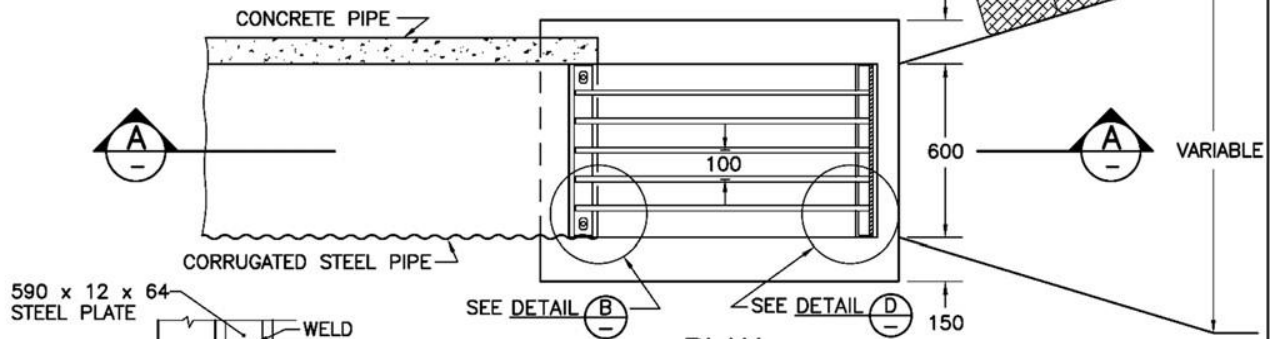
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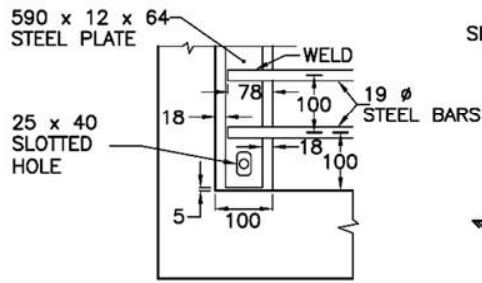
CAST IN PLACE INLET STRUCTURE

SP582-04.01

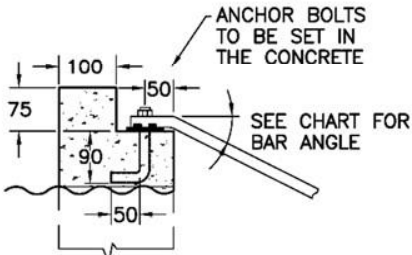
WHEN DITCH BOTTOM IS WIDER THAN STRUCTURE, TRANSITION SLOPE SHALL BE PROTECTED WITH CONCRETE FILLED SAND BAGS



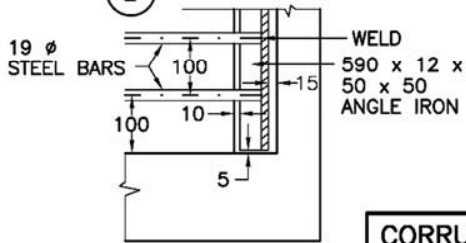
PLAN



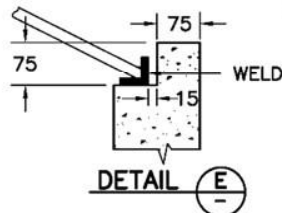
DETAIL B



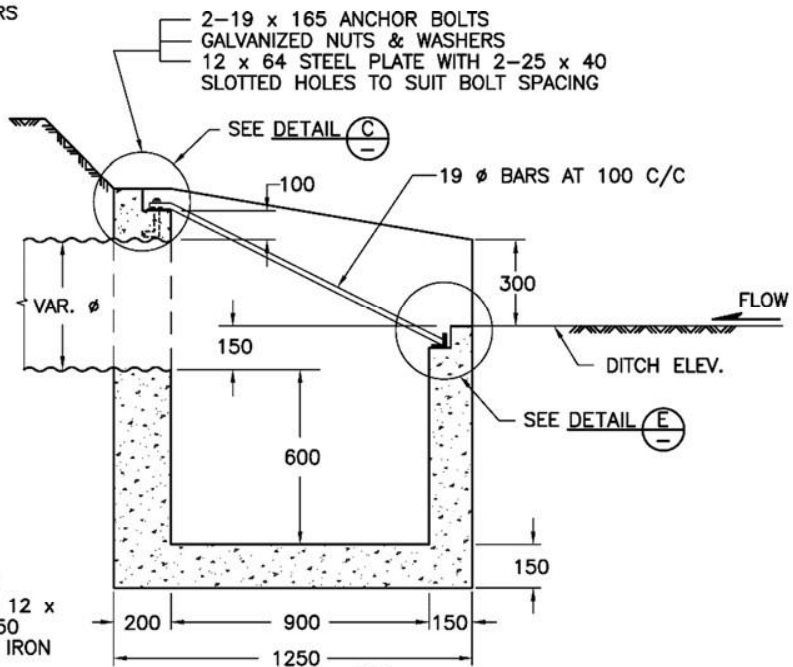
DETAIL C



DETAIL D



DETAIL E



SECTION A

CORRUGATED STEEL PIPE		
PIPE DIAM.	BAR LENGTH	BAR ANGLE
300	1073	19°
400	1112	24°
500	1157	29°
600	1209	34°

CONCRETE PIPE		
PIPE DIAM.	BAR LENGTH	BAR ANGLE
300	1073	19°
375	1101	23°
450	1134	27°
525	1170	30°
600	1209	34°

NOTES:

1. ALL STEELWORK TO CONFORM TO CSA SPECIFICATION G40.21M GRADE 300W.
2. ALL STEELWORK TO BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH CSA SPECIFICATION G164 TABLE 1.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

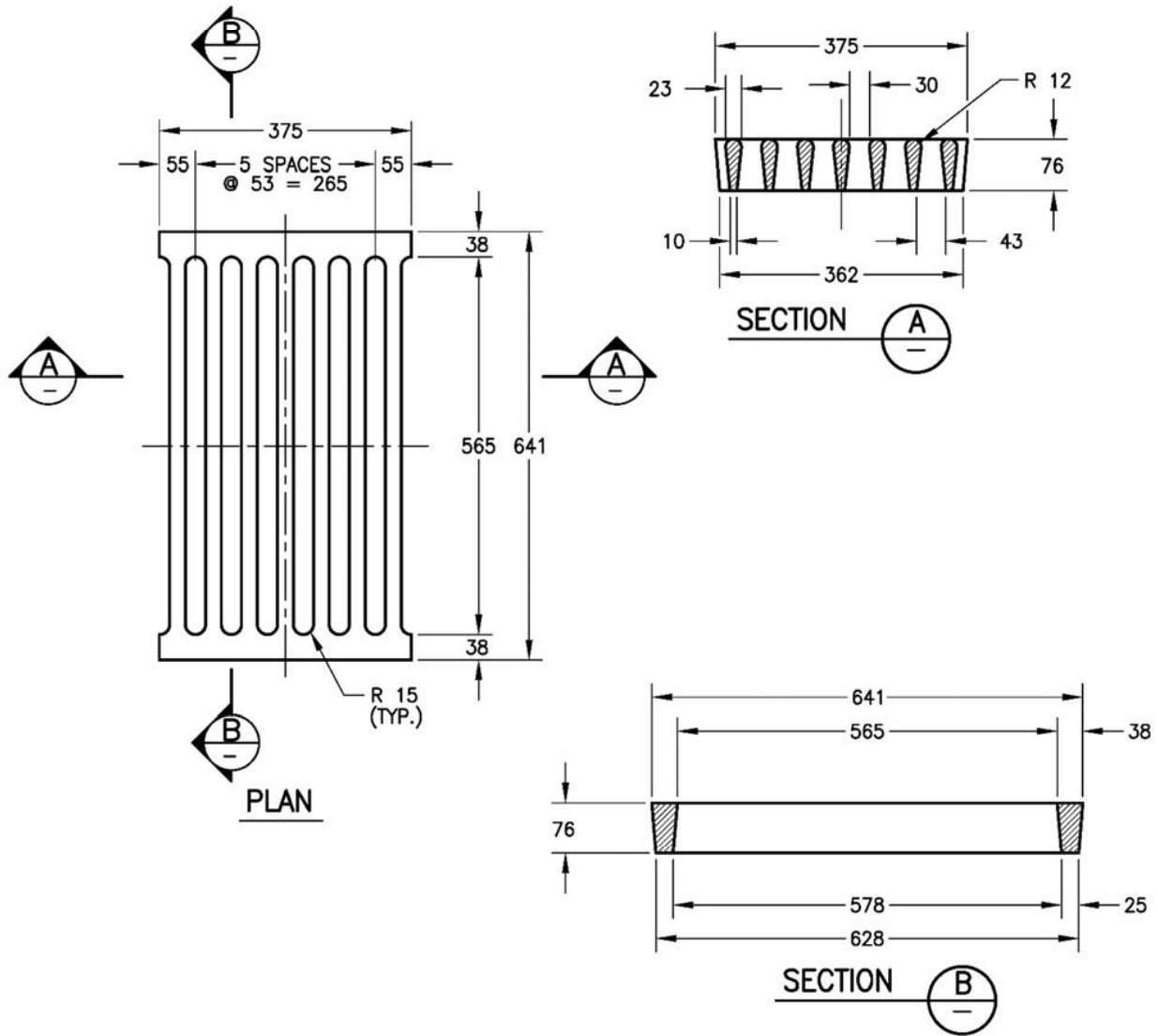


BRITISH COLUMBIA

Ministry of Transportation & Infrastructure

TYPE 'A' CATCH BASIN GRATE (FREEWAY GRATE)

SP582-05.01



NOTES:

1. GRATES TO FIT CATCH BASIN FRAMES (SEE SP582-05.03 & SP582-05.04).
2. ALL MATERIAL TO BE CAST IRON.

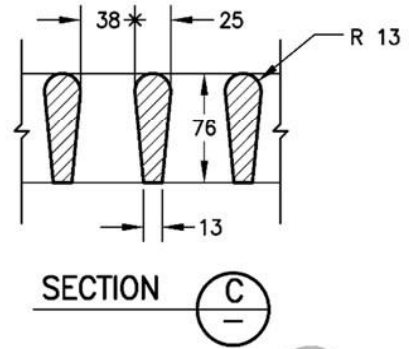
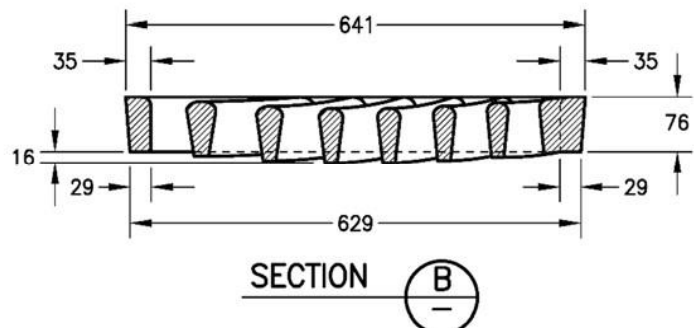
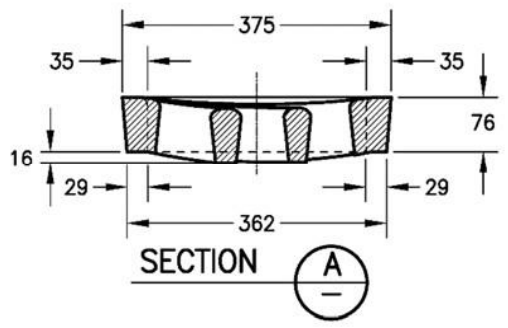
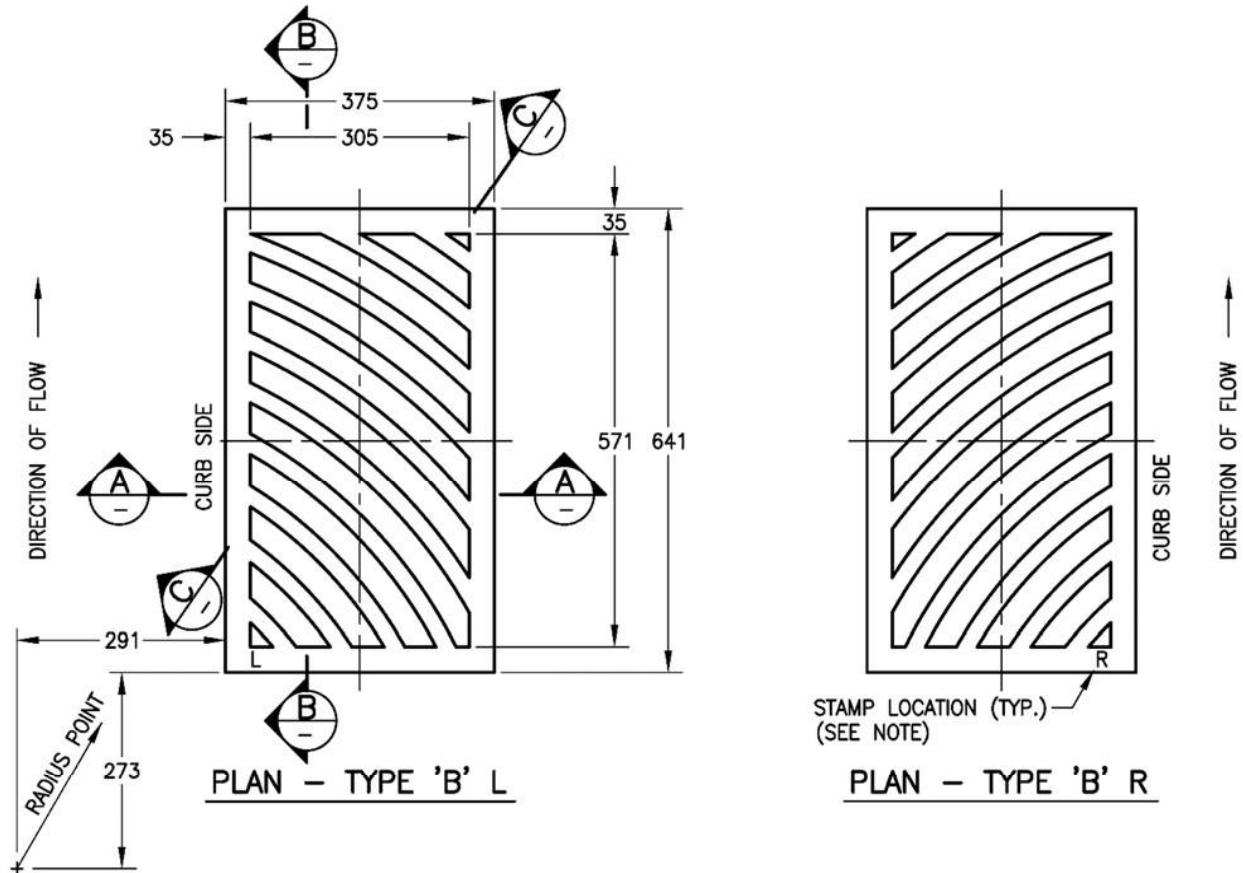
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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Ministry of Transportation & Infrastructure

TYPE 'B' CATCH BASIN GRATE (BICYCLE SAFE GRATE)	SP582-05.02
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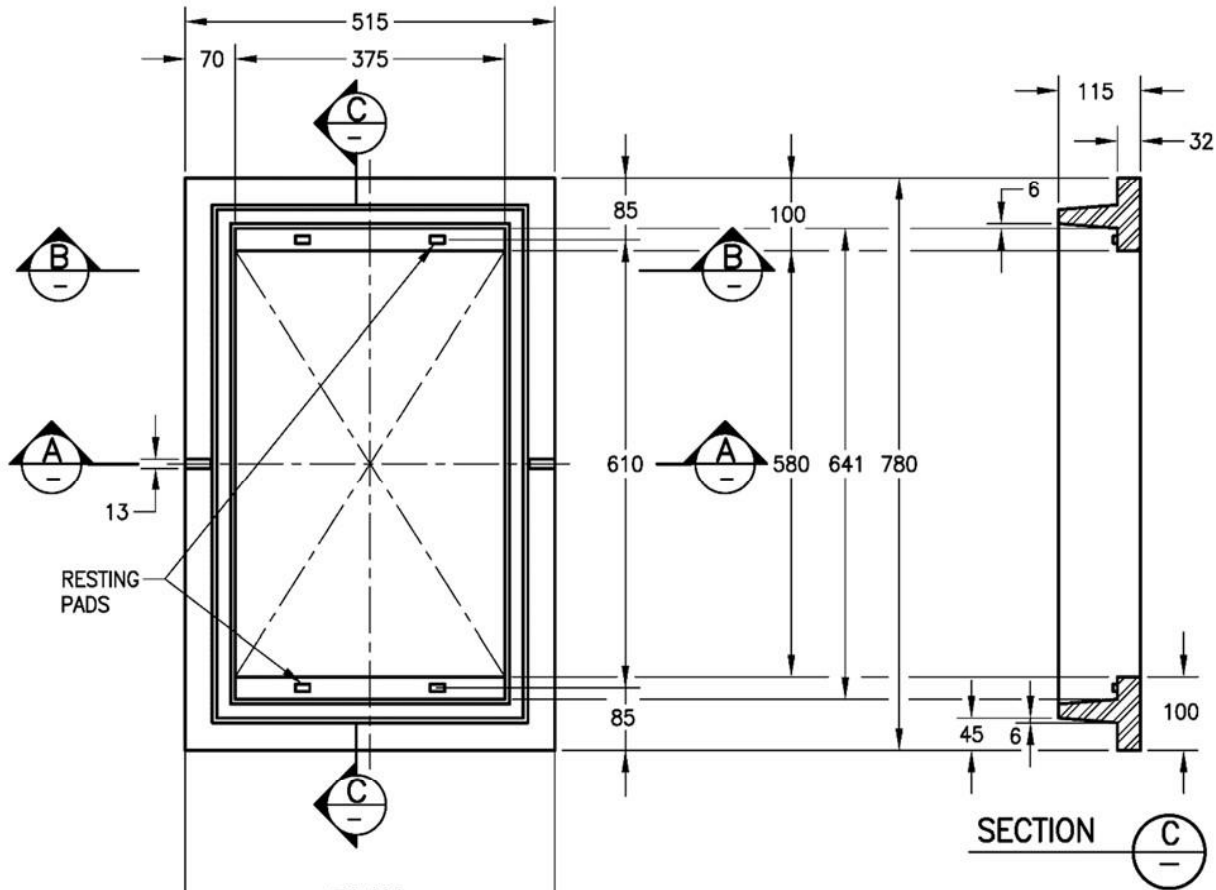
- NOTES:**
1. GRATES TO FIT CATCH BASIN FRAMES (SEE SP582-05.03 & SP582-05.04).
 2. ALL MATERIAL TO BE CAST IRON.
 3. GRATES TO BE STAMPED 'L' OR 'R' AS APPLICABLE, AT TIME OF MANUFACTURE.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

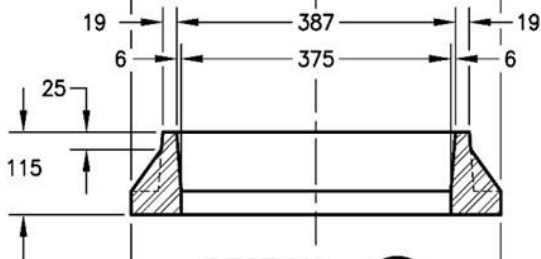


SINGLE INLET CATCH BASIN FRAME

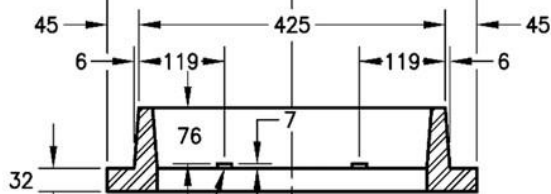
SP582-05.03



PLAN



SECTION A



SECTION B

4 - 20 x 10 RESTING PADS (MACHINED)

NOTES:

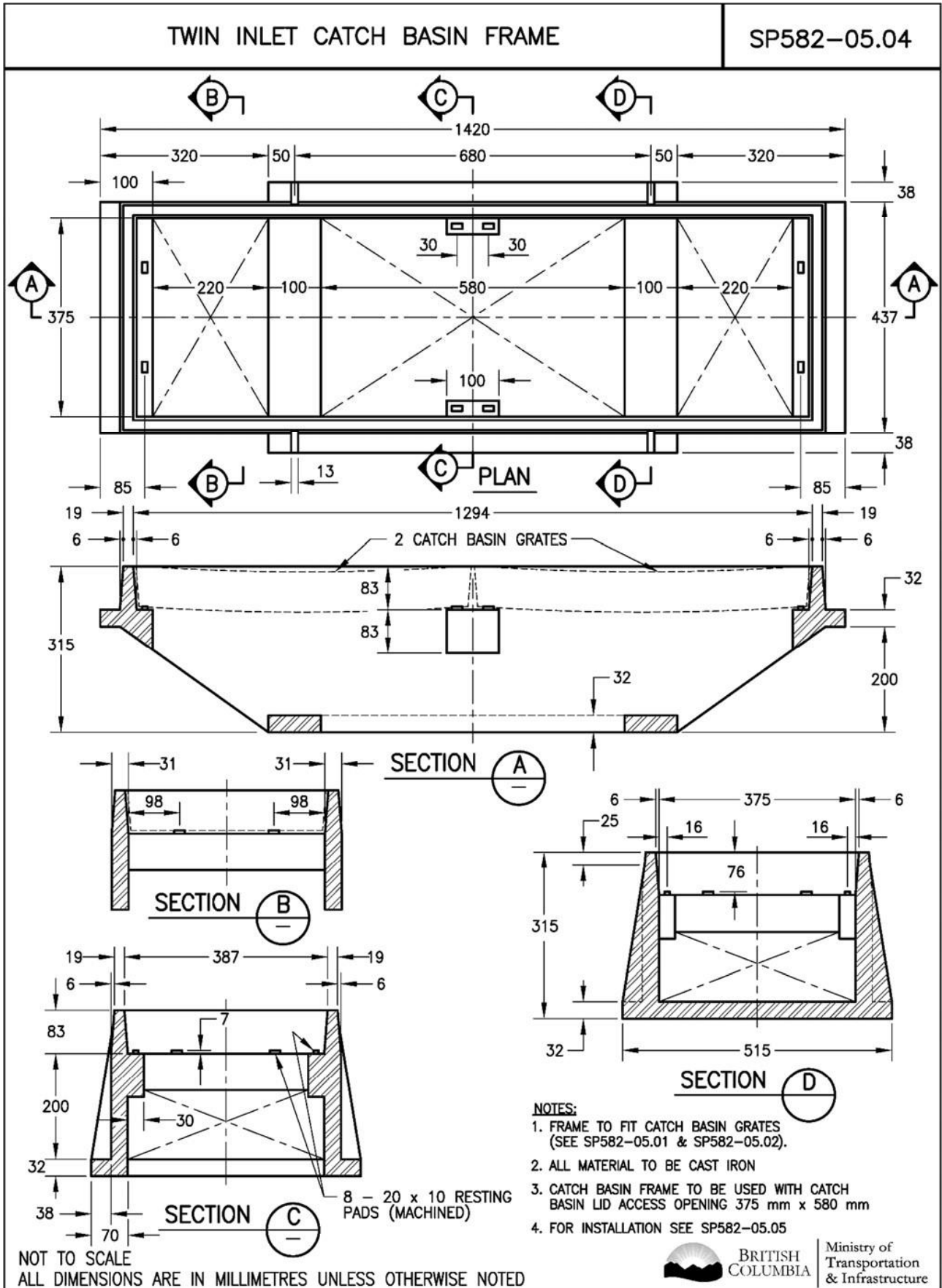
1. FRAME TO FIT CATCH BASIN GRATES (SEE SP582-05.01 & SP582-05.02).
2. ALL MATERIAL TO BE CAST IRON
3. CATCH BASIN FRAME TO BE USED WITH CATCH BASIN LID ACCESS OPENING 375 mm x 580 mm
4. FOR INSTALLATION SEE SP582-05.05

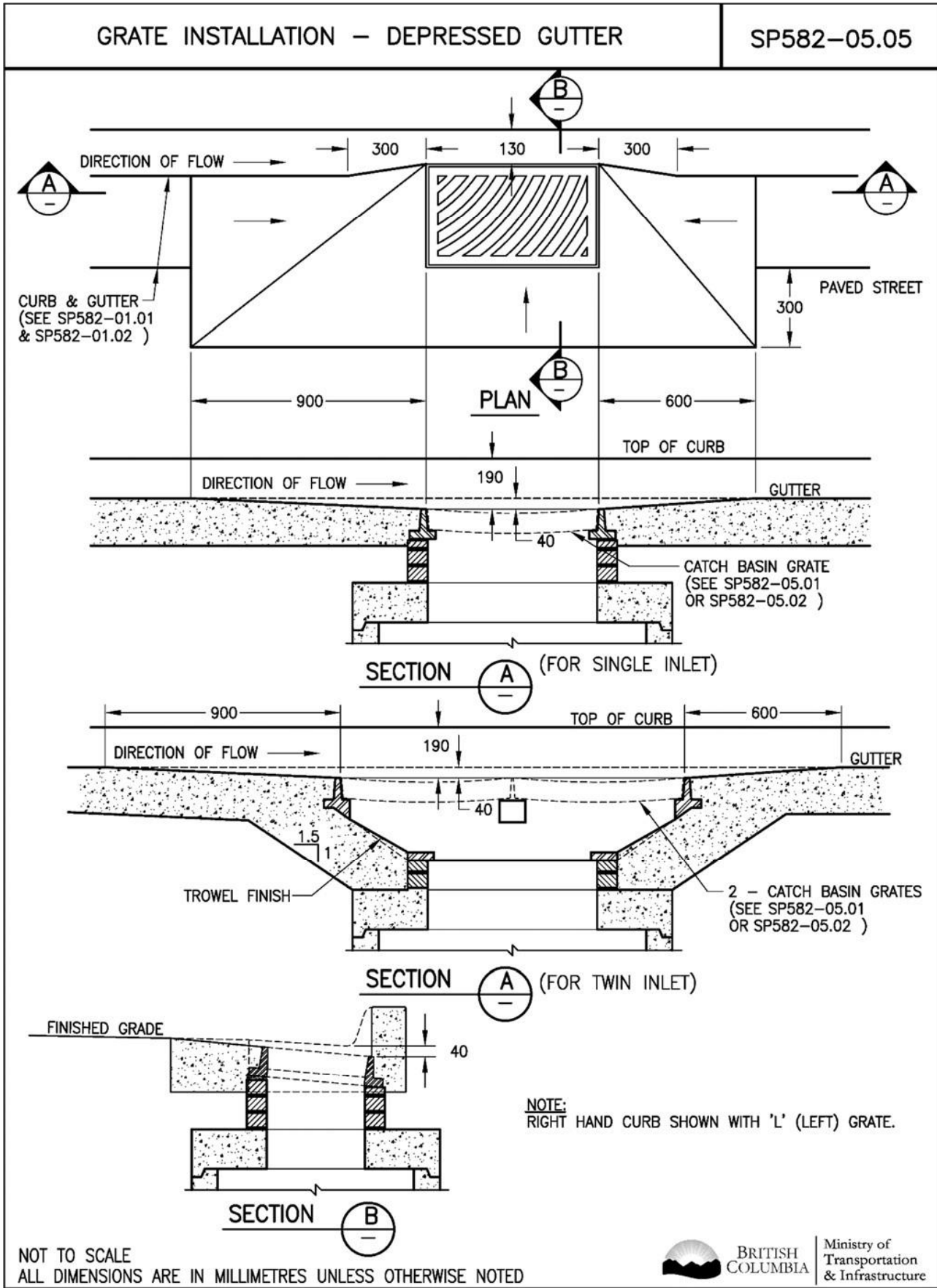
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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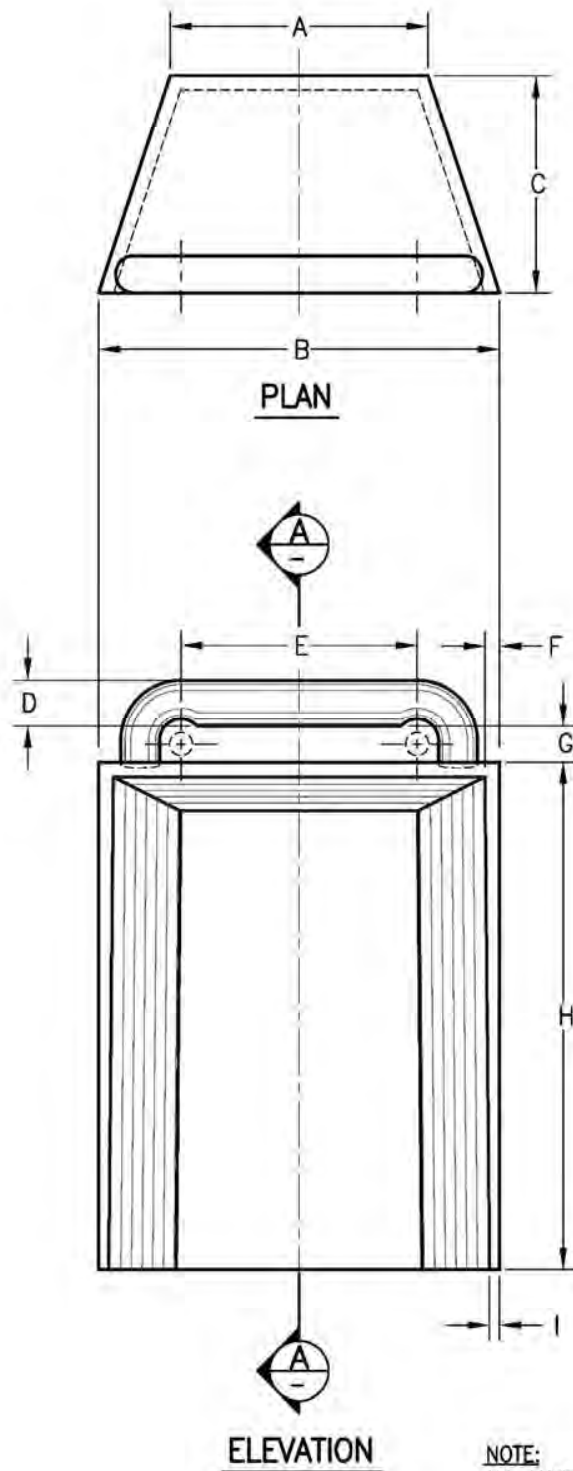
Ministry of Transportation & Infrastructure



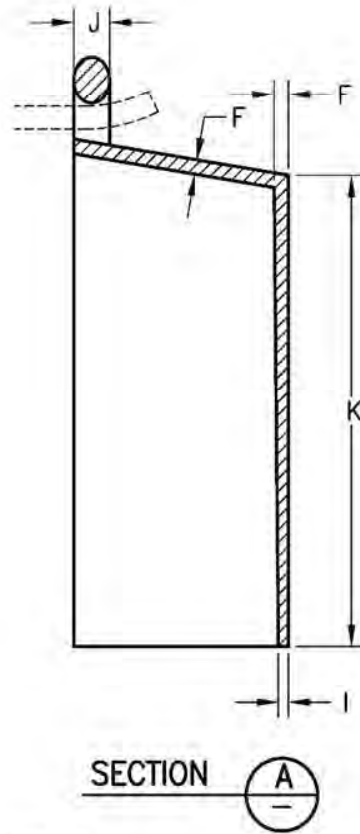


TRAPPING HOOD

SP582-05.06



DIM.	1A 150 OUTLET	9 200 OUTLET	9A 300 OUTLET
A	180	215	320
B	280	345	445
C	150	215	255
D	32	32	38
E	165	190	305
F	10	13	13
G	25	25	25
H	350	425	535
I	7	10	10
J	25	29	29
K	325	370	470
MASS	16 kg	25 kg	49 kg



NOTE:
ALL MATERIAL TO BE CAST IRON.

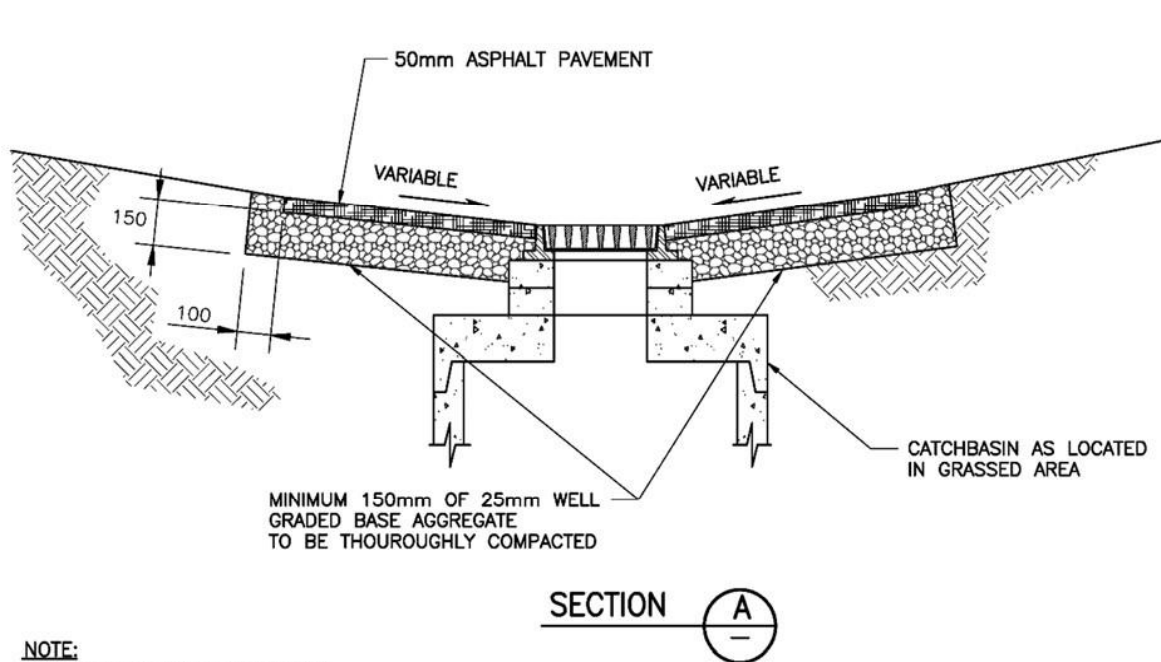
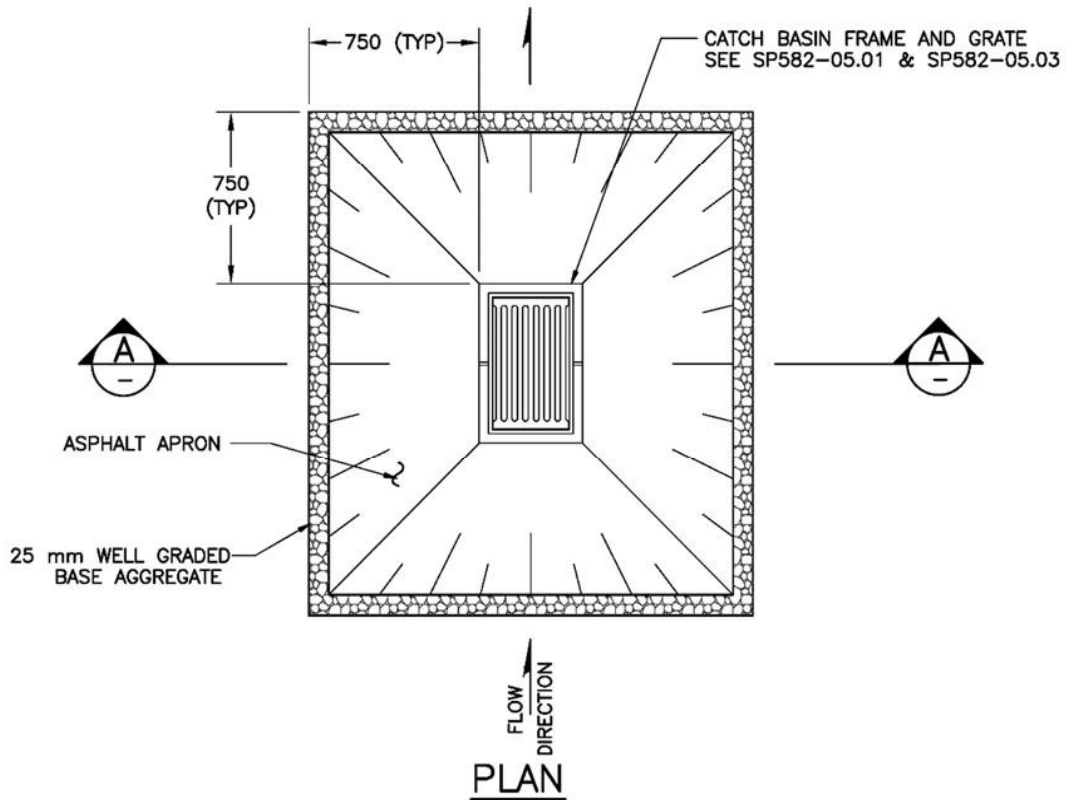
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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& Infrastructure

ASPHALT APRON

SP582-05.07



NOTE:
FOR DETAILS OF CATCH BASIN
SEE SP582-02.01 THROUGH SP582-02.05.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

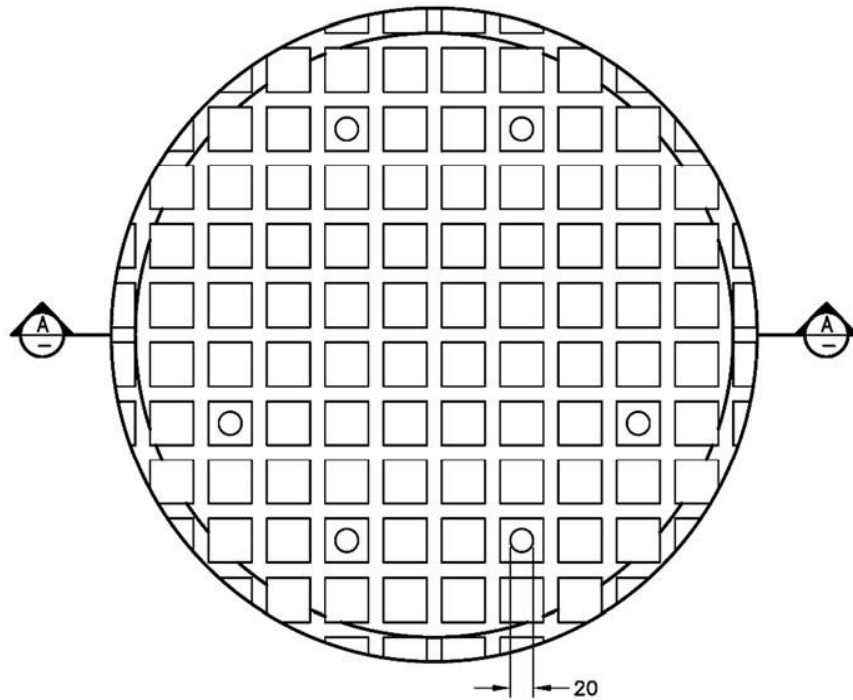


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COLUMBIA

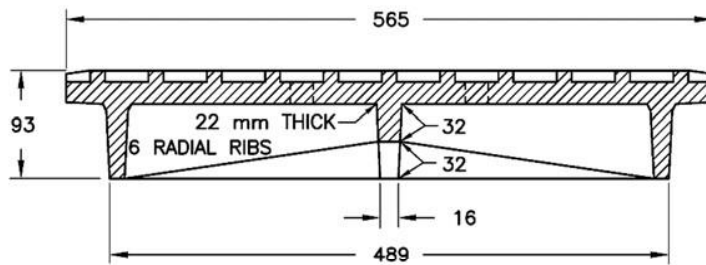
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& Infrastructure

MANHOLE COVER

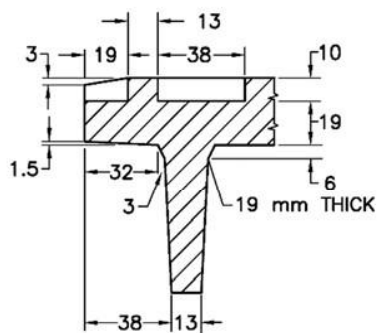
SP582-06.01



MATERIAL: CAST IRON



SECTION A



DETAIL OF RIM

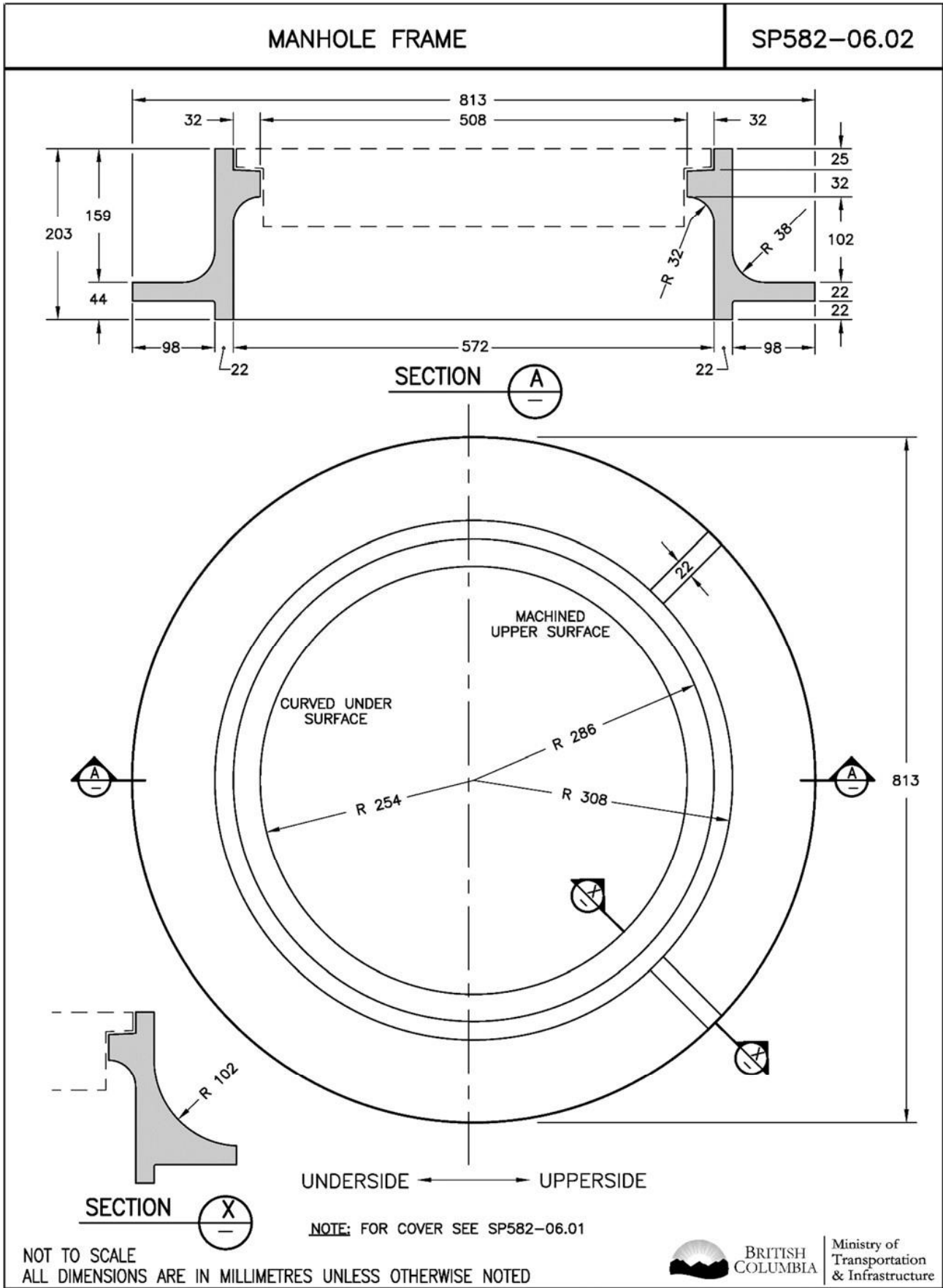
NOTE: FOR FRAME SEE SP582-06.02

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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CONCRETE SIDEWALK & DRIVEWAY ENTRANCE

SP582-07.01

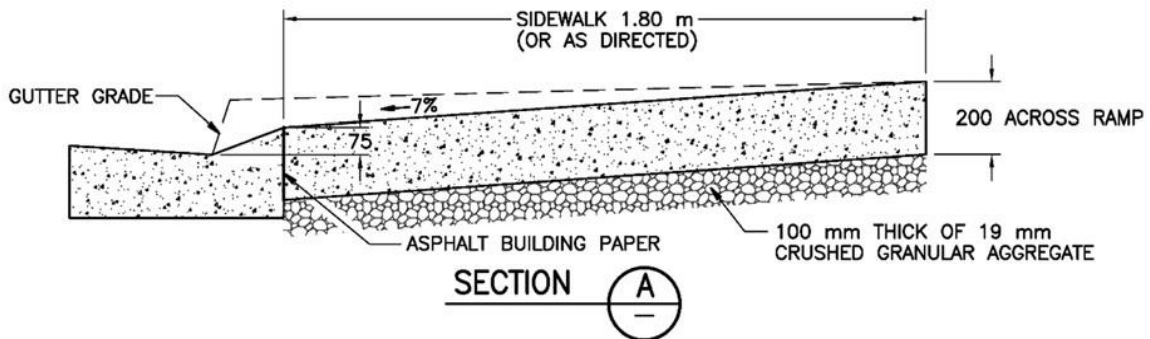
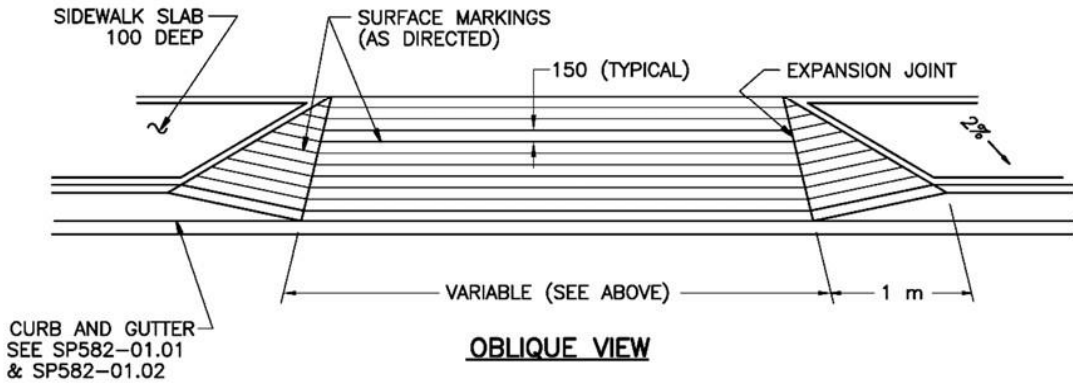
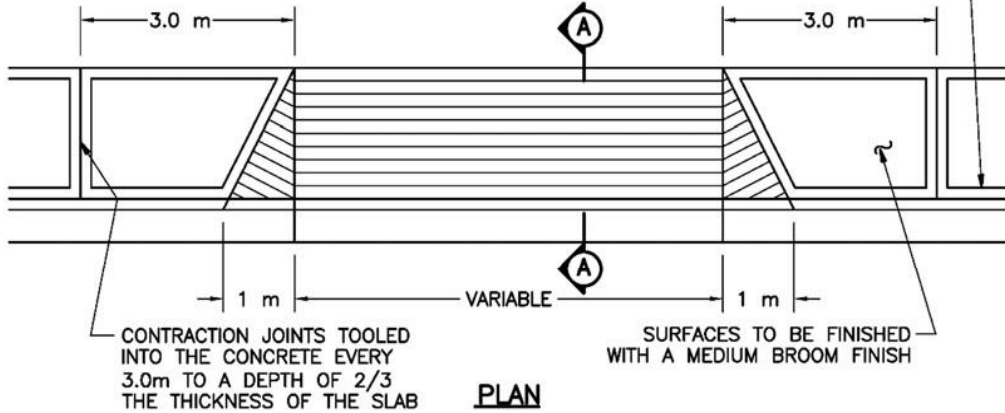
EXPANSION JOINTS

- AT 15 m INTERVALS ACROSS SIDEWALK.
- AROUND UTILITY POLES, CONCRETE STRUCTURES AND BUILDINGS.
- AT EACH END OF THICKENED SIDEWALK SECTION AND AT MID-SPAN WHEN THE LENGTH IS GREATER THAN 9 m.

DRIVEWAY WIDTHS

- COMMERCIAL 2 WAY TRAFFIC 7.5 - 15 m
- COMMERCIAL 1 WAY TRAFFIC 6 - 10 m
- RESIDENTIAL.....6 m

ALL EDGES TO BE FINISHED WITH A 50 mm WIDE FINISHING TOOL WITH A 6 mm BULLNOSE RADIUS



NOTES:

1. FOR OPEN SHOULDER DRIVEWAY DETAIL SEE B.C. SUPPLEMENT TO TAC.
2. ADD 150 mm TO WIDTH WHEN SIDEWALK FENCE IS REQUIRED.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

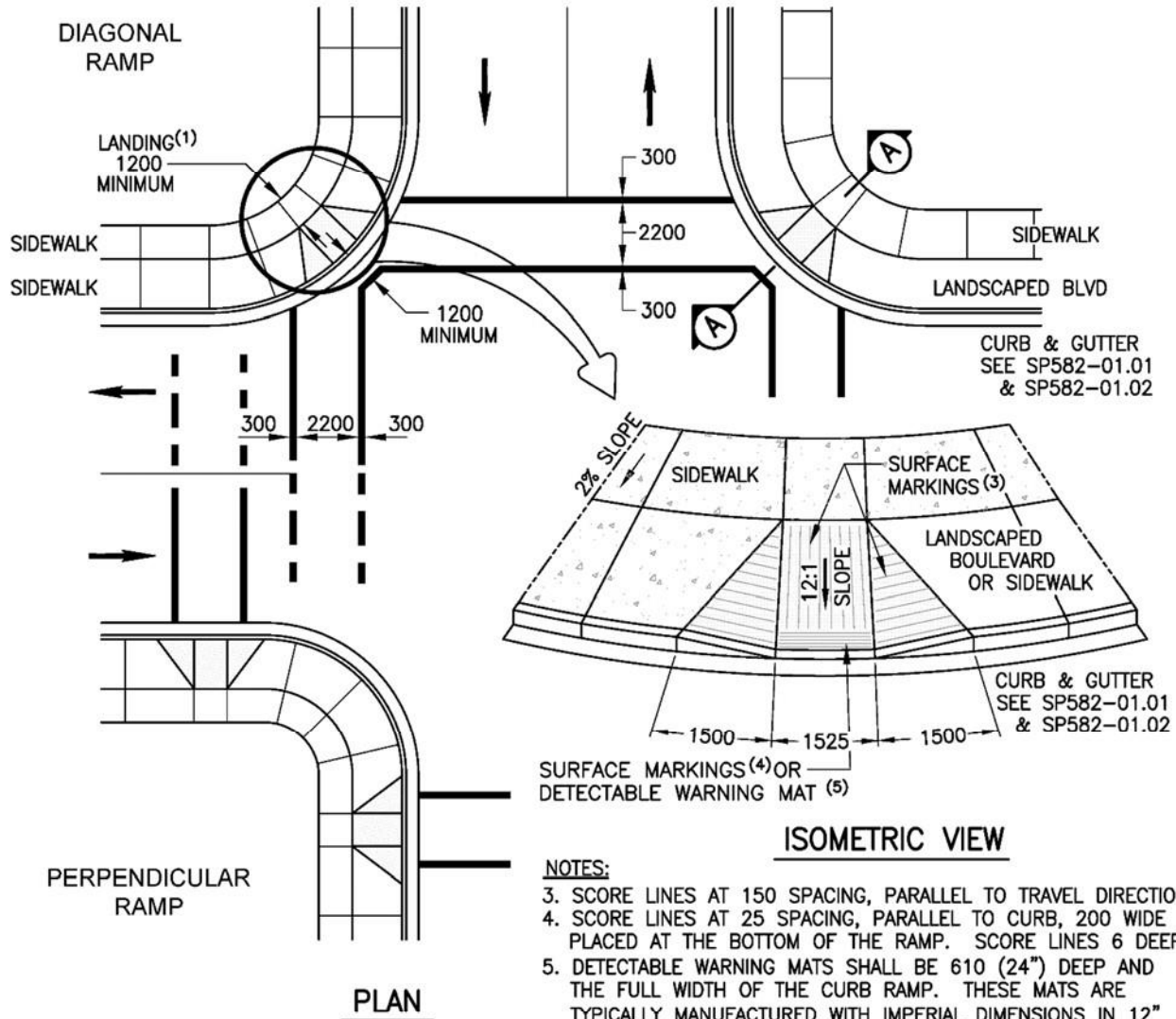


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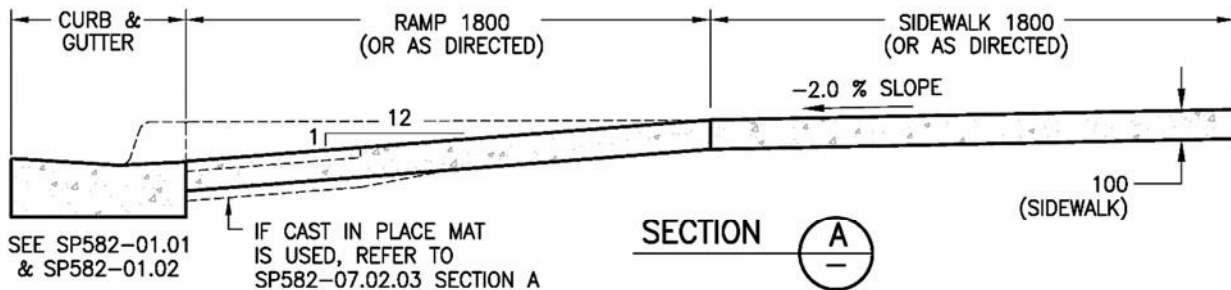
SIDEWALK RAMP – DIAGONAL AND PERPENDICULAR

SP582-07.02.01



NOTES:

3. SCORE LINES AT 150 SPACING, PARALLEL TO TRAVEL DIRECTION.
4. SCORE LINES AT 25 SPACING, PARALLEL TO CURB, 200 WIDE PLACED AT THE BOTTOM OF THE RAMP. SCORE LINES 6 DEEP.
5. DETECTABLE WARNING MATS SHALL BE 610 (24") DEEP AND THE FULL WIDTH OF THE CURB RAMP. THESE MATS ARE TYPICALLY MANUFACTURED WITH IMPERIAL DIMENSIONS IN 12" INCREMENTS; THEREFORE, CONSTRUCT THE CURB RAMP WIDTH TO SUIT.



NOTE:

1. PERPENDICULAR RAMPS ARE PREFERRED. IF IT IS NOT POSSIBLE TO PROVIDE A TOP LEVEL LANDING, DIAGONAL AND PERPENDICULAR RAMPS SHOULD NOT BE INSTALLED; USE SP582-07.02.02 PARALLEL RAMPS.

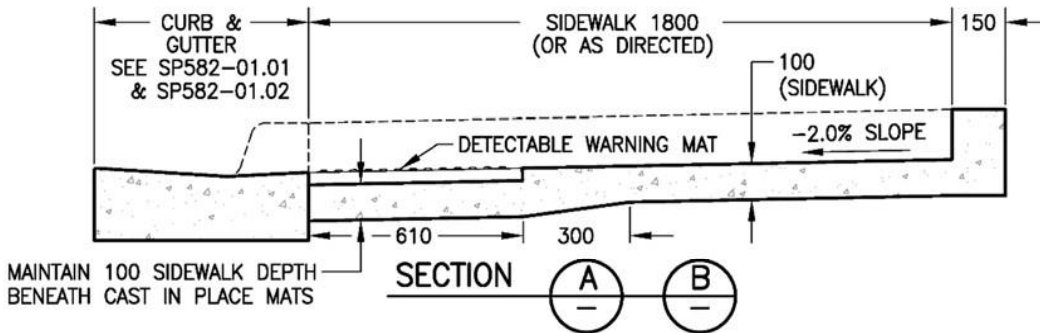
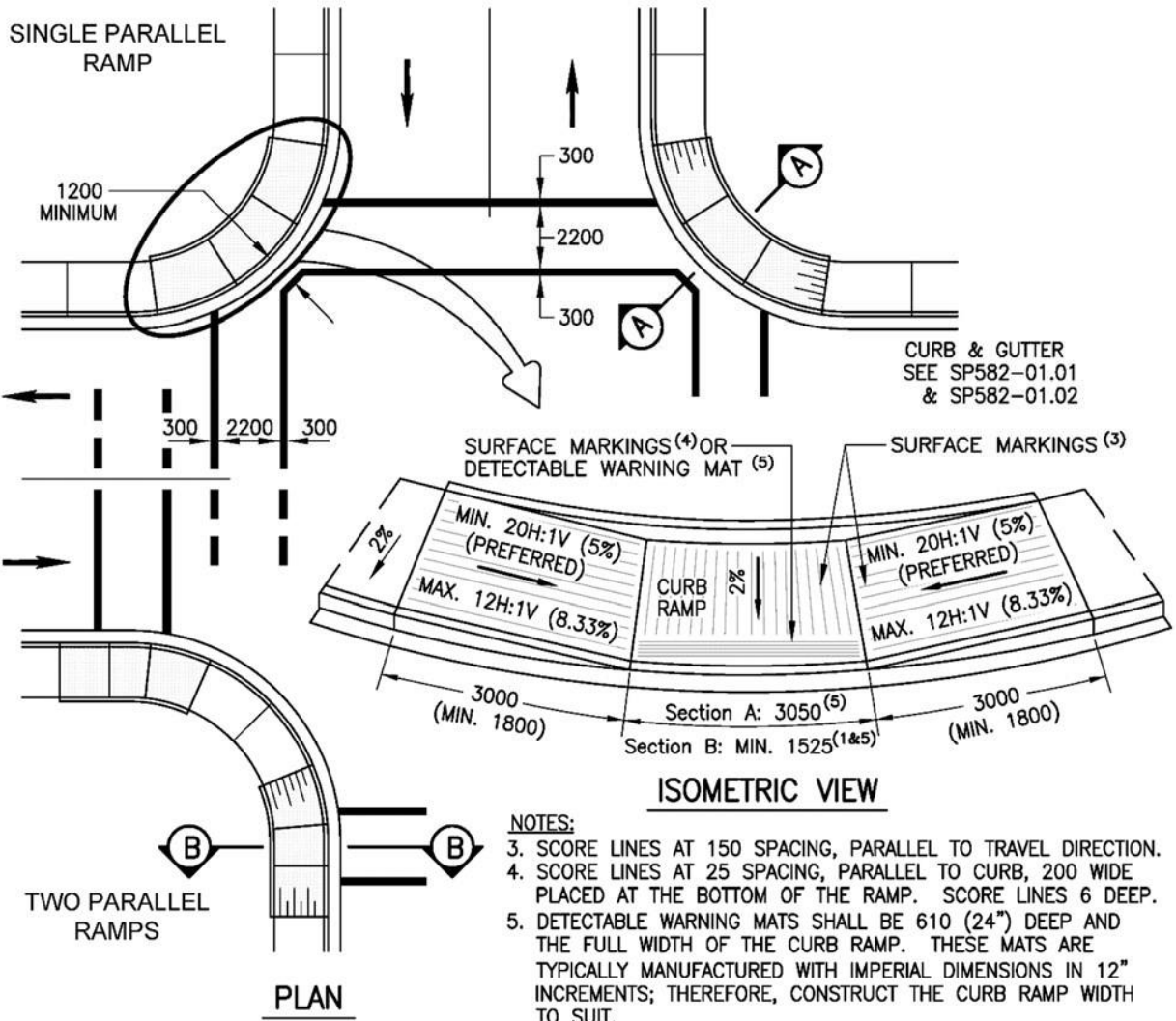
NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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SIDEWALK RAMP - PARALLEL

SP582-07.02.02



NOTE:

- FOR SECTION B RAMPS, MINIMUM WIDTH OF RAMP IS 1525. IT MAY BE NECESSARY TO BUILD WIDER RAMPS IN BUSY URBAN AREAS WHERE THE VOLUME OF PEDESTRIAN TRAFFIC IS HIGH.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED

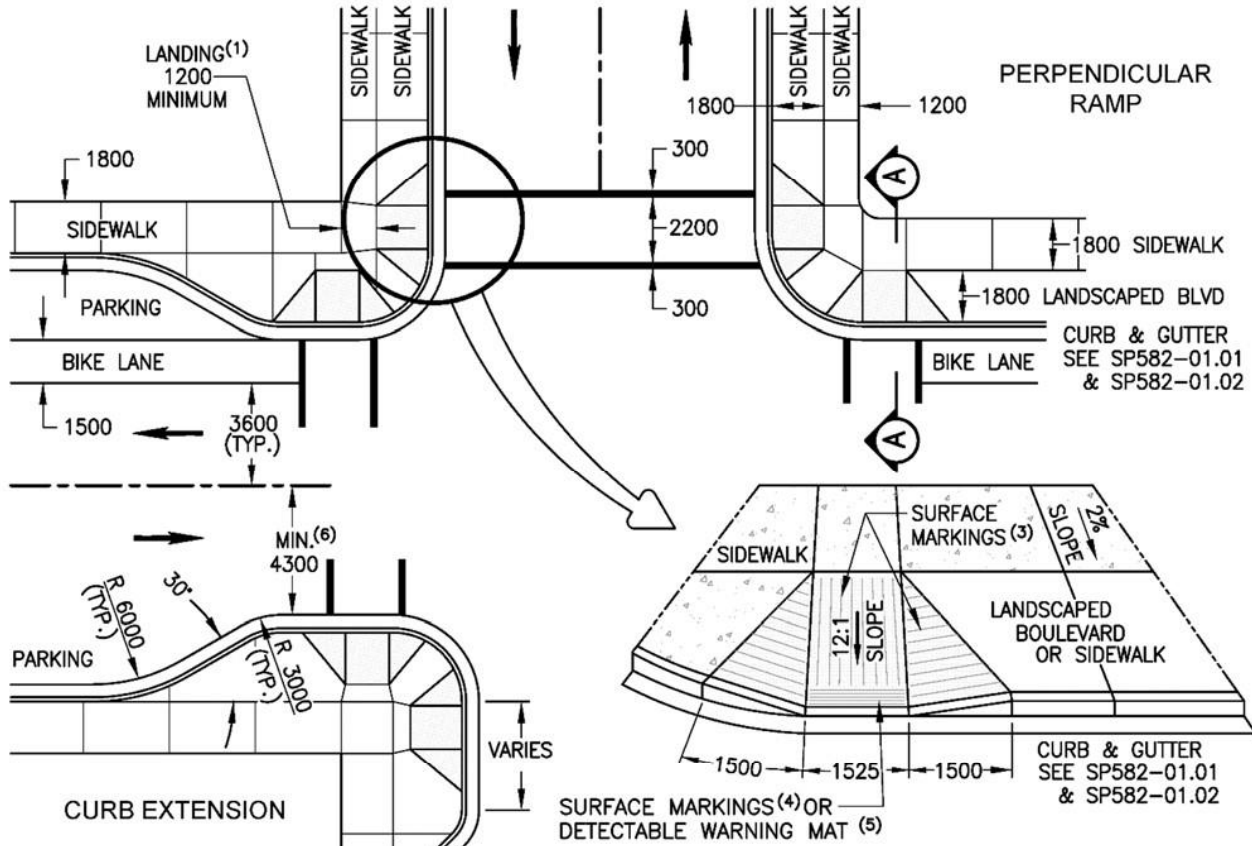


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SIDEWALK RAMP – CURB EXTENSION

SP582-07.02.03



NOTES:

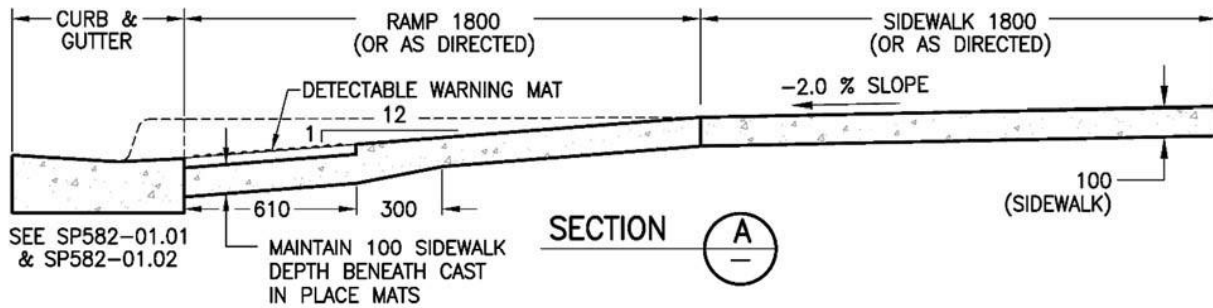
- 6. SHARED ROADWAY WIDTH WHERE BIKE LANES ARE NOT PROVIDED.

PLAN

NOTES:

- 3. SCORE LINES AT 150 SPACING, PARALLEL TO TRAVEL DIRECTION.
- 4. SCORE LINES AT 25 SPACING, PARALLEL TO CURB, 200 WIDE PLACED AT THE BOTTOM OF THE RAMP. SCORE LINES 6 DEEP.
- 5. DETECTABLE WARNING MATS SHALL BE 610 (24") DEEP AND THE FULL WIDTH OF THE CURB RAMP. THESE MATS ARE TYPICALLY MANUFACTURED WITH IMPERIAL DIMENSIONS IN 12" INCREMENTS; THEREFORE, CONSTRUCT THE CURB RAMP WIDTH TO SUIT.

ISOMETRIC VIEW



SECTION A

NOTE:

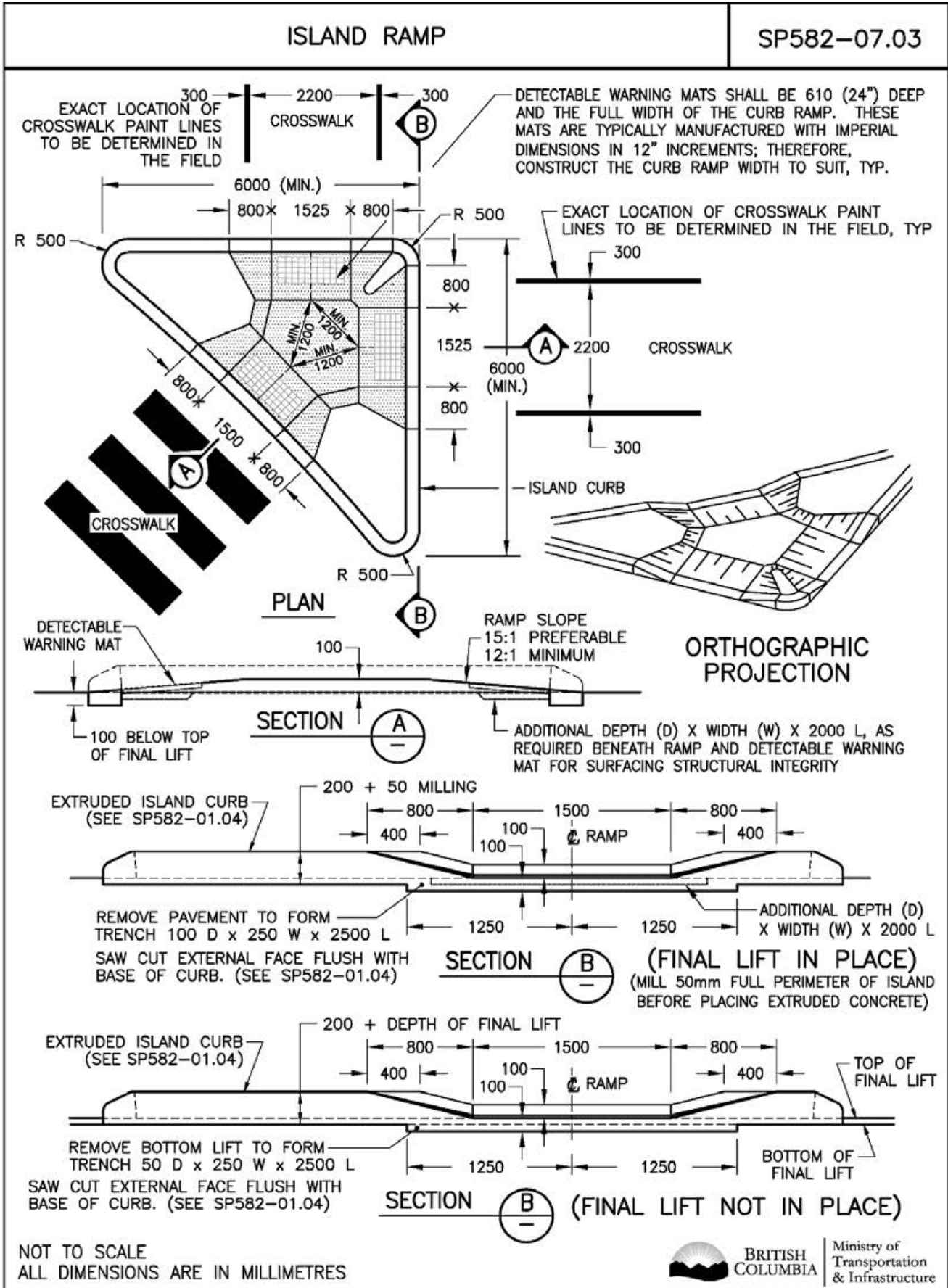
- 1. IF IT IS NOT POSSIBLE TO PROVIDE A TOP LEVEL LANDING, PERPENDICULAR RAMPS SHOULD NOT BE INSTALLED; USE SP582-07.02.02 PARALLEL RAMPS.

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



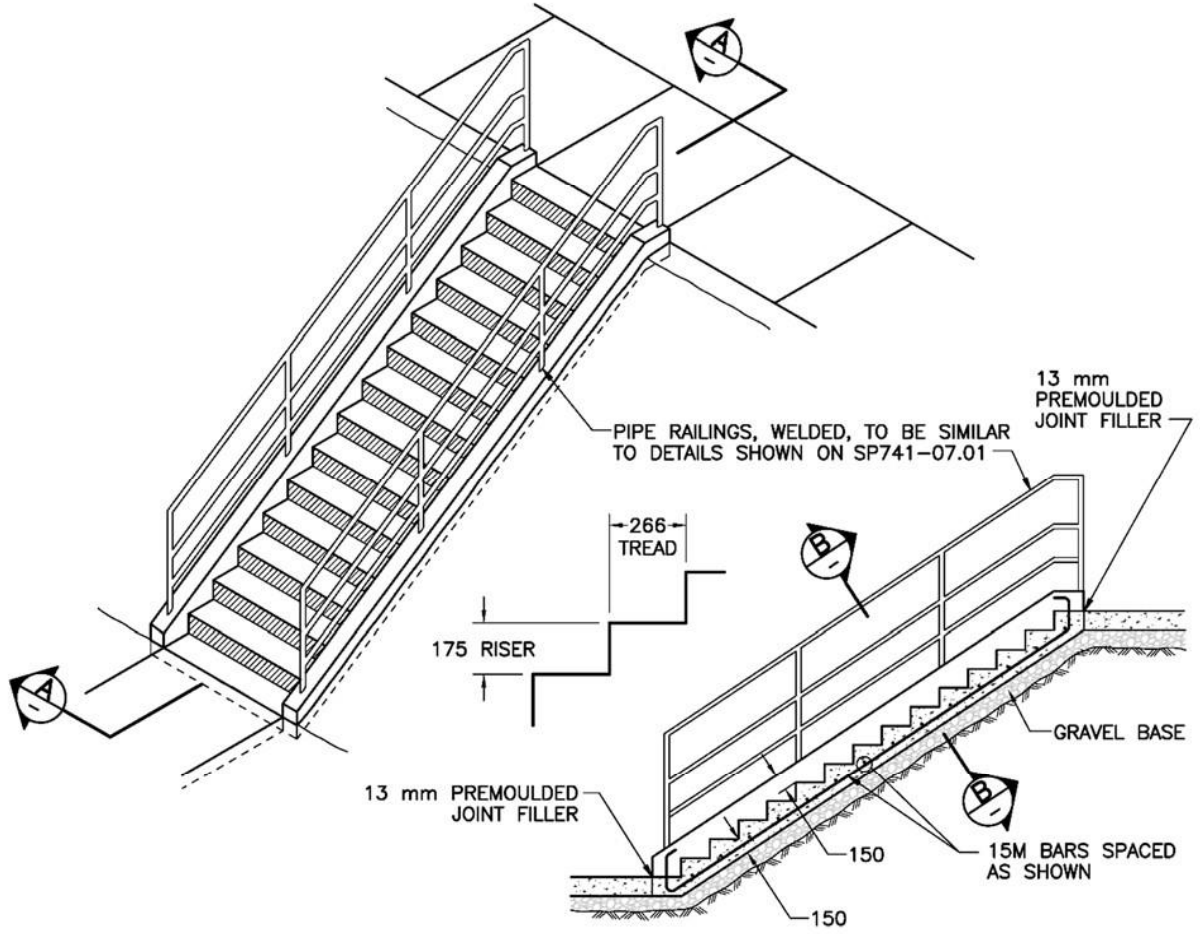
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REINFORCED CONCRETE STEPS

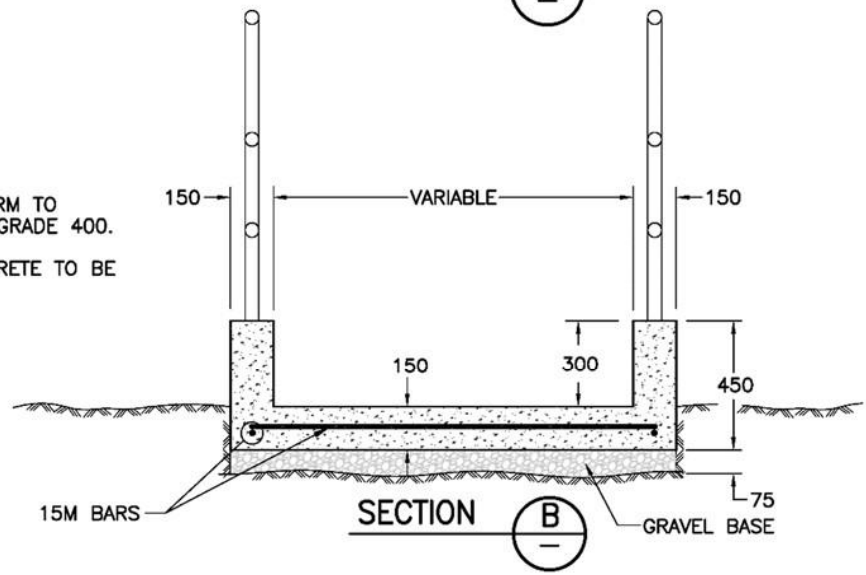
SP582-08.01



SECTION A

NOTES:

1. REINFORCING STEEL TO CONFORM TO CSA SPECIFICATION G30.18-M GRADE 400.
2. ALL EXPOSED EDGES OF CONCRETE TO BE ROUNDED TO 13 mm RADIUS.



SECTION B

NOT TO SCALE
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED



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SECTION 586

PLANT MIXED PORTLAND CEMENT TREATED BASE COURSE

DESCRIPTION

586.01 Preliminary and General – This Section describes the materials, plant equipment and quality of work required for the stabilization of base course material with Portland cement.

586.02 General Description of Work – The cement treated base shall consist of prepared aggregate, Portland cement and water uniformly mixed to meet the criteria specified, in a central mixing plant of satisfactory type, hauled out and laid on the prepared subgrade and compacted to established line and grade. The Contractor shall supply all materials.

MATERIALS

586.11 Cement – Portland Cement to be supplied for the work will comply with the requirements of SS 211 and tests of the CSA A3000 "Cementitious Materials Compendium".

586.12 Water – The water shall be clean and free from substances deleterious to the hardening of the cement stabilized base course.

586.13 Aggregate – The aggregate supplied shall be composed of inert durable material uniform in quality and free from organic impurities and soft or disintegrated particles. When tested according to ASTM C117, the aggregate shall meet the gradation requirements for 25 mm WGB as listed in Section 202.

The material shall also be tested in accordance with ASTM D4318 and shall have:

- a liquid limit not greater than twenty-five (≤ 25);
and
- a plasticity index not greater than six (≤ 6).

All aggregate shall be stockpiled in horizontal layers spread and levelled to not more than 1.0 m in thickness. The stockpile area shall be cleared of all vegetation, trees, brush, rock and other debris and a uniform ground surface prepared before the aggregate is deposited on the site.

The material shall be loaded from the stockpile into the plant in such a manner as to ensure that a uniform cut of the pile is obtained.

Mixing may not commence until a minimum of 30% of the total quantities of aggregate required is in the stockpile.

Subsequent addition of prepared aggregate to the pile shall be organized to ensure continued uniformity of the aggregate supply to the mixing plant.

EQUIPMENT

586.21 Mixing Plant – The Portland Cement treated base course shall be mixed in a stationary plant employing either continuous or batch type equipment.

The plant shall be equipped with interlocked feeding and metering devices which will add the cement, water and aggregates in proportions established by the Ministry Representative to a tolerance of $\pm 3\%$ by weight. The plant shall be capable of mixing the ingredients uniformly. Suitable storage shall be provided for bulk cement, and if required, bulk mineral filler. The capacity of the mixing plant shall be such that a minimum of 0.4 km of full width cement treated base can be laid per daily shift, i.e., sustained mixing capacity shall normally be not less than 250 tonnes per hour.

Details of the plant and its operation shall be submitted to the Ministry Representative for approval a minimum of seven (7) days prior to commencing construction.

586.22 Cement Treated Base Spreader – The spreading device may be self-propelled, pushed, or towed; and shall be so constructed and operated as to produce without segregation or surface tearing, a layer of material which is uniform in thickness and surface contour, either longitudinal or transverse, and free from irregularities in density. Whichever type of spreader may be used, it is mandatory that it be provided with a positive mechanical means for distributing the cement treated base transversely across the front of the cut-off blade or screed.

Spreading equipment which rides on the freshly spread material and produces tracks or partially compacted areas behind the cut-off blade or screed will be acceptable provided no displacement of material or filling of the tracks occurs, and provided further that the tracks are not of such depth as to be visible after compaction is completed. The use of a motor grader or any other spreading device, which requires repeated trips over the same area, will not be permitted for spreading.

586.23 Water Distributors – Water distributors shall be the pressure type, equipped with spray bar mounting nozzles similar to those used on asphalt distributors and capable of applying the water in accurate quantities and as a uniform spray. Splash plate type distributors or those equipped with spray bars that eject fine streams of water will not be permitted. The distributor must be provided with a satisfactorily accurate means of measuring the quantity of water sprayed.

CONSTRUCTION

586.31 Mixing – The mix proportions shall be determined by the Contractor, from samples taken at the commencement of aggregate production and in accordance with the mix design criteria provided in the Contract, and shall be subject to approval by the Ministry Representative.

The aggregate, mineral filler and cement shall be fed into the pugmill accurately, according to the proportions required, and shall be thoroughly dry mixed. Water shall then be added in the required amount and mixing continued until uniformity is achieved.

586.32 Spreading and Compaction

586.32.01 Surface – The surface of the subgrade shall be thoroughly moistened just prior to placing the cement treated base mixture thereon and shall be kept moist but not excessively wet until covered by the mixture.

586.32.02 Cement Treated Base Mixture – The cement treated base mixture shall be hauled to the roadbed in trucks equipped with protective covers and shall then be laid using a spreading device as per SS 586.22 to produce a layer of uniform density and cross section and in sufficient quantity to provide a compacted base conforming to the grade and cross section set by the Ministry Representative.

End dumping on the roadway followed by spreading will not be permitted.

Depositing and spreading the material on the roadway shall progress continuously without breaks, except as otherwise directed by the Ministry Representative.

At any break, including the end of each day's full width construction, a straight transverse construction joint shall be formed by cutting back into the completed work to form a true vertical face and by installing a temporary wooden header, if required by the Ministry Representative.

586.32.03 Compaction – Immediately following spreading, the material shall be compacted to a minimum 100% of the Standard Proctor Maximum Dry Density obtained by ASTM D698.

586.32.04 Compaction Equipment – The Contractor shall supply compacting equipment having sufficient capacity to produce the specified degree of compaction within the time limits specified. The use of compacting equipment which marks, ruts, cracks or otherwise loosens or damages the surface of the cement treated base will not be permitted.

Wherever compaction planes are formed in the cement treated base during spreading, shaping or compaction, they shall be removed by scarifying or tilling the surface to the required depth to completely remove the compaction plane and in such a manner as to thoroughly break up the compaction planes. Scarifying and pulverization may be performed by any suitable equipment – nail drag, weeder, disk harrow or rotary tiller that will satisfactorily perform the work required. The scarified material will be regraded

and recompact, to the satisfaction of the Ministry Representative.

586.32.05 Water – Water shall be incidental to the Work and shall be applied as required.

The surface of the uncompacted, partially compacted, and completely compacted material shall always be kept moist. An adequate water supply shall be maintained at the point of compacting.

586.32.06 Speed – The speed of operations shall be such that not more than forty-five minutes shall elapse between the time of starting mixing and the time of starting compaction. Shaping and final compaction shall be completed within two hours of commencement of mixing at the plant.

586.32.07 Finished Surface – The finished surface of the compacted base shall at all points be accurate to ± 15 mm of the grade and shall not, at any point, deviate by more than 10 mm from the bottom of a 3 m straight edge laid in any direction on the surface on either side of the crown of the roadway.

586.32.08 Temperature – No construction shall be undertaken when the temperature is below 4.0°C or when the conditions indicate that the temperature will fall below 4.0°C in the next 24 hours or if the subgrade is frozen.

586.33 Protection and Cover

586.33.01 General – Protection and cover shall be provided by one of the methods, A or B, given below and as directed in the Special Provisions.

(a) Method A: Crushed Gravel Course – A 25 mm crushed gravel shall be laid not more than 24 hours after the treated base is laid. The surface of the treated base and the overlying gravel shall be kept moist by the application of water as directed by the Ministry Representative.

(b) Method B: Bituminous Seal – As soon as possible after the cement treated base course has been finished, in accordance with preceding subsections, a bituminous seal shall be applied to the surface and sanded to protect the cement treated base course from drying out during the seven day curing period. The finished base course shall be kept continuously moist until the seal has been applied.

The bituminous sealing material shall be applied at a rate between 0.7 L/m² to 1.0 L/m² or as directed by the Ministry Representative in order to give complete surface coverage without excessive runoff.

Generally, this work shall be governed by the applicable provisions of SS 502.21. At the time of sealing, the surface of the cement treated base must be dense, free from all loose and extraneous material and shall contain sufficient moisture to prevent penetration of the bituminous material. If needed, water shall be

applied in sufficient quantity to fill the surface voids of the soil cement immediately before the bituminous material is applied. The seal cover shall be effectively maintained by the Contractor during the seven day curing period.

586.33.02 Protection – Any finished portion of the base course adjacent to construction, which is travelled by equipment used in constructing an adjoining section, shall be protected in such a manner as to prevent equipment from marring or damaging the completed work.

586.33.03 Temperature – At any time when the air temperature may be expected to fall to the freezing point, during the day or the night, sufficient above protection shall be given to the cement treated base to prevent its freezing for seven days after placement and until the soil cement has hardened.

586.34 Maintenance – The Contractor shall maintain the entire base course in good condition satisfactory to the Ministry Representative from commencement of construction until it has been paved with asphalt concrete. Maintenance shall include immediate repairs to any defects that may occur and shall be repeated at the Contractor's expense as often as may be necessary to keep the section continuously intact.

MEASUREMENT

586.81 Cement Treated Base – Cement treated base in place will be measured by the TONNE for the total mass actually accepted and incorporated in the base of the road.

PAYMENT

586.91 Cement Treated Base – Payment for CEMENT TREATED BASE will be at the Contract Unit Price per tonne for the total mass actually accepted and incorporated in the base of the road. Payment shall provide full compensation for supply or production of aggregate; supply, handling, hauling and storage of bulk cement and filler; provision of mixing water and plant mixing of cement treated base; haulage, spreading, watering, grading and compaction of the base material in place; and for all labour, equipment, plant, tools, superintendence, traffic control and all other incidentals necessary to complete the work in accordance with the specifications.